UNITED STATES OF AMERICA

BEFORE THE FEDERAL ENERGY REGULATORY COMMISSION

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| In Re: | ) | Docket No. 77-332 |
| | ) | |
| Potter Valley Hydroelectric | ) | |
| Project | ) | |

# COMMENTS OF THE COUNTY OF LAKE TO PG&E'S PROPOSED SURRENDER, DECOMMISSIONING, AND NON-PROJECT USE OF PROJECT LANDS AND SCOPE OF ISSUES TO ADDRESS IN THE NEPA DOCUMENT

Pursuant to the Federal Energy Regulatory Commission's (hereinafter, "FERC" or "Commission") May 22, 2026 "Notice Of Scoping Meetings And Request For Comments On Proposed Surrender, Decommissioning, And Non-Project Use Of Project Lands" and the "Scoping Document 1 for the proposed Potter Valley Hydroelectric Project surrender and decommissioning," the County of Lake ("County") hereby submits its updated comments to Pacific Gas & Electric Company's (hereinafter, "PG&E") proposed surrender, decommissioning, and non-project use of project lands (hereinafter, "Application") and scope of issues to address in the NEPA document. This updated comments document supersedes the County's previous protest and comments document filed August 25, 2025, under P-77-000 and P-77-332 (Accession No. 20250825-5174), and its protest and comments document filed December 1, 2025 (Accession No. 20251201-5616).

As members of your Commission will be aware, the County of Lake has long opposed Decommissioning of Scott Dam and the Potter Valley Project. To date, it has not been satisfyingly demonstrated that the project, which would be historic in scale, can be safely undertaken. In 2027 General Rate Case testimony before the California Public Utilities Commission (May 15, 2025), PG&E represented Decommissioning

would cost $532 million. The true cost, particularly when considering the need to develop alternative water infrastructure and restore the currently inundated area, will multiply that figure; as the report from SLR International Corporation (hereinafter, "SLR") discussed herein and attached to these comments demonstrates, based on PG&E's own assumptions, which neglect multiple potential cost drivers, Scott Dam Decommissioning, alone, could reach $358 million, while throwing into doubt water supply relied on by 600,000 Californians; population numbers similar to the States of Wyoming and Vermont.

Should projections of climate scientists come to pass, water storage will only grow in importance in coming years. California's Sites Reservoir project has received hundreds of millions in both State and Federal commitments (greater than $1.5 Billion, in total), in recognition of the potentially profound implications of failing to adequately store water during the wettest seasons. Members of your Commission may likewise be aware, California's SB 72 (2025) charged our State's Department of Water Resources with a, "target of identifying 9 million acre-feet of additional water supply by 2040 to offset the anticipated loss from warmer ambient temperatures in the future." Decommissioning Scott Dam would be the largest single dam removal in California's history, and decrease reliably stored water supply by at least 53,000 acre-feet. As Honorable Bureau of Reclamation Acting Commissioner, Scott Cameron, noted in June 2025 testimony before the Senate Subcommittee on Energy and Water Development (Committee on Appropriations), the Department of the Interior is implementing President Trump's Memorandum, "Putting People over Fish: Stopping Radical Environmentalism to Provide Water to Southern California," through multiple

efforts, including the "Central Valley Projects" which have received greater than $3 Billion in taxpayer investment, over time. Creation of another "water desert" in California could further strain taxpayer resources and frustrate the Administration's efforts to make essentials more affordable for Americans.

Not only would these water supply implications be consequential for every American, frequent lightning-caused fires near the current Lake Pillsbury inundation area could grow to larger wildfire events, over-burdening emergency services resources in an area that has already seen some of the largest megafires in California's history. Prevention now, in the form of your Commission demanding PG&E submit an alternative plan leaving existing diversion infrastructure in place or meaningfully and adequately replacing it with fully funded projects that can be enacted prior to dam removal, can save *Billions* in future federal allocations. With FEMA reform, "active and ongoing," and earnestly seeking to meaningfully "accelerate disaster response and recovery" while "[reducing] administrative costs and federal burden," acting upstream to stem megafires before they start by maintaining the most critical existing water storage asset abutting the Mendocino National Forest is more than sensible – it will save lives and property and enhance the legacy of ongoing reforms.

Management of sediment, concrete, and other materials associated with removal of Scott Dam would also constitute the single largest disposal project in Lake County's history, yet PG&E has not engaged in substantive planning discussions with local officials. PG&E has consistently underestimated costs associated with sediment control, and a robust sediment and vegetation management plan could reach $200-500 million. Allowing PG&E's plan to proceed with such elements left in the realm of

conjecture and assumption would be truly unprecedented and may invite avoidable and expensive environmental disaster.

As elucidated by SLR Reports attached hereto, Scott Dam may be presently stabilizing a hillside in an area known for geological instability, as well. If there is a landslide consequent to removal of the Potter Valley Project facilities (relatively immediately, or even 20 years down the road), and it blocks the Eel River, your Commission must make it clear that PG&E will remain responsible, for the long haul. Fundamentally: 1) PG&E received 100 years of benefit from the Potter Valley Project; 2) PG&E elected not to appropriately maintain and retrofit the project to maximize hydroelectric generation; and 3) PG&E has now proposed Decommissioning – PG&E must take *full responsibility* for the consequences of their own decisions. Thank you for your earnest consideration of these, and the following Comments, as well as the substantive attachments to this submittal. Taken in whole, your Commission will find PG&E's Final Application lacks details critical to assessing the best path forward. With a project this consequential from regional water supply stability and wildfire suppression and disaster management standpoints, it may not be reasonable to expect one private, Investor-Owned Utility, to independently uphold and balance the full complement of implications for all American taxpayers. PG&E has pushed short-sighted Memoranda of Understanding and other Agreements that prejudice and limit the ability of some key stakeholders (with broad-based interests) to engage in this very Proceeding, and that fact demonstrates, without qualification, PG&E should not be in the driver's seat in determining what comes next. Decisions as consequential as investments in water infrastructure should be made by prudent consideration of the

right complement of parties, not driven by fear of a private utility leaving a region without water. Reason must prevail (in project design and implementation timelines), or it will cost tax- and ratepayers *Billions*.

Therefore, we respectfully invite your Commission to recognize the urgent and compelling need to promote American interests by finding an alternative way forward; one that places people first and takes every reasonable step to avert avoidable disaster. Your vital role in ensuring a good outcome cannot be overstated.

## I. INTRODUCTION

The County of Lake is a political subdivision of the State of California. Provisions for the government of California counties are contained in the California Constitution and the California Government Code. A County is the largest political subdivision of the state having corporate powers. It is vested by the Legislature with the powers necessary to provide for the health, safety, and welfare of the people within its borders. Located in Northern California, Lake County is home to Clear Lake, the largest, natural freshwater lake in California with 68 square miles of surface area. There are three major watersheds in the County: Cache Creek (including the Clear Lake basin), Putah Creek and Eel River. Agriculture is the predominant industry in the County, including the Lake Pillsbury Area formed by Scott Dam. Other prominent industries include tourism/hospitality and outdoor recreation, both presently found in the Lake Pillsbury area.

PG&E and its predecessors created Lake Pillsbury on the Eel River as part of the Potter Valley Project. Located on the Upper Lake Ranger District in Lake County, this lake has 31 miles of shoreline. Fishing, boating, sailing and waterskiing are

popular activities.

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in PG&E's Potter Valley Hydroelectric Project. Scott Dam is located on the Eel River in Lake County, within the Mendocino National Forest.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project (PVP), which includes Potter Valley Powerhouse, Cape Horn Dam, Scott Dam and other associated facilities. On July 25, 2025, PG&E submitted its final Application requesting approval of decommissioning. The removal of Scott Dam and loss of Lake Pillsbury is expected to have complex impacts on various aspects of the environment, economy, and community, particularly within the County of Lake.

## II. COUNTY OF LAKE'S ARGUMENTS AGAINST PG&E'S PROPOSED SURRENDER, DECOMMISSIONING, AND NON-PROJECT USE OF PROJECT LANDS AND COMMENTS ON THE SCOPE OF ISSUES TO ADDRESS IN THE NEPA DOCUMENT

### 1. THE SCOPING PROCESS SHOULD TAKE INTO ACCOUNT PG&E'S MISREPRESENTATIONS AND INACCURACIES IN ITS APPLICATION.

In filings related to this proceeding, PG&E has repeatedly made misleading statements; the veracity and credibility of claims made in this Final Application should therefore be closely scrutinized. As recently as 2023, PG&E filed requests to leave the gates of Scott Dam open as an interim risk reduction measure due to seismic stability concerns (*See Attachment 3* – SLR *Comments on PG&E's Final Application for Surrender of License and Application for Non-Project Use of Project Lands*, Section 3.1, at 0528-0530). However, a more recent seismic report from the California Department of Water Resources confirmed that Scott Dam "reservoir, and the appurtenances are

judged safe for continued use (*See Attachment 4* – Division of Safety of Dams - *Inspection of Dam and Reservoir in Certified Status* (October 18, 2023), at 0545-0555).

The Application also miscalculates (by approximately $765,000) the amount of property taxes collected in connection with the Lake Pillsbury area (*PG&E Final Application for Surrender of License and Application for Non-Project Use of Project Lands,* Volume II, at 2.2.4.14-16 (FERC Docket No. P-77-000, July 25, 2025); *See Attachment 3*, Section 3.13, at 0539-0540). This exemplifies PG&E's recurrent tendency to characterize substantial negative impacts to Lake County as "negligible." For example, PG&E states:

> Dividing by the estimated 34,385 housing units in the county (U.S. Census Bureau 2023) translates to approximately $253,000 in assessed value per housing unit across the entire county. This value includes land and housing improvements; the taxable value of much of the housing at Lake Pillsbury reservoir is just for the housing improvement as most of the land is owned by USFS and is not taxable. The property tax rate in Lake County varies from 1.0 to 1.112 percent (Herrington 2023) or up to approximately $285 annually per home. This indicates that county property taxes paid by the 300 homes in Lake County may be at most approximately $85,000 annually, or less than 0.1 percent of property tax collections (totaling over $101.3 million) in Lake County in fiscal year 2022–2023. Removal of Project infrastructure would also reduce property tax revenues associated with improvements located on Project lands. In fiscal year 2022–2023, the property tax for the Project was $202,536.

(*PG&E Final Application for Surrender of License and Application for Non-Project Use of Project Lands*, Volume II, (FERC Docket No. P-77-000, July 2025), at 2.2.4.14-26).

These estimates are significantly lower than those of the Lake County Treasurer-Tax Collector's office; County officials have repeatedly raised this issue with PG&E. In their draft application, PG&E's estimate did not even include the $202,536 in property taxes PG&E paid on the Potter Valley Project. In their final application, errors persist that

misrepresent the impact to Lake County. Even using PG&E's own assumption that the average home has an assessed value of $253,000, with a property tax rate of 1.0 to 1.112 percent, the property tax paid is not $285 per home as PG&E states. A $253,000 home at a 1.112 percent rate would require payment of approximately 10 times that amount, or $2,813.

This also means PG&E's assertion "county property taxes paid by the 300 homes in Lake County may be at most approximately $85,000 annually, or less than 0.1 percent of property tax collections," is also off by a factor of 10, meaning the described loss could be approximately $844,000. Coupled with the $202,536 PG&E in property tax PG&E acknowledges having paid, that puts the potential loss in property tax alone at over $1 million per year. That can hardly be characterized as a negligible impact to Lake County. PG&E's persistent inclusion of these known inaccurate calculations underscores the need for your Commission to robustly scrutinize conclusions drawn throughout their Final Application - truly, it would be most appropriate to entirely reject the application as insufficiently considered and incomplete.

When PG&E submitted its Draft Application for comment, Lake County and SLR each expressed concerns over the methodology used by PG&E to find, "Implementation of the Proposed Action is not expected to result in disproportionate or adverse impacts to [Environmental Justice] communities in the study area or surrounding region." (*PG&E Final Application for Surrender of License and Application for Non-Project Use of Project Lands* (FERC Docket No. P-77-000, July 2025), at 3.4.1.15-5). Rather than addressing these concerns, PG&E now appears to

have entirely omitted consideration of County of Lake-focused Environmental Justice implications.

Furthermore, the Application makes only conclusory attempts to address environmental impacts. One glaring example of PG&E's approach is found in its discussion of water sedimentation. Acknowledging the release of sediment from Scott Dam removal would increase, "turbidity and suspended sediment and adversely impact water quality parameters that would affect environmental and cultural resources below Scott Dam," while refusing any mitigating measures as these, "would not meet PG&E's interest to remove the dams as quickly as possible," is wholly inadequate to resolve this significant environmental concern (*PG&E Final Application for Surrender of License and Application for Non-Project Use of Project Lands*, Volume 1A (FERC Docket No. P-77-000, July 25 2025), at 5-11). In contrast, SLR's report provides measures to mitigate turbidity and water quality concerns surrounding the proposed removal of Scott Dam (*See Attachment 3,* Sections 3.1 - 3.2, at 0528-0530).

Significant effects on air quality could likewise very reasonably result if decommissioning proceeds as envisioned by PG&E, including from naturally occurring asbestos emissions and construction materials within the Scott Dam structure; this should also be considered in support of denying the Application (*See Attachment 3, Section* 3.6, at 0531-0533).

The potential loss of water supply to fight wildfires remains a significant concern (*See Attachment 3,* Section 3.8, at 0534-0537). Lake County residents have repeatedly faced the horrors of wildfires and more than seventy percent (70%) of the County's land mass has been impacted by wildfire in the past decade, alone. Yet, PG&E

proposes to significantly reduce a critical resource (i.e., water for fire suppression) that can extinguish these horrors. *Id*. PG&E's Application provides no comparably reliable alternative sources for water, and it is undisputed that even indefinitely leaving the spillway gates open would significantly reduce the storage capacity of Lake Pillsbury (*See Attachment 3,* Section 3.1, at 0528-0530). Any amount of increased wildfire risk associated with PG&E's proposed decommissioning should not be tolerated in drought-prone Northern California.

Robust environmental review and development of a detailed and objective Environmental Impact Statement are essential to responsible decision-making in this proceeding; Decommissioning of Scott Dam and the Potter Valley Project carries the potential to throw water supply into question for hundreds of thousands of Californians, increase the probability of mega fires with profound regional consequences, and leave portions of Lake County in a state of environmental disaster.

As members of the Commission will be aware, billions in federal funds have already been invested to secure California's water supply through the Sites Reservoir and "Central Valley Projects." Creation of another "water desert" in California would heavily strain taxpayer resources and frustrate efforts to make essentials more affordable for Americans. At least 600,000 Californians currently access vital household and agricultural water facilitated by the Potter Valley Project (and some estimates have ranged as high as 750,000). Common understanding must be secure (*among all affected constituents*) regarding the long-term viability of any proposed alternative for regional water supply prior to *considering* any Order to Decommission Scott Dam and other Potter Valley Project infrastructure.

In light of the foregoing and for purposes of this scoping process, all relevant

environmental and socioeconomic factors must be independently reviewed, including, but not limited to the following:

- Implications of fractured rock and gravel in the Pillsbury basin for groundwater must be well understood. Drought conditions with Scott Dam in place have already severely affected local wells; were Lake Pillsbury gone, many would go dry.
- As SLR reports previously submitted by Lake County (and attached hereto) demonstrate, Scott Dam may presently be stabilizing a hillside; the probability of landslides needs to be better understood.
- All hydrology, bathymetric and other relevant data sets should be made publicly available, as well, so assumptions can be fully investigated; PG&E has been reluctant to make this data accessible to all interested parties.
- Lake County's Air Pollution Control Officer has indicated PG&E's structures may contain asbestos, and serpentine soils in the project area lend further cause for air quality concern, as just two examples. PG&E has not engaged our Air Quality team to respond to these and other concerns, to date.
- The quality of any water ultimately released and/or diverted, should the Potter Valley Project be removed is also profoundly consequential. Management of sediment, concrete, and other materials associated with removal of Scott Dam would constitute the single largest disposal project in Lake County's history, yet PG&E has not engaged in substantive planning discussions with Lake County officials. PG&E has instead consistently underestimated costs associated with sediment control, and a robust sediment and vegetation management plan could reach $200-500 million.

- Frequent lightning-caused fires near the current Lake Pillsbury inundation area could grow to larger wildfire events, over-burdening emergency services resources in an area that has already seen some of the largest megafires in California's history. United States Forest Service and Lake Pillsbury Fire Protection District leadership well understand how critical water supply from Lake Pillsbury has been to fire suppression in wildfire events large and small; robust environmental review must include input from those agencies that are most directly involved in wildfire response in the area. On or about March 1,2025, the Lake Pillsbury Fire Protection District expressed its opposition towards PG&E's claim that Lake Pillsbury water may be adequately substituted with water from Clear Lake or Lake Mendocino for fire suppression.

Allowing decommissioning to proceed without robust environmental review could invite avoidable and expensive environmental disasters affecting residents of Lake County and other neighboring counties.

2. PG&E'S SURRENDER APPLICATION HAS FAILED TO COMPLY WITH THE REQUIREMENTS OF 18 C.F.R. SECTION 6.1.

18 C.F.R. Section 6.1 expressly states:

> Every application for surrender of a license shall state the reason therefor; and, except in the case of an application for surrender of a license for a minor project, or for a transmission line only, shall be executed by the licensee and filed in the same form and manner as the application for license, and shall be accompanied by the license and all amendments thereof.

PG&E did not include copies (electronic or otherwise in Volumes IA, IB, and all parts of Volume II) of the initial license and all amendments in its Application. In Volumes IA and IB, PG&E merely cites the following in its References section:

3. FERC (Federal Energy Regulatory Commission). 2004. Order amending license "January 28, 1983. Opinion and Order Denying Appeal, Approving Settlement,

and Issuing New License. October 4.

4. "NMFS (National Marine Fisheries Service). 2002. Biological Opinion for the Proposed License Amendment for the Potter Valley Project (FERC Project No. 77-110). Southwest Region. November 26.

The documents cited in both of these references were not included in the set of Application documents filed with FERC. Furthermore, these references remain insufficient to cover "the license and all amendments thereof" required to be included under 18 C.F.R. Section 6.1.

3. PG&E HAS FAILED TO MEANINGFULLY CONSULT IN GOOD FAITH WITH THE COUNTY OF LAKE IN MATTERS RELATED TO THE PROPOSED DECOMMISSIONING OF SCOTT DAM AS REQUIRED BY THIS COMMISSION'S ORDER IN THE JULY 29, 2022 LETTER.

This Commission's July 29, 2022 letter unequivocally required any surrender application filed by PG&E to include a "documentation of consultation with relevant federal and state agencies, Tribes, and other interested parties."

Since the service date of PG&E's initial draft application for the proposed decommissioning of the Potter Valley Project, the County of Lake and PG&E met approximately three (3) times in a span of five (5) months, with the last meeting done at the request of U.S. Congressman Mike Thompson. It was clear from the first meeting that PG&E representatives construed "consultation" to mean only appearing at these meetings. In each of these sporadic meetings, PG&E repeatedly expressed that FERC's approval of its Application was inevitable, with no regard over any good faith meaningful discussions of the economic and environmental effects of decommissioning the Project.

PG&E has kept its pattern of excluding Lake County from any substantive

discussions to date. To date, PG&E has not requested to have any of their scoping meetings in Lake County, opting instead to hold both meetings in Mendocino County, despite Lake Pillsbury and Scott Dam being located in Lake County, California. As part of its NEPA review process, this Commission should take into account PG&E's constant disregard and exclusion of Lake County and its affected residents (spanning several years), related to Potter Valley Project.

PG&E's attempt to fast-track this Application has resulted in its intentional disregard of the economic and environmental concerns raised by the County of Lake during these few meetings. Each inquiry related to these concerns raised by the County of Lake has been ignored or outright dismissed by PG&E. In light of the foregoing and this Commission's Order, PG&E's documentation of these "consultations" are a gross misrepresentation of what actually occurred.

4. PG&E'S POSITION THAT DECOMMISSIONING IS AN INEVITABLE RESULT CHALLENGES THIS COMMISSION'S AUTHORITY TO DENY DAM REMOVAL PROPOSALS.

It's clear from their repetitive use of the phrase "unavoidable adverse effects" in the Application that PG&E does not duly consider the potential and likely environmental effects of the proposed decommissioning of the Potter Valley Project; PG&E's Final Application references "unavoidable adverse effects" in a greater number of environmental categories than earlier drafts. By characterizing these effects as "unavoidable," PG&E attempts to predetermine conclusions of environmental review prior to submitting any formal project environmental application, as required under the National Environmental Protection Act. This is not at all surprising in light of PG&E's desire to fast track any and all approvals in the decommissioning process, even

if it means making erroneous presumptions in its Application. Your Commission must ensure appropriate and robust environmental review occurs.

In previous licensing proceedings, PG&E advocated that FERC consider the regional benefits of water storage and water diversion facilitated by the Potter Valley Project, as well as other environmental benefits. PG&E successfully lobbied in comments to FERC for inclusion of expanded language in the Final Environmental Impact Statement's "project purpose," stating water from the Project contributes significantly to summer recreation flows in the Russian River, and, "the diversion of Eel River water into the Russian River has been responsible to a large measure for extensive resort developments along the Russian River that would not otherwise be possible." (*See Attachment 1 – Pacific Gas and Electric Company – FERC Project 77, Potter Valley, Comments on Staff Draft Environmental Impact Statement* – Section 1.1 - July 12, 1978, at 0002)

From the early stages of its participation, the County of Lake has been consistent in challenging the inevitability of decommissioning the Potter Valley Project. The Application describes only two (2) possible scenarios: (1) a "No-Action Alternative" which retains the existing Project (*which "Scoping Document 1" signaled was* not *"a reasonable alternative," despite substantive concerns remaining surrounding environmental impacts of PG&E's proposed approach to Decommissioning, as discussed herein and demonstrated in the attachments hereto*); or (2) approval of the Application and the proposed decommissioning plan (*Final Application for Surrender of License and Application for Non-Project Use of Project Lands*, at Section 2.1 (FERC Docket No. P-77-000, July 25, 2025)). This limited view fails to account for other viable scenarios as

outlined in the attached report submitted by SLR, who conducted an extensive study and comprehensive evaluation aimed at assessing the potential impacts of decommissioning Scott Dam and draining Lake Pillsbury on various resources and stakeholders in the surrounding community (*See Attachment 2 – SLR Impact Analysis and Adaptation Strategy Evaluation Report*, at 0011-0519). SLR's report goes beyond PG&E's limited scope in outlining five (5) further alternative scenarios that would address and mitigate these "unavoidable adverse effects":

1. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
2. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
3. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
4. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
5. Scenario E describes the removal of Scott Dam and installation of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

(*See Attachment 2*, Sections 3.0 – 4.2.3, at 0032-0058).

Each of these alternative scenarios are discussed in greater detail in SLR's report and should be taken into consideration by this Commission prior to entering any decision on the Application. *Id*. Furthermore, any scope of issues in the NEPA document should require PG&E to submit models that show how each of these alternative scenarios would have an effect on the Potter Valley Project over time. Such models should include the following:

1. Renderings of downstream water flows of Scott Dam and Lake Pillsbury if FERC were to approve the Application;

2. Bathymetry models of Lake Pillsbury existing conditions and pre-dam conditions;

3. Conceptual renderings of each alternative scenario showing water levels and bathymetry / topography from the baseline;

4. Sediment renderings showing four (4) sediment layers from the 2023 bathymetry baseline.

PG&E's unwillingness to consider and provide these hydrological models for any of these alternative scenarios in its Application underscores its preference to ignore important environmental considerations under NEPA.

Presumption by PG&E that decommissioning is inevitable underappreciates the critical role your Commission plays in ensuring a positive outcome for all Americans. Contrary to PG&E's assertions and insinuations, your Commission has the discretion to find dam removal is not feasible, as it has done in prior proceedings. On January 14, 2025, the United States Court of Appeals, District of Columbia Circuit, in *American Whitewater v. FERC,* 125 F. 4th 1139 (D.C. Cir 2025) (Case No. 23-1291), found your Commission had acted appropriately in ordering that Salmon Falls River dams remain in place. In *Whitewater*, your Commission found Stone Dam's reservoir was a source of water supply for Somersworth and Berwick and used for local firefighting, and its removal could affect local infrastructure; FERC reasonably denied the request for dam removal, determining removal of the dams was not feasible. In

that ruling, the Court held FERC reasonably determined removal of Stone Dam was unfeasible because local municipalities relied on the reservoir for key water supply and fire suppression needs; differently stated, FERC appropriately rejected dam removal because known and immediate benefits to local municipalities of keeping the dams outweighed theoretical benefits to the environment and recreation from removal. *Id*. This case established precedent for scenarios where public interests, namely water supply, water storage and fire suppression benefits, could outweigh speculated future environmental benefits of dam removal.

In the case of this particular application, a strict Conservation Easement adds complexity to your Commission's review, and consideration of whether dam removal is viable. At their most recent meeting with the County of Lake, held April 16, 2025, PG&E representatives indicated they would provide a response to the County's concerns over the Conservation Easement between PG&E and Mendocino Land Trust that wholly restricts most types of development in and around Lake Pillsbury. PG&E provided no substantive resolution to concerns surrounding the Conservation Easement, leaving the County (*and other interested parties*) unable to reliably plan for responsible restoration of the current Lake Pillsbury inundation area, should decommissioning proceed. In PG&E's Final Application, they state the following: "The removal of FERC jurisdictional boundaries means the lands would no longer be under FERC jurisdiction and thus not be subject to the terms and conditions of the FERC license. Potential impacts to changes to land use associated with the removal of FERC jurisdictional boundaries are considered negligible" (*PG&E Final Application for Surrender of License and Application for Non-Project Use of Project Lands*, Volume II, at

2.2.4.9-6 (FERC Docket P-77-000, July 25, 2025)). In numerous discussions with stakeholders in the Upper Eel, including the CalWild-facilitated visioning process, there has been near universal agreement the Conservation Easements present serious complications to restoration and recreational opportunities. Plainly, impacts are far from negligible, and this statement, alone, should cast serious doubt on the transparency and reliability of PG&E's application.

Particularly given approval would terminate FERC's jurisdiction over the easement area, it is critical any potential issues associated with the Conservation Easement are addressed in FERC's Surrender Order; this appears to be the surest path to ensure any substantive proposals for restoration and adaptation of the area to facilitate recreational opportunity would not violate this Conservation Easement. Undertaking this work now, and ensuring environmental review is sufficient to consider all implications, is critical. Even if short-term funding, for example, were provided to make immediate improvements, any trail systems, camping grounds, river access points (and/or fire response infrastructure, for that matter) will need to be supported by sustainable revenues, and understanding these burdens (much less mitigating them) will not be possible without robust environmental review, documentation of mitigations in FERC's Surrender Order, and sufficient coordination to allow for adequate and reliable planning.

On January 24, 2025, President Trump signed Executive Order 14181: "Emergency Measures To Provide Water Resources in California and Improve Disaster Response in Certain Areas." (https://www.federalregister.gov/documents/2025/01/31/2025-02174/emergency-measures-to-provide-water-resources-in-california-and-improve-disaster-response-in-certain). In this Executive Order, President Trump requests the

Secretary of the Interior and the Secretary of Commerce, "immediately take actions to override existing activities that unduly burden efforts to maximize water deliveries." By approving Decommissioning of Scott Dam, FERC would "unduly burden" many communities that rely on Lake Pillsbury, meaningfully affecting water supply reliability and sufficiency for agricultural and other purposes, including water flows for fire suppression.

This Executive Order continues by highlighting the need to, "deliver more water and produce additional hydropower, including by increasing storage and conveyance . . . to high-need communities." While this statement is specific to the Central Valley Projects, parallels are concerning surrounding the current needs of Southern California and the constant needs of Lake County and Northern California where large and devastating wildfires have affected our communities for the past decade. Lake County, alone, has endured more than 10 devastating wildfires in that timeframe, with many receiving State and Federal Emergency Declarations. Reducing water storage is not what California needs, and could be truly catastrophic in Lake County; especially in the lightning-prone areas surrounding Lake Pillsbury.

On January 20, 2025, President Trump signed Executive Order 14156: "Declaring a National Energy Emergency." https://www.govinfo.gov/content/pkg/DCPD- 202500123/html/DCPD-202500123.htm). In this Executive Order, President Trump requests an increase to our supply of energy, stating our "generation capacity of the United States are all far too inadequate to meet our Nation's needs." The Executive Order, in Section 7, speaks to "the Department of Defense's ability to acquire and transport the energy needed to protect the homeland . . .

with a focus on such vulnerabilities within the Northeast and West Coast regions of the United States."

The State of California likewise issued its own Executive Order related to the preservation of water. On January 31, 2025, California Governor Gavin Newsom signed Executive Order N-16-25, directing the Department of Water Resources, the State Water Resources Control Board, the Natural Resources Agency, and the Environmental Protection Agency to maximize storage of excess flows due to winter storms (https://www.gov.ca.gov/wp-content/uploads/2025/01/Executive-Order-N-16-25- FINAL.pdf). In light of this Executive Order, the County recognized the potential loss of Scott Dam, the headwaters of the Eel River, would be detrimental to public safety and water supply security in a fire-prone region.

The U.S. Department of Agriculture also filed its own comments in this proceeding requesting that FERC reject PG&E's Application "because of the profoundly negative and irreversible impact on local farmers, ranchers, agricultural producers, communities, and USDA equities." *See USDA Comments on the Federal Energy Regulatory Commission (FERC) Notice of Application for Surrender of License and Non-Project Use of Project Lands, Accepted for Filing, Soliciting Comments, Motions to Intervene, and Protests; Project Number 77-332* (December 19, 2025).

For each "unavoidable adverse effect" referenced in an environmental category discussed in PG&E's Application, mitigating measures and alternative scenarios discussed in SLR's report should be considered by FERC. If this Commission grants weight to PG&E's Application (despite its inaccuracies and misrepresentations), then this Commission should also consider mitigating measures and the aforementioned five (5)

alternative scenarios described by SLR.

Should Decommissioning of the Potter Valley Project facilities proceed despite substantial concerns raised by the County of Lake and other interested parties, mitigation measures must be undertaken to reduce the potential for environmental disaster. Your Commission has the regulatory authority to order PG&E to explore these alternative mitigation scenarios and incorporate them in its Application for full consideration.

5. INDEPENDENT ASSESSMENT OF COSTS FOR PROPOSED SCOTT DAM DECOMMISSIONING DEMONTRATES SIGNIFICANT RATEPAYER BURDEN

The enclosed independent cost-analysis study conducted by SLR reveals the total estimated cost in 2025 dollars (using PG&E's own assumptions and incomplete information) is approximately $179 million, with a potential accuracy range from $143 million to $358 million, depending on actual site conditions and implementation challenges. *See Attachment 5 – SLR Technical Memorandum Re: Cost Estimate Scott Dam Rapid Removal; Potter Valley Hydroelectric Project, FERC Project No. 77; The County of Lake Resource Assessment, Impact Analysis, and adaptation Strategy Evaluation Project* (November 24, 2025), at 0557-0572. Indirect costs such as, general conditions, mobilization, overhead, profit, engineering, project oversight, bonding, and sales tax are applied to the direct construction costs. *Id*. In SLR's study, the offsite disposal of debris and sediment constitutes the most significant potential cost escalation factor, primarily due to the considerable volume of material requiring removal. *Attachment 5,* Table 1, at 0563-0564.

The aforementioned estimate excludes costs for permitting, engineering design,

legal services, and administrative overhead, focusing solely on construction-related expenses with respect to Scott Dam demolition. *Attachment 5*, at 0561. This estimate also excludes potential lost revenues to the County and other parties. *Id*. To date, PG&E has not properly published a total cost estimate for the proposed removal of Scott Dam. PG&E makes no attempt to explore any alternatives to the decommissioning plan that would mitigate these costs, and it has effectively acknowledged that ratepayers shall bear the costs on and after the completion of the decommissioning process. *Id*.

## III. CONCLUSION

PG&E does not desire to continue operating Scott Dam, and is urging your Commission to expedite the decommissioning process at the expense of the other parties to this docket proceeding. By PG&E's own characterization, if this Commission approves the Application, there will be "unavoidable adverse effects" to Lake County's infrastructure, economy, and public safety. Clearly, PG&E has not demonstrated dam removal is a feasible alternative in the decommissioning process; the County of Lake staunchly opposes this plan. Further, the County of Lake shares substantive technical concerns documented by SLR in its report (*See Attachment 3*, at 0521-0543).

Allowing this process to proceed without requiring PG&E to first more fully attend to substantial risks associated with its proposal would set a dangerous precedent. Your Commission must not allow the financial interests of a publicly-regulated utility to be prioritized to the detriment of the broader interests of the people of Lake County, and the 600,000+ Californians who rely on the Potter Valley Project for water, as just

one example.

Further discussion and salient concerns are presented in the attached County of Lake Comments submitted to the State Water Resources Control Board *(See Attachment* 6 – Potter Valley NOP Comments – the County of Lake (November 3, 2025), which likewise includes highly detailed SLR reports and other supportive materials of interest to your Commission. This important collection of comments and documents is likewise accessible here: https://www.lakecountyca.gov/DocumentCenter/View/15333/Potter-Valley-NOP-Comments-of-the-County-of-Lake

Alternatives and mitigating measures must be considered prior to this Commission rendering a determination. Therefore, in consideration of the foregoing, PG&E's Application should be deemed incomplete and denied.

As part of this scoping process, we ask that all of the County's substantive concerns (including those contained in the State Water Resources Control Board comments and other attachments hereto) be nonetheless considered and responded to as Comments pursuant to your Commission's solicitation of Public Comments.

Dated: July 24, 2026

Respectfully submitted,

_____

Lloyd C. Guintivano
County Counsel
County of Lake

CERTIFICATE OF SERVICE

I hereby certify that Comments of the County of Lake has been served upon each person designated on the attached official P-77-000 service list compiled by the Commission in this proceeding via electronic mail or first-class mail.

Dated: July 24, 2026

_____

Lloyd Guintivano,
County Counsel
County of Lake
255 N. Forbes Street
Lakeport, CA. 95453
Telephone: 707-263-2321
Lloyd.Guintivano@lakecountyca.gov

# ATTACHMENT 1

PACIFIC GAS AND ELECTRIC COMPANY

PG&E + 77 BEALE STREET • SAN FRANCISCO, CALIFORNIA 94106 • (415) 781-4211 • TWX 910 372-6587

W. M. GALLAVAN
VICE PRESIDENT - RATES AND VALUATION

RECEIVED
JUL 24 1978
DIVISION OF PUBLIC UTILITIES

July 12, 1978

RECEIVED
JUL 21 1978
OFFICE OF ELECTRIC POWER REGULATION
OFFICIAL MAIL

Mr. Kenneth F. Plumb
Secretary
Federal Energy Regulatory Commission
825 North Capitol Street, N.E.
Washington, D. C. 20426

| TO | INIT. | DATE |
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| DOCKET | CJ | 7-14 |
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FEDERAL POWER COMMISSION
14 10 33 AM '78

Dear Mr. Plumb:

FERC Project 77, Potter Valley
Comments on Staff Draft
Environmental Impact Statement

Pacific Gas and Electric Company (PGandE), the Applicant for New License for the Potter Valley Project (FERC 77), hereby submits its comments on the FERC's Draft Environmental Impact Statement (DEIS).

Section 1.1 - Purpose - Page 1-1, Paragraph 4

The paragraph discusses beneficial uses of the water after it is utilized by the Powerplant and flows into Lake Mendocino. PGandE believes the statement should be expanded to include the contribution the project makes to summer recreation flows along the main stem of the Russian River. The diversion of Eel River water into the Russian River has been responsible to a large measure for extensive resort developments along the Russian River that would not otherwise be possible.

This information has been noted i the FEIS.

For further disucssion refer to Section 2.5.

In addition, the paragraph states that water releases into the Eel River are maintained at 2 cfs. It should be noted that this release is below Van Arsdale Reservoir. On many occasions the California Department of Fish and Game has made special requests for higher flow releases from the reservoir during fish migration periods. PGandE has always provided the requested releases. Releases from Scott Dam are much higher; a minimum of about 20 cfs is released continuously.

This point has been clarified in the FEIS.

Section 1.3 - Land Requirements - Page 1-2

The paragraph states that there are a total of 3,577.71 acres in the project and that PGandE owns 2,488.09 acres. These acreages were shown in Exhibit F of our Application for New License, dated April 29, 1970. On February 17, 1978, we filed an Amendment to our Application, deleting 52.23 acres of our land. The correct acreages should be a total of 3,525.48 acres with PGandE owning 2,435.86 acres. See also page 2-1 (Paragraph 2) and 3-1, Section 3.1.2.

Acreages have been corrected in the FEIS.

FEDERAL ENERGY REGULATORY COMMISSION
DOCKETED
JUL 14 1978



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Section 1.4.1 - Dams and Reservoirs - Page 1-4

The area-capacity curve for Van Arsdale Reservoir should show the normal maximum water surface as the top of the flashboards, elevation 1494.3. The water surface elevation 1490.3 should be labeled, "Spillway Crest." Please refer to the revised Exhibit K-5 filed on February 17, 1978. See also Page 16 (Paragraph 1).

These corrections have been made in the FEIS.

Section 1.4.5 - Traveling Fish Screen - Page 1-14

The description of the fish screen is what was originally installed. However, the traveling screen had continuous operating and maintenance problems resulting in costly repairs. In 1975, the screen was temporarily taken out of service and the California Department of Fish and Game placed louvers in front of a portion of the screen and removed the screen panels immediately behind the louvers, creating a stationary louver-type fish screen. The louvers were installed on an experimental basis. The louver-type screen was tested in the spring of 1976; efficiency of the screen was found to be high at diversion flows of 70 cfs. The screen will be tested at higher flows in 1978.

See revised Section 1.4.5.

Section 1.4.6 - Operation Procedures - Page 1-17, Paragraph 3

When inflow to Van Arsdale Reservoir exceeds the capacity of the diversion, the excess water flows downstream past the reservoir. Streamflows as low as 2 cfs released below the dam normally occur only during summer and fall months.

See revised Section 1.4.6.

Section 1.5 - Proposed Facilities and Construction Procedures Page 1-18

Exhibit R is presently being revised in cooperation with the U. S. Forest Service.

The facilities along the Eel River were intended to be parking spurs for self-contained vehicles which would not require public sanitary facilities. A trash can would, however, be provided.

The Exhibit R (Page 6) specifically indicated that portable sanitary facilities are being placed at key points along the Eel River as the need arises.

Section 1.6 - Project Operational Agreement - Page 1-19, Paragraph 2

The penultimate sentence states that PVID also diverts additional quantities of water from the East Fork Russian River. PVID diverts and/or pumps water from the waterway below the Powerhouse which includes the tailrace canal, Adobe Creek, and East Fork Russian River.

This information has been included in Section 1.6.

Section 2.1 - Land Features and Uses - Page 2-3, Paragraph 2

The area's regional transportation facilities were greatly enhanced by bridge construction and the widening and general improvement of the Forest Service Road from the mill at Van Arsdale Reservoir to Lake Pillsbury.

This is reflected in the FEIS.



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Originally the road was restricted to logging trucks, but recently the road has been opened to the public. This road has made travel by recreation vehicle and/or vehicles towing trailers considerably easier and, thus, increased recreation use at the lake.

Section 2.4.2 - Employment and Earnings - Pages 2-18 and 2-21

The paragraphs under the subheadings of Agriculture, Manufacturing, Outdoor Recreation and Tourism, and Retirement and Vacation Housing use the term "Project Area." We feel this is misleading in that the areas described encompass a region whereas the "Project Area" is defined as those lands located within the project boundary.

Refer to Section 2.4.1 for definition of project area as used in the FEIS.

Section 2.5.1 - Recreational Supply - Page 2-26, Paragraph 2

Land capability for recreational development is not substantiated. The USFS and PGandE are curtently studying variables which affect the capability of the area to support recreational development.

This is clarified in the FEIS.

Section 2.5.1 - Recreational Supply - Page 2-27

Table 2-15 states it is a Recreation inventory. However, the Otter Flat area is not yet developed and the existing Trout Creek Campground is not listed.

Table 2-15 has been amended to show that Otter Flat is an undeveloped overflow camping area and that Trout Creek Campground has been added to the list.

Section 2.5.1 - Recreational Supply - Page 2-31, Paragraph 2

The Potter Valley Project diverts an average of 154,700 acre-feet of water annually, as noted in Appendix A-1.

Correction made in the FEIS.

Section 2.6.4 - Geologic Hazards - Page 2-36, Paragraph 2

The November 22, 1977 earthquake referred to had its epicenter about 15 miles west of Scott Dam. The peak recording accelerograph in the dam gallery recorded 0.075g.

Refer to revised Section 2.6.4.

Section 2.10.2 - Fish Communities - Page 2-47, Paragraph 2

According to Lee and Baker (1975), the following anadromous game species are present in the Eel River system: steelhead trout, chinook salmon, coho salmon, coastal cutthroat trout, American shad, and green sturgeon. The following anadromous game species are present in the Russian River system: steelhead trout, chinook salmon, coho salmon, American shad, striped bass, white sturgeon, and green sturgeon.

This correction has been made in Section 2.10.2.

Section 2.10.2 - Fish Communities - Page 2-48, Paragraph 2

Sentence 3: Anderson (1972) estimated that about 6,200 chinook salmon and 11,800 steelhead trout spawn annually in the main stem and tributaries of the Eel River in that portion of the drainage above the English Ridge dam site which is located 18 miles downstream of Cape Horn Dam. This reach of stream is approximately thirty miles in length.

This correction has been made in Section 2.10.2



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| Sentences 6 and 7: Coho salmon rarely occur in the main stem above the confluence with the North Fork, which is located approximately sixty miles downstream from Van Arsdale Reservoir. | This correction has been made in Section 2.10.2. |
| Section 2.10.2 - Fish Communities - Page 2-48, Paragraph 3 | |
| Day (1968) also reported that river lamprey (Lamptera ayresii) reach Van Arsdale Dam. | This information has been included in Section 2.10.2. |
| Section 2.10.2 - Fish Communities - Page 2-48, Paragraph 4 | |
| Adult steelhead trout that migrate to the upper Eel River basin begin entering the river in late October and November, and they may continue to enter through the winter. | This information has been included in Section 2.10.2. |
| Section 2.10.2 - Fish Communities - Page 2-49, Paragraph 1 | |
| Those steelhead fry that are spawned in the Eel River remain in the river after emergence. Juveniles remain in freshwater up to four years. | Refer to Lee and Baker (1975). |
| Section 2.10.2 - Fish Communities - Page 2-49, Paragraph 3 | |
| Sentence 3: The mouth of Outlet Creek is located approximately thirty miles downstream from Van Arsdale Reservoir. | This correction has been made in Section 2.10.2. |
| Sentence 5: If adequate flows do not occur, chinook salmon will attempt to spawn elsewhere. | This information has been included. |
| Section 2.10.2 - Fish Communities - Page 2-50, Paragraph 3 | |
| It is more correct to state that chinook salmon spawn in the main stem upstream from Hopland. | This statement has been included. |
| Section 2.10.2 - Fish Communities - Page 2-50, Paragraph 4 | |
| It is stated that 20 resident fish species have been sampled in Lake Pillsbury, Lake Mendocino, the Eel River from Lake Pillsbury to the mouth of Outlet Creek, and the East Fork of the Russian River above Lake Mendocino; however, not all 20 species are mentioned in this and the succeeding paragraphs. It would be helpful to list in table format the species sampled in each area. | This portion of Section 2.10.2 has been revised. |
| Golden shiners are not native. They were introduced accidently while being used as bait. California roach may be native to both basins. | Status of these two species has been revised in Section 2.10.2. |
| Section 2.10.2 - Fish Communities - Page 2-50, Paragraph 5 | |
| The number of catchable-size rainbow trout planted in Lake Pillsbury each year is usually greater than 15,500. The average annual planting allotment from 1968 through 1977 was 23,490. The number planted in both 1976 and 1977 was 25,000. | This correction has been made in Section 2.10.2. |



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Section 2.10.2 - Fish Communities - Page 2-51, Paragraph 2

It should also be mentioned that Lake Mendocino supports an important catfish fishery, consisting of white catfish, Ictalurus catus, and brown bullhead, Ictalurus nebulosus.

This information has been included in Section 2.10.2.

Section 2.12 - Historic and Archeological Features - Pages 2-56 and 2-57, Figures 2-8 and 2-9

We question the need to display archeological site location maps in a public document. It is PGandE's practice to not publicize such site location maps in order to reduce the probability of illegal "pot-hunting".

These two figures have been deleted from the FEIS.

Section 3.1.2 - Operation and Maintenance - Page 3-2, Paragraph 5

Environmental effects of the proposed action are not limited to specific sites as suggested here. Development of formalized recreational facilities provide accommodations to facilitate use of general recreational activity zones. Therefore, some environmental effects can be anticipated in association with various activity zones, i.e., shoreline, erosion and noise from boaters, and slope erosion, noise, and wildlife displacement by ORV users (as referenced on page 3-14, paragraph 3.)

This section has been revised to indicate broader impacts to the project recreational lands and water affected.

Section 3.8 - Plants and Plant Communities - Page 3-17, Paragraph 2

In 1978, Mendocino National Forest conducted a rare plant survey of potential recreation development sites. Exhibit R will give consideration to the survey.

No response required.

Section 3.9.1 - Wildlife - Page 3-17, Paragraph 2

Mendocino National Forest has developed a draft Management Plan for the Bald Eagle Nesting Site at Lake Pillsbury. The Recreation Plan will give consideration to the management plan.

No response required.

Section 3.9.2 - Fisheries - Page 3-18, Paragraph 1

Diversions from the Eel River also supply water to Lake Mendocino, providing habitat for populations of striped bass and several other warmwater game species.

This information has been included in Section 3.9.2.

Section 3.9.2 - Fisheries - Page 3-18, Paragraph 4

According to Anderson (1972), chinook salmon which are forced to congregate at the mouth of Tomki Creek due to inadequate streamflow seek suitable spawning areas downstream or in the Tomki Creek drainage.

This information has been included in Section 3.9.2.



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| Section 3.9.2 - Fisheries - Page 3-20, Paragraph 2 | |
| Sentence 4: It would be more correct to state: As flows declined after the spill, the fish either were stranded in the deeper pools, where they perished if the spill did not resume in time to allow their continued upstream migration, or they retreated to the mouth of Tomki Creek. | Same as stated in the FEIS. |
| Section 3.9.2 - Fisheries - Page 3-20, Paragraph 3 | |
| The percentages of available spawning area reported by Anderson (1972) are for a single sampling area located 20 miles downstream from Cape Horn Dam and should not be considered to represent the entire stream section between Tomki and Outlet Creeks. Much of this section consists of long, deep pools which cannot be used for spawning. The percentages listed represent only areas that are adequate for spawning. | This point has been clarified in the FEIS. |
| Section 3.9.2 - Fisheries - Page 3-21, Paragraph 3 | |
| Sentence 3: Temperatures are also satisfactory for trout in areas of cool spring or tributary inflow. | This point has been noted in Section 3.9.2. |
| Section 3.9.2 - Fisheries - Page 3-21, Paragraph 4 | |
| Please see our comments for Setcion 1.4.5, Page 1-14. | Refer to revised Sections 1.4.5, 3.9.2 and 4.2. |
| Section 4.2 - Environmental Restoration and Enhancement - Page 4-2, Paragraph 1 | Additional information has been included in Section 4.2. |
| See comments for Section 1.4.5, Page 1-14. | |
| Section 4.2 - Environmental Restoration and Enhancement - Page 4-2, Paragraph 2 | |
| Project streamflow between Scott Dam and Van Arsdale Reservoir is greater than the estimated unimpaired natural flow in this stream section during the summer and fall. Fisheries habitat has been enhanced in this section by the increased flows. | This has been noted in Section 4.2. |
| Section 5.2 - Water Quality - Page 5-1, Paragraph 1 | |
| Stream habitat destruction resulting from sediment deposition has not been documented below the project and may not necessarily be occurring. | Staff observed during field investigations in November 1977, habitat destruction caused by sedimentation. |
| Section 5.3 - Fish Resource - Page 5-2, Paragraph 1 | |
| Chinook salmon have not necessarily been eliminated above Tomki Creek; however, their numbers have been reduced. | Statement has been clarified in Section 5.3. |



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Section 8.1 - Need for Project Power - Page 8-1, Paragraph 4

The paragraph states that the 1976 cost of energy from Avon steamplant is 60 mills per kwh. This value is used in several places throughout the report to evaluate project alternatives. It should be noted that Avon is a co-generation plant which is connected to an oil refining process and is not a typical generating plant. The paragraph should continue by stating that Applicant's thermal steam plants in 1976 generated energy at an average cost of 21 mills per kwh.

Refer to revised Section 8.1.

The value of 21 mills per kwh (instead of 60) should be used to develop values of power on the following pages of the report:

Page 7-1, last paragraph
Page 8-10, first paragraph
Page 8-16, next to last paragraph
Page 8-19, first paragraph

These values have been corrected.

Section 8.4.1 - Land Features, Cover, Use and Visual Resources - Page 8-3, Paragraph 3

The paragraph states that the historical annual diversion from the Eel River to the East Branch Russian River is about 184,000 acre-feet per year. The actual diversion for the period 1923-1970 averages 154,700 acre-feet per year as shown in Appendix A-1. The 184,000 acre-feet figure is an adjusted figure to compensate for the diversion tunnel enlargement in 1950, and provides an estimate of the maximum diversion, based on theoretical operation studies, that could have been expected under the historical project operation.

Correction made in FEIS.

Section 8.5 - Alternative Operating Programs - Page 8-6

We note that the Staff DEIS reviews only three alternative operating programs (DWR 2E, DWR 2B, and CDF&G) in Section 8.5. PGandE's analysis or comments about DWR's operating plans were not included. The DEIS should be supplemented to indicate that there are other flow release variations that would generally accomplish the same goals, yet provide less reduction in water to Potter Valley Powerhouse, and that licensee should be allowed to work with the State agencies to arrive at mutually agreeable releases.

A settlement conference was convened on October 24, 1978, in San Francisco in an attempt to get agreement among the parties on a mode of project operation. An operation proposal was presented by PG&E and discussed by the by the attendees. This proposal has been included in Section 8 of the FEIS. Future conferences are scheduled.

We have attached excerpts from PGandE's July 30, 1975 letter from Mr. A. P. Kilroy to Mr. Edwin J. Barnes of DWR commenting on the Alternative 2E discussion in DWR's report, "Eel-Russian River's Streamflow Augmentation Study, June 1975". These comments would also be applicable to the DEIS's discussion of fish water release alternatives.

Refer to revised Section 8.5.1.1.

Section 8.5.1.1 - Power Production - Page 8-8, Paragraph 1

We question why the period April, 1964 through March, 1965 was chosen to represent a dry period. The State and other agencies have generally accepted a dry year criteria based on 50 percent or less of

Applicant's October 1978 proposal would define a dry year as one ranging from less than 8 percent of cumulative normal flow from October to December to less than 40 percent of cumulative flow from October to July.



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normal runoff. Such a criteria has been accepted by FERC, the State, and PGandE for other projects. With this criterion, 1924, 29, 31, 33, 34, 39, 44, 47, 55, and 64 were designated as dry years. DWR's revised dry year criteria (40 percent or less of cumulative runoff of the Eel River at Van Arsdale Reservoir) would reduce the dry years to 1924, 29, 31 and 39. Using Table B-1 of the DEIS, the unimpaired runoff for the suggested "dry period", April, 1964 to March, 1965, is 767,200 acre-feet, as compared to a 48-year average of 468,300 acre-feet. This suggests that the period April, 1964 to March, 1965 would not be representative of a dry period.

Section 8.5.1.1 - Power Production - Page 8-10, Paragraph 1

It is stated, "During the dry period of April, 1964 through March, 1965, the total annual power production would be reduced by about 10 percent or 4,400,000 kwh at an annual cost to the applicant of $264,000." To be consistent with the previous sentence in the same paragraph where 5 percent loss is 3,050,000 kwh (based on a 61,000,000 kwh/year average yield), 10 percent should be 6,100,000 kwh/year.

Refer to revised Section 8.5.1.1.

Section 8.5.1.2 - Socio-Economic Considerations - Page 8-11, Paragraph 2

It is stated, "The amount of water that could be diverted during the May 1 through October 31 period would be reduced by 4,500 acre-feet (9 percent)". Table 8-3 on page 8-12 shows the cumulative diversion from May, 1964 through September, 1964 to be a 4,500 acre-feet difference under the DWR Alternative 2E operation. It is not clear why the month of October is not included in this calculation.

Table 8-3 has been revised.

Page 8-12, Table 8-3

We suggest that this Table should show the units for water flow, and explain more clearly how these data are formulated and the source for this information.

Table 8-3 has been revised.

Very truly yours,

W. J. Gallery

Attachment

cc: Mr. Eugene Noblett
Regional Engineer
Federal Energy Regulatory Commission
555 Battery Street
San Francisco, California



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# ATTACHMENT 2

# Impact Analysis and Adaptation Strategy Evaluation Report

## Lake County Resource Assessment – Proposed Scott Dam Decommissioning

### County of Lake

255 N. Forbes Street
Lakeport, CA 95453
Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

August 22, 2025

Revision: 00

Making Sustainability Happen

# Cover Letter

August 22, 2025

County of Lake
255 N. Forbes Street
Lakeport, CA 95453

**RE: Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project, Proposed Scott Dam Decommissioning by Pacific Gas and Electric (PG&E)**

To whom it may concern:

On behalf of the County of Lake, SLR International Corporation (SLR) has prepared this Report as part of our effort to support the Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project. SLR is conducting a comprehensive evaluation aimed at assessing the potential impacts of decommissioning Scott Dam and draining Lake Pillsbury on various resources and stakeholders in the surrounding community. The grant project, funded by the Department of Water Resources (DWR) Agreement Number 4600015984, dated June 12, 2024 with the County of Lake aims to provide a thorough understanding of the existing conditions that would be affected by the removal of the dam and lake. The following tasks define Grant scope of work:

1. Project Administration
2. Existing Condition Evaluation
3. Impact Analysis and Adaptation Strategy Evaluation

This report has been prepared by SLR International Corporation (SLR) to meet the requirements of the DWR agreement. The material and data in this report were prepared under the supervision and direction of the undersigned.

Sincerely,

**SLR International Corporation**

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# Executive Summary

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in Pacific Gas and Electric Company (PG&E)'s Potter Valley Hydroelectric Project (facilities shown in Exhibit 1). Scott Dam is located on the Eel River in Lake County, within the Mendocino National Forest. PG&E is proposing to decommission the Potter Valley Hydroelectric Project (PVP), which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam and other associated facilities. In this regard, PG&E has submitted a Final Surrender Application (PG&E, 2025c) to the Federal Energy Regulatory Commission (FERC). requesting approval of the decommissioning.

The decommissioning process proposed by PG&E includes the removal of Scott Dam and draining Lake Pillsbury. The removal of Scott Dam and loss of Lake Pillsbury is expected to have complex impacts on various aspects of the environment, economy, and community in particularly within the County of Lake. SLR has been contracted by the County of Lake to support them on evaluating these impacts to the county as per a grant provided by the Department of Water Resources of the State of California (DWR). SLR's scope of work is to review existing conditions in the project area, identify the potential impacts of the dam decommissioning and then develop appropriate adaptation strategies to mitigate them to meet county and community needs.

SLR has reviewed the PG&E's Final Application for Surrender of License (PG&E, 2025c) and their suggested mitigation approaches for the PVP and specifically for the Scott Dam and Lake Pillsbury decommissioning to identify the potential impacts and PG&E's suggested mitigation options. SLR then have additionally developed other applicable and feasible mitigation options to achieve project goals desired by both the County of Lake, community and PG&E.

Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Five alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Current Operational Conditions Scenario covers operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Final Application for Surrender of License dated July 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habermatolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.

7. Scenario E describes the removal of Scott Dam and installation of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

Based on SLR's evaluation, PG&E's Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by previous studies, indicating shortcomings in its anticipated outcomes. In contrast, we identified five other alternatives that rank more favorably, and should be further evaluated for implementation before FERC issues a License Surrender Order.

This report summarizes the evaluation of the potential impacts of removing Scott Dam and above scenarios as preliminary adaptation strategies. This report is prepared to meet the deliverable requirements of the DWR grant work plan as well.

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# Acronyms and Abbreviations

| | |
|---|---|
| AACE | Association for the Advancement of Cost Engineering |
| ac-ft | acre-feet |
| BIOS | Biogeographic Information and Observation System |
| CDFW | California Department of Fish and Wildlife's |
| CEED | Center for Environmental Economic Development |
| CESA | California Endangered Species Act |
| CEQA | California Environmental Quality Act |
| cf | cubic feet |
| cfs | cubic feet per second |
| cy | cubic yards |
| CDEC | California Data Exchange Center |
| CNDDB | California Natural Diversity Data Base |
| DSOD | Division of Safety of Dams |
| DWR | Department of Water Resources |
| EIR | Environmental Impact Report |
| EIS | Environmental Impact Statement |
| ESA | Endangered Species Act |
| FERC | Federal Energy Regulatory Commission |
| ft | Foot or feet |
| GRC | General Rate Case |
| HEC-RAS | Hydrologic Engineering Center’s River Analysis System |
| LAT | Large Airtankers |
| NEPA | National Environmental Policy Act |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NERF | New Eel Russian Facility |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NIDIS | National Integrated Drought Information System |
| NMFS | National Marine Fisheries Service |
| NOA | Naturally Occurring Asbestos |
| NOAA | National Oceanic and Atmospheric Administration |
| PAD | Pre-Application Document |
| PG&E | Pacific Gas and Electric Company |

| | |
|---|---|
| PVP | Potter Valley Project |
| PVID | Potter Valley Irrigation District |
| SLR | SLR International Corporation |
| USACE | United States Army Corps of Engineers |
| USDA | United States Department of Agriculture |
| USFS | United States Forest Service |
| USGS | United States Geological Survey |
| VLAT | Very Large Airtankers |
| WY | water year |

# 1.0 Introduction

SLR International Corporation (SLR) has been contracted by the County of Lake (the County) to provide technical assistance needed to better understand the proposed Scott Dam Decommission by the Pacific Gas & Electric Co. (PG&E) as part of the overall decommissioning proposed for the Potter Valley Hydroelectric Project (PVP; *Federal Energy Regulatory Commission [FERC] Proceeding No. 77*). PVP facilities are shown in Exhibit 1. On July 25, 2025, PG&E submitted a Final Application for Surrender of License and Application for NonProject Use of Project Lands for the PVP to the FERC indicating the project is not economically feasible. The Final Application for Surrender of License includes information on the decommissioning process for the Potter Valley Hydroelectric Project, and for the construction of a New Eel Russian Facility, which is identified as non-project use of project lands by the Eel-Russian Project Authority (ERPA). SLR has carefully reviewed the information available to date for this proposed action by PG&E and prepared this report to document several mitigation concepts for discussions between SLR and the County.

Based on our review and project understanding we have listed PG&E's project goals below as applicable to Scott Dam and Lake Pillsbury (located within the County) and the desired goals of the County (including some local community interests) with respect to the proposed actions.

## 1.1 Report Organization

The report is organized into several sections to facilitate understanding and navigation:

**1.0 Introduction:** This section provides an overview of the report's purpose and scope, and an overview of the project goals.

**2.0 Summary of Background Information:** This section provides an overview of the background information for the project.

**3.0 Scott Dam Scenario Description:** This section specifies scenarios for adaptation.

**4.0 Preliminary Comparison of Scenarios:** This section presents a preliminary comparison based on relevant criteria identified in the previous sections.

**5.0 Climate Vulnerability:** This section summarizes climate change effects on Lake County and cumulative effects if Scott Dam is decommissioned.

**6.0 Conclusions:** This section summarizes the key findings and insights derived from the analysis.

**7.0 Remarks:** This section specifies professional scope and remarks in the report.

**8.0 References:** This section provides a list of sources cited in the report.

**Figures:** Figures created by SLR present relevant information.

**Exhibits:** Figures and diagrams created by others are included as exhibits.

**Tables:** Tables summarize information, and are included in text and appended.

**Appendix A:** Presents detailed information on existing information and models relevant to hydraulic aspects.

**Appendix B:** Provides additional information on fire suppression.

**Appendix C:** Provides detailed evaluation on fish passage for each scenario.

**Appendix D:** Presents considerations related to Climate Vulnerability.

**Appendix E**: Presents considerations related to the permitting process.

## 1.2 Decommissioning Project Goals

According to PG&E, its overall project goal is the decommissioning of the PVP as the hydropower generation is no longer economically viable. PG&E therefore expects to obtain FERC's approval for the decommissioning as proposed in the Application for Surrender of License and complete the PVP decommissioning including the Scott Dam and Lake Pillsbury. The County of Lake main goal is to accommodate best and most suitable mitigation options to benefit the county and the community with respect to the Scott Dam decommissioning. More specific discussions of these goals are provided below.

### 1.2.1 PG&E's Project Goals

As stated in their Application for Surrender of License, PG&E's goals at the conclusion of the decommissioning process are:

1. To remove the Project facilities and features including but not limited to Scott Dam and Cape Horn Dam,
2. To remove the Project from FERC jurisdiction, and
3. no longer operate or maintain the Project and Project features in the future.

SLR's general understanding is that the following additional goals, facts and interests played a role in PG&E's proposed decommissioning approach:

- PG&E recognized the PVP as uneconomic for its customers. On January 25, 2019, PG&E rescinded its Notice of Intent and Pre-Application Document for License renewal.
- Operation and Maintenance (O&M) of the PVP is no longer economically feasible and therefore the need for water storage previously created by Scott Dam and Lake Pillsbury for hydropower plant is no longer needed. With FERC's approval of the Application for Surrender of License, PG&E would start the decommissioning process for Scott Dam which would result in the removal of Lake Pillsbury. Once the decommissioning and restoration process is complete, PG&E expects to limit their financial and regulatory responsibilities.
- The initial design capacity of Lake Pillsbury was 76,876-acre feet (ac-ft) at Elevation 1,910 feet (ft)[1]. PG&E reduced this capacity in 2023 due to concerns of dam instability, to a maximum elevation at the spillway at 1,900 ft.
- The CA Division of Dam Safety (DSOD) has agreed with PG&E's seismic evaluation and granted approval to operate at the current spillway crest at Elevation 1,900 ft to reduce risk of failure[2]. In April 2023, DSOD provided comments for the simplified seismic stability analysis and proposed interim risk reduction measures. On July 28, 2025, FERC once again asked for updates to risk reduction measures currently implemented or proposed since this letter was issued.
- Due to the presence of Scott Dam, fish migration along the Eel River to the upper reaches has not been possible. Several interest groups along the Eel River have voiced

[1] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. Elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.

[2] DSOD, 2023. Scott Dam, No. 97-101, Lake County. April 12.

support for the removal of the dam to create an unrestricted fish passage (primarily anadromous salmonids, including chinook salmon, coho salmon and steelhead) upstream of the Scott Dam. A working group evaluated alternatives for the Potter Valley Project, including adding fish passage to the existing Scott Dam structure[3]. The working group parties[4] determined the preferred alternative was to decommission Scott Dam and to add fish passage to Cape Horn Dam[5]. The County of Lake and other stakeholders were not meaningfully considered in these discussions and did not participate in the selection of alternatives.

- FERC has allowed PG&E to formally start the license surrender process to proceed following the FERC procedures. A Final Application for Surrender of License was submitted to FERC on July 25, 2025. As planned, PG&E would begin the detailed planning and design of the decommissioning of the PVP (including Scott Dam) following the final application submittal, and they anticipate one to two years to prepare Management Plans[6].
- PG&E and the Eel-Russian Project Authority (ERPA)[7] have signed an agreement to proceed with the construction of the New Eel-Russian Facility (NERF) to continue Eel River water diversions to the Russian River.

### 1.2.2 County of Lake Project Goals

The presence of Scott Dam and Lake Pillsbury has been a vital resource for local counties and the state of California for over one hundred years. PG&E has not considered the true loss to the community in their planning of the Scott Dam decommissioning, and that is not acceptable to the County of Lake, local communities, and other agencies. The County of Lake would like PG&E and FERC to address their concerns as part of the decommissioning process. If they address the County of Lake's concerns, FERC would include certain mitigation procedures proposed by the County in their final Regulatory Order to PG&E to comply. The County of Lake requested PG&E to accommodate mitigation measures that are important to the County and find ways to integrate those measures into final decommissioning plans and requirements so that lost values can be replaced with new options including compensating for financial losses.

The County of Lake has identified several major areas that would be affected by the removal of Scott Dam and Lake Pillsbury. Those areas include:

1. Water Supply to the nearby users and commercial facilities.
2. Fire Suppression for wildfire management both from aerial attacks and ground attacks (like during the last two major wildfires that happened in the area).
3. Loss of revenue from decrease in land value and the recreation facilities and activities.
4. Recreation in and around the lake (see current facilities in Figure 1).
5. Ecosystem that has been well established over one hundred years around the lake (wildlife, terrestrial, avian, and lake fish).
6. Recharge to groundwater in the vicinity of Lake Pillsbury.

[3] Stillwater Sciences, et. al, 2021. Potter Valley Project Feasibility Study: Capital Improvements. April.

[4] The working group parties include California Trout, Humboldt County, Mendocino County Inland Water and Power Commission, Round Valley Indian Tribes, and Sonoma County Water Agency.

[5] Stillwater Sciences, et. al, 2021. Potter Valley Project Feasibility Study: Alternatives Description and Project Plan. April

[6] Information verbally presented by PG&E in an August 11, 2025 townhall.

[7] A joint powers authority formed by a joint exercise of powers agreement between the County of Sonoma, Sonoma County Water Agency (Sonoma Water), and the Mendocino County Inland Water and Power Commission.

7. Environmental impacts due to the sediment release and debris management and other soil management.
8. Other impacts that may be generated, for example the release of asbestos during deconstruction, impacts on infrastructure, noise and aesthetics.

## 1.3 Other Considerations for Mitigation Options

Based on SLR's review of the PG&E's Final Application for Surrender of License, existing conditions and County provided information, we have identified scenarios that would provide more equitable mitigation options for all the stakeholders.

From a technical perspective, including geological, geotechnical and slope stability around Scott Dam and Lake Pillsbury, there is a need to better address long term site stability. Also, construction concerns associated with the presence of naturally occurring asbestos (NOAs), need to be considered in mitigation. A better plan for road infrastructure improvements is also needed.

From the information provided by PG&E and other parties with respect to the decommissioning of Scott Dam and removal of Lake Pillsbury, PG&E's decision lacks consideration of local community needs and inclusion of other mitigation options. It appears that only certain stakeholders' views were taken into account, and PG&E's decision was not representative of any outreach to the County and local community members residing around Lake Pillsbury. Additional considerations should be given to the County of Lake's goals and that can be accomplished by further evaluation of options as presented in this report. Not only the County and local communities but also entities like CalWild[8] have identified and stated the loss of water storage at Lake Pillsbury is a great concern to all parties, and therefore PG&E needs to invest in additional mitigation measures.

In this report, SLR presents several mitigation option scenarios considering the overall need of both PG&E and the County of Lake. We recommend that these options be evaluated and compared to the Scott Dam removal alternative. The scenarios identified below allow for a comparative analysis and support the selection of a more acceptable mitigation option to address stakeholder needs.

## 1.4 Impact Analysis and Adaptation Strategy Evaluation Scope

SLR has identified various impacts to the County of Lake and the local community based on the Scott Dam decommissioning process proposed by PG&E as presented in their Final Application for Surrender of License published in July 2025. SLR and the County of Lake submitted joint comments on the Final Application for Surrender of License (SLR, 2025a, County of Lake, 2025) and those comments include general comments and specific comments with details to document identified impacts under various technical and resources categories as presented in the application.

SLR's Existing Conditions Report provides a summary assessment of the present conditions in the vicinity of Scott Dam and Lake Pillsbury (SLR, 2025b). Existing conditions were documented in relationship to proposed PG&E decommissioning in the following areas as related to the

[8] CalWild is a California-based public benefit corporation, which goal is to protect and restore California's landscape. As part of the PVP decommissioning process their role is to develop collaborative consensus, their work is funded by grants from the California Coastal Conservancy and the Resources Legacy Fund.

proposed decommissioning: Recreation, ecosystem, wildfire suppression, power, geotechnical, sediment, water supply, groundwater, hydrology, roadways and economic aspects.

Based on the above findings from SLR's reviews to date, we have focused on the following key impact areas to further document impacts and evaluation of those impacts as the basis of identifying preliminary mitigation options to mitigate or eliminate such impacts.

**Recreation**: PG&E's proposed decommissioning plan does not address the need to minimize the recreational impacts or replace the recreational facilities to fully benefit the local economy and community.

**Ecosystem (Wildlife, Botanical and Aquatic resources**): The decommissioning of the Scott Dam and Lake Pillsbury removes many ecosystem resources that have been vital to the current ecosystem elements in the area. Even though the Eel River restoration benefits fish migration upstream along the river, other fish resources in the lake would be lost.

**Wildfire Suppression**: Lake Pilsbury provides a crucial water supply needed for major forest fires in the County of Lake. No consideration is given by PG&E for the replacement of this water supply need in the Final Application for Surrender of License.

**Infrastructure/Water Supply** (including downstream water needs and groundwater use): Lake Pilsbury has been used to maintain the downstream water supply need (through minimum flow releases through needle valve control at Scott Dam) but the reliability of downstream water flow under natural summer conditions cannot meet such downstream requirements. The impact on local groundwater use due to loss of lake recharge would also be lost with the removal of Lake Pillsbury. Proper understanding of the local hydrology/hydraulic are key aspects in evaluating the impacts to water supply needs.

**Restoration** (includes water quality and sediment management): PG&E's current plan considers preparing a sediment and water quality management plan, but no details have been provided. There are various environmental and reuse benefits on sediment reuse that can be incorporated into mitigation options that have not been considered by PG&E.

**Economic and Cost Conditions** (both from project implementation costs as well as revenue lost to the County of Lake and the local community): The current information provided by PG&E is not accurate and incomplete. However, the cost evaluations completed previously by Stillwater Sciences and McMillen Jacobs are summarized where applicable in this report.

**Other Impact Areas** (including geotechnical and roadway infrastructure): Scott Dam is currently operated at 1,900 ft spillway crest elevation to mitigate seismic risk, which has been approved by FERC and DSOD[9]. Roadway improvements or replacements needed for the benefit of local communities are not addressed adequately as part of the decommissioning process.

SLR has considered the above key impact areas and evaluated several mitigation scenarios that would better address the County of Lake goals, local community concerns as well accomplish PG&E goals to achieve more equitable outcome of the proposed decommissioning project. The purpose of this report is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of adaptation strategies required under the Department of Water Resources (DWR) grant. Specifically, this report outlines how potential decommissioning scenarios may affect the Eel River system and

[9] In a virtual townhall meeting on August 11, 2025, PG&E stated with the current operational level there is no immediate safety risk and Scott Dam can continue to operate safely.

identifies potential restoration and adaptation opportunities within the Lake Pillsbury area for different mitigation scenarios.

A range of high-level options were developed and organized into a set of preliminary alternatives. This report presents the initial analyses conducted to assess the feasibility of these alternatives, with a focus on the effects on the Eel River hydrology and hydraulics. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource. Mitigation strategies presented in this evaluation may be used as mitigation measures associated with the proposed PG&E remedy.

This analysis does not represent a final decision or commitment to any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

This report represents an initial step in a broader, ongoing process. In addition to the further work to be done by PG&E after submitting the Application for Surrender of License to FERC in July 2025, additional studies, analyses, and synthesis will be needed to refine the proposed scenarios to make go forward decisions by the County of Lake and other stakeholders. Community and regulatory outreach will also be needed to better accommodate their concerns with respect to selected mitigation options.

SLR's proposed mitigation scenarios are based on our current understanding of the relevant project background information, our evaluation of hydrology and hydraulic conditions, knowledge of local wildfire suppression, recreation, ecosystem specifically understanding of fish passage needs, and the knowledge of local economic conditions. We have provided applicable summaries of these aspects below.

# 2.0 Summary of Background Information

The construction of Scott Dam started in 1920 and was completed in 1922, creating Lake Pillsbury. Snow Mountain Water and Power Company was the original owner. They acquired a Department of Agriculture Power permit in 1920. Then, the Federal Power Commission (the *precursor to FERC*) granted a 50-year license for the Project in 1922. In 1930, PG&E acquired the Project from Snow Mountain Water and Power and assumed the license (PVID, 2024; McMillen Jacobs Associates, 2021).

Scott Dam is a concrete, gravity-type, ogee-shaped structure. It is 130 ft high and 805 ft long and classified as a high hazard dam[10] according to the U.S. Army Corps of Engineer's Institute of Water Resources National Inventory of Dams database. The spillway crest elevation is 1,900 ft[11] built in 1922. Additionally, each of the five radial gates surmounting the spill crest is 32 ft wide by 10 ft high, made of 26 steel slide gates, each 10 ft high and varying width from 7.5 ft to 10.08 ft. The storage releases are 72-inch diameter, riveted-steel outlet pipe passing through the dam at an invert elevation of 1,812 ft. The storage releases are controlled by a 42-inch Lauren-Johnson needle valve.

At initially constructed, Lake Pillsbury had a surface area of approximately 2,275 acres, with an elevation of 1,910 ft and maximum storage capacity of 76,876 ac-ft. Since inception, Lake

[10] High hazard dam classification means that that dam failure could pose serious threat to downstream communities and infrastructure.

[11] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. PG&E also presents elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.

Pillsbury's capacity has reduced based on operational changes, sediment accumulations, and most recently in 2023 due to concerns of dam instability during seismic activity, to a maximum elevation at the spillway at 1,900 ft. Therefore, the reservoir is currently operated for a storage capacity of 66,876 ac-ft. Operational footprints are shown in Figure 2.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The decommissioning process considers removal of Scott Dam and draining Lake Pillsbury. PG&E prepared an Updated Final Application for Surrender of License and Application for Non-Project Use of Project Lands in July 2025 (mentioned as "Final Application for Surrender of License") which has been circulated to agencies and interested parties for comments (PG&E, 2025c). The proposed decommissioning of Scott Dam is also considered by some interested parties as part of a broader initiative to restore the Eel River's ecosystem and improve fish passage.

In addition to power generation, the PVP has been providing water supply for agriculture and recreation in Mendocino and Sonoma Counties. A key aspect of the Potter Valley Project is the Cape Horn Dam, where water diversions to the Russian River basin occur. As part of the license surrender a New Eel Russian Facility (NERF) to maintain water diversions is proposed by PG&E as a non-project use of project lands by the Eel-Russian Project Authority (ERPA). Since these diversions take place downstream of the Lake County's boundary, it is expected that the impact on the County of Lake will be minimal.

SLR's Existing Conditions Report provides a summary assessment of the present conditions in the vicinity of Scott Dam and Lake Pillsbury (SLR, 2025a). Existing conditions were documented in relationship to proposed PG&E decommissioning in the following areas as related to the proposed decommissioning: Recreation, ecosystem, wildfire suppression, power, geotechnical, sediment, water supply, groundwater, hydrology, roadways and economic aspects.

Regulatory flow requirements are specified in the FERC license and the National Marine Fisheries Service's (NMFS's) Reasonable and Prudent Alternative (NMFS, 2002). These minimum flows range from 20 to 60 cfs between June 1 and November 30 and range from 20 to 100 cfs between December 1 and May 31 depending on water year classification.

In the July 2025 Final Application for Surrender of License, PG&E states that they have historically diverted around 39,000 ac-ft from the Eel River to PVID and the East Branch of the Russian River (PG&E, 2025c). ERPA proposes annual diversions from the Eel River of 3,000 – 7,500 ac-ft to PVID and 24,800 – 27,500 ac-ft to the East Branch of the Russian River, (PG&E, 2025c). The combined flows would equal between 55 – 120 cfs diverted during the wet season, and 1 – 6 cfs diverted during the dry season (PG&E, 2025c). ERPA's goal for adjusting diversions seasonally is to support ecological restoration following the dam removal, whereas PG&E's goal with diversions has been to generate hydropower and maintain infrastructure (Sonoma Water, 2024). If PG&E's application results in a transfer of responsibility to ERPA, ERPA will oversee the operations of Cape Horn Dam, Van Arsdale Reservoir, the Van Arsdale diversion and fish screening facility, PVID, tunnels and conveyance systems, and water rights and authorities that currently fall with PG&E. ERPA would coordinate this transfer of rights and responsibilities with PG&E and aim to align this transfer with PG&E's proposed decommissioning schedule. ERPA's next steps, in the case of PG&E's surrender, are to coordinate with PG&E on the transition and SCADA integration, perform geotechnical and hydraulic studies, risk assessments, and life cycle cost analyses for the facilities.

## 2.1 Hydrology and Hydraulic Conditions

To support the hydrologic and hydraulic evaluation of PG&E's proposed Scott Dam removal, SLR reviewed a broad set of technical studies, datasets, and modeling tools. These included PG&E's surrender application, consultant reports, and publicly available data from United States Geological Survey (USGS), National Oceanic and Atmospheric Administration (NOAA), National Integrated Drought Information System (NIDIS), National Centers for Environmental Information (NCEI), and California Data Exchange Center (CDEC). The evaluation was conducted focusing on potential impacts to water supply, flood risk, and seasonal hydrology.

SLR's analysis incorporated reconstructed water balance modeling, flood frequency assessments, and climate condition evaluations. The Stillwater Sciences (2021e) hydraulic model projected a potential increase in 100-year flood water surface elevations by up to 1.2 ft following dam removal. However, the model lacked detailed operational inputs and was not available for independent review.

Climate conditions were evaluated using both precipitation records and impaired gage flow data. While precipitation-based analysis suggested wet seasons from November through March and dry seasons from April through October, gaged outflow data indicated a slightly different seasonal pattern. To maintain consistency with prior studies and operational data, the gage-based wet (December–April) and dry (May–November) seasons were used in scenario evaluations.

SLR also reviewed how Scott Dam responded to historical flood and drought events. During the flood of record in Water Year 1982, the dam attenuated peak flows—limiting downstream impacts—despite outflows reaching 15,400 cfs. The maximum recorded outflow of 45,300 cfs in 1964 did not correspond to a major flood, suggesting operational releases may have been anomalous and due to operational, maintenance , or extreme snow melt conditions. In drought years, such as 1976–1977 and 2012–2016, the dam maintained minimum outflows even as storage dropped below operational thresholds, though temporary flow variances were required. These responses demonstrate the dam's historical role in buffering climate extremes, while also highlighting operational limitations under prolonged stress.

Standardized Precipitation Index (SPI) data from NIDIS indicates increasing frequency and intensity of droughts in recent decades. These trends, along with declining snowpack and greater precipitation variability, reinforce the need for adaptive water management strategies.

SLR documented Lake Pillsbury's current role in supporting aerial wildfire suppression and summarized minimum water storage requirements for future pond design. This information was used to inform conceptual sizing for firefighting support but was not evaluated beyond that scope.

The evaluation considered multiple potential scenarios in response to PG&E's surrender application. Each scenario was assessed under the four defined climate conditions—wet, dry, flood, and drought—to support Lake County's decision-making regarding dam removal and long-term water resource planning. Detailed findings are presented in Appendix A.

## 2.2 Fire Suppression

Lake Pillsbury is currently an optimal resource for current support for fire suppression and operates as an extensive use of water source for wildfire firefighting operations in the Lake County. The SLR Team (SLR and Aanko) has developed a summary of information regarding fire suppression and the use of aerial resources and their needs to support the scenario feasibility analysis.  The information is summarized herein and provided in Appendix B. There

are currently several main types of aerial firefighting support aircraft applicable for combating large-scale wildfires in Lake County, California: rotary-wing Type 1 helicopter aircraft like the Boeing CH-47 Chinook and Sikorsky S-70 Firehawk, multiengine scoopers like the Canadair CL-415 (Super Scooper), Large Airtankers (LATs), and Very Large Airtankers (VLAT's). When determining the suitability of a reservoir or water source for aerial firefighting there are three main considerations: water capacity, accessibility, and operational safety for the aircraft based upon the operational need.

The LATs and VLATs, play a critical role in wildfire suppression by delivering substantial volumes of water or fire retardant—typically 2,000-4,000 gallons for LATs and over 8,000 gallons for VLATs. These aircraft are primarily deployed during large, prolonged wildfire events due to their high capacity and strategic value. However, they do not scoop water from local reservoirs; instead, they rely on established airbases for refilling. Amphibious aircraft like the Super Scooper are an exception, capable of scooping over 1,000 gallons of water from large bodies of water, requiring at least one mile of open water for safe operation as the craft needs 10-12 seconds during a scoop. Lake Pillsbury is currently able to support fixed-wing aerial firefighting operations.

Helicopters can carry 1,000 to over 3,000 gallons of water and are critical for wildfire suppression, especially in the absence of large water bodies. Type 1 helicopters can be utilized for large-scale, sustained operations and effective for initial attack and support ground crews by delivering water directly to fire lines. Helicopters are also more maneuverable, but they are ideal when used near the water source to reduce fuel costs and travel time.

When constructing a new reservoir which can support helicopter operations, specific size and maneuverability criteria should be met. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet (sq. ft) at an average depth of 6 ft. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223-669 sq. ft. These considerations are summarized in Table 1 (below), listed by operational need for aerial firefighting.

**Table 1: General Aerial Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Gallons [ac-ft] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Extensive Use | 100,000+ [0.31] | 6 ft | 2,230 sq. ft. | 47 ft × 47 ft or 30 ft × 75 ft |
| Limited Use (Low Volume) | 10,000 [0.031] | 6 ft | 223 sq. ft. | 15 ft × 15 ft or 10 ft × 23 ft |
| Limited Use (High Volume) | 30,000 [0.092] | 6 ft | 669 sq. ft. | 26 ft × 26 ft or 15 ft × 45 ft |

In the absence of a reliable water supply, forest management practices must prioritize methods that enhance ecosystem resilience and mitigate wildfire risk through non-irrigative means. A primary strategy involves managing stand density via mechanical thinning, which reduces competition for scarce resources and lowers the accumulation of combustible biomass.

Prescribed burning is also a critical tool; when carefully planned and executed under suitable weather conditions, it replicates natural fire regimes, reduces surface fuel loads, and promotes the regeneration of fire-adapted native species.

## 2.3 Recreation

Recreation has relied on the scenic qualities of Lake Pillsbury for many decades. Lake Pillsbury is the largest waterbody in the Mendocino National Forest and attracts numerous visitors each year who generate revenue for the local economy. Figure 1 shows the location of recreational facilities in the vicinity of Lake Pillsbury.

Recreation taking place in and around the lake, includes camping, boating (motorized and non-motorized), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking (USFS, 2018). Lake fishing includes largemouth bass, Sacramento pikeminnow, bluegill and Rainbow trout (Fish Brain, n.d.). Whitewater rafting has historically occurred downstream of Lake Pillsbury in October and November when PG&E released water from Pillsbury Reservoir to make room for winter rain and snowmelt (California Creek). There are currently six campgrounds, two with group camping facilities along with three boat ramps around Lake Pillsbury (summarized in Figure 1). According to the Forest Service's website "The campgrounds at Lake Pillsbury are operated and maintained by Pacific Gas and Electric," excepting Lake Pillsbury Resort, which is privately owned (Lake Pillsbury Resort, 2024).

The annual visitation estimate for the Mendocino National Forest is 254,000 individuals, based on National Visitor Use Monitoring Data Collected FY 2010, FY 2011, FY2012, FY2013, FY2014, FY2015, FY2016, FY2017, FY2018, FY2019 (Visitor Use Report Pacific Southwest Region (R5) USDA Forest Service Region 5) A visit is defined as "A National Forest Visit is defined as the entry of one person upon a national forest to participate in recreation activities for an unspecified period. A National Forest Visit can be composed of multiple Site Visits" (USFS, 2018).

## 2.4 Ecosystem

An ecosystem is defined as a large community of living organisms (plants, animals and microbes) in a particular area. Lake Pillsbury has its own unique ecosystem – with threatened and endangered species, including Tule Elk, and fish – that attracts visitors to the area. SLR has conducted further research on these species to understand the existing ecosystem surrounding Lake Pillsbury as discussed below.

### 2.4.1 Wildlife Resources

The focus of the ecosystem evaluation is on threatened and endangered species. Verifying the presence or absence of these species within the project study area and determining potential impacts will require additional protocol surveys.

The predominant plant communities surrounding Lake Pillsbury and Van Arsdale Reservoir are yellow pine forest, mixed evergreen forest, foothill woodland, and chaparral (FERC, 1978).

SLR conducted a preliminary database review through the California Department of Fish and Wildlife's (CDFW's) BIOS Viewer, part of the CA Fish and Wildlife Wetland – CA Natural Diversity Database (CDFW, 2018). Preliminary research identified 15 total species within 0.5 miles of Lake Pillsbury's perimeter as shown in Exhibit 3A. Among the list of species identified,

3 of 15 are considered endangered according to State criteria. Bald eagle, steelhead, and Humboldt marten are the 3 endangered species located within the 0.5-mile distance radius.

The preliminary database review also located Tule Elk in areas surrounding Lake Pillsbury, as shown in Exhibit 3B. Exhibit 3B demonstrates the likelihood of elk presence within the outlined contours on the map. Contours from the database suggest there is a 50% likelihood that elk would be within the parameters of 0.5 miles, around the southern area of Lake Pillsbury. The second contour demonstrates a 90% likelihood of elk presence within its outlined contours, which exceeds the 0.5-mile distance radius scope for this database review.

PG&E has also identified rare, threatened, and endangered resources in the vicinity of the Project in the Final Application for Surrender of License (PG&E, 2025c). Species known to occur on Lake Pillsbury and Van Arsdale reservoirs include:

- Bald Eagle
- Northern Spotted Owl
- Northern Goshawk
- Fisher-West Coast
- Pacific Marten
- Special—Status bats
- Black-Tailed Deer
- Tule Elk
- Western bumble bees
- Monarch butterflies

### 2.4.2 Botanical Resources

There are four special-status plant species that are known to occur in the FERC Project boundary [see Exhibit 3C, specifically the area of Lake Pillsbury] or are adjacent to Project features, as mentioned in the Final Application for Surrender of License including:

Two populations of three-fingered morning glory (*Calystergia colling ssp. Tridactylosa*). One population located along the tunnel between Cape Horn Dam and Potter Valley Powerhouse; and one population located along the southern shoreline of Lake Pillsbury. One population of Green's narrow-leaved daisy (*Erigeron greeni*).

PG&E's Final Application for Surrender of License has acknowledged and stated various plans to mitigate construction related impacts on botanical resources.

### 2.4.3 Fish and Aquatic Resources

The alteration of the Eel River's natural flow has disrupted local ecosystems and fish migration patterns. The dam has been a barrier to native fish species, such as salmon and steelhead, preventing them from accessing upstream habitats. Disruptions to the system can have significant ecological consequences, affecting the health and biodiversity of the river system on a temporary and long-term basis.

The Eel River downstream of Lake Pillsbury, including the lower river and estuary, provides important aquatic habitat for numerous special-status species. This includes several anadromous salmonids listed as threatened under the Endangered Species Act (ESA), such as Chinook salmon (*Oncorhynchus tshawytscha*), coho salmon (*O. kisutch*), and steelhead (*O. mykiss*), as well as the threatened Pacific eulachon (*Thaleichthys pacificus*) and the

endangered tidewater goby (*Eucyclogobius newberryi*). Species proposed for ESA listing, such as the northwestern pond turtle (*Actinemys marmorata marmorata*) and the longfin smelt (*Spirinchus thaleichthys*), also inhabit this area.

Other notable species include those listed under the California Endangered Species Act (CESA), like the endangered summer steelhead (*O. mykiss irideus*), the candidate species white sturgeon (*Acipenser transmontanus*), and species of special concern such as the green sturgeon (Acipenser medirostris), Pacific lamprey (*Entosphenus tridentatus and Lampetra spp.),* foothill yellow-legged frog (*Rana boylii, Northwest/North Coast clade*), coastal tailed frog (*Ascaphus truei*), southern torrent salamander (*Rhyacotriton variegatus*), California floater (*Anodonta californiensis*), western pearlshell (*Margaritifera falcata*), and the northern red-legged frog (*Rana aurora*).

The river also supports a variety of non-listed native freshwater fish, including sculpin (*Cottus spp.),* stickleback (*Gasterosteus spp.),* and sucker (*Catostomus spp.).* Additionally, occasional runs of pink salmon (*O. gorbuscha*) and chum salmon (*O. keta*) have been documented (Yoshiyama and Moyle 2010).

The construction of Scott Dam created a 130-foot-high barrier that prevents upstream fish migration. Currently, there are no facilities at the dam to allow fish passage. Estimates of the blocked river habitat and potential returning fish numbers vary widely. A 1982 VTN study identified 35.7 miles of primary spawning habitat above the dam, estimating possible returns of 1,250 Chinook salmon and 1,499 steelhead. NMFS (2002) referenced this and suggested that historically, 2,000 to 4,000 fall-run Chinook and winter-run steelhead may have spawned upstream. Other studies estimate 100–150 miles of blocked habitat, while Cooper et al. (2020) projected even greater totals—up to 288 miles for steelhead spawning.

Considerations for fish and aquatic resources, including fish passage alternatives, are further evaluated in Appendix C.

## 2.5 Economic and Cost Conditions

Economic analyses of varied levels of detail and geographic focus have been completed over several decades, documenting the economic resources in the affected region and the projected economic impacts of the PVP. The focus on Lake County and Lake Pillsbury in these analyses is limited and/or analysis needs to be updated with the latest information. This section summarizes the County of Lake specific tax revenue information that can be used in estimating future loss of tax revenue to the County with respect to the decommissioning of Scott Dam and Lake Pillsbury.

In the areas immediately surrounding Lake Pillsbury, county tax revenues are related to secured and unsecured property taxes, forest service cabins, sales tax and PG&E unitary tax payment related to property they occupy. SLR has reviewed the tax revenue information provided by the County of Lake and summarized information as presented below.

The tax revenue for the year 2024 is approximately $872,000, showing a 3.5% increase from the actual 2023 values. These costs are broken down in Table 2 (below).

**Table 2: 2024 Taxes Due**

| Tax Source | Taxes collected |
|---|---|
| Current Secured (2024) | $ 581,976 |
| Current Unsecured (2023) | $ 6,700 |
| Current Unsecured Forest Service (2024) | $ 64,380 |
| Transient Occupancy Tax (TOT) + Sales Tax (2019-2020 estimate) | $ 16,400 |
| PG&E (2023) | $ 202,356 |
| **Total** | **$ 871,812** |

**Source:** Lake County Tax Collector, May 2025

Alternative analysis cost evaluations were completed previously by Stillwater Sciences (2021b) and McMillen Jacobs (2018), and are summarized where applicable in this report.

# 3.0 Scott Dam Scenario Description

PG&E's current proposed action under the Final Application for Surrender of License (PG&E, 2025c) includes the decommissioning and removal of most PVP facilities, including Scott Dam, Cape Horn Dam and the Potter Valley powerhouse. The evaluation below focuses only on the Scott Dam and Lake Pillsbury area.

Replacing Lake Pillsbury with a smaller reservoir can preserve the benefits of recreation, fire suppression, ecosystem maintenance and water supply to the community. Considering the above background and other information discussed, SLR has considered the following aspects in development of mitigation options:

1. Preserving recreational use by maintaining a water source.

2. The water needs for downstream water supply and supporting existing ecosystems in and near the Lake Pillsbury reservoir.

3. The potential for fire suppression using aerial machinery, such as helicopters.

4. Design pathways to increase fish passage to upper reaches on the Eel River.

5. Employ strategies to minimize environmental impacts and ecosystem benefits.

6. Accommodate short- and long-term economic benefits to maintain the current level of revenues for the County of Lake.

SLR has identified several scenarios to accommodate the above objectives as described in this section. For each scenario, a description of the scenario, our analysis where applicable and findings of the scenario evaluation are presented.

## 3.1 Existing Conditions Scenario

The current operational conditions scenario represents the Scott Dam and Lake Pillsbury operational period from 2019 to the present. Based on observed operational data, the maximum

operational water surface elevation for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 ft (PG&E datum) with a correspond water storage volume of 56,322 ac-ft (See **Error! Reference source not found.** in Appendix A). Historically, the maximum operational water surface elevation for Lake Pillsbury was at top elevation of the spillway gates of 1,910 ft. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the water surface elevation at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained. It is not known if there have been operational changes to the needle valve.

### 3.1.1 Existing Conditions Findings

#### 3.1.1.1 Water Supply

The existing conditions provide the highest reliability for water supply in terms of downstream flow, groundwater recharge, and operations due to the operability control provided by the needle valve and spillway gates. Lake Pillsbury has historically operated with an average storage volume of over 45,000 ac-ft (14 billion gallons) of water availability. Even though Lake Pillsbury was designed for hydropower support, floods have historically been attenuated by the reservoir, reducing the flood risk to downstream infrastructure.

Drought trends during the 21st century have been increasing in frequency, intensity, and duration (see also Appendix D). This historical water storage capacity has been reduced over time due to losses from sediment accumulation.

The average water surface elevation and storage in Lake Pillsbury for the current operational storage conditions (established in 2023) are lower than the historical operational conditions due to the reductions of the maximum operational water surface elevation and the passive operation of the spillway. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation.

As currently operated, Lake Pillsbury is expected to continue operating with an average available storage volume over 45,000 ac-ft (14 billion gallons).

Reducing the storage capacity, as sediments continue to build up, will reduce the drought tolerance and it is likely that under drought conditions the storage and outflow will be lower than the historic conditions observed values in the future for this scenario.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.1.1.2 Fire Suppression

The current conditions provide the most dependability for fire suppression support. Lake Pillsbury historically satisfied the operational requirements to extensive fire suppression use (see Appendix B).

#### 3.1.1.3 Ecosystem

Lake Pillsbury historically has provided habitat for various wildlife, lake aquatic and botanical species for many decades. Under the current conditions, the presence of Scott Dam as a fish barrier has been a concern for many fishing advocates for a long period of time.

#### 3.1.1.4 Recreation

Lake Pillsbury has historically allowed recreational use to be maintained. Recreation taking place in and around the lake includes camping, boating (motorized and non-motorized), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking (USFS, 2018).

#### 3.1.1.5 Economic and Cost Conditions

The presence of Lake Pillsbury generates tax revenue and tourism from recreation, benefiting local businesses and the community. The economic conditions described in Section 2.5 are applicable for the existing conditions.

#### 3.1.1.6 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Existing biological communities and habitat conditions would remain unchanged.
- Public recreational areas and activities would continue without disruption.
- Ongoing water supply benefits for local users, including irrigation, fire suppression, flood control, drought resilience, and municipal use, would be preserved.
- Groundwater recharge would continue to occur to support well users.
- The local economy would continue to benefit from the activities associated with the Lake
- Reservoir storage would be maintained, supporting continued water diversion to the Russian River watershed.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.1.1.7 Disadvantages

- Sediment would continue to accumulate in the reservoir, gradually reducing its flow storage capacity.
- Scott Dam would remain a complete barrier to fish passage, preventing access to upstream habitat in the upper watershed.

## 3.2 Scenario A1– PG&E Proposed Removal

PG&E's current proposed action under the Final Application for Surrender of License (PG&E, 2025c) includes the decommissioning and removal of Scott Dam and associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. The pre-dam waterways would be restored, as shown in Figure 3. The recreational facilities would be removed alongside the removal of the dam and would be restored later. A list of the specific Project Facilities/Features that would be removed, left in place, and removed without restoration are outlined in Exhibit 4 (PG&E, 2025a).

The Proposed Action under the Application for Surrender of License includes the following for the Scott Dam and Lake Pillsbury area:

- Decommissioning and removal of Scott Dam and associated facilities and features;
- Removal and restoration of certain Project recreational facilities (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands;
- Restoration of the remnant inundation zone of Lake Pillsbury.

PG&E has chosen a Rapid Dam Removal approach, expected to span three seasons. The timeline may vary depending on flow conditions, potentially extending beyond two consecutive years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards [cy]) downstream of the Eel River. Exhibit 2 illustrates the cross-section of the dam removal plan. The decommissioning plan has been split into three seasons:

1. Initial low-flow season (June–October) to dewater and remove the upper portion of the dam
2. First high-flow season following removal of the upper portion of the dam for sediment flushing (November–May)
3. First low-flow season after sediment flushing to complete final dam removal.

During the low-flow season of the first year, the reservoir drawdown would be initiated when the inflows are within the capacity of the needle valve (generally below 400 cfs). Additionally, Scott Dam Road would be connected to the temporary barge launch downstream of the terminus of the existing plunge pool below the dam by a temporary road for easy access. An adit tunnel would also be built at the base of the spillway, utilizing a temporary plug on the upstream face of the dam. Then, the upper portion of the dam would be removed using dam lowering and notching and the sediments would be dredged near the new tunnel intake. During the high-slow season between the first and second year, the adit plug would be removed, and sediment flushing would begin. Finally, during the low-flow season in the second year, dam removal and sediment flushing would be completed (See Exhibit 2).

Under the Proposed Action, PG&E would develop restoration plans to be implemented in the Scott Dam, Lake Pillsbury and recreation facilities following completion of construction activities. PG&E has not provided detailed restoration plans, and it is expected that final restoration plans would be developed between 2025 and 2027. The duration of construction, currently anticipated to be 2 years (not necessarily consecutive), and it may change based on final engineering design.

The disadvantages associated with this scenario are that there are mentions of various activities associated with removal of recreation facilities, some surveys to document ecosystem aspects, sediment management goals but no specific information provided to address the true impacts. PG&E identified a long list of unavoidable adverse impacts from the decommissioning of Scott Dam.

This scenario corresponds to the complete dam removal option as presented by PG&E (2025c). In the application there are mentions of various activities associated with removal of recreation facilities, some surveys to document ecosystem aspects, sediment management goals but no specific information provided to address the true impacts. PG&E identified a long list of unavoidable adverse impacts from the decommissioning of Scott Dam.

### 3.2.1 Scenario Findings

#### 3.2.1.1 Water Supply

This scenario would eliminate all water storage from Scott Dam, and any benefits of flood control. Water supply to the Russian River watershed would be limited to winter months and during wetter dry seasons where water is not diverted for irrigation. Under baseline existing conditions, the average monthly dry season flows are approximated between 147-222 cfs and the wet season average flows range from 296-375 cfs. In comparison, the wet and dry season average flows would be much higher at 1,237 cfs (wet) and a bit lower at 133 cfs (dry).

This scenario would increase flood risk and likely increase the frequency, intensity, and duration of flood events seen under existing conditions operations. This is attributed to storage and dam controls attenuating the flows and retaining much of the flood waters. This scenario has very low drought tolerance when compared to the baseline scenario where average monthly flows are projected to fluctuate between 17 and 376 cfs during the dry season. Flood and drought risk are difficult to quantify due to the limited unimpaired data analysis assumptions and would require more analysis to provide a higher confidence in the projected response.

The drought conditions for this scenario are difficult to project without more detailed unimpaired flow data. The Eel River, Habematolel Creek, and Rice Fork are classified by the USGS as perennial streams, while the Smokehouse and Salmon Creeks are classified as intermittent. Smokehouse and Salmon Creek are likely not to have flows during a drought condition. The Eel River, Habematolel Creek, and Rice Fork may be more drought tolerant under specific conditions, however, the Eel River is likely to have the most reliable source of flow.

The passive sediment removal will cause flow to fluctuate and become unreliable to predict until the watercourse and sediment are semi stabilized. Semi stabilization should be assumed when there are no major changes to the channel geometry and floodplain limits as well as when vegetation has been re-established and unaffected during normal wet and dry season flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed and recharging would be limited to the watercourse contributions. This could result in a reduction to the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

The changes in hydrology based on the Stillwater HEC-RAS model projected up to a 1.2-foot increase in flood waters during a 100-year event (Stillwater Sciences, 2021e). This was assuming the longitudinal profile of the Eel River would not change from the existing conditions, which is unlikely as the downstream has been sediment starved since the dam's inception and once the dam is removed, will undergo changes to the bed elevation, slope, and roughness. The modeling was also limited to just encompassing the Eel River just downstream of the Dam and utilized hydrology that was not based on precipitation gages within the watershed. The removal would have a potential higher risk for flood damage to downstream infrastructure.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.2.1.2 Fire Suppression

This scenario would eliminate any potential for fire suppression that is currently available through storage at Lake Pillsbury. There are no other alternatives to water storage included as part of this proposed scenario.

#### 3.2.1.3 Ecosystem

The decommissioning would have a high risk for adverse environmental impact during construction activities and during sediment flushing years, particularly without a robust sediment management plan. PG&E proposes passive management of sediment that could take years to decades before the sediment is no longer a concern to water quality and the aquatic and riparian habitats of the Eel River. Stillwater Sciences noted that the removal of the dam will likely result in increased sediment deposition in the Van Arsdale Reservoir in the long-term (Stillwater Sciences, 2021a). In the short-term, brief periods of significant sediment deposition and subsequent flushing by flood events are likely. Long term, there would be a positive environmental impact once the sediment and watercourses have reached a quasi-equilibrium post removal. The removal of the dam would allow for fish passage upstream to the Eel River upstream of dam and therefore provides a biological advantage.

The loss of the reservoir would likely change the ecosystem composition and biodiversity of vegetation. The moisture content of the soil will likely affect the existing and established vegetation buffers and zones where specific plant species thrive. During the transition period after the dam is removed, if no restoration efforts are put in place, a loss in vegetative cover could occur. Without mitigation and restoration, the vegetation can dry out and experience cover loss. This could lead to colonization by either upland or invasive species that are more drought tolerant but also introduce a higher risk for fire. This loss of vegetation and exposure of the reservoir bed can also create a potential risk for erosion. Over time, natural succession may lead to the development of new plant communities that are more adapted to the altered hydrology.

#### 3.2.1.4 Recreation

This scenario would remove recreation associated with activities on Lake Pillsbury such as boating and swimming. The existing piers, docks, and marina would cease to function under current use. Other recreation uses would need to be modified and may be limited in the offerings when compared to the baseline in existing conditions. This scenario could provide opportunities for other types of recreation should an agreement or provisions be made on the existing land trust.

#### 3.2.1.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Early cost estimates indicated Scott Dam rapid removal would likely cost $ 48 (Stillwater Sciences, 2021b).

PG&E's current estimate for the cost of decommissioning the overall PVP project is $532 millions (SFGate, 2025). The 2027 General Rate Case (GRC) application from PG&E suggests costs for the PVP decommissioning project will be recovered over 8 years or two GRC periods. PG&E hydroelectric decommissioning projects, including PVP, were applied a 25 percent contingency to estimates derived from a Class 5 project study. Estimates for the projected cost of $532 million were calculated through internal PG&E cost estimating models and project-specific studies of hydroelectric facilities which follow the Association for the Advancement of Cost Engineering (AACE) guidelines, corresponding to AACE Level 5 (PG&E, 2025b). This estimate is not differentiated for initial vs long-term costs, neither for Scott Dam nor other PVP areas.

#### 3.2.1.5.1 Initial Implementation Costs

This scenario would have a high initial cost for the decommissioning. A risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain. The diversion to the Russian River watershed would also need to be modified to adapt to the change in water supply reliability.

#### 3.2.1.5.2 Long-term Operation and Maintenance Costs

This scenario removes costs associated with the dam operations and maintenance. Long-term, once the sediment within Lake Pillsbury has been released and the watercourses reach a quasi-equilibrium, this scenario would have infrequent maintenance associated with the watercourses upstream of the Van Arsdale reservoir. The NERF, if established, would require sediment management and potential periodic dredging to accommodate for the long-term sediment flushing.

#### 3.2.1.5.4 Other Economic Impacts

The local economy will likely be impacted by the loss in revenue from the previous tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury.

### 3.2.1.6 Complexity

Medium-to high relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.2.1.7 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

### 3.2.1.8 Disadvantages

- Risk of Sacramento Pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and reliable flow diversion capabilities to the Russian River Watershed.

- Requires large-scale removal and disposal of dam structures and materials.
- The removal would have a potential higher risk for flood damage to downstream infrastructure.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment transport downstream of Scott Dam.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of tax revenue associated with lowering of the property values and sales tax. Lowering of property values for nearby residents.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users and lower drought resilience and fire suppression in terms of water supply.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- Less resilient to droughts and floods because storage would no longer be maintained and the outflow would not be operationally controlled.

## 3.3 Scenario A2 – Scenario A1 with Additional Mitigation

Under this scenario, PG&E proposes the full decommissioning of Scott Dam with additional mitigation efforts implemented to address County of Lake and local community concerns. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the region, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and will have higher ecosystem benefits to the community.

As mentioned before, the County of Lake, local recreation communities, fire marshals, other agencies and CalWild have identified the need to find an alternative water source to address the lost benefits currently provided by Lake Pillsbury, for firefighting needs.

The main advantage associated with this scenario is that it addresses the County’s concerns with the full decommissioning approach suggested by PG&E in the Final Application for Surrender of License.

The disadvantage associated with this scenario is that it is not feasible to adequately address all the County’s concerns with the full decommissioning of the dam and removal of Lake Pillsbury.

### 3.3.1 Scenario Findings

Scenario A2 analyzes PG&E's Scott Dam removal and the projected impacts on water supply, fire suppression, the environment, recreation, and associated costs. SLR projects the same outcomes as Scenario A1 for water supply and fire suppression.

#### 3.3.1.1 Ecosystem

Similarly to Scenario A1, the decommissioning would introduce a high risk for adverse environmental impacts during construction activities and during sediment flushing years, but with a more robust sediment management plan including restorative practices, environmental impacts would likely be less harmful for this option. SLR recommends implementing a robust sediment management plan that would designate a timeframe for when all sediment would be removed or flushed downstream and the Eel River would reach its quasi-stable state. In addition to the sediment management plan, SLR recommends performing a wetland, riparian, and channel restoration for Eel River and its tributaries. Revegetation of disturbed areas using native plants and locally sourced plants and seeds, with implementation of adaptive measures such as irrigation, is also recommended. This will help expedite the restoration process so that there are minimal impacts to the ecosystem.

#### 3.3.1.2 Recreation

As in Scenario A1, this scenario would remove recreation associated with activities on Lake Pillsbury such as boating and swimming. The existing piers, docks, and marina would cease to function under current use. Under this option, there would likely be more new recreational opportunities compared to Scenario A1, with the restored Eel River and other major creeks available for kayaking, fishing, and trail access. This scenario could provide opportunities for other types of recreation should an agreement or provisions be made on the existing land trust.

#### 3.3.1.3 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Initial capital costs to fully decommission the dam and provide sediment management were estimated at $65 million, and full dam removal with complete sediment removal and full channer restoration was estimated at $112 million (McMillen Jacob, 2018). Other cost estimates indicate Scott Dam rapid removal will likely cost $ 48 million, additional $ 39 million for sediment management including a pilot channel option and Lake Pillsbury revegetation for $ 75 million (Stillwater Science, 2021b).

##### 3.3.1.3.1 Initial Implementation Costs

This scenario would have a high initial cost for the decommissioning and sediment management, and an associated cost with the sediment management and revegetation plan. A risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain. The diversion to the Russian River watershed would also need to be modified to adapt to the change in water supply reliability.

#### 3.3.1.3.2 Long-term Operation and Maintenance Costs

This scenario does remove costs associated with the dam operations and maintenance. Long-term, once the sediment within Lake Pillsbury has been released and the watercourses reach a quasi-equilibrium, this scenario would have infrequent maintenance associated with the watercourses upstream of the Van Arsdale reservoir. The Van Arsdale reservoir would require sediment management and potential periodic dredging should it remain in place.

#### 3.3.1.3.3 Other Economic Impacts

The local economy will likely be impacted by the loss in revenue from the previous tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury.

The County of Lake will lose revenue from Forest Service cabins and the PG&E unitary tax.

### 3.3.1.4 Complexity

High relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.3.1.5 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Reduced volume of sediment transported downstream.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

### 3.3.1.6 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and flow diversion capabilities to the Russian River Watershed.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.

- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of business and lowering of property values for local residents and reduction of tax revenue for the County.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users and lower drought resilience and fire suppression in terms of water supply.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Less resilient to droughts and floods because storage would no longer be maintained and the outflow would not be operationally controlled.

## 3.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

This scenario is similar to the current operational conditions described in Section 2.1, with the addition of a fish ladder, as previously evaluated by Mead and Hunt (2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, seismic retrofitting would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules, and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures.

To maintain operational conditions would require the dam to be retrofitted and for dam operations and maintenance to be taken over by other agencies or entities. Immediate actions to the dam would require a seismic retrofit which would induce an increase in maintenance costs. This scenario also has the highest long-term costs from maintenance and operations. Permitting considerations would need to be evaluated. This scenario would continue the positive economic impacts on local businesses and tax revenue due to recreational use associated with Scott Dam and Lake Pillsbury.

For the purposes of this initial evaluation the scenario was considered unfeasible, and no detailed evaluation was conducted at this moment. Pending analysis of Scenario C, this scenario may be reconsidered.

### 3.4.1 Scenario Findings

Scenario B maintains the same operational characteristics as the current conditions described in Section 3.1 in terms of hydrology and hydraulic conditions, fire suppression, recreation, ecosystem, and local economic benefits.

The key distinction lies in the seismic retrofitting, addition of a fish ladder and associated infrastructure upgrades. These improvements introduce increased initial implementation costs due to the construction of the fish ladder and necessary seismic retrofitting of the dam. Over the long-term, Scenario B also would entail additional operations and maintenance efforts to ensure the continued functionality of the fish ladder, dam structure, and flow control systems.

#### 3.4.1.1 Cost Conditions

Initial cost estimates valued this scenario to have a capital cost of $26 million for upstream fish passage and $45.5 million for downstream fish passage (McMillen Jacobs, 2018). Other early estimates for adding a modified conventional fish pathway to Scott Dam may add $101 million (Stillwater Sciences, 2021b) and additional maintenance costs for fish ladder operations.

#### 3.4.1.2 Complexity

Medium relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.4.1.3 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Existing biological communities and habitat conditions would remain unchanged.
- Public recreational areas and activities would continue without disruption.
- Potential increase in resident/anadromous fish production.
- Ongoing water supply benefits for local users, including irrigation, fire suppression, flood control, drought resilience, and municipal use, would be preserved.
- Groundwater recharge would continue to occur to support well users.
- Reservoir storage would be maintained, supporting continued water diversion to the Russian River Watershed.
- Would provide upstream fish passage over the dam.
- Construction and maintenance access from existing access road on southern dam abutment.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.4.1.4 Disadvantages

- Sediment would continue to accumulate in the reservoir, gradually reducing its flow storage capacity.
- Uncertain fish passage effectiveness, particularly for downstream migration.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level would need to be maintained within a set range to support operation.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.

- Would require extensive excavation and construction activities on the left abutment of the dam.
- Tunnel through concrete dam face may introduce additional seismic concerns.

## 3.5 Scenario C – Lowered Scott Dam with Fish Ladder

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 ft (PG&E datum) and the installation of a fish passage system such as a fish ladder structure. Scenario C is similar to Scenario B, and it is considered more feasible as the capital improvements costs would be lower for a reduced height dam, and the water pressure reduction would increase stability.

The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 ft would create a reservoir of approximately 40 percent of the current storage volume at water surface elevation 1,900 ft. Potentially the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury water surface elevations and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage as the current maximum operational water surface elevation of 1,900 ft, the spillway invert may be further reduced in height by 25 ft below the existing spillway invert which would result in a new lake footprint (Figure 4). At that reduced maximum operational water surface elevation the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam by placing the concrete material in both or either of the downstream and upstream sides of the lowered dam along either the river or lake embankments. Further evaluation would be required to understand the effects on the adjacent land area, and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation and ecosystem benefits as compared to full decommissioning.

Recreational facilities will need to be relocated to the new lake shore. Groundwater levels will also likely be reduced. Slope stability for the new lake shore needs to be considered, as PG&E currently has a minimum lake level for slope safety.

### 3.5.1 Design and Construction Considerations

Design aspect would need to include additional Geotechnical Feasibility and other material reuse evaluations.

### 3.5.2 Scenario Findings

#### 3.5.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury by 35,870 ac-ft by lowering the maximum operation water surface elevation from 1,900 ft to 1,875 ft. Flood control, downstream water diversion requirements, and operational reliability would be impacted. Flood control and downstream water diversion flow requirements would be affected due to reduced dam operational control and the passive operation of the spillway. An updated low-level outlet structure that would be able to operate at low water surface elevations and have the capacity to maintain minimum ecological flow requirements to Eel River.

This scenario would generally increase flood risk due to the stopping of active operations of the spillway gates which allowed for greater control, attenuation, and retention of flood inflows. This scenario has reduced drought tolerance due to a lower average water surface elevation and storage in Lake Pillsbury than the current operational conditions. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation. It is recommended that existing sediment be mitigated to increase the available water supply.

The Lake Pillsbury reservoir's ability to recharge the groundwater would likely be reduced, due to the reduced reservoir size. Groundwater recharge would continue to occur, but the water table may be lowered as the reservoir level lowers.

Additional evaluation of the slope stability of the existing slope would need to be considered. Currently PG&E has maintained a minimum water surface elevation due to slope safety concerns.

This scenario proposes that seasonal fish passage be provided during two seasons per year. The first season would be during November-December, and the second season would be during April-May to meet fish passage requirements. Lake Pillsbury is expected to be able to provide the required flowrates to properly allow fish passage.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.5.2.2 Fire Suppression

This scenario will be able to provide dependability for providing fire suppression. While Lake Pillsbury water storage and surface area would be reduced when compared to the baseline scenario it is expected to continue to operate as an extensive fire suppression use water source. This results in satisfying the operational requirements for extensive fire suppression use. Expected to allow for full use in support of fire suppression requirements for plane and helicopter methods.

#### 3.5.2.3 Ecosystem

Lake Pillsbury has historically provided habitat for various species which would continue for this scenario. The proposed fish ladder would allow for both fish passage and mitigation of existing sediments. This scenario involves implementing a robust sediment management plan with a timeframe and method for the removal or flushing of sediments in Lake Pillsbury. A low-level outlet structure that would be operational at a lower water surface elevation and allow for managed sediment flushing behind the dam. Sediment management would reduce the impacts to downstream ecosystems when compared to the full removal of Scott Dam.

#### 3.5.2.4 Recreation

This scenario would have an impact on recreational activities on Lake Pillsbury such as boating and swimming. Facilities at the northwestern portion of Lake Pillsbury and Salmon Creek may cease to function under the current use. A portion of other recreational facilities will potentially need to be relocated to the lowered Lake Pillsbury shoreline such as existing piers, docks, and marina. Other recreation uses would need to be modified and may be limited in the offerings when compared to the baseline scenario.

#### 3.5.2.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Costs for lowering of Scott Dam have not been estimated. Fish passage costs are anticipated to be similar to the current elevation of Scott Dam, with initial cost estimates of a capital cost of $26 million for upstream fish passage and $45.5 million for downstream fish passage (McMillen Jacobs, 2018). Other early estimates for adding a modified conventional fish pathway to Scott Dam may add $101 million (Stillwater Sciences, 2021b) and additional maintenance costs for fish ladder operations.

#### 3.5.2.6.1 Initial Implementation Costs

This scenario would have a high initial cost for the lowering of Scott Dam which would include a new dam spillway, construction of the fish ladder structure, and updating to a low-level outlet structure. The required fish ladder structure for the lowered dam height configuration would be of reduced cost compared to a potential fish ladder structure at the current existing dam height configuration. To continue long-term operations of Scott Dam under this scenario would require the dam operations and maintenance to be taken over by other agencies or entities.

#### 3.5.2.6.2 Long-term Operation and Maintenance Costs

An assessment is recommended to better understand if the existing needle valve structure could be modified or if it would need to be removed to provide adequate scenario requirements. The use of a low-level outlet that has the capability for managed sediment on a periodic basis would potentially reduce the economic impact to downstream structures, facilities and watershed due to large sediment flushing volumes when compared to full removal of Scott Dam.

#### 3.5.2.6.3 Other Economic Impacts

This scenario would provide a slight reduction in benefits to local businesses and tax revenue due to reduced recreational use associated with Scott Dam and Lake Pillsbury.

### 3.5.2.6 Complexity

Medium relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.5.2.7 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Would provide upstream and downstream fish passage.
- Potential increase in resident/anadromous fish production.
- Restoration of historical ecosystem function and dynamics.
- Minimizes sediment release from Lake Pillsbury reservoir.
- Potential improvement to water quality and temperature due to reduction of reservoir.
- May reduce seismic instability concerns.
- Maintain some benefits of water supply, fire suppression, flood control, drought resilience, and recreation.
- Groundwater recharge would continue to occur, potentially with a lower groundwater table, to support well users.

- Construction and maintenance access from existing access road on southern dam abutment.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.5.2.8 Disadvantages

- Potential modifications to the existing recreational activities may need to be maintained or reduced.
- Existing biological communities and habitat conditions will be impacted in the short term from the change in lake footprint and depth.
- Exposed and saturated steep slopes along the adjusted lake perimeter will be susceptible to failure and sloughing.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level has to be maintained within a set range to support operation.
- May not be fully effective during summer periods due to change in water temperature due to the surface draw of the fishway and lower-level release to the river.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.
- Partial Dam removal and material disposal.
- Impacts to downstream water users and reduced flood control and drought response.
- Reducing the storage capacity of Lake Pillsbury would make the area more prone to droughts, and reducing the height of the dam would make the area more prone to floods.

## 3.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure. A new 3,100-foot-long earthen impoundment dam would be established in the Salmon/Smokehouse and Squaw Valley Creeks watershed area to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir (Figure 5). The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within Lake Pillsbury footprint can also be reused for the new impoundment construction. Low permeability clay/silt sediments can be used in the core of the dam and gravelly sediments can be used in the embankment/slopes of the new dam.

The water storage will be based on the Salmon/Smokehouse and Squaw Valley Creeks watershed area, and it is assumed the maximum capacity (approximately 9,990 ac-ft) would be provided at a maximum operational water surface elevation of 1,885 ft. For this scenario, the approximate surface area would be 580 acres, approximately 30 percent of the current size (water surface elevation 1,900 ft). The new impoundment dam would be constructed at the

narrowest location possible that would not impair the flows through the Eel River. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational water surface elevation. An active low-level outlet structure would not be included, though a passive outlet is considered for potential draining. This option will support continued use of recreations facilities, ecosystem benefits, fire suppression and the upstream fish pathway along the Eel River as Scott Dam will be removed as a fish barrier.

### 3.6.1 Design and Construction Aspects

If this scenario is considered, the design would involve:

1. Considerations on geotechnical feasibility.
2. Volumes of materials required for this scenario and cross sections.

A constructability review will be needed to identify challenges, complexity and cost efficiencies.

### 3.6.2 Scenario Findings

#### 3.6.2.1 Water Supply

This scenario would reduce water storage within the area to approximately 30 percent of the current water storage in Lake Pillsbury. Flood control, downstream water diversion flow requirements, and operational reliability would be impacted, as passive management is proposed.

This scenario would generally increase flood risk due to the higher flows from the Upper Eel River watershed. The water supply capacity will not be actively managed.

Groundwater recharge would continue to occur on the north shore, providing some reliability to existing well owners.

The storage feasibility needs to be evaluated, as the alluvial deposits in the north area are not well understood. Geotechnical feasibility needs to be evaluated due to the faults in the area.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.6.2.2 Fire Suppression

This scenario will be able to provide dependability for providing fire suppression. While water storage and surface area would be reduced when compared to the current availability of Lake Pillsbury, the smaller reservoir is expected to continue to operate as an extensive fire suppression use water source. This results in satisfying the operational requirements for extensive fire suppression use. Expected to allow for full use in support of fire suppression requirements for plane and helicopter methods.

#### 3.6.2.3 Ecosystem

Lake Pillsbury has historically provided habitat for various species which would continue for this scenario.

Implementing a robust sediment management plan that designates a timeframe and method for the removal or flushing of would reduce the impacts to downstream ecosystems when compared to the full removal of Scott Dam. The sediment management plan could be similar to what was proposed in Scenario A2.

Due to the smaller watershed area that is being impounded, a fish passage for this watershed is not deemed necessary since the larger Eel River and Rice Fork watersheds would be unimpaired.

#### 3.6.2.4 Recreation

This scenario would have an impact on recreation associated with activities on Lake Pillsbury such as boating and swimming. Facilities in the northwestern portion of Lake Pillsbury and Salmon Creek may experience a reduction in use, unless dredging occurs to accommodate extension of the footprint. A portion of other recreational facilities will potentially need to be relocated to the lowered Lake Pillsbury shoreline such as existing piers, docks, and marina. Other recreation uses would need to be modified and may be limited in the offerings when compared to the current conditions.

#### 3.6.2.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

This scenario would include similar initial costs to Scenario A2. Initial capital costs for full decommissioning of the dam with sediment management were estimated at $65 million (McMillen Jacobs, 2018). Additional costs, pending assessment, will be required for the earthen dam construction.

##### 3.6.2.5.1 Initial Implementation Costs

This scenario would have a very high initial cost for removal of Scott Dam and construction of the earthen dam. The reuse of material from Scott Dam, residual sediments within Lake Pillsbury footprint would provide construction materials at lower costs than importing from other sources. The complexity of the engineering design and construction costs need to be further evaluated. The required dam length will need to be approximately 3,100 ft. The quantity of materials to construct the proposed dam are large resulting in high construction costs.

##### 3.6.2.5.2 Long-term Operation and Maintenance Costs

To continue long-term operations of the new earthen dam under this scenario, the long-term costs would be low. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational water surface elevation.

##### 3.6.2.5.3 Other Economic Impacts

This scenario would provide a minor reduction in economic activity, and minor effects on local businesses and tax revenue.

#### 3.6.2.6 Complexity

Highest relative complexity due to design of a new earthen dam, when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.6.2.7 Advantages

- Reduced dam operations and maintenance.
- A portion of the existing biological communities and habitat conditions would remain unchanged or undergo short term changes while the new dam is being constructed.

- Public recreational areas and activities would continue albeit with a small reservoir. Recreation activities may be limited to a shorter duration of the year, dependent upon water supply.
- Eliminates dam-specific operations, maintenance, and long-term safety liabilities of Scott Dam.
- Maintenance of a reduced reservoir for recreation and fire suppression.
- Groundwater recharge would continue to occur to support well users.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat within the Eel River and Rice Fork.
- Restores historic river ecosystem functions and natural hydrologic dynamics within the Eel River and Rice Fork.
- Over time, parts of the Eel River ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### 3.6.2.8 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of controlled water storage and flow diversion capabilities to the Potter Valley Powerhouse.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment plume.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the altered reservoir footprint.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- Requires extensive engineering and construction of new dam.

- Reducing the storage capacity of Lake Pillsbury would make the area more prone to droughts, and reducing the height of the dam would make the area more prone to floods.

## 3.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2 but with specific mitigation added to enhance fish passage while maintaining fire suppression, and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[12], the off-channel ponds would be constructed.

Two potential pond locations have been identified as shown in Figure 6 based on the 1922 pre-dam topography (McBain Associates, 2018). The 1922 topography was georeferenced to the Lake County hillshade to predict how the site will look upon restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table 3 summarizes the potential footprints and volumes of ponds A and B assuming a 10-foot normal depth (Figure 6). The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design.

Figure 7A shows a longitudinal profile along the centerline of the Historic Eel River Upstream from Scott Dam (adapted from Stillwater Sciences' November 2021) with the approximate locations and proposed depths of Ponds A and B.

The design function similarly to the example provided in Figure 8, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table 3: Potential Footprint and Storage Volume of Ponds A and B under Scenario E.**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

The disadvantages associated with this scenario are high constructability costs.

### 3.7.1 Design and Construction Aspects

Considerations on volumes of materials required for sediment removal for this scenario and constructability aspects will be required.

### 3.7.2 Scenario Findings

The predicted scenario outcomes are similar to A2 but with additional benefits and outcomes with the inclusion of the ponds.

#### 3.7.2.1 Water Supply

The water supply for this scenario is of higher benefit than scenarios A1 and A2 but significantly lower than the current conditions. This scenario allows for storage from the proposed ponds.

The benefits of this scenario include the positive environmental impacts, the preservation of capacity for fire suppression and downstream use for municipalities.

Under extreme runoff conditions, the following design considerations need to be evaluated:

- Ponds would only allow for helicopter fire suppression support.
- Ponds may alleviate certain flood flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed and recharging would be limited to the watercourse contributions. This could result in a reduction to the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

This scenario is not likely to drastically change flows during the extreme flood conditions as the ponds will be located within the floodplain. Drought conditions would likely see a reduction from the Scenarios A1 and A2 predictions but are difficult to quantify or predict with the currently available data. The reduction or impacts to the extreme climate conditions would be dependent upon the infiltration rates, design goals, usage (fire suppression), and groundwater tables.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.7.2.2 Sediment and Channel Restoration

SLR reproduced Stillwater Sciences' longitudinal profiles to show the sediment accumulation from 1921-2016, showing how the dam's presence for 94 years has contributed to the deposition of sediment in Lake Pillsbury's upstream tributaries. Up to 60 ft of sediment has been deposited in Eel River upstream of Scott Dam in some locations (Figure 7A). Up to 10 ft of sediment has been deposited in Salmon Creek upstream of Scott Dam in some locations (Figure 7B). Prior to construction of the ponds, the sediment needs to be managed, channels need to be restored to natural conditions, and the ponds would be constructed on the natural riverbed.

#### 3.7.2.3 Fire Suppression

Depending on the area and depth of the adjacent ponds, any of the three operational scales presented in Table 1 (Section 2.2) could be feasible. For extensive use, there would likely be a need to excavate the ponds to create the depths needed for the high storage capacity. The preliminary pond sizes considered would allow for fire suppression with helicopters.

This option has the added benefit of creating off-channel floodplain habitat and open water sources sufficient in size to be used in fire suppression operations.

#### 3.7.2.4 Ecosystem

The ponds would not function as impoundments and therefore would not impede fish passage in the Eel River. This scenario would eliminate the existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage.

In addition to removal of the existing fish barrier, the proposed off-channel ponds adjacent to the Eel River would create spawning grounds for fish and other aquatic species, which provide additional ecological benefits. However, during the period between dam removal and the system reaching quasi-equilibrium, sediment flux is likely to stall the flourishing of the aquatic ecosystem. Therefore, it is important to develop and follow a robust sediment management plan.

#### 3.7.2.5 Recreation

Restoring natural river channels, along with creating off-channel ponds, can significantly enhance recreational opportunities in comparison to Scenarios A1 and A2. These changes may support expanded fishing, kayaking, birdwatching, and nature-based tourism. Off-channel ponds can provide calm-water areas for recreational activities, while restored channels may improve habitat diversity, attract more outdoor enthusiasts and boost local ecotourism.

#### 3.7.2.6 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

This scenario would include similar initial costs to Scenario A2. Initial capital costs for full decommissioning of the dam and completely remove sediments were estimated at $112 million (McMillen Jacobs, 2018). Additional costs, pending assessment, will be required for construction of the ponds.

##### 3.7.2.6.1 Initial Implementation Costs

This scenario would have a higher initial cost for the decommissioning and sediment management compared to Scenario A2. Associated costs for the sediment management plan, channel restoration and pond creation are the factors making this option more expensive. Similarly to Scenario A2, a risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain.

##### 3.7.2.6.2 Long-term Operation and Maintenance Costs

Following initial sediment management and pond construction, this scenario would have infrequent maintenance associated with the watercourses in the footprint of Lake Pillsbury.

The ponds would be designed to be sustainable and resilient so that they would reasonably withstand larger flow events and require little to no maintenance.

##### 3.7.2.6.3 Economic Impacts

Like scenarios A1 and A2, the local economy will likely be impacted by the loss of revenue from tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury. There may be opportunities for minor recreational activities in the vicinity of the newly built ponds that may attract some visitors.

#### 3.7.2.7 Cost/Complexity

High relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.7.2.8 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Maintains some benefits of water supply, fire suppression flood control, drought response, and recreation through establishment of standing water in off-channel ponds.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat. This enhancement is increased from scenario A1 by adding ponds to support spawning and shelter for fish and aquatic species.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Reduced volume of sediment transported downstream in the long-term due to active sediment management.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.
- This option provides the highest potential benefit to water supply when compared to the other scenarios where the dam is being removed.
- The wetland ponds would be resilient to flooding, since they act like sponges and absorb flood waters effectively.

#### 3.7.2.9 Disadvantages

- Risk of Sacramento Pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and flow diversion capabilities to the Russian River Watershed.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment plume.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of business and lowering of property values for local residents and reduction of tax revenue for the County.

- Need for long-term ecosystem monitoring and potential mitigation measures post-removal. This would be of a lesser degree with active sediment management and restoration efforts than scenario A1.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- The ponds would be vulnerable to drought, since they would be vulnerable to evaporation.

# 4.0 Preliminary Comparison of Scenarios

## 4.1 Criteria for Comparison

McMillen Jacobs (2018) completed a high-level feasibility study of potential alternatives for modifications to the Potter Valley Project. Different criteria were identified and organized in a matrix. SLR has considered similar criteria to prepare a comparison matrix for our evaluation of alternatives and added additional criteria to address the County's specific criteria. These criteria are presented in the sections below.

## 4.2 Rating System

The rating system used by SLR to compare the alternative decommissioning scenarios is adapted from the rating system used in the McMillen Jacobs (2018) evaluation matrix. However, the McMillen Jacobs ranking system was modified to be better suited for the County specific scenarios evaluated. The SLR ranking system is presented below.

| Ranking | Description |
|---|---|
| Very Good | Would be successful or have no impact. |
| Good | Would have a high likelihood of success or minimal impact. |
| Average | Would have a moderate likelihood of success or a significant impact. |
| Poor | Would have a poor likelihood of success or a significant impact. |

### 4.2.1 Modified 2018 McMillen Jacobs' Criteria

Complete descriptions for the criteria below can be found in *Section 4.3.2 Evaluation Matrix* of in McMillen Jacobs (2018). SLR's modified criteria are described below:

**Biological Efficiency (Fish Passage)**

This criterion presents a measure of the ability of the scenario to attract, guide, and pass fish over the dam and/or fishway passage efficiency.

A "poor" for this criterion would indicate that the scenario poorly supports fish passage and. A "very good" for this criterion would indicate that the scenario supports fish passage, both upstream and downstream.

**Constructability Challenges**

This criterion is intended to identify construction challenges. The construction challenges associated with a specific scenario or alternative may lead to elimination of the alternative due to construction challenges or geotechnical stability issues.

Considerations for this may include access availability, such as adequate routes for construction, geotechnical stability issues during construction, availability of utilities to support project construction and other construction considerations.

A “poor” for this criterion would indicate that the scenario faces construction challenges that may require complex solutions to overcome. A “very good” for this criterion would indicate that the scenario can be constructed minimal difficulty, using standard methods and equipment, with few or no geotechnical or logistical constraints.

**Environmental Impact – During Construction**

This criterion is intended to identify environmental impacts during construction that could make the project difficult or costly to construct or difficult to permit. The criterion would be applied considering only the construction phase of the alternative implementation. Consideration relates to riparian habitats, degradation of water quality, disruption to wildlife, and visual or aesthetic impacts.

A rating of “poor” under this criterion indicates that construction activities are likely to cause significant environmental harm, requiring extensive mitigation measures and potentially complicating the permitting process. A rating of “very good” indicates that construction would result in minimal environmental disturbance, with limited or no adverse effects on sensitive resources, and would likely face fewer regulatory hurdles.

**Environmental Impact – Post Construction**

Post-construction environmental considerations relate to the final project configuration and operation, and its impact on the natural resources and environment. This criterion identifies potential long-term impacts to riparian habitats, degradation of water quality, disruption to wildlife, and visual or aesthetic impacts.

A rating of “poor” under this criterion indicates that post-construction activities are likely to cause significant environmental harm, requiring extensive mitigation measures and potentially complicating the permitting process. A rating of “very good” indicates that construction would result in minimal environmental disturbance, with limited or no adverse effects on sensitive resources, and would likely face minimal regulatory hurdles.

**Operational Impact (Long-term operations)**

This criterion captures the potential impacts of long-term operations under the option or alternative operation as well as overall general operation complexity and challenges. It considers water storage and release for downstream users, sediment management, and water quality.

A rating of “poor” indicates that the scenario presents significant operational challenges—such as complex water management, high maintenance demands, or persistent water quality issues—that could be costly or difficult to manage over time. A rating of “very good” indicates that the scenario supports reliable, low-complexity operations with minimal maintenance needs and effective management of sediment and water quality, ensuring long-term sustainability and regulatory compliance.

**Design Approach**

The design approach criterion is intended to determine which scenarios would require relatively simple system designs with proven technologies.

A rating of “poor” indicates that the scenario requires a highly complex or unproven design. A rating of “very good” indicates that the scenario can be implemented using well-established, low-complexity design approaches.

**Initial Implementation Cost**

This criterion considers the anticipated level of capital investment including construction, engineering, planning, regulatory and permitting, and administration that would be associated with the scenario implementation.

A “poor” for this criterion would indicate a high construction/implementation cost. A “very good” for this criterion would indicate low construction/implementation cost.

**Long-term Operation and Maintenance Costs**

This criterion evaluates the long-term operational costs and level of effort associated with each scenario.

A “poor” for this criterion would indicate high operational costs and/or level of effort, and a “very good” for this criterion would indicate low operational costs and/or level of effort.

**Criteria not Considered**

The following criteria, although included in McMillen Jacobs (2018), were not considered in the evaluation of SLRs. These criteria may be revisited and assessed in future phases of this project.

- Cost sub criteria: Certainty in Capital Estimate and Life Span
- Regulatory and Permitting
- Safety Risk

### 4.2.2 County of Lake Specific Criteria

Based on additional considerations not included in the 2018 evaluation by McMillen Jacobs, the following additional criteria is included in this high-level feasibility evaluation.

**Water Supply**

The County is in an ecosystem that historically oscillates between wet and dry years. With changes in the climate, droughts have increasingly become a problem in this region.

Presently, Lake Pillsbury provides residents with access to drinking water via groundwater recharge. This criterion evaluates how each scenario impacts resident accessibility to groundwater as a source of drinking water. This criterion also considers the impact of each scenario on groundwater recharge and how it will affect the residents’ local wells used to access drinking water.

Lake Pillsbury also currently provides the residents of the County with a consistent water storage source. This criterion discusses the impacts of each scenario on the ability to keep a consistent, reliable source of water storage for the residents to use. This criterion offers a comprehensive view of water storage as opposed to the criterion “fire suppression” and “access to drinking water” which have a more specific lens.

A “poor” for this criterion would indicate that the scenario has low potential for access by local residents and other users and also scenario provide no or minimal water storage to manage drought and wet year impacts. A “very good” for this criterion would indicate that the scenario has a higher potential for water user access and provide necessary storage to manage wet and drought tolerances.

**Fire Suppression**

Wildfires are common in Lake County. The Mendocino National Forest and surrounding areas have a long history of significant wildfires. Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts (Friends of the Eel River, 2023).

Many common fire mitigation strategies such as helicopter bucket operations, fixed-wing aircraft water scooping, and ground-based drafting for fire engines and water tenders require a minimum volume of water to function. This criterion evaluates each scenario and its effects on fire suppression strategies.

A “poor” for this criterion would indicate that the scenario poorly supports fire suppression and mitigation efforts. A “very good” for this criterion would indicate that the scenario supports fire suppression and mitigation efforts.

**Recreation**

Lake Pillsbury serves as a recreational facility in and around the lake, including camping, boating, swimming, fishing, picnicking, wind surfing, and bird watching. This criterion evaluates the effects of each scenario on recreational activity, and the complexity of improving/relocating recreational facilities.

A “poor” for this criterion would indicate that the scenario poorly supports current recreational activity and/or future recreational activity. A “very good” for this criterion would indicate that the scenario supports current recreational activity and/or future recreational activity.

**Effects on Local Economy**

Each scenario has broader economic consequences for the local community. Changes in water availability, land use, and recreational opportunities can influence property values, business activity, and municipal tax revenues. This criterion evaluated these economic trade-offs, both for the community and the County of Lake.

A “poor” for this criterion would indicate that the scenario poorly supports homeowners, local businesses and the local economy in the County. A “very good” for this criterion would indicate that the scenario supports property values, local businesses and the local economy in the County.

### 4.2.3 Scenario Comparison Matrix

SLR has developed a Mitigation Scenario comparison table using the above discussed McMillen Jacobs and County of Lake criteria and included in this report (Table 5, appended). Descriptions to support the criteria rating are provided in the scenario outcomes in Section 3 and summarized in the table. The ranking is based on SLR subject matter expertise.

Based on SLR’s evaluation, PG&E’s Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by McMillen Jacobs (2018) as part of a consultation group. In contrast, we identified five other alternatives that rank more favorably and should be further evaluated for implementation.

# 5.0 Climate Vulnerability Considerations

Appendix D includes a description of the California Adaptation Planning Guide, PG&E's identified cumulative impacts from Scott Dam removal, and considerations from County of Lake's Climate Vulnerability Analysis.

Maintaining a safe and reliable water supply is essential for climate resilience in Lake County. The historical operations of Scott Dam and Lake Pillsbury provided strong protection against drought, flooding, wildfire, and extreme temperatures. As climate stressors intensify, the ability to store and manage water becomes increasingly critical for public health, agriculture, infrastructure, and emergency response.

This climate vulnerability analysis shows that scenarios which retain or enhance water storage and operational control offer the highest resilience. In contrast, scenarios involving full dam removal without replacement introduce significant vulnerabilities unless paired with robust mitigation strategies.

# 6.0 Permitting and Conservation Considerations

## 6.1 Permitting

Future permitting aspects are discussed in Appendix E. The main permitting aspects will include the following:

- Federal Permitting Process (FERC Lead), to comply with the National Environmental Policy Act (NEPA). An Environmental Impact Statement (EIS) is anticipated.
- California Permitting Process (State Water Resources Control Board as Lead and DODS) to comply with the California Environmental Quality Act (CEQA). An Environmental Impact Report (EIR) is anticipated.

## 6.2 Conservation Easement

A conservation easement for 5,660 acres of PG&E-owned land, of which approximately 2,234 acres are within the FERC Project boundary, was recorded on June 28, 2022, to permanently protect beneficial public values on lands owned by PG&E (PG&E, 2025a, PG&E, 2022a). The conservation easement is held by Mendocino Land Trust.

PG&E owned lands are already encumbered by easements held by Mendocino Land Trust. The easement permanently restricts development and protects conservation values (habitat, recreation, cultural resources) regardless of PG&E's future use. The conservation easement includes PG&E's right for continued operation and maintenance of hydroelectric facilities and associated water delivery facilities, PG&E's unequivocal ability to comply with any FERC license, FERC license renewal, or other regulatory requirements.

If PG&E ceases power operations, the land could be sold or transferred, but any new owner must comply with the conservation easement. This ensures no future industrial or residential development inconsistent with the agreed conservation purposes. Mendocino Land Trust is to enforce conservation restrictions; they do not own the land but can limit certain uses.

The easement aims to protect, preserve, or enhance the Beneficial Public Values on the property, including:

- Habitat for fish, wildlife, and plants

- Forest resources
- Scenic viewshed
- Outdoor recreation
- Historical and cultural values
- Agricultural values

There may be potential limitations for appropriate restoration activities as proposed in some of the mitigation scenarios discussed in this document. Additional evaluation, beyond the scope of this study, and negotiations with the Mendocino Land Trust for changes in the conservation easement may be required for appropriate restoration.

# 7.0 Conclusions

SLR has completed a preliminary evaluation of the impacts on the Lake County based on the Scott Dam decommissioning activities proposed by PG&E's Application for Surrender of License to FERC. SLR identified impacts on several key areas including water supply, recreation, fire suppression, ecosystem, restoration and economic conditions. Each of these impact areas were evaluated for some details of technical aspects as applicable, such as hydrology and hydraulics, geotechnical consideration and construction. Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Five alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Current Operational Conditions Scenario covers operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Final Application for Surrender of License dated July 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel Creek watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and installation of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

The above scenarios were evaluated for meeting the County of Lake and PG&E goals and a comparison of scenarios provided to help support the County of Lake's decision-making process.

Based on SLR's evaluation, PG&E's Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by previous studies, indicating shortcomings in its anticipated outcomes. In contrast, we identified five other alternatives that rank more favorably and should be further evaluated for implementation as part of FERC's PVP surrender.

The information provided in this report may also be helpful in the following activities.

- Provide comments to FERC on the Final Application for Surrender of License
- Evaluation of future climate vulnerability aspects associated with the project
- Submit this report to comply with final deliverable per DWR grant requirements

# 8.0 Remarks

The services described in this work product were performed in accordance with generally accepted professional consulting principles and practices. No other representations or warranties, expressed or implied, are made. These services were performed consistent with our agreement with our client. This work product is intended solely for the use and information of our client unless otherwise noted. Any reliance on this work product by a third party is at such party's sole risk.

Opinions and recommendations contained in this work product are based on conditions that existed at the time the services were performed and are intended only for the client, purposes, locations, time frames, and project parameters indicated. The data reported and the findings, observations, and conclusions expressed are limited by the scope of work. We are not responsible for the impacts of any changes in environmental standards, practices, or regulations subsequent to performance of services. We do not warrant the accuracy of information supplied by others, or the use of segregated portions of this work product.

This work product presents professional opinions and findings of a scientific and technical nature. The work product shall not be construed to offer legal opinion or representations as to the requirements of, nor the compliance with, environmental laws rules, regulations, or policies of federal, state or local governmental agencies.

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# Figures

## Impact Analysis and Adaptation Strategy Evaluation Report

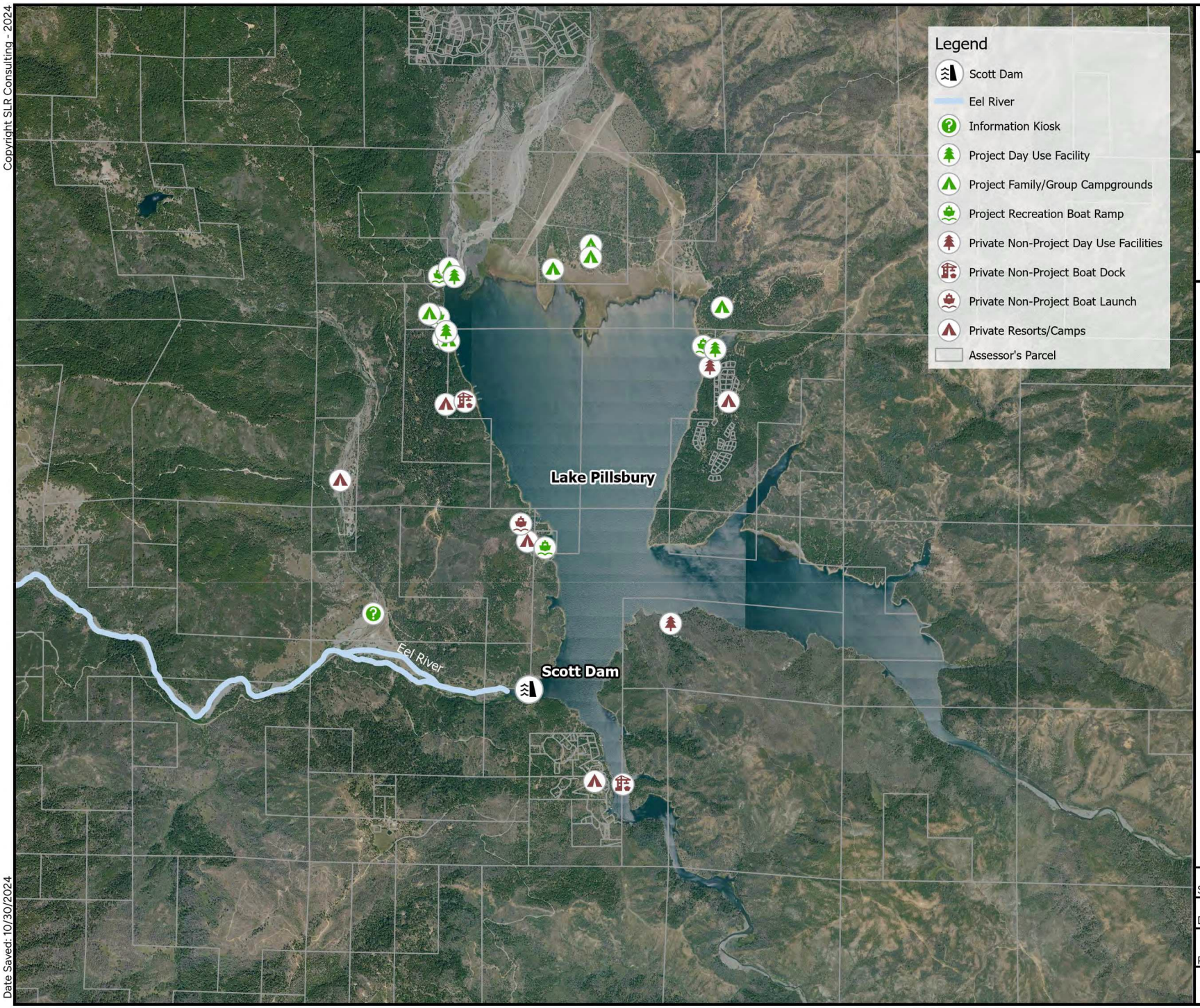
Lake County Resource Assessment – Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

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Date Saved: 10/30/2024

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0 4,000

1 in. = 4,000 ft

SLR

SITE MAP (RECREATION)

Potter Valley Project Decommissioning

County of Lake

SCALE 1:48,000

DATE 10/30/2024

102.021249.00001

PROJ. NO.

**FIG. 1**

0066

Sources:
California Division of Regional Assistance
California Department of Water Resources
PG&E Initial Surrender Application

**Measured Historical Scott Dam Outflow to Eel River (2019-2024)**

**Minimum:** 42 cfs
**Average Annual:** 522 cfs
**Average Dry Season:** 230 cfs
**Average Wet Season:** 954 cfs
**Maximum:** 26,600 cfs

N

**Legend**

Scott Dam Location

**Measured Historical Lake Pillsbury Level**

Minimum - 1871 ft (16,800 ac-ft)

Average - 1895 ft (48,100 ac-ft)

Maximum - 1909 ft (75,400 ac-ft)

**PG&E Operational Lake Pillsbury Levels**

Maximum since 2023 at Spillway Elevation 1900 ft (56,300 ac-ft)

Minimum 1860 ft (10,000 ac-ft)

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 2**

**CURRENT CONDITIONS (WY 2019-2024)**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR

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Layer Credits: Esri, NASA, NGA, USGS, Earthstar Geographics, Bureau of Land Management [2025]

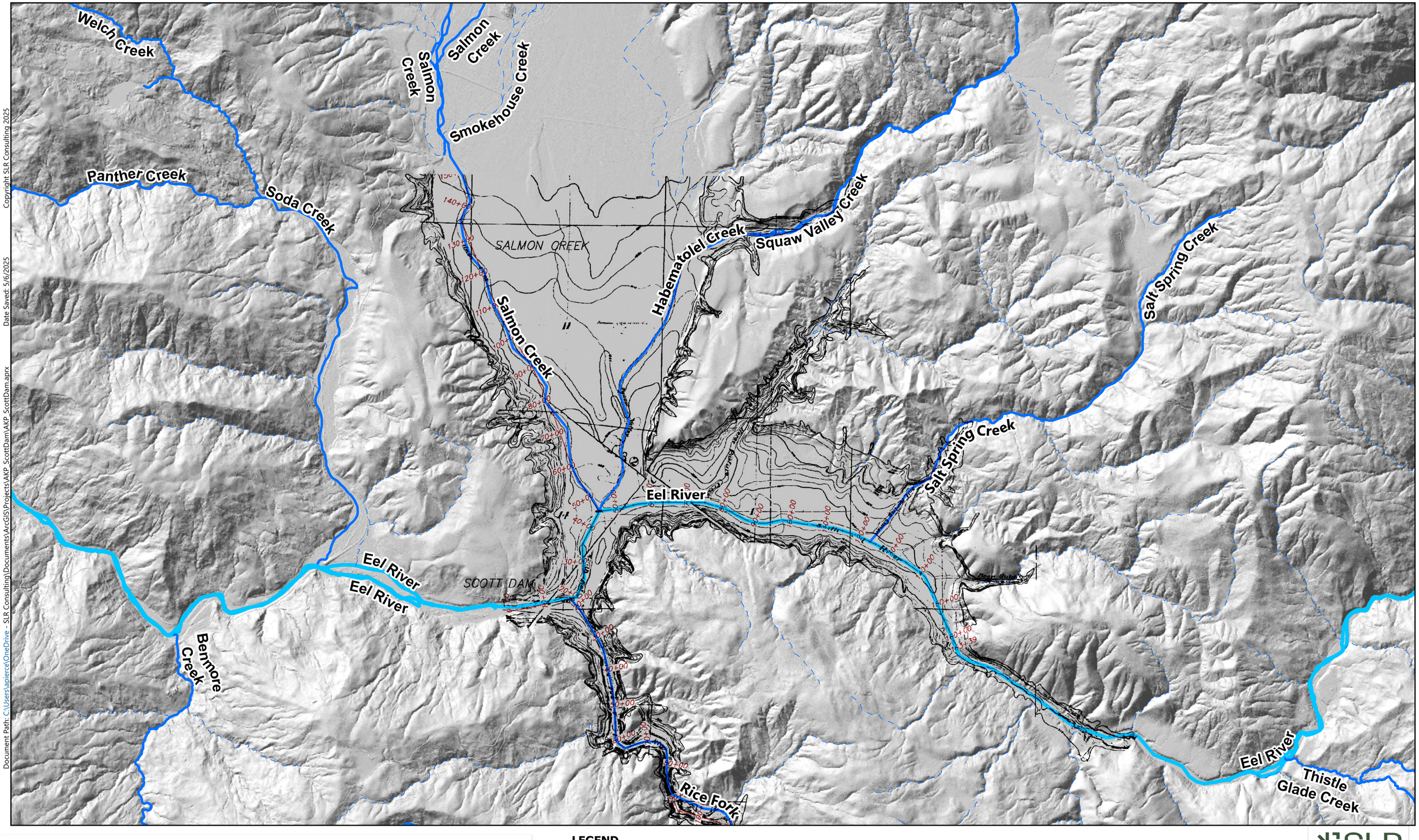


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**Figure 3:** SCENARIO A1/A2 – SCOTT DAM DECOMISSIONING

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

- Rivers
- Creeks
- Seasonal Creeks

Hillshade
High : 254
Low : 0

1922 Pillsbury Bathymetry
Red: Band_1
Green: Band_2
Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

SLR

0068

N

**Legend**

Scott Dam Location

**Conceptual Operational Lake Pillsbury Level**

Lowered Spillway Elevation 1875 ft (20,000 ac-ft)

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 4**

**SCENARIO C - LOWERED SCOTT DAM WITH FISH LADDER CONCEPTUAL LAKE FOOTPRINT**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR

COUNTY OF LAKE STATE OF CALIFORNIA

N:\Irvine\GIS\Projects\Irvine\300144 County of Lake\102.021249.00001 Scott Dam\Map\2025 04 Meeting with Council\2025 04 Meeting with Council.aprx

Salmon Creek, Smokehouse Creek, and Habematolel Creek Tributaries = 14% of Scott Dam Watershed

N

Salmon Creek
Smokehouse Creek
Habematolel Creek
Eel River
Salt Spring Creek
Eel River
Eel River
Rice Fork
Eel River

**Legend**

**Conceptual Salmon Creek Dam Layout**

- Crest
- Footprint

**Scenario D Calculated Lake Pilsbury Levels**

- Minimum: 1875 ft (5,000 ac-ft)
- Average: 1885 ft (9,500 ac-ft)
- Maximum: 1900 ft (20,000 ac-ft)

**Watercourse Feeding Lake Pilsburry**

- Eel River
- Tributary

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 5**

**SCENARIO D - SMALLER RESERVOIR IN THE SALMON CREEK**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR

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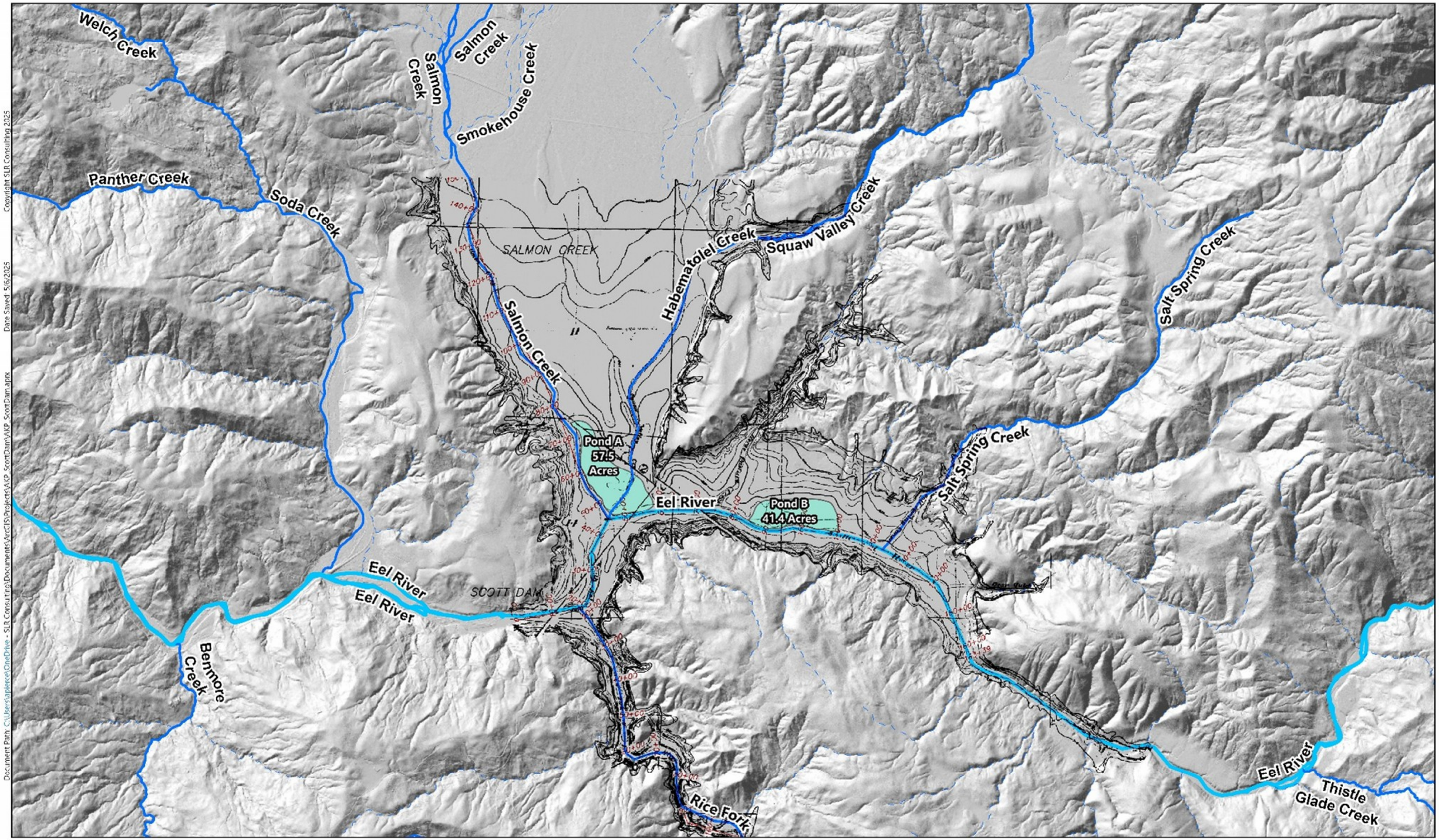



**Figure 6:** SCENARIO E - OFF-CHANNEL PONDS

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

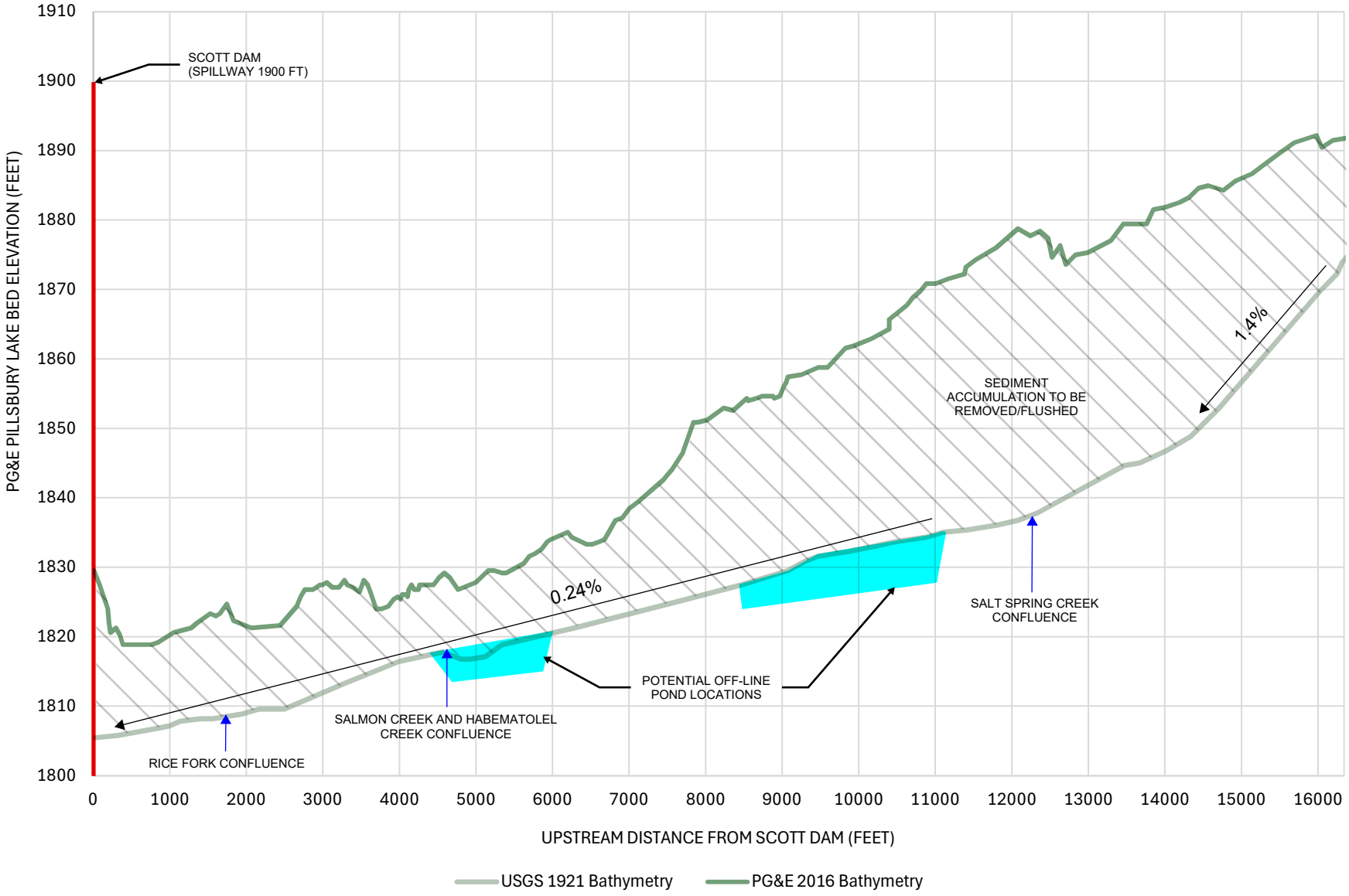
- Rivers
- Creeks
- Seasonal Creeks
- Ponds
- Hillshade
  - High : 254
  - Low : 0
- 1922 Pillsbury Bathymetry
  - Red: Band_1
  - Green: Band_2
  - Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

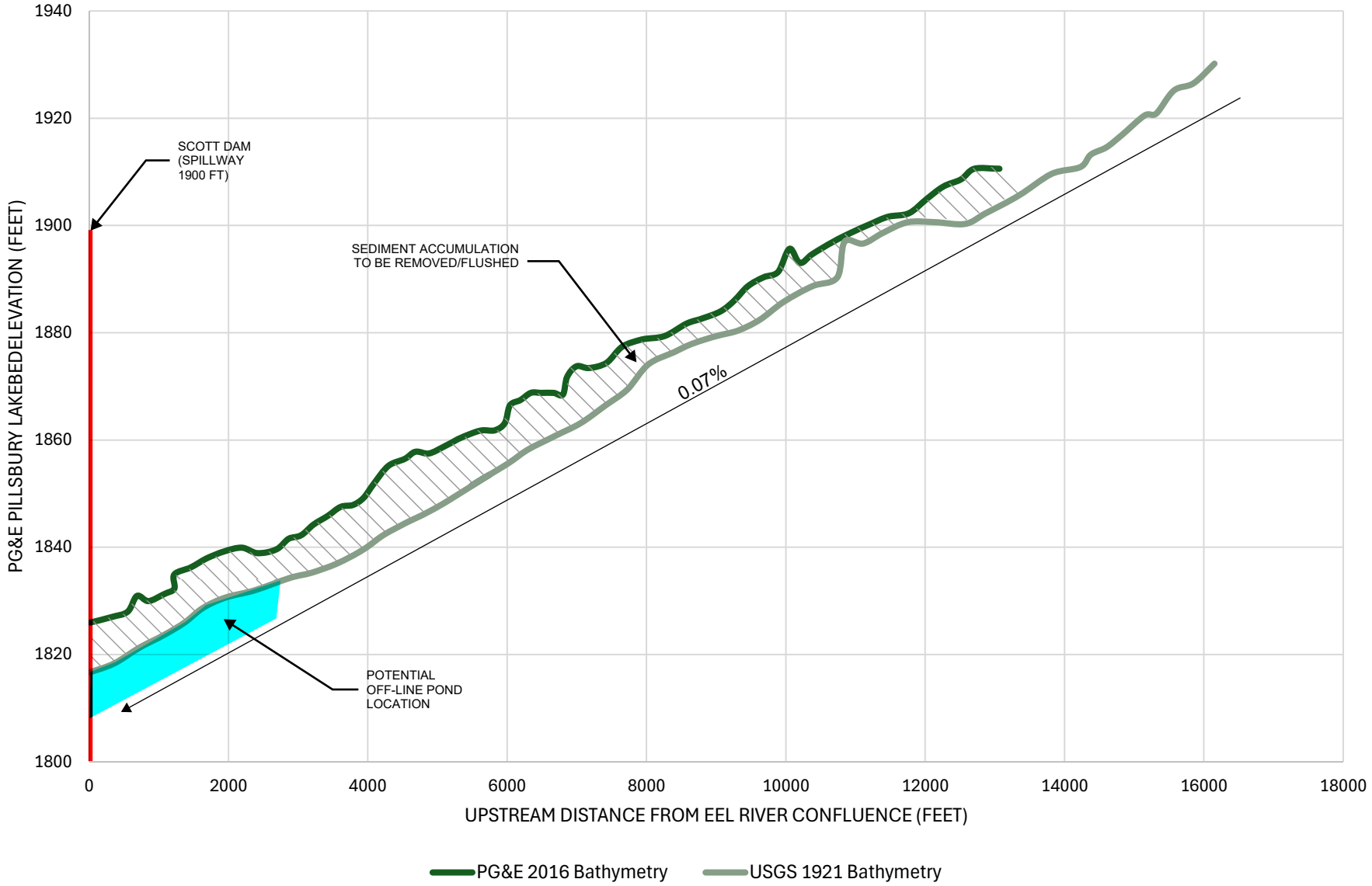
SLR

**FIGURE 7A:** LONGITUDINAL PROFILE ALONG THE CENTERLINE OF THE HISTORIC EEL RIVER UPSTREAM FROM SCOTT DAM




SLR

**FIGURE 7B:** LONGITUDINAL PROFILE ALONG THE CENTERLINE OF THE HISTORIC SALMON CREEK UPSTREAM FROM EEL RIVER CONFLUENCE




SLR

0073

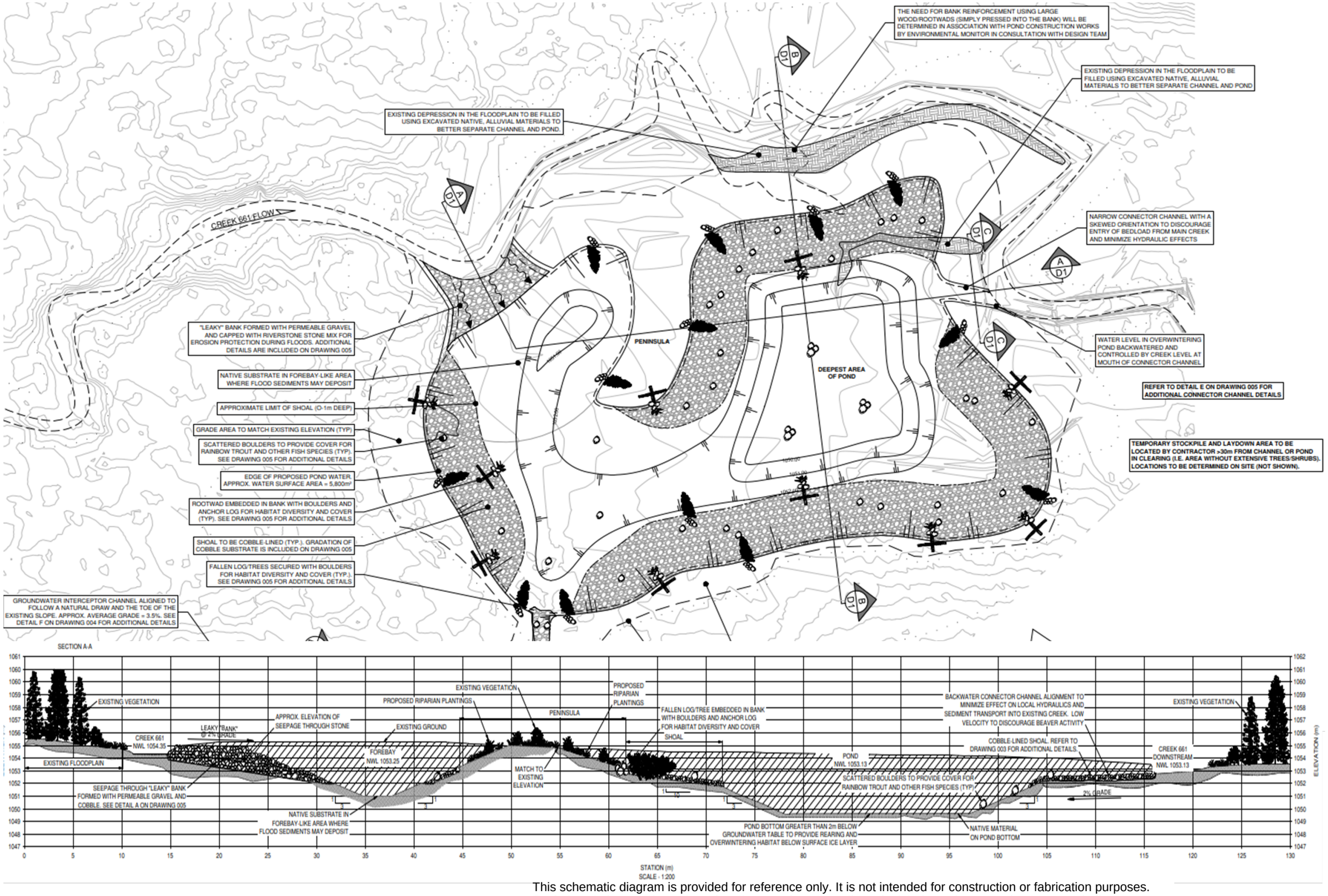



**Figure 8: Example Design of Off-Channel Pond**

This schematic diagram is provided for reference only. It is not intended for construction or fabrication purposes. Scale is not shown. Dimensions, locations, and configurations depicted may be approximate or symbolic. Diagram is based on Palmer/SLR's *Blackwater Gold Fish Habitat Offsetting. Creek 661 Overwintering Pond Detailed Design* dated February 28, 2023.

# Exhibits

## Impact Analysis and Adaptation Strategy Evaluation Report

Lake County Resource Assessment – Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

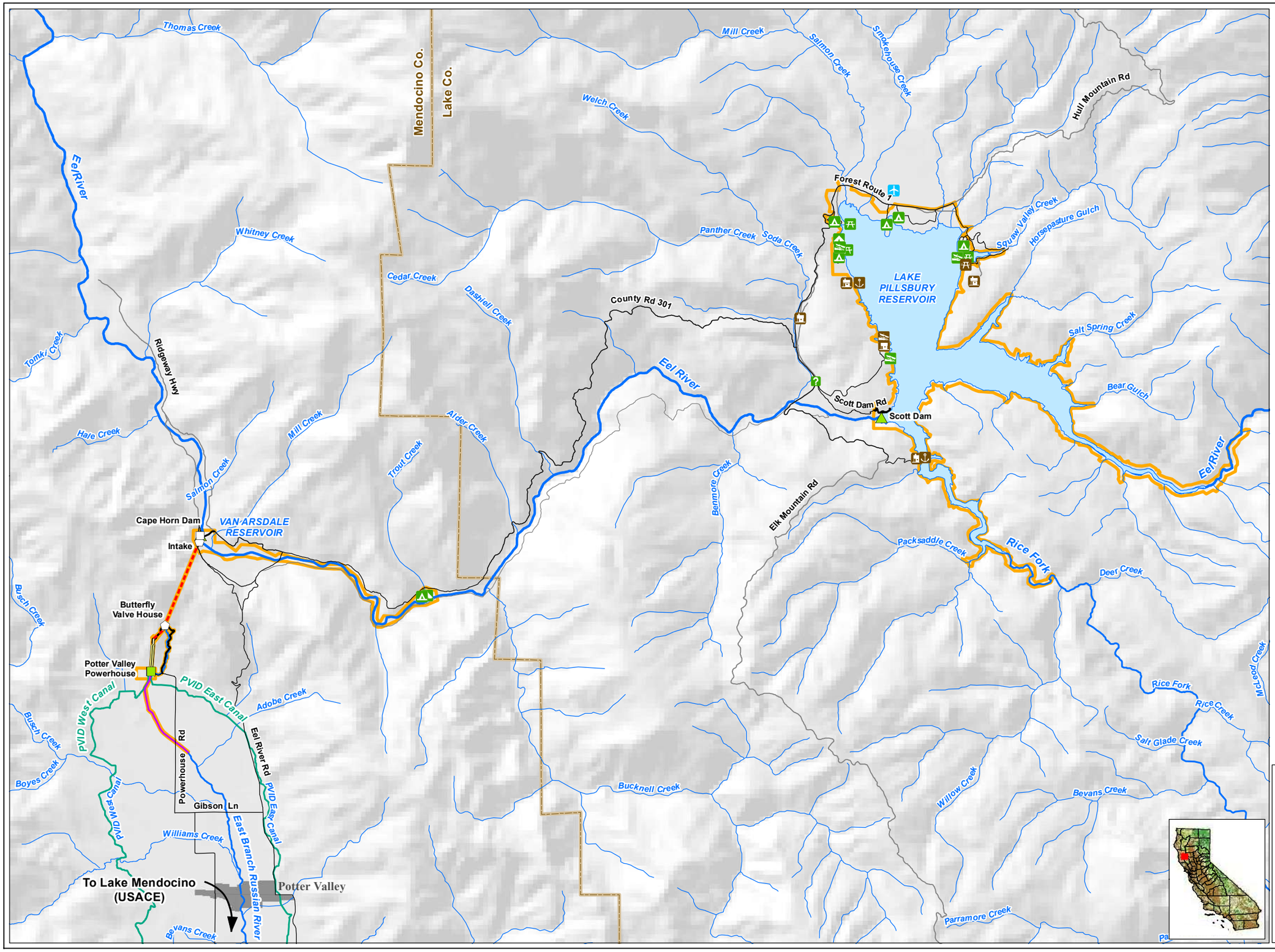
SLR

# Exhibit 1 Potter Valley Hydroelectric Project Site Plan

SLR

**Exhibit 1**

Potter Valley Hydroelectric Project Site Plan




**PG&E Project Facilities**

- Powerhouse
- Dam
- Water Conveyance Feature
- Water Conduit
- Penstock
- Powerhouse Discharge Canal
- FERC Project Boundary

**Project Recreation Features**

- Boat Ramp
- Family Campground
- Group Campground
- Day Use Facility
- Information Kiosk

**Private Non-Project Recreation Features**

- Boat Launch
- Boat Dock
- Day Use Facility
- Resort, Camp, or Tract

**Other Features**

- Airstrip
- Access Road
- General Access Road
- Other Road
- Canal
- Watercourse
- Waterbody

Pacific Gas and Electric Company®

PG&E Potter Valley Project
FERC Project No. 77

**Map 1-1**

**Project facilities and features.**

0 0.5 1 Miles

Projection: UTM Zone 10 N
Datum: NAD 83

Date: 7/15/2025

C:\Users\kgabel\OneDrive - Stantec\gisnt\185706343\map\LandMaps\Map 1-1_ProjectFacilities&Features_17i11i_10.mxd

Source:

PG&E, 2025. Final Application for Surrender of License VOLUME IA PUBLIC INFORMATION. July.

# Exhibit 2 Scott Dam Removal Cross Sections

SLR

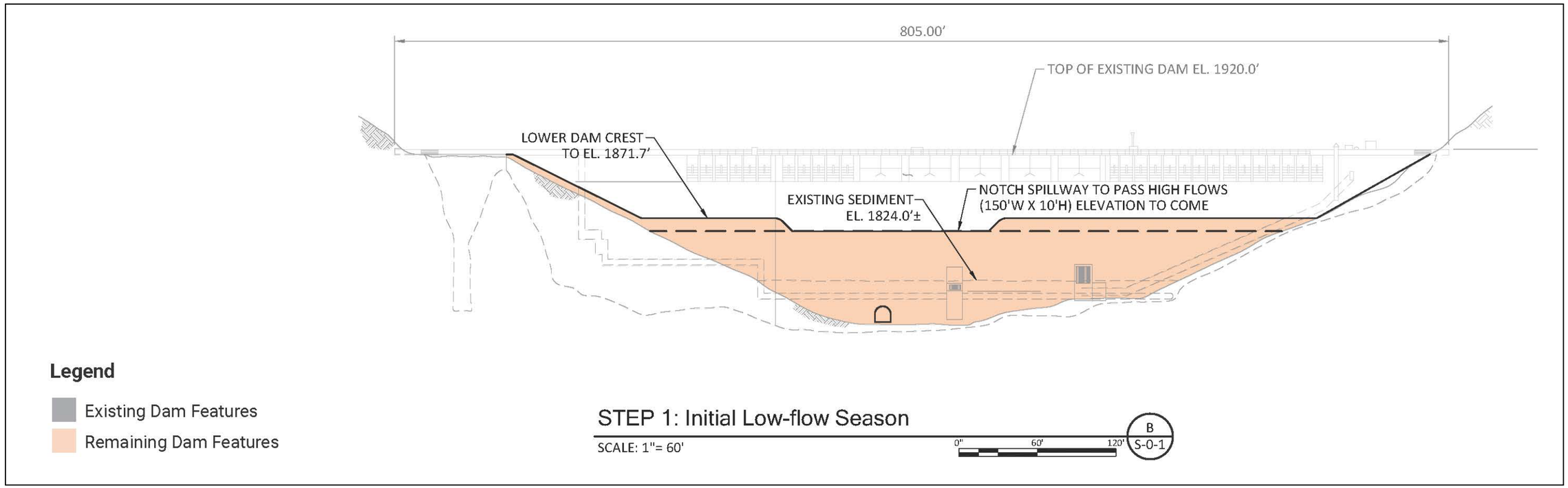



**Figure 2-2. Scott Dam removal—initial dam removal cross section (cross section—initial low-flow season).**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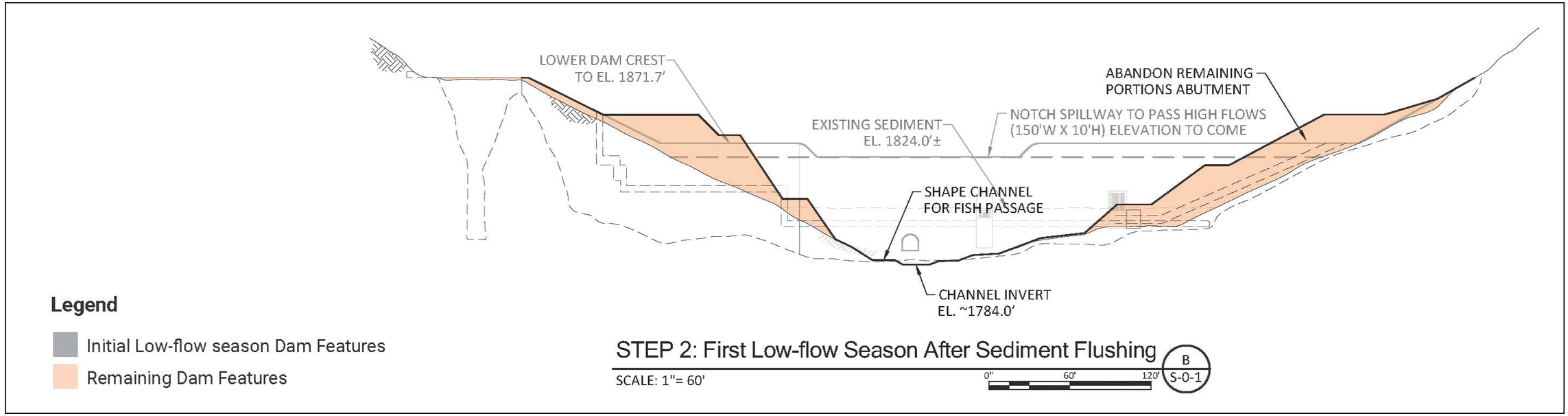


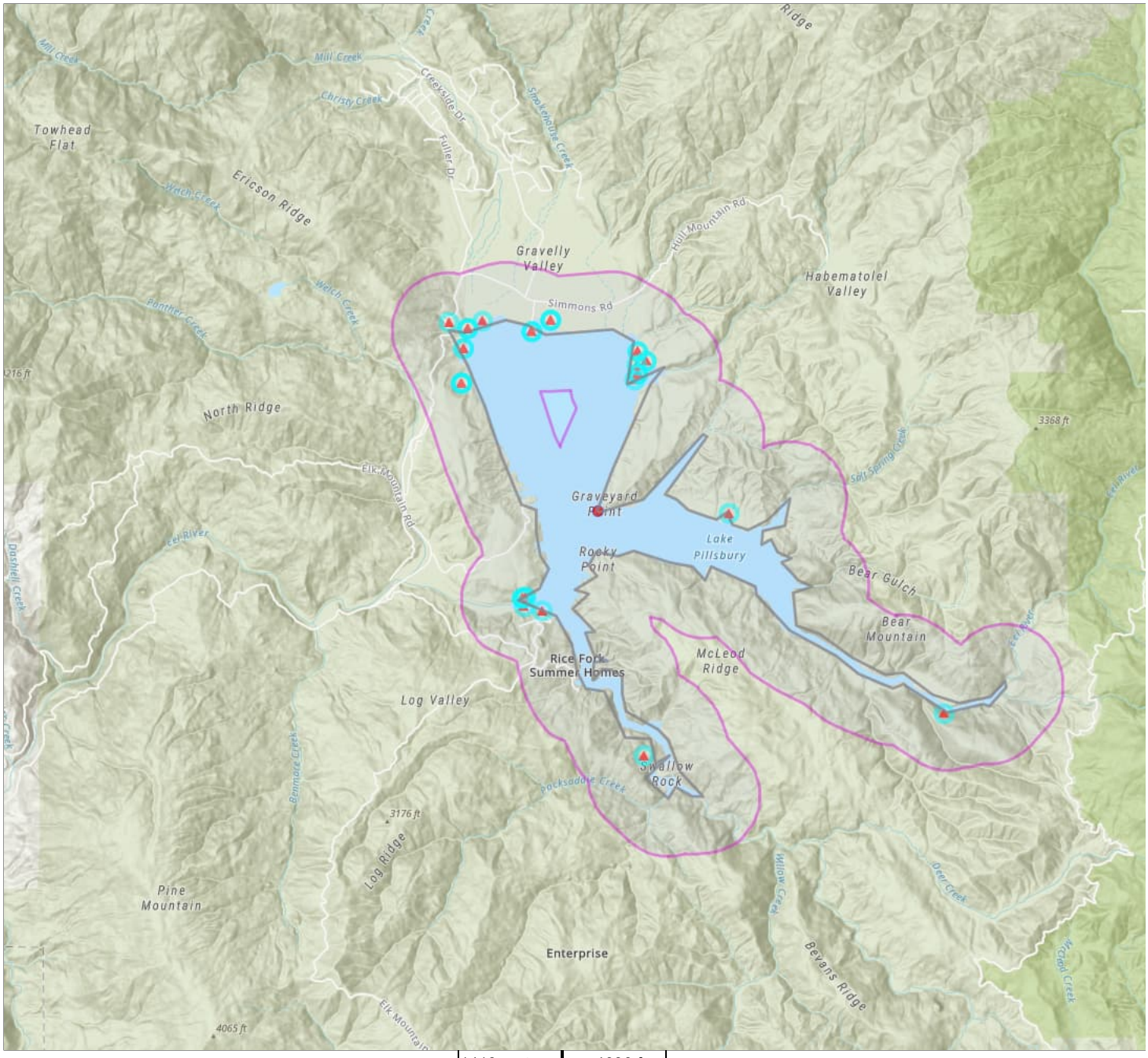

**Figure 2-3. Scott Dam removal—final dam removal cross section (cross section—first low-flow season after sediment flushing).**


©2025, Pacific Gas and Electric Company

# Exhibit 3A Identified Species Near Lake Pillsbury (0.5 Mile Radius)

**Exhibit 3A**

Identified Species Near Lake Pillsbury (0.5 Mile Radius)




| 1413 meters | 4636 feet |
|---|---|

Endangered Species identified within 0.5 miles of Lake Pillsbury Permitter

**Map Legend**

**Unprocessed Data from CNDDB Online Field Survey Form [ds1002]**

Unprocessed Data from CNDDB Online Field Survey Form _ ds1002

- Animal
- Plant

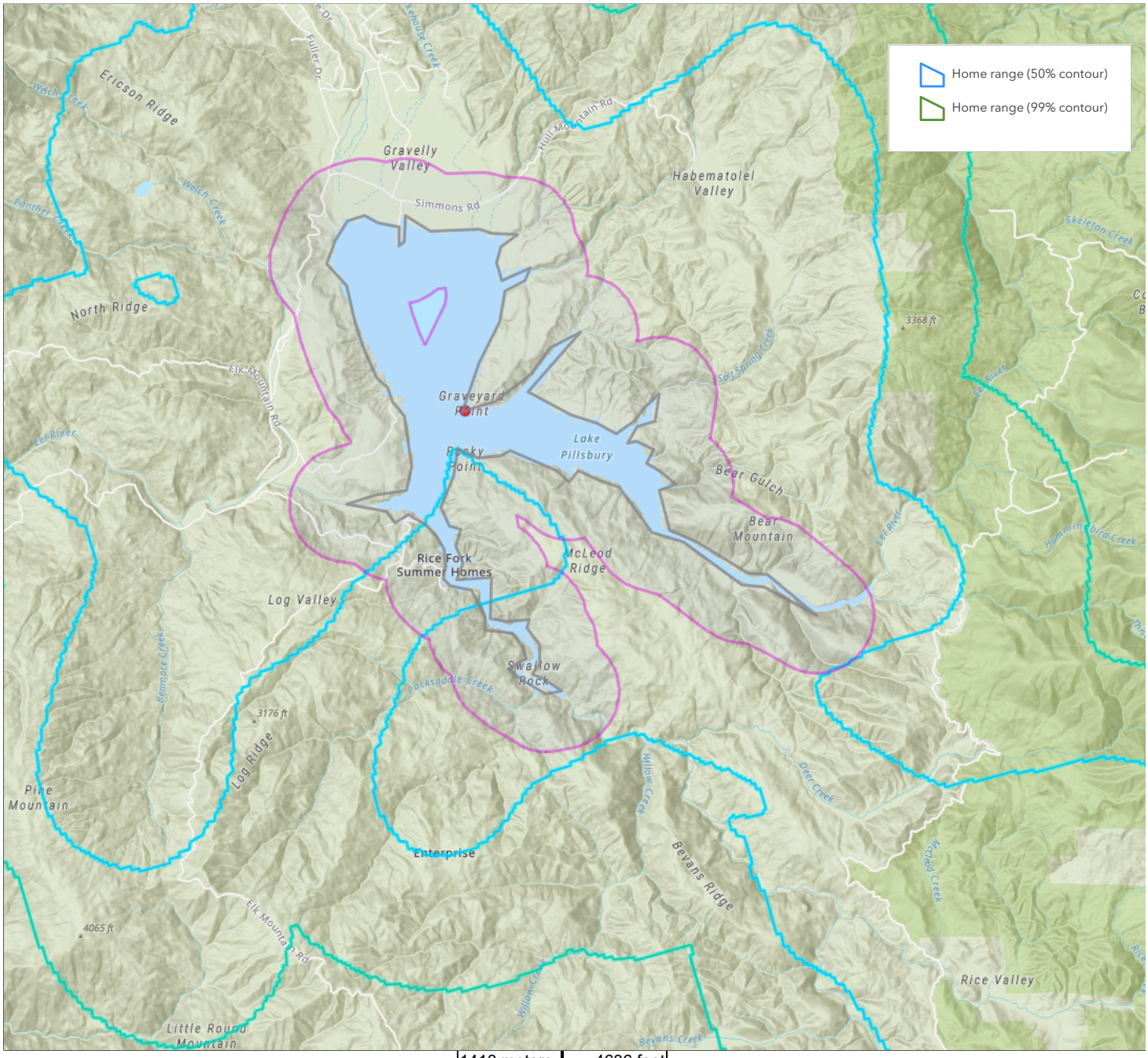
Source:

California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System. https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

# Exhibit 3B Tule Elk Contours

**Exhibit 3B**

Tule Elk Contours




| | |
|---|---|
| 1413 meters | 4636 feet |

Description: Elks identified nearby Lake Pillsbury that may enter the 0.5 mile radius range.

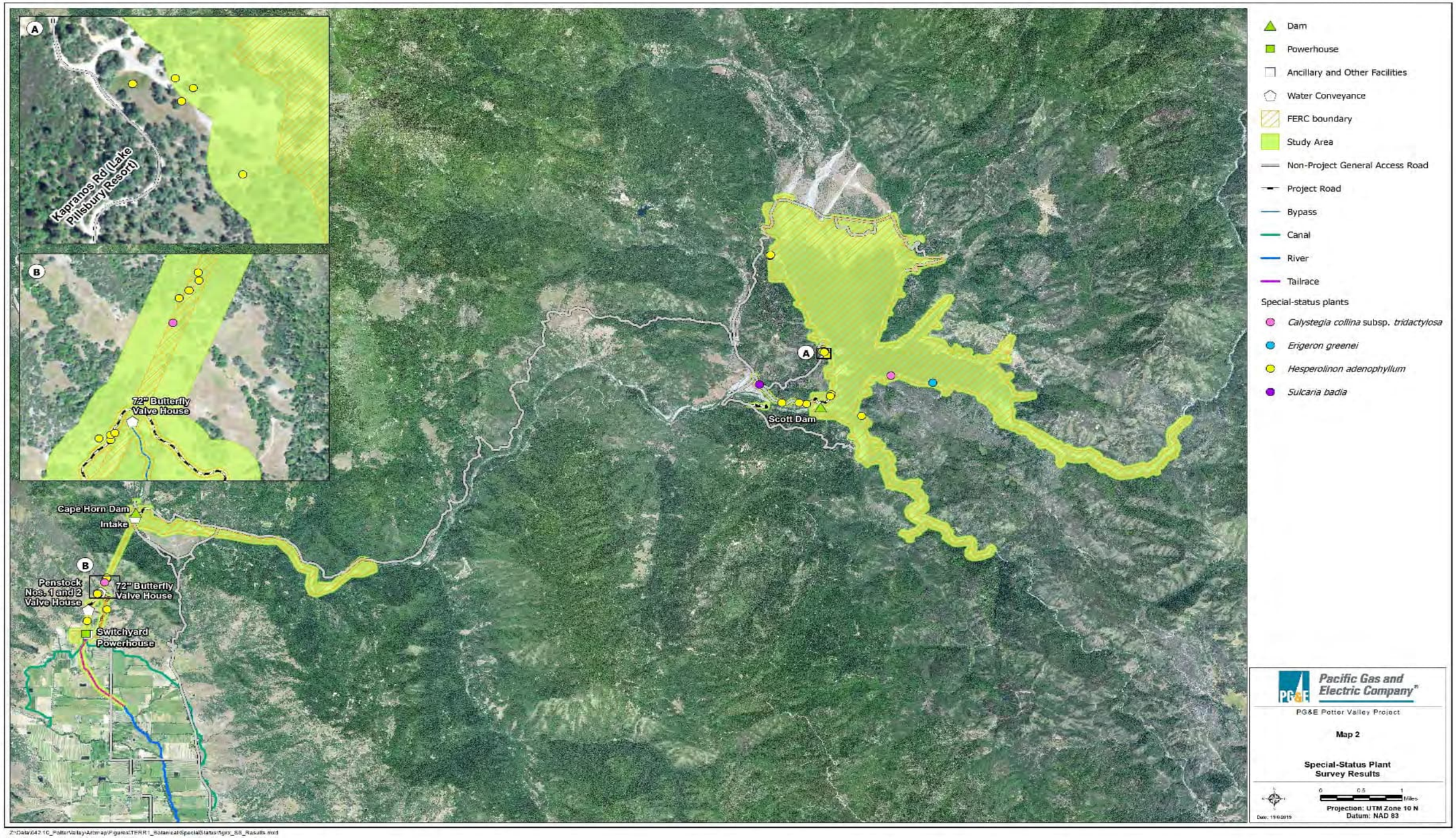
Source:

California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System.https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

# Exhibit 3C Special-status Species Plants Documented in the Study Area During the 2018 Botanical Survey

**Exhibit 3C**

Special-status species plants documented in the Study Area During the 2018 Botanical Surveys




Source:

Cardno and Stillwater Sciences, 2019. TERR 1 – Botanical Resources Data Memorialization, Potter Valley Project (FERC Project No. 77). December 5

# Exhibit 4 Decommissioning of Project Facilities and Features in the Scott Dam Area

***Scott Dam Area***

The following provides a description of decommissioning of Scott Dam and associated Project facilities and features and Project recreation facilities. Table 2-8 provides a summary of decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area and the associated existing land ownership. Decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area includes leaving the facility in place (L), removal of the facility with restoration (RR), and removal of the facility with no restoration (X).

**Table 2-8. Decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area.**

| Project Facility/Feature or Project Recreation Facility | Land Ownership | Decommissioning |
|---|---|---|
| **Dam and Associated Facility/Features** | | |
| Scott Dam | PG&E | RR |
| Valve Control House | PG&E | X |
| ***Reservoir*** | | |
| Lake Pillsbury (storage reservoir) | PG&E/USFS | RR |
| ***Reservoir Gage*** | | |
| E1—Lk Pillsbury NR Potter Valley CA (11470000) | PG&E | X |
| ***River Gages*** | | |
| E2—Eel R BL Scott Dam NR Potter Valley CA (11470500) | PG&E | L |
| **Leakage Weirs and Piezometers and Associated Trail[1]** | | |
| Scott Dam Leakage Weirs | PG&E | RR |
| Scott Dam Piezometers | PG&E | L (cap) |
| Scott Dam Piezometers and Leakage Weir Access Trail | PG&E | L |
| **Project Communication Line** | | |
| Scott Dam Block Building Communication Line | PG&E | X |
| **Ancillary and Support Facilities** | | |
| Scott Dam Block Building | PG&E | L (remove equipment only) |
| Scott Dam Boat Barrier | PG&E | X |
| **Access Roads[1]** | | |
| Scott Dam Rd (portion) | PG&E | L |
| Upper Scott Dam Access Rd | PG&E | L |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| Project Facility/Feature or Project Recreation Facility | Land Ownership | Decommissioning |
|---|---|---|
| **Project Recreation Facilities** | | |
| ***Family Campgrounds*** | | |
| Fuller Grove Campground | USFS | RR |
| Navy Campground | USFS | L |
| Oak Flat Campground | USFS | L |
| Pogie Point Campground | USFS | L |
| Sunset Point Campground | USFS | L |
| ***Group Campgrounds*** | | |
| Fuller Grove Group Campground | PG&E | RR |
| **Day-Use Facilities** | | |
| Eel River Visitor Information Kiosk | USFS | RR |
| Fuller Grove Day-Use Area and Boat Launch | PG&E | RR |
| Pillsbury Pines Day-Use Area and Boat Launch | PG&E | RR |
| Pogie Point Day-Use Area | USFS/PG&E | L |

[1] Not listed as a Project road or trail in the license.

Notes: L = leaving the facility in place
RR = full or partial removal of the facility with restoration
X = removal of the facility with no restoration.

*Scott Dam and Associated Project Facility/Feature Removal*

PG&E will implement the Rapid Dam Removal approach to decommission Scott Dam. The description of Rapid Dam Removal is based on the *Scott Dam and Cape Horn Dam Removal Alternatives Technical Memorandum* (McMillen Jacobs Associates 2021), existing site conditions, and technical expertise and is described in more detail below. Scott Dam Area elevations cited herein are based on PG&E's datum, which equals NGVD29 + 81.7 ft.

Rapid Dam Removal entails expedited removal of Scott Dam (approximately 2 years in duration, but the 2 years may not be consecutive depending on site conditions and flows) such that no water is impounded and volitional fish passage and unimpaired flows occur once decommissioning is completed. The structure would no longer be under the jurisdiction of FERC. Rapid Dam Removal would result in the flushing of a large volume of sediment (approximately 12 million cubic yards [cy]) downstream of the remnant reservoir into the Eel River. See Maps 2-8a,b and Figures 2-2 and 2-3 for plan and section views of the dam removal. Provided below is a description of the construction area and rapid removal of Scott Dam and associated Project facilities/features.


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# Tables

## Impact Analysis and Adaptation Strategy Evaluation Report

Lake County Resource Assessment – Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

**Table 4: Summary of Scenario Hydrology and Hydraulics under the Existing and Extreme Climate Conditions**

| Scenario | Existing Climate Conditions | | | | Extreme Climate Conditions | | | |
|---|---|---|---|---|---|---|---|---|
| | Wet Season (December - April) | | Dry Season (May - November) | | Flood | | Drought | |
| | Storage (ac-ft) | Outflow (cfs) | Storage (ac-ft) | Outflow (cfs) | Max Storage (ac-ft) [Date] | Max Outflow (cfs) [Date] | Min Storage (ac-ft) [Date] | Min Outflow (cfs) [Date] |
| Historical Conditions WY 1922-2019 & 2005-2019 | **1922-2019**<br>Min: 10<br>Median: 65,900<br>Average: 59,810<br>Max: 94,900<br><br>**2005-2019**<br>Min: 7,840<br>Median: 55,400<br>Average: 49,078<br>Max: 80,200 | **1922-2019**<br>Min: 2<br>Median: 375<br>Average: 1,033<br>Max: 45,300<br><br>**2005-2019**<br>Min: 37<br>Median: 296<br>Average: 820<br>Max: 26,600 | **1922-2019**<br>Min: 2,100<br>Median: 59,700<br>Average: 58,240<br>Max: 94,900<br><br>**2005-2019**<br>Min: 11,300<br>Median: 45,200<br>Average: 45,911<br>Max: 80,800 | **1922-2019**<br>Min: 0.1<br>Median: 177<br>Average: 222<br>Max: 10,200<br><br>**2005-2019**<br>Min: 27<br>Median: 147<br>Average: 165<br>Max: 2,400 | **1922-2019**<br>Flood of record:<br>76,260 [1/26/1983]<br>Max gage reading:<br>94,900 [4/24/1932]<br><br>**2005-2019**<br>Flood of record:<br>72,470 [12/31/2005]<br>Max gage reading:<br>80,200 [4/29/2005] | **1922-2019**<br>Flood of record:<br>15,400 [1/27/1983]<br>Max gage reading:<br>45,300 [12/22/1964]<br><br>**2005-2019**<br>Flood of record:<br>33,500 [12/31/2005]<br>Max gage reading:<br>33,500 [12/31/2005] | **1922-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>10 [12/9/1931]<br><br>**2005-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>7,840 [2/5/2014] | **1922-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>0.1 [9/8/1924]<br><br>**2005-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>27 [11/5/2015] |
| Current Conditions Historical WY 2019-2024 & Projected | **2019-2024**<br>Min: 16,800<br>Median: 53,200<br>Average: 45,439<br>Max: 67,300<br><br>**Projected**<br>Min: 13,870<br>Average: 48,500<br>Max: 78,910 | **2019-2024**<br>Min: 67<br>Median: 207<br>Average: 768<br>Max: 13,200<br><br>**Projected**<br>Min: 40<br>Average: 1,052<br>Max: 51,230 | **2019-2024**<br>Min: 21,100<br>Median: 44,100<br>Average: 43,789<br>Max: 62,100<br><br>**Projected**<br>Min: 13,910<br>Average: 45,070<br>Max: 67,380 | **2019-2024**<br>Min: 42<br>Median: 115<br>Average: 201<br>Max: 2,150<br><br>**Projected**<br>Min: 40<br>Average: 258<br>Max: 15,110 | **Projected**<br>78,910 | **Projected**<br>51,230 | **Projected**<br>13,870 | Not enough data to adequately predict. Likely <40. |
| Scenario A1/A2 - PGE Proposed Removal | No storage | Min: 791<br>Average: 1,216<br>Max: 1,535 | No storage | Min: 17<br>Average: 134<br>Max: 376 | No storage | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | No storage | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <17. |
| Scenario C - Lowered Scott Dam with Fish Ladder | Min: 5,420<br>Average: 18,769<br>Max: 28,460 | Min: 20<br>Average: 1,116<br>Max: 47,470 | Min: 7,340<br>Average: 18,080<br>Max: 31,840 | Min: 20<br>Average: 220<br>Max: 14,923 | 31,840 | 47,470 | 5,420 | Not enough data to adequately predict. Likely <20. |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Min: 9,100<br>Average: 10,110<br>Max: 13,500 | Min: 684<br>Average: 1,051<br>Max: 1,327 | Min: 9,060<br>Average: 9,920<br>Max: 11,860 | Min: 15<br>Average: 43<br>Max: 245 | 13,500 | 51,686 | 9,060 | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <15. |
| Scenario E – Dam Removal + Off Channel Pond Restoration | ≥863 | Min: 682<br>Median: 1,067<br>Average: 1,048<br>Max: 1,324 | <863 | Min: 15<br>Median: 43<br>Average: 80<br>Max: 244 | ≥863 | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | <<863 | Not enough data to adequately predict. Restoration design can provide resiliency. Likely <15. |

Notes:
1. All outflow values predicted at the location just downstream of the Scott Dam within the Eel River.
2. All storage values are limited to predicted storage within Lake Pillsbury upstream of Scott Dam except scenario D where values are based within the reduced impoundment footprint.
3. Scenario B hydraulic and hydrologic parameters were not analyzed or reported in this table.

| | Current Conditions | Scenario A1 | Scenario A2 | Scenario B |
|---|---|---|---|---|
| | | PG&E Proposed Removal | Scenario A1 with Additional Mitigation | Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder |
| **Similar Scenarios** | | Proposed action by PG&E in 2025 | Sediment management alternatives (McMillen Jacobs, 2018) | Option A (McMillen Jacobs, 2018) |
| **Biological Efficiency (Fish Passage)** | **Poor** | **Very Good** | **Very Good** | **Average** |
| | No fish passage upstream of Scott Dam | Unimpeded upstream and downstream fish migration through restored Eel River. | Unimpeded upstream and downstream fish migration through restored Eel River. | Upstream passage through fish ladder. Uncertain downstream passage. |
| **Constructability Challenges** | **Not Applicable** | **Average** | **Average** | **Average** |
| | No New Construction | To be addressed in PG&E Management Plan | To be addressed in PG&E Management Plan | PG&E Management Plan should address the fish ladder |
| **Environmental Impact – During Construction** | **Not Applicable** | **Poor** | **Average** | **Average** |
| | No construction under current conditions. | Significant short term sediment release from reservoir | Minor impacts | Minor impacts |
| **Environmental Impact – Post Construction** | **Good** | **Average** | **Good** | **Good** |
| | No major impacts | Long term sediment impacts; Release of non-native species from Lake Pillsbury. | Minor impacts | Minor impacts |
| **Operational Impact** | **Average** | **Very Good** | **Very Good** | **Good** |
| | Same as Current Conditions | To be addressed in PG&E Management Plan | To be addressed in PG&E Management Plan | PG&E Management Plan should address the fish ladder |
| **Design Approach** | **Not Applicable** | **Good** | **Good** | **Average** |
| | No New Design | Additional Designs to be addressed within Management Plan | Additional designs to be addressed within Management Plan | Additional design to be addressed wtihin Management Plan |
| **Initial Implementation Cost** | **Very Good** | **Average** | **Average** | **Average** |
| | No initial implementation cost to maintain current conditions. | Dam removal costs. | Active sediment management will have high short-term costs. | High seismic retrofit cost and fish passage construction cost. |

| | Scenario C | Scenario D | Scenario E |
|---|---|---|---|
| | Lowered Scott Dam with Fish Ladder | Reduced Reservoir within Salmon Creek as Main Tributary | Dam Removal with Off-Channel Pond Restoration |
| **Similar Scenarios** | Option C (McMillen Jacobs, 2018) | | |
| **Biological Efficiency (Fish Passage)** | **Average** | **Good** | **Very Good** |
| | Upstream passage through fish ladder. Downstream passage over lowered spillway. | Unimpeded upstream and downstream fish migration through restored Eel River. | Unimpeded upstream and downstream fish migration through restored Eel River. Additional fish and aquatic habitat provided in off-channel ponds. |
| **Constructability Challenges** | **Average** | **Average** | **Good** |
| | PG&E Management Plan should address the fish ladder | New Design will address constructability | To be addressed in PG&E Management Plan |
| **Environmental Impact – During Construction** | **Average** | **Average** | **Poor** |
| | Significant short term sediment release from reservoir | Significant short term sediment release from reservoir | Significant short term sediment release from reservoir |
| **Environmental Impact – Post Construction** | **Good** | **Good** | **Good** |
| | Long term sediment impacts; Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. | Long term sediment impacts; Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. | Long term sediment impacts partly mitigated by sediment management and revegetation; Release of non-native species from Lake Pillsbury. |
| **Operational Impact** | **Good** | **Very Good** | **Very Good** |
| | PG&E Management Plan should address the fish ladder | New Design will address operational impacts | To be addressed in PG&E Management Plan |
| **Design Approach** | **Average** | **Average** | **Very Good** |
| | Additional design to be addressed within Management Plan | The new design will be included in the Management Plan | The new design will be included in the Management Plan |
| **Initial Implementation Cost** | **Average** | **Average** | **Average** |
| | High seismic retrofit cost and fish passage construction cost. | Active sediment management will have high short-term costs. High construction costs | Active sediment management will have high short-term costs. Medium expected construction costs |

| | Current Conditions | Scenario A1 | Scenario A2 | Scenario B |
|---|---|---|---|---|
| | | PG&E Proposed Removal | Scenario A1 with Additional Mitigation | Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder |
| **Similar Scenarios** | | Proposed action by PG&E in 2025 | Sediment management alternatives (McMillen Jacobs, 2018) | Option A (McMillen Jacobs, 2018) |
| **Long-term Operation and Maintenance Costs** | **Average** | **Very Good** | **Very Good** | **Average** |
| | High maintenance and operational costs. | Minimal maintenance or operational costs. | Minimal maintenance or operational costs. | High maintenance and operational costs. |
| **Water Supply** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Most reliable for flood control, irrigation, and drinking water to Russian River watershed. | No flood control. Limited reliability in providing water for irrigation and drinking water. Dependent upon flows. Limited reliability in the dry season and likely no reliability during a drought. | No flood control. Limited reliability in providing water for irrigation and drinking water. Dependent upon flows. Limited reliability in the dry season and likely no reliability during a drought. | Less reliable than current conditions but still provides a majority of current needs. |
| **Fire Suppresion** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Most reliable option. Provides a high availability of fire suppression support. | No option for fire suppression. | No option for fire suppression. | Less reliable than current conditions, but still provides a high availability of fire suppression support |
| **Recreation** | **Very Good** | **Poor** | **Average** | **Very Good** |
| | Boating, fishing, marina, swimming, camping, hiking. | Loss of many recreational uses including boating and swimming. Loss of related infrastructure such as marina, docks, and piers. | Loss of many recreational uses including boating and swimming. Potential for creation of recreation alternatives as part of mitigation. | Should maintain most baseline recreation with limited use during droughts. |
| **Effects on Local Economy** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Supports current local economy. | Loss of recreational uses creates less income from tourism. Loss of jobs. | Loss of recreational uses creates less income from tourism. Loss of jobs. | Maintains economic benefits to current business, landowners and county tax revenue. |

| | Scenario C | Scenario D | Scenario E |
|---|---|---|---|
| | Lowered Scott Dam with Fish Ladder | Reduced Reservoir within Salmon Creek as Main Tributary | Dam Removal with Off-Channel Pond Restoration |
| **Similar Scenarios** | Option C (McMillen Jacobs, 2018) | | |
| **Long-term Operation and Maintenance Costs** | **Average** | **Good** | **Very Good** |
| | High maintenance and operational costs. | Low with proper design. | Minimal with proper design. |
| **Water Supply** | **Very Good** | **Average** | **Poor** |
| | Limited flood control, irrigation, and drinking water reliability. | Limited flood control. Limited reliability for irrigation and drinking water. | Limited flood control, irrigation and drinking water reliability. Mostly dependent on flows. Limited reliability during dry season and no reliability during drought. |
| **Fire Suppresion** | **Very Good** | **Very Good** | **Average** |
| | Less reliability on fire suppression but still likely allowable for extensive use with planes. | Potential for extensive use for fire suppression, including use of planes. | Less reliable than current conditions, a range of fire suppression measures are possible for use of helicopters. |
| **Recreation** | **Good** | **Good** | **Average** |
| | Should maintain most baseline recreation with limited use during droughts. Recreational facilities need to be relocated to new shoreline, or dredging could expand lake footprint. | Provides limited recreation use when compared to baseline based on Salmon and Habematolel Creek watershed water supply. High fluctuations in the WSEs during dry season are probable. | Recreational facilities could be created in the vicinity of ponds or riverbanks. |
| **Effects on Local Economy** | **Very Good** | **Good** | **Average** |
| | Maintains economic benefits to current business, landowners and county tax revenue. Some reductions expected due to reduced reservoir. | Maintains economic benefits to current business, landowners and county tax revenue. Some reductions expected due to reduced reservoir. | Pond recreational could suport tourism and jobs. |

# Appendix A - Hydrologic and Hydraulic Scenario Evaluation

## Analysis of Effects of Proposed Scott Dam Decommissioning

### County of Lake

Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

August 22, 2025

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# Appendices

Appendix A.A Supporting Figures for Hydrologic and Hydraulic Analysis

Appendix A.B PG&E Provided Data

# Acronyms and Abbreviations

| | |
|---|---|
| ac-ft | acre-feet |
| AEP | annual exceedance probability |
| CADSOD | California Division of Dam Safety |
| CDEC | California Data Exchange Center |
| COY | Coyote Lake Mendocino Weather Station |
| cfs | Cubic feet per second |
| DWR | Department of Water Resources |
| ELP | Eel River below Lake Pillsbury Weather Station |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| FFA | flood frequency analysis |
| ft | feet |
| HEC-RAS | Hydrologic Engineering Center's River Analysis System |
| LAT | Large Airtanker |
| LPY | Lake Pillsbury Weather Station |
| MSL | mean seal level |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NIDIS | National Integrated Drought Information System |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NOAA | National Oceanic and Atmospheric Administration |
| NMFS | National Marine Fisheries Service |
| NERF | New Eel Russian Facility |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| SCD | Scott Dam Weather Station |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| SPI | Standardized Precipitation Index |
| USACE | United States Army Corps of Engineers |
| USDM | U.S. Drought Monitor |
| USGS | United States Geologic Survey |
| VLAT | Very Large Airtanker |

| | |
|---|---|
| WSE | water surface elevation |
| WY | water year |

# A1.0 Introduction

SLR International Corporation (SLR) conducted an evaluation of the existing hydrologic and hydraulic conditions of the Eel River and Scott Dam system within Lake County as part of the *County of Lake Impact Analysis and Adaptation Strategy Evaluation Report*. The primary objective was to assess the potential water supply impacts of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam and the removal of Lake Pillsbury. In addition to evaluating PG&E's proposal, SLR developed and analyzed several alternative mitigation scenarios designed to better align with Lake County's objectives. Each scenario, including the historical baseline representing past operational conditions, was assessed under four climate conditions: wet season, dry season, flooding, and drought.

The baseline scenario covers the Historical Operational Conditions during water years[1] (WYs) 1922-2019 and a focused WY period from 2005-2019.

Mitigation Scenarios:

1. Existing Operational Conditions Scenario covers current operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 follows PG&E's proposal per the PG&E Surrender Application dated January 2025, for the complete removal of Scott Dam.
3. Scenario A2 follows PG&E's proposed removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggestion for additional mitigation (including active sediment management).
4. Scenario B assuming Scott Dam to remain as operated currently (i.e. Spillway gates open at elevation 1,900 ft [PG&E datum[2]] and operation of the needle valve to provide water supply downstream) and retrofitted with a fish passage structure.
5. Scenario C assumes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device, and the inclusion of a fish ladder structure.
6. Scenario D assumes Scott Dam is removed and a new earthen dam is placed to restrict flows from only the Salmon and Habematolel Creek watersheds for a smaller reservoir footprint. The spillway is assumed to be at an elevation of 1,885 ft.
7. Scenario E assumes Scott Dam is removed and one or two small off-channel ponds are built along the Eel River mainstem allowing for water storage and natural fish passage.

SLR analyzed available hydrologic and hydraulic data to provide a high-level summary of each scenario's anticipated performance under the four climate conditions. The resulting statistics include approximated ranges for storage availability, outflows, and water surface elevations (WSEs) within Lake Pillsbury, where applicable. These results are intended to help Lake County

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

compare the projected limitations and benefits of each scenario and inform the viability of Scott Dam and Lake Pillsbury alternatives for future study.

## A1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with approximately 2,275 acres of surface area and 31 miles of shoreline at the maximum WSE of 1,910 feet corresponding to the top of the spillway gates (Stillwater, 2021) (PG&E, 2023). The Lake's main flow contributor is the Eel River's upper watershed[3] and is located in Lake County, California. Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam is classified by the U.S. Army Corps of Engineer's Institute of Water Resources National Inventory of Dams database as a high hazard dam. Dams classified as high hazard potential means that dam failure could pose serious threat to downstream communities and infrastructure. The dam primarily provided water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley (in the Russian River watershed).

Scott Dam has also historically provided water to meet the minimum instream flow requirements in the Eel River and East Fork Russian River, and irrigation to the Potter Valley Irrigation District (PVID) (Stillwater, 2021) (PG&E, 2025). The Lake also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID is diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pillsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the Russian River. Scott Dam is currently considered a fish barrier as there are no mechanisms by which fish can safely pass to the upper Eel River watershed.

Scott Dam historically operated with an approximate maximum capacity of 76,876 acre-feet (ac-ft) at the WSE of 1,910 feet and a usable storage capacity was 66,876 ac-ft (Stillwater, 2021). In 2023, due to dam stability concerns, PG&E reduced the maximum WSE to the spillway invert elevation of 1,900 feet with a resulting storage capacity of 55,948 ac-ft. The Lake Pillsbury reservoir is operated to maintain a minimum storage of at least 10,000 ac-ft as the reservoir banks become unstable and the potential for material to slough off and block the dam outlet (needle valve) and introduce sediments downstream (PG&E, 2023).

The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs) according to PG&E (2023) however, the gage observations indicate ranges between 20 and 400 cfs. Water can be released from the dam through two structures: a needle valve[4] located at the base of the dam and the spillway[5]. The flow capacity of the needle valve is approximately 400 cfs but it can operate at varying flows for each WSE. The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[6]) controls.

[3] An area of land drained by a river, river system, or other body of water.

[4] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[5] A structure used to provide controlled release of water downstream of a dam or levee.

[6] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

The spillway consists of multiple sliding and radial gates. Historically, the spillway gates were typically fully opened between November 1 and April 1 and closed for the remainder of the year. Flow through the spillway will only occur when the WSE within Lake Pillsbury exceeds the spillway invert elevation of 1,900 feet. The top of all the spillway gates are at an elevation of 1,910 feet. Slide Gate #13 and one of the five radial gates are SCADA-controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained open.

As part of the license surrender a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E (as non-project use of project land) and the Eel-Russian Project Authority (ERPA). On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023).

## A1.2 Existing Lake Pillsbury and Scott Dam Usage

Scott Dam and Lake Pillsbury play a crucial role in the Potter Valley Project, which diverts water from the Eel River to the Russian River. This diversion supports various essential functions and provides significant benefits to the surrounding communities. Scott Dam and Lake Pillsbury are resources for the region supporting agriculture, providing drinking water, aiding in fire suppression, and offering environmental and recreational benefits. However, there are significant detriments, including environmental impacts and the financial burden of maintaining aging infrastructure. Balancing these benefits and detriments is crucial for ensuring the continued prosperity and safety of the surrounding areas. This analysis focuses on the hydrology and the effects on water supply, while other aspects described below were considered in this evaluation, detailed impact assessment are presented in the Existing Conditions Report prepared by SLR (SLR, 2025).

### A1.2.1 Water Supply

Scott Dam and Lake Pillsbury are integral to the water supply system in the region. According to Lake County (Huettl, 2025), Lake Pillsbury supports local agriculture worth approximately $743 million per year. This water is vital for irrigation, ensuring the productivity and sustainability of local farms. Additionally, the project supplies drinking water to around 600,000 people downstream in inland Mendocino, Sonoma, and Marin counties (Huettl, 2025). This reliable water source is essential for meeting the daily needs of residents and maintaining public health. Additionally, Lake Pillsbury provides groundwater recharge for the area and a consistent water supply for nearby wells. There are 38 recorded groundwater wells in the vicinity of Lake Pillsbury. Further evaluation on the extent of water supply from Lake Pillsbury and its effects on the groundwater and existing wells should be conducted (SLR, 2025).

Regulatory flow requirements are specified in the Federal Energy Regulatory Commission (FERC) license and the National Marine Fisheries Service's (NMFS's) Reasonable and Prudent Alternative (FERC, 2003). These minimum flows range from 20 to 60 cfs between June 1 and November 30 and range from 20 to 100 cfs between December 1 and May 31 depending on water year classification. Historically, PG&E has released substantially higher flows from Scott Dam than the minimum flows due to the required releases for downstream compliance points on the Eel and Russian Rivers and for water deliveries to PVID (PG&E, 2025).

### A1.2.2 Fire Suppression

Lake Pillsbury serves as a critical resource for aerial firefighting operations. The lake provides a reliable water source for helicopters and planes, which are essential for combating wildfires in

the region (Appendix B). The availability of water from Lake Pillsbury allows firefighting aircraft to quickly refill and return to the fire, enhancing the efficiency and effectiveness of fire suppression efforts. This capability is particularly important during the extended fire season, when rapid response and sustained operations are necessary to protect lives and property.

### A1.2.3 Ecosystem and Environmental Impact

Lake Pillsbury supports a diverse range of wildlife, including nesting bald eagles, tule elk, and migratory waterfowl. The lake's ecosystem has developed over nearly a century, creating a habitat for many species. While the lake and surrounding areas currently provide habitats for various wildlife species, this condition has evolved away from natural, pre dam conditions. The alteration of the Eel River's natural flow has disrupted local ecosystems and fish migration patterns. The dam has been a barrier to native fish species, such as salmon and steelhead, preventing them from accessing upstream habitats. Disruptions to the system can have significant ecological consequences, affecting the health and biodiversity of the river system on a temporary and long-term basis.

### A1.2.4 Recreational Impact

The presence of Scott Dam and Lake Pillsbury offers multiple recreational benefits. Lake Pillsbury is a popular destination for recreational activities such as fishing, boating, and camping. These activities attract visitors and support the local economy, providing income for businesses and enhancing the quality of life for residents. There are six Forest Service campgrounds with 153 campsites, which according to the Forest Service’s website "[..] are operated and maintained by Pacific Gas and Electric" (US Forest Service, 1999). There is also a private resort that operates a marina as well as private piers and docks that depend on the reservoir water levels to function throughout the recreational season.

### A1.2.5 Economic Impact

Scott Dam and Lake Pillsbury have a substantial economic impact on the region. Properties with a Lake Pillsbury address paid over $500,000 in taxes due to County of Lake 2024 (SLR, 2025). The availability of water is crucial for sustaining local economies, particularly in agriculture and tourism. The benefits provided by Scott Dam and Lake Pillsbury are essential for the economic well-being of the communities in Mendocino, Sonoma, and Lake counties.

Under the surrender conditions proposed by PG&E, there is no need to maintain Scott Dam and Lake Pillsbury as no hydropower generation is anticipated to occur following decommissioning of the Potter Valley Project. PG&E will be responsible for the decommissioning of the Scott Dam and removal of lake Pillsbury including restoration of the decommissioned areas as approved by FERC after the final surrender application is accepted by FERC. If Scott Dam remained in place for other purposes, the financial responsibility of future operation and maintenance of the dam and lake would be taken on by another party which is yet to be determined. The financial burden of maintaining the dam and associated infrastructure is expected to be high.

## A2.0 Existing Data

Various reports, analyses, and data have been made available for review as part of the Scott Dam Decommissioning Analysis completed by PG&E, PG&E consultants, and other interested or affected parties. Notable information includes hydrology and project operations modeling to project the unimpaired flows post dam removal utilizing a mass or water balance approach, analysis of water supply under various operating conditions and dam alternatives, a hydraulic

model to project the effects of flood waters on the Eel River downstream of the Scott Dam, fish pass ability assessments, and bathymetric data to approximate storage within Lake Pillsbury.

SLR reviewed the existing data and reproduced select data to support the proposed scenarios and climate change assessments. The existing data and assumptions were generally used to maintain continuity between assumptions and the reporting consistency. Raw data and models were not provided for this assessment and limits the highest accuracy of reproduction. Raw data that was provided via public sources were utilized when available and to validate any of the assumptions and analyses. Public records and resources were taken from the United States Geologic Survey (USGS), the National Oceanic and Atmospheric Administration (NOAA), National Integrated Drought Information System (NIDIS), National Centers for Environmental Information (NCEI), and the California Data Exchange Center (CDEC).

## A2.1 Existing Gage and Weather Station Data

PG&E has continuously monitored the storage and outflows from Scott Dam to meet the supply and demands of the system. Other federal and state agencies have recorded climate data such as temperature, precipitation, evaporation, and snowfall from different time periods within the Upper Eel River basin and retrieved from the NOAA NCEI dataset and the California Department of Water Resources CDEC which include the Lake Pillsbury (LPY), Scott Dam (SCD), Eel River below Lake Pillsbury (ELP), and Coyote Lake Mendocino (COY). The PG&E operated stations include USGS 11470500, and USGS 11470000.

A list of the active and inactive gaging stations are noted below:

1. LPY station data records reservoir elevation and reservoir storage reported on a daily basis covering the time period from March 2007 to the present. Data is available from the CDEC (CDWR, 2025c).
2. SCD station data records precipitation reported on a daily basis covering the time period from October 2011 to the present. Data is available from the CDEC (CDWR, 2025d).
3. ELP station data records mean flow reported on a daily basis covering the time period from March 2007 to the present. Data is available from the CDEC (CDWR, 2025b).
4. COY station operated by the United States Army Corps of Engineers (USACE) records regional pan evaporation data on a daily basis covering the time period from January 1994 to the present. Data is available from the CDEC (CDWR, 2025a).
5. USGS gage 11470500 located at Eel River below Lake Pillsbury records the daily average discharge from the Scott Dam in cfs and water temperature in degrees Celsius. The period of record is October 1, 1922, to September 30, 2024. Data is available from the USGS (USGS, 2025a).
6. USGS gage 11470000 located within Lake Pillsbury records the observed daily average storage in ac-ft. The period of record is October 1, 1928, to September 30, 2024. Data is available from the USGS (USGS, 2025b).
7. The NOAA recorded historical data at weather station LAKE PILLSBURY 2, CA US USC00044698, located northwest of the Soda Creek confluence with the Eel River, reports statistics on evaporation, precipitation, temperature, and snowfall/depth based on daily observations. The period of record is January 1, 1966, to December 31, 1968. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
8. The NOAA recorded historical data at weather station LAKE PILLSBURY, CA US USC00044156, located near Lake Pillsbury, reports daily statistics on evaporation,

precipitation, temperature, snowfall/depth, fog, and thunderstorms. The period of record is January 1, 1909, to December 31, 1937. Data can be accessed through NOAA's NCEI (NOAA, 2025a).

9. The NOAA recorded historical weather station HULLVILLE 7 SE, CA US USC00044158, located near Rice Creek above the Rice Fork confluence, reports statistics on evaporation, precipitation, temperature, and snowfall/depth based on daily observations. The period of record is from January 1, 1908, to December 31, 1909. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
10. The NOAA records data at weather station SODA CREEK CALIFORNIA, CA US USR0000CSDC, located along Soda Creek, reports statistics on temperature based on daily observations. The period of record is from January 1, 1994, to December 31, 2024. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
11. The NOAA records data at weather station POTTER VALLEY 8.4 ENE, CA US US1CALK0019, located near Packsaddle Creek, reports statistics on precipitation, snowfall, and snow depth based on daily observations. The period of record is October 24, 2021, to April 21, 2025. Data can be accessed through NOAA's NCEI (NOAA, 2025a).

## A2.2 USGS StreamStats

The USGS StreamStats Application reports watershed statistics and characteristics based on a user-specified delineation point on a specified watercourse within the US (USGS, 2025c). The application treats the delineation point as the most downstream location in the study area and assumes that the watershed is undeveloped (no dam) and computes a regional regression analysis based on the USGS gage data available. StreamStats reports that the Eel River watershed immediately downstream of the Scott Dam has a drainage area of approximately 288.9 square miles. The watershed is assumed to have approximately 42.5% area covered by forest, 3.6% developed, and the remaining coverage consists of a combination of other land coverage such as wetlands, open water, and/or shrubland. The mean annual precipitation is reported to be 53.8 inches. The summary of the StreamStats Peak Flow Statistics Report for the Eel River at Scott Dam is summarized in Table A2-1. The flows are reported based upon their annual exceedance probability (AEP) which is a statistical measure representing the probability of a flood of a certain size occurring or exceeded in any given year. An example would be a 1% AEP flood has a 1 in 100 chance of occurring in any given year and has an average recurrence interval of 100 years and is why this flood often referred to as the 100-year flood.

**Table A2-1: StreamStats Generated Statistical Peak Flow Report for the Eel River Watershed at Scott Dam**

| AEP Flood [Recurrence Interval] (cubic feet per second or cfs) | | | | | | | |
|---|---|---|---|---|---|---|---|
| 50% [2-year] | 20% [5-year] | 10% [10-year] | 4% [25-year] | 2% [50-year] | 1% [100-year] | 0.5% [200-year] | 0.2% [500-year] |
| 15,300 | 26,800 | 34,700 | 44,900 | 52,500 | 60,100 | 67,300 | 76,900 |

The data has limited accuracy when analyzing a watershed that has been at least partially developed or major changes in ground cover. Development within the Eel River watershed above Scott Dam is very low and unlikely to contribute to a high degree of variation. However, the application will also not account for timber harvesting or fires that would impact extreme changes in watershed runoff conditions or how quickly runoff water will route to the watercourse.

It should also be noted that there are limited USGS gages within the area, and those gages are based on impaired flows from Scott Dam, which are used to predict the AEP. There are no other active USGS stream gages or precipitation gages within the watershed above Scott Dam that are used by the StreamStats calculation. StreamStats data should only be used as a reference for flood frequency and more data is required with other known precipitation gages to more accurately project the flood flows and frequency for the watershed above Scott Dam.

## A2.3 Flood Modeling

The Federal Emergency Management Agency (FEMA) has posted several Flood Insurance Studies (FIS) for the County of Lake, with the most recent effective revision completed October 10, 2024, and previous effective FIS reports published in September 2005 and March 1998. The 2024 FIS reports a summary of Pillsbury Lake stage-frequency relationships for the 10-, 50-, 100-, and 500-year floods which are 1,826.3; 1,829.8; 1,831.0; and 1,834.0 stage feet respectively (FEMA, 2024a). The stage-frequency has remained unchanged since the 1998 FIS (FEMA, 1998). The 2005 and 2024 FISs do not detail the hydrology or hydraulics for the Eel River. The 2005 FIS reports approximate studies for the Eel River, Soda Creek, and Smokehouse Creek with a detailed study of the Lake Pillsbury Perimeter (FEMA, 2005). The special flood hazard area (SFHA) inundation boundaries developed as part of the FIS for the Eel River are defined as a zone A, meaning the area inundated by the 1% annual chance floodplains were determined by approximate methods and no base flood elevations (BFEs) or depths are shown per FEMA Flood Insurance Rate Map (FIRM) panels 06033C0140D and 06033C0125D, per the FEMA National Flood Hazard Layer Viewer.

The preliminary July 2024 FEMA FIS for Mendocino County references a more detailed analysis of the Eel River from the Cape Horn Dam downstream approximately 1.8 miles that was completed in 1981. The 1981 study identified the recurrence intervals for the 10%, 2%, 1%, and 0.2% annual chance floods, predicted to be 41,000; 70,000; 82,500; and 112,000 cfs respectively. The recurrence intervals were determined using a USGS Bulletin 17B analysis (FEMA 2024b).

A One-dimensional hydraulic USACE Hydrologic Engineering Center's River Analysis System (HEC-RAS) model was developed and reported by Stillwater Sciences in 2021. Model development included a 49-mile reach of the Eel River from Scott Dam downstream to the Middle Fork Eel River and required gathering and analyzing various data to create the model inputs, boundary conditions, and river geometry. The Stillwater memorandum included a summary of the previous hydrologic assumptions and their own Bulletin 17C analysis for the 1% AEP floods which are summarized in Table A2-2 along with the most recent StreamStats projections (Section A2.2).

**Table A2-2: Summary of Previous 1% AEP Projected Flows by Source and Method**

| Year of Analysis | Source | 1% AEP (100-year Recurrence) (cfs) | Analysis Method |
|---|---|---|---|
| 2012 | USGS StreamStats | 60,200 | Regional Regression |
| 2018 | McMillen Jacobs Associates | 59,200 | Bulletin 17B |
| 2021 | Stillwater Science | 56,417 | Bulletin 17C |
| 2025[1] | USGS StreamStats | 60,100 | Regional Regression |

| Year of Analysis | Source | 1% AEP (100-year Recurrence) (cfs) | Analysis Method |
|---|---|---|---|
| Notes:<br>1. USGS StreamStats 1% AEP reported for 2025 used the same method of analysis for 2012 with a longer period of record to project the peak flood flow | | | |

The previous studies have used either a regional regression or Bulletin 17 methodology for estimating the flood return periods. The regional regression analysis is typically used at sites with few or no stream gages by relating streamflow characteristics to basin and climate variables. The regional regression analysis method recommends excluding or adjusting streamflow data that is heavily regulated with varying operations. The Bulletin 17C superseded 17B in 2018 and is a standard method for determining flood frequency at specific sites using long term streamflow data (USACE, 2025). A Bulletin 17C analysis would ideally use a homogeneous record, where the statistical parameters should not change over time, however regulated flows can be used when there are adjustments made to the impaired flows based on a high confidence in the operations. Alternatively hydrologic models can be used to approximate natural conditions runoff and peak flows, which at the time of this report development were not completed. The operations of the Scott Dam are also not replicated or understood with a high degree of confidence. The previous 1% AEP projections also all utilize the current impaired or controlled streamflow gages which are likely to differ from the actual peak flood flows.

Stillwater Sciences (2021b) developed one-dimensional existing conditions and future conditions hydraulic models for the Eel River downstream of the Scott Dam. The models assumed an in-line weir for the Cape Horn dam where the dam dimensions were to simulate the spillway based on data provided by PG&E and did not include detailed dam operations. The models were used to assess the existing conditions and future flood conditions post Scott Dam removal using the impaired hydrologic FFA results summarized in Table A2-3. The existing conditions model was calibrated using the impaired gaged WSEs. The existing condition 100-year WSEs were within 0.02 to 1.5 feet when compared to the recorded WSEs. When modeling the FEMA flood flows under the existing conditions, the inundation limits were relatively in agreement, with an exception near Todd Road. The existing and future conditions water surface elevation profile is shown in Figure A2-1.

**Table A2-3: Sillwater (2021b) Flood Frequency Analysis (FFA) Flow Report for the Eel River Watershed below Scott Dam**

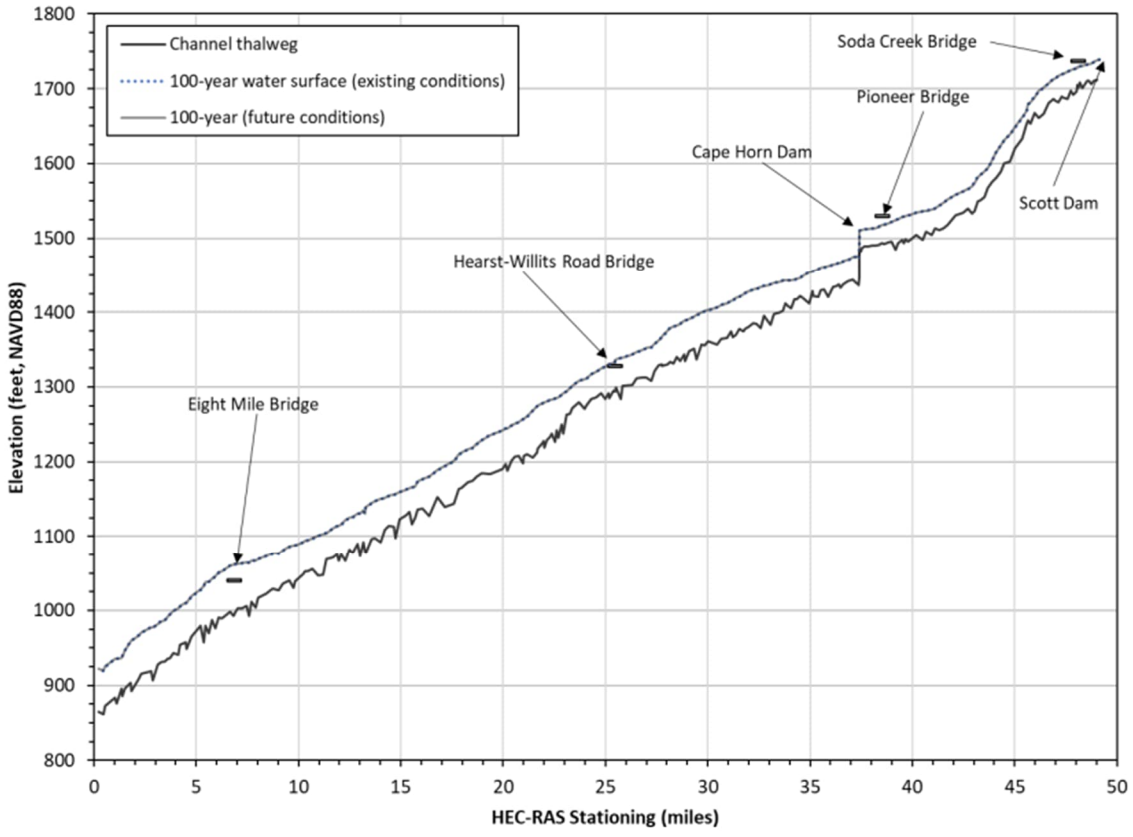
| AEP Flood [Recurrence Interval] (cubic feet per second or cfs) | | | | |
|---|---|---|---|---|
| 50% [2-year] | 10% [10-year] | 4% [25-year] | 2% [50-year] | 1% [100-year] |
| 11,205 | 34,034 | 42,026 | 50,883 | 60,100 |

The future conditions Stillwater hydraulic model (2021b) assumes Cape Horn Dam remains in place and Scott Dam is removed. The model used unimpaired hydrology calculated via StreamStats (Table A2-2, 2012 results) and then uniformly increases the impaired discharges at all downstream flow locations by the difference between impaired and unimpaired flows (roughly 4,000 cfs → 60,200 – 56,417 = 3,783 cfs). The proposed channel geometry was assumed to remain unchanged from existing conditions based on the sediment transport model, projecting the 100-year flows would flush out all sediment released from the removal of Scott Dam (Stillwater, 2021a). The results of the future conditions model projected an increase in water surface elevations for the 100-year event to show a maximum increase in flood waters by up to 1.2-feet (Figure A2-1).

Stillwater concludes that risk is not expected to substantially increase following Scott Dam removal during a 100-year flood event and that additional data is expected to refine these conclusions. Stillwater's results assume that the 100-year event will mainly be categorized by the hydrology and not by coarse sediment bed transport based on a previous Stillwater sediment transport analysis (2021a). However, these conclusions rely on assumptions that lack analytical support. The 2021 Stillwater sediment transport analysis specifically states that "No model was developed to simulate gravel transport dynamics because there is inadequate data to do so." The assumption that the Eel River's longitudinal profile will remain unchanged is speculative and does not account for potential scouring of bridge abutments or infrastructure impacts from coarse sediment and higher flows. The projected 1.2-foot WSE rise, combined with newly accessible sediment and flow magnitudes not seen in over a century, could pose significant risks to downstream infrastructure.

Figure A2-1 shows two crossings that are projected to be overtopped during the 100-year event. Increased river flows and debris movement would likely exacerbate these vulnerabilities. These uncertainties suggest that flood risk may be underestimated and warrant further analysis. Additionally, the Stillwater (2021b) report provides limited visualizations of the increased flood profile and inundation area near Scott Dam. The HEC-RAS model used to generate these results was not made available for review, and further data could not be extrapolated to assess flood risk from the Scott Dam removal. Without access to the model or more detailed outputs, it is difficult to validate the assumptions or evaluate the full extent of potential infrastructure impacts.

**Figure A2-1: Stillwater Sciences (2021a) Existing and Future Conditions 100-year Water Surface Elevation Profiles [HEC-RAS stationing of Eel River from Scott Dam (station 49) downstream to Middle Fork Eel River confluence (station 0)]**



# A3.0 Existing Climate and Hydrology

The climate of the Upper Main Eel watershed is relatively moderate, with occasional extreme temperatures during both the winter and summer. According to the NOAA (2025a), the summer temperatures often exceed 100°F, while winter temperatures usually fall below freezing. Summer temperatures typically range from 70°F to 90°F, while winter average temperatures range from 40°F to 60°F. However, these temperatures vary with elevation and have become more extreme over the past century.

The Soda Creek weather station has been reporting on temperature data since 1992, with observed extremes ranging from -32°F to 114°F (NCEI, 2025). Due to the lack of a consistent long-term dataset covering both precipitation and temperature throughout the lifetime of Scott Dam, only limited inferences can be made. Comparisons rely on overlapping timeframes between station activity and dam operations.

A continuous-record precipitation station operated by the U.S. Weather Bureau (now NOAA) near Lake Pillsbury from 1907 to 1936 recorded mean annual precipitation of 47.62 inches, with monthly totals ranging from 0 to 18.10 inches. During this period, recorded temperatures ranged from 15°F to 107°F. (NOAA, 2025a).

Rainfall in the watershed is relatively high, averaging 50-60 inches per year along the eastern boundary and 70-80 inches per year around Bald Mountain, north of the Salmon and Smokehouse Creek watershed (NOAA, 2025a). In the valleys, rainfall averages 40-50 inches per year, with the lowest amounts in the vicinity of Lake Pillsbury.

Snow occasionally falls at the 2,500-foot level but seldom builds any significant snowpack below 5,000 feet. Higher elevations can receive substantial snowfall, often accumulating several feet during the winter months, often accumulating several feet during winter months. Average snow depths range from 26 to 39 inches (NOAA, 2025b), and spring snowmelt from these accumulations significantly contributes to the Eel River watershed (US Forest Service, 1999).

While the historical and contemporary data presented here offer insights into the climate of the Upper Main Eel watershed, they are limited in their ability to fully reflect long-term climate change trends. The lack of a continuous, multi-variable dataset spanning the full operational history of Scott Dam makes it difficult to draw definitive conclusions about how climate change has influenced local temperature, precipitation, and snowpack.

Observed extremes at the Soda Creek station and historical records from Lake Pillsbury suggest increasing variability, but without consistent overlap or standardized measurements, these datasets cannot be used to quantify climate-driven shifts with high confidence. As such, while the data hint at intensifying seasonal extremes and reduced snowpack at lower elevations—patterns consistent with broader regional climate trends—any assumptions about climate change impacts in this specific watershed must be made cautiously and ideally supplemented with downscaled climate modeling or regional projections such as Cal-Adapt or Integrated Climate Adaptation and Resiliency Program (ICARP) tools; see further details in Appendix D.

## A3.1 Stream Types

Lake Pillsbury has several water courses contributing to water storage availability. These watercourses were historically tributaries that routed directly into the Eel River prior to the

construction of Scott Dam. A majority of the tributaries to Lake Pillsbury are ephemeral[7] or intermittent[8] streams with Eel River, Habematolel Creek (also known as Squaw Valley Creek), and Rice Fork identified as entirely perennial[9] by the USGS's 3D Hydrography Program (USGS, 2024). Other notable named watercourses that have large perennial reaches and short stretches that are intermittent, specifically as they reach Lake Pillsbury, are Smokehouse Creek, Salt Spring Creek, and Salmon Creek.

## A3.2 Climate Conditions

Each scenario was analyzed under four climate conditions: normal wet season, normal dry season, flood, and drought. The full period of storage and outflow gages, along with any precipitation, flood, and drought data was used to project the climate conditions. The precipitation data available is not continuous over the period of analysis (1900- present), and no flow gage data was made or known to be available pre-dam (pre-1922). Assumptions were based on the observed trends over the period of record for which data was available and primarily when the dam was in place.

### A3.2.1 Wet and Dry Season

The approximate wet and dry seasons were predicted for the region using the available data from the NOAA NCEI (Menne, et al, 2012). The annual total precipitation is approximately 44.90 inches and 40.4 inches respectively for the Lake Pillsbury and SCD station, respectively, which is lower than the reported average via StreamStats (Section A2.2). The daily precipitation data from the LAKE PILLSBURY, CA US USC00044156 weather station was used to determine the average monthly precipitation over with a period of record from 1909-1937 as summarized in Figure A3-1. The incremental daily precipitation from the SCD station was used to determine monthly average precipitation from 2011-2025 as shown in Figure A3-2. The average precipitation over the period of record was determined to be approximately 0.12 inches for both stations. Months where the daily precipitation was greater than average precipitation are assumed to be a part of the wet season and include November through March. Months where the daily precipitation was less than the average precipitation includes April through October and are assumed to be a part of the dry season. Figure A3-1 and Figure A3-2 show the average precipitation threshold (average over the period of record), monthly average precipitation, and the assumed wet and dry seasons based on the precipitation records. The Lake Pillsbury and SCD stations are consistent in the assumed wet and dry season.

The approximate wet and dry seasons were also predicted from the Eel River outflow gage below Scott Dam. The average outflow for the period of record (1922-2024) based on daily average discharge[10] reported at USGS gage 11470500 from the Scott Dam was determined to be approximately 553 cfs. The gaged data was used to determine monthly averages over the period of record and compared to the period of record average as shown in Figure A3-3. Months with average daily outflows greater were than the average outflow include December through

[7] This stream flows only briefly after rainfall events. It is usually dry most of the year and only carries water for a short duration following precipitation. Ephemeral streams do not have a continuous source of water and are highly dependent on immediate weather conditions.

[8] This type of stream flows only during certain times of the year, typically when there is enough rainfall or snowmelt to sustain flow. It may have water for weeks or months at a time, but it dries up during periods of low precipitation.

[9] A perennial stream is one that flows continuously throughout the year, having a consistent and reliable source of water, such as groundwater, springs, or sustained rainfall.

[10] Outflow and discharge used interchangeably; the volume of water flowing through a cross-section at a point in time.

April. Months with average daily outflow less than the annual average outflow include May through November (Figure A3-3). Dam operators likely increase the outflow in December, following the start of the wet season in November, and decrease flows in April, following the end of the wet season in March.

Storage[11] within Lake Pillsbury follows a more opposing trend throughout the year, being at its highest during from March through July and the lowest between August through January as observed in Figure A3-5. This is presumably due to PG&E keeping the storage capacity available for irrigation, water supply to Lake Mendocino, increased recreation, and greater fire suppression during the dry season. The wet and dry season periods were generalized, and it should be noted that there is some variability from year to year when the wet and dry season starts and ends.

Using the PG&E controlled outflows and storage provided an initial trend in the wet and dry season assumptions for the watershed. However, as the storage and outflows are controlled, they will dampen the precipitation events, introducing inaccuracy when projecting weather patterns. When comparing the precipitation data to the outflow and storage trends, it was observed that as the outflow and storage are operationally controlled, the flows are not predictive of the seasons themselves. However, to remain consistent with previous studies, the scenario analyses reference the wet and dry season based on the outflow seasonal trends (wet: December through April and dry: May through November), as opposed to precipitation (wet: November through March and dry: April through October). The previous studies and hydrological studies are based on the outflows and follows a similar seasonal trend. This trend is shown in the unimpaired monthly average flow data estimates developed as part of the Results of Initial Water Supply Modeling for PVP and Russian River Alternatives which follow the outflow trends for the seasonality and not the precipitation seasonal trends (Addley et al, 2019) recreated in Figure A3-4. Analytically, unimpaired flows should more closely follow the precipitation trends for seasonal variations.

[11] The volume of water filling a basin (Lake Pillsbury), measured in acre-feet.

**Figure A3-1: Average Monthly Precipitation Calculated from the Lake Pillsbury Weather Station over the Period of Record from 1909-1937**

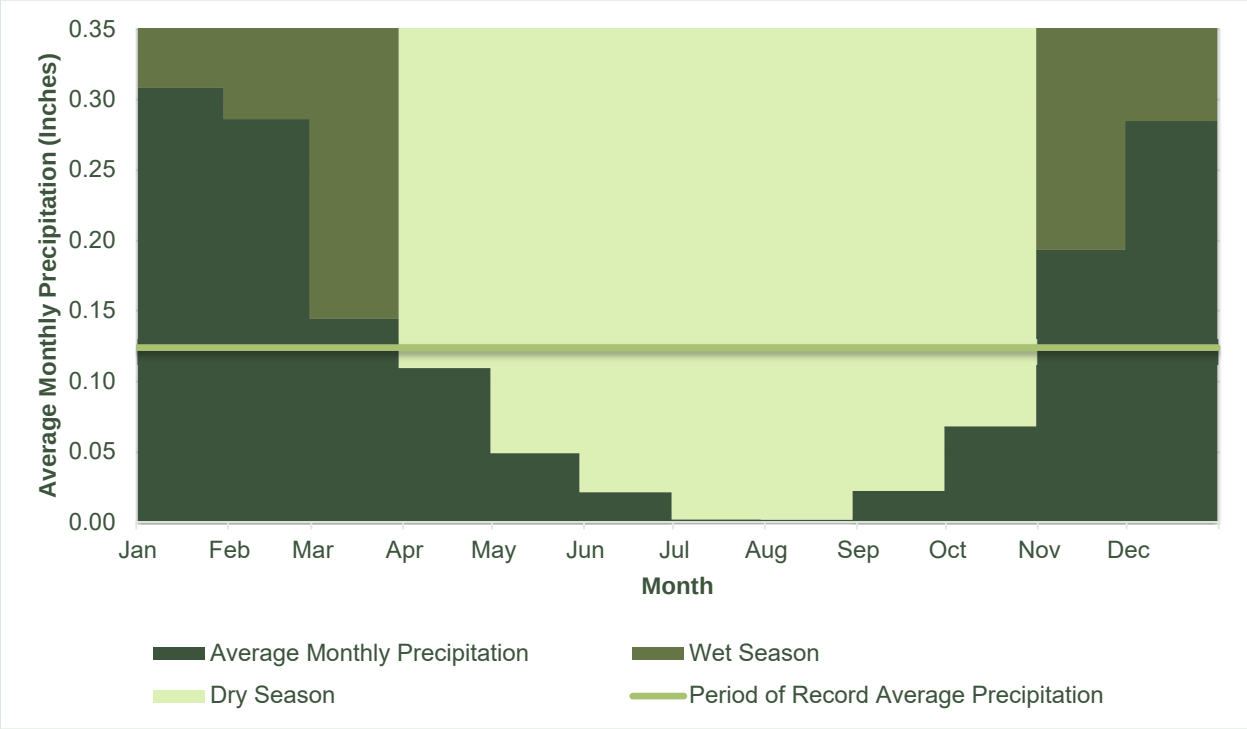



**Figure A3-2: Average Monthly Precipitation Calculated from the SCD Station over the Period of Record from 2011-2025**

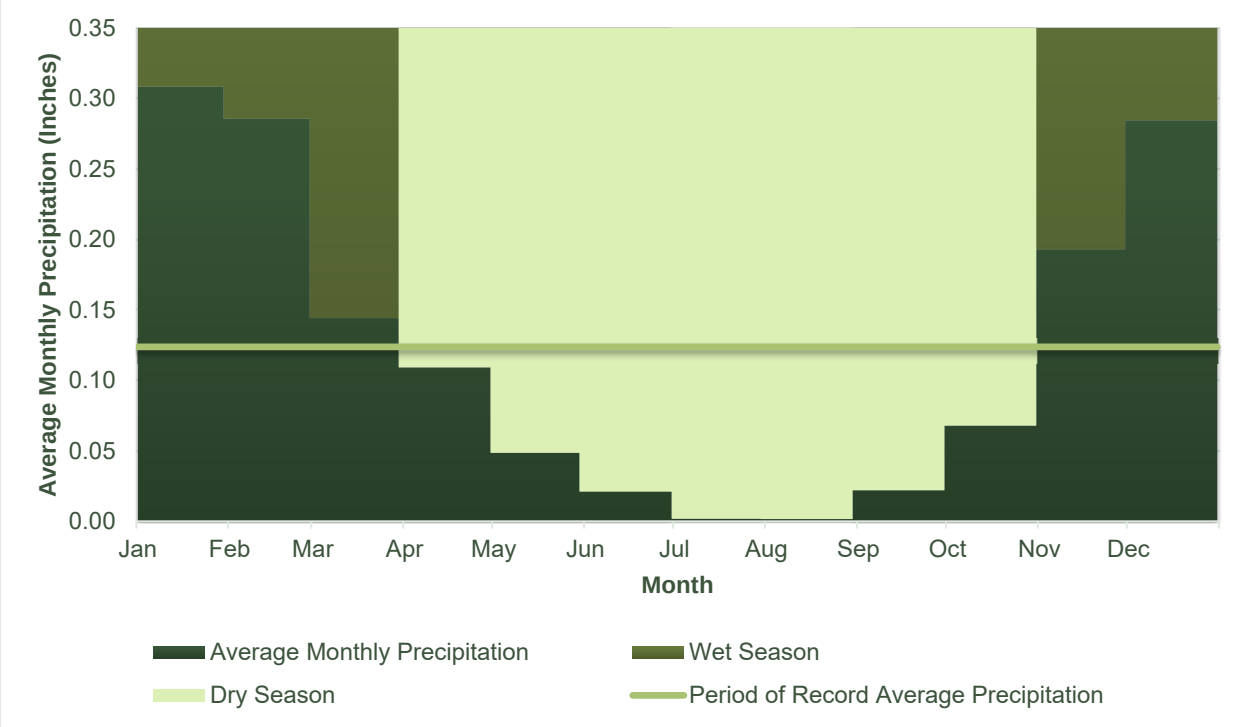


**Figure A3-3: Average Monthly Gage Flows Calculated from the Eel River below Scott Dam Flow Gage over the Period of Record from 1911-Present**

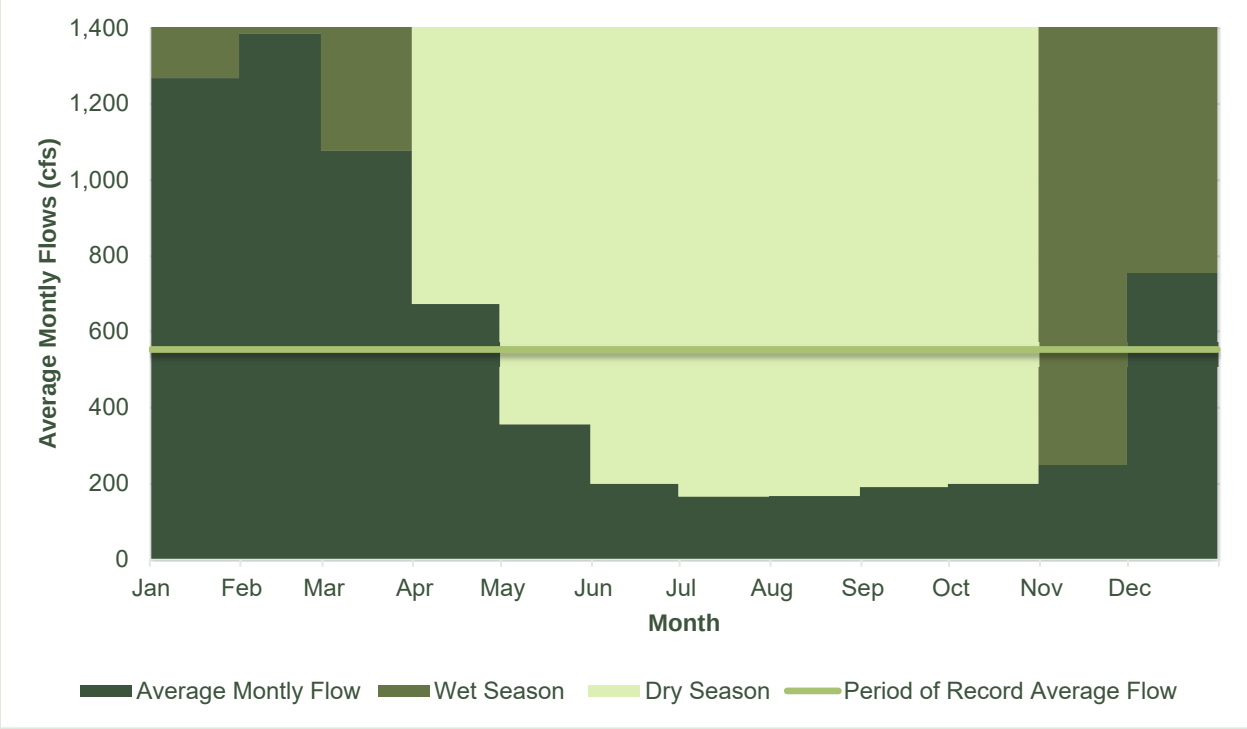



**Figure A3-4: Unimpaired Monthly Average Flows Downstream of Scott Dam (Cardno 2019 & Addley et al. 2019)**

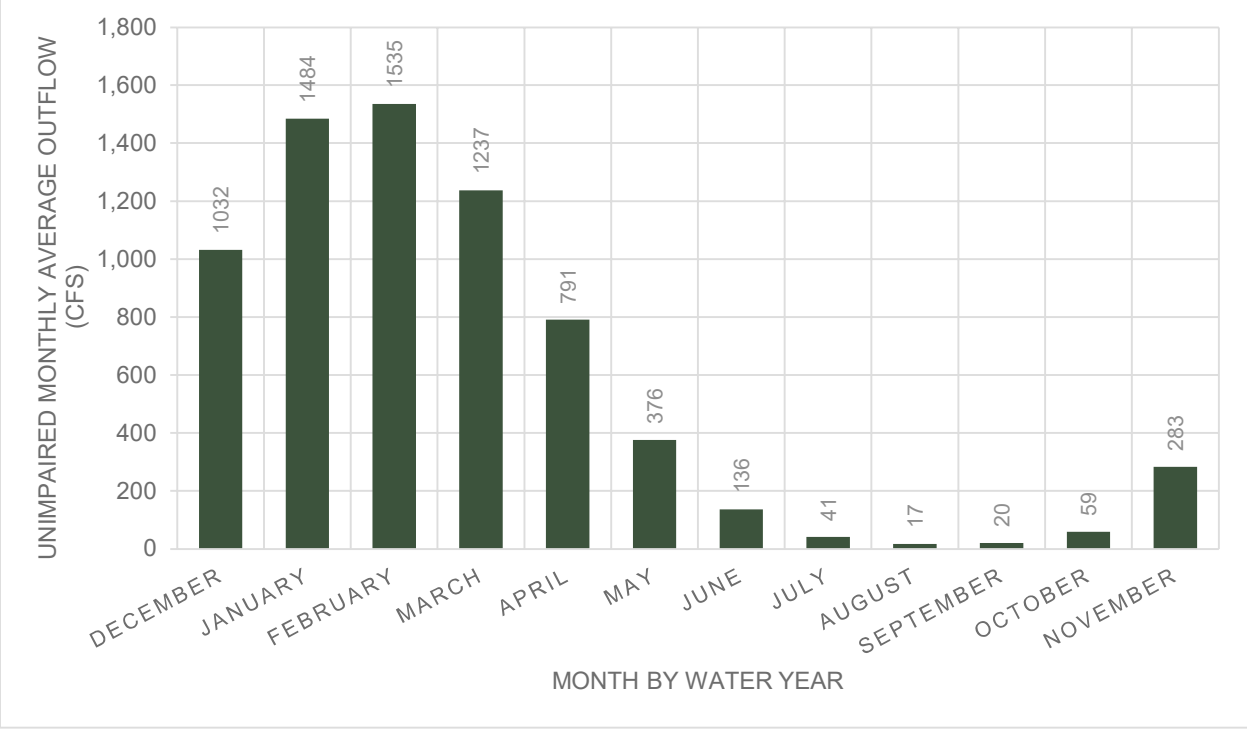
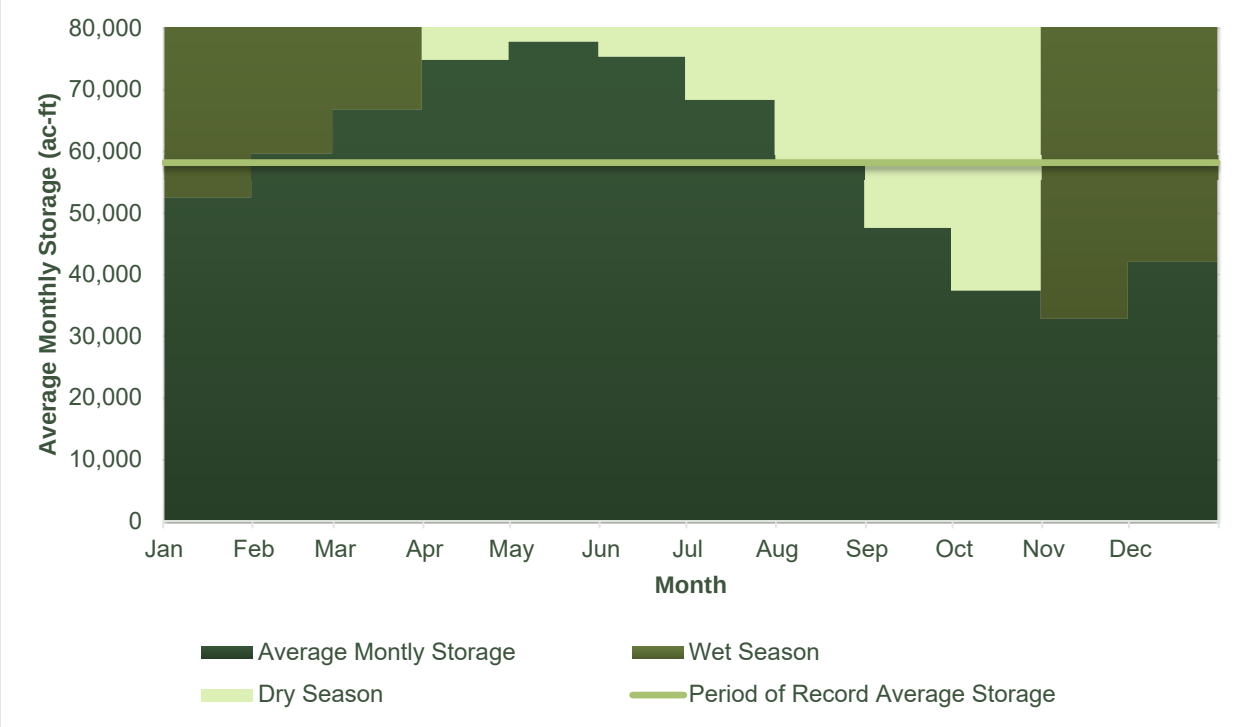


**Figure A3-5: Average Monthly Gage Storage Calculated from the Lake Pillsbury Storage Gage over the Period of Record from 1928-Present**




## A3.2.2 Flood and Drought of Record

The flood and drought of record were determined by using the NIDIS wet and dry conditions monitoring data for California State and County of Lake. NIDIS calculates flood and drought data using a combination of historical climate records, drought indices, hydrologic models, and advanced modeling and mapping techniques. Two key tools used in this analysis are the U.S. Drought Monitor (USDM) and the Standardized Precipitation Index (SPI), which evaluate how much a region's observed precipitation deviates from the climatological average and classify it within a standardized range. The SPI, in particular, quantifies precipitation anomalies by expressing them in terms of standard deviations from the historical mean—negative values indicate below-average precipitation (drought), while positive values suggest above-average precipitation (potential flood conditions). Figure A3-6 shows Lake County's SPI from 1895-Present (NIDIS, 2025) with classifications summarized in Table A3-1. This data is useful for understanding past climate events and trends within Lake County.

**Figure A3-6: Standard Precipitation Index from 1895-Present for Lake County, CA as of April 2025 from Drought.gov**

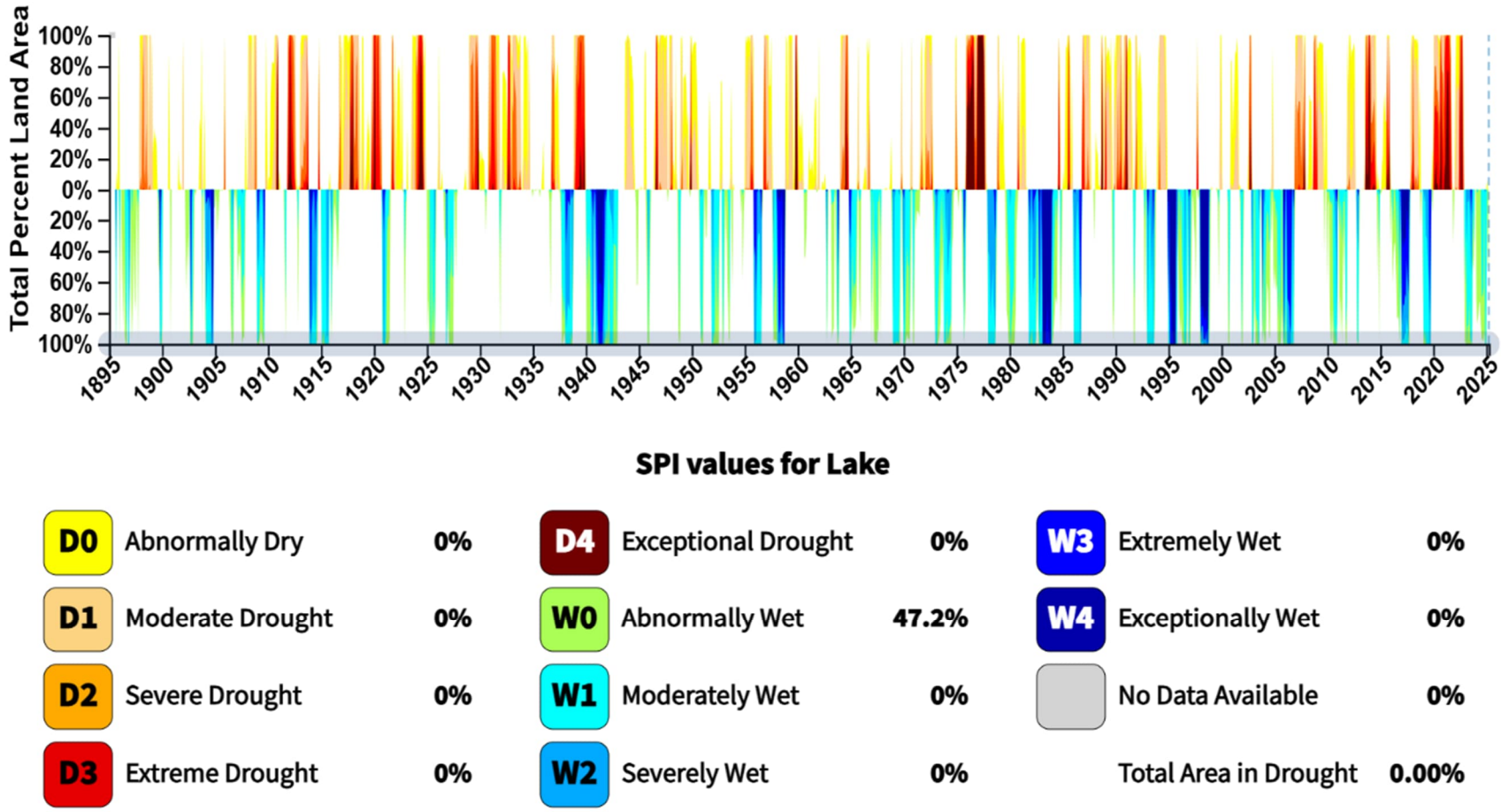


**Table A3-1: NIDIS Wet and Dry Conditions Classifications**

| Conditions | Classification | SPI |
|---|---|---|
| Normal | N/A | -0.4 to +0.4 |
| Abnormally Dry | D0 | -0.7 to -0.5 |
| Moderate Drought | D1 | -1.2 to -0.8 |
| Severe Drought | D2 | -1.5 to -1.3 |
| Extreme Drought | D3 | -1.9 to -1.6 |
| Exceptional Drought | D4 | -2.0 or less |
| Abnormally Wet | W0 | 0.5 to 0.7 |
| Moderate Wet | W1 | 0.8 to 1.2 |
| Severe Wet | W2 | 1.3 to 1.5 |
| Extreme Wet | W3 | 1.6 to 1.9 |
| Exceptional Wet | W4 | 2.0 or greater |

According to historic news archives (Metzler, 1983), the flood of record in Lake County took place during WY 1982 with another large flood event happening in WY 1983. These 1982 and 1983 events are remembered as a substantial flood caused by exceptionally heavy rainfall with flooding that destroyed infrastructure and agricultural land resulting in the loss of 19 lives. The NIDIS SPI classification identifies this period as an exceptionally wet period. A focused SPI can be found in Appendix A.A, Figure A.A9-9 along with Figure A.A 9-2 showing the daily average outflows and storage during the 1982-1983 flood period. The peak outflow from Lake Pillsbury during the flood of record was observed at 15,400 cfs observed on January 27, 1983. The peak flow did not reach the projected StreamStats 50% (2-year) flood level. This is likely due to Scott Dam attenuating the flood flows through Lake Pillsbury. Other prominent floods took place in 1937 (Figure A.A9-8, Appendix A.A) and 1995 in Lake County (Figure A.A 9-3 and Figure A.A9-8, Appendix A.A).

There are several notable droughts within Lake County according to NIDIS over the past century. The driest period in California history took place between 2012-2016, following California state-wide trends (Figure A.A9-10, Appendix A.A). Before the 2012-2016 drought, the most severe drought in Lake County and in California took place from 1976-77 (Figure A.A9-1 and Figure A.A9-11, Appendix A.A), which is attributed to very low precipitation over a long duration. Other notable droughts took place from 1928-1934 (Figure A.A 9-6, Figure A.A 9-7, and Figure A.A9-12; Appendix A.A) which was significant due to its duration, and 2020-22 (Figure A.A 9-4, Figure A.A 9-5, and Figure A.A9-10; Appendix A.A), which was less severe than the 2012-16 drought but alarming for California to have experienced two droughts in quick succession. Several factors made the 2012-2016 drought the most severe, including the duration, temperature extremes exacerbated by climate change, population growth, runoff, and ecological stress. (USGS, 2017). Drought trends over the past two decades have been observed in the NIDIS to show an increase in the frequency, intensity, and duration of drought with lower overall precipitation and snow accumulations.

### A3.2.3 Maximum and Minimum Outflows

Since 1922, the recorded maximum daily average outflow from the PG&E gages at Scott Dam took place on December 22, 1964, reaching 45,300 cfs. The period between December 1964

and January 1965 correlates to between a 4-10% annual chance of exceedance (approximate 10-25 year recurrence interval) flood according to StreamStats as indicated in Figure A3-7. However, the NIDIS historical wet and dry trends do not show any significant precipitation during the 1964-65 wet season (Figure A3-6). The minimum average daily outflow occurred on September 8, 1924, reaching 0.1 cfs as observed in Figure A3-8. The minimum average daily outflow coincidentally occurred during the drought from 1928-34.

**Figure A3-7: Daily average outflow from Lake Pillsbury and inflow from contributing streams, and daily average storage from December 1964 - January 1965 (period of record outflow)**

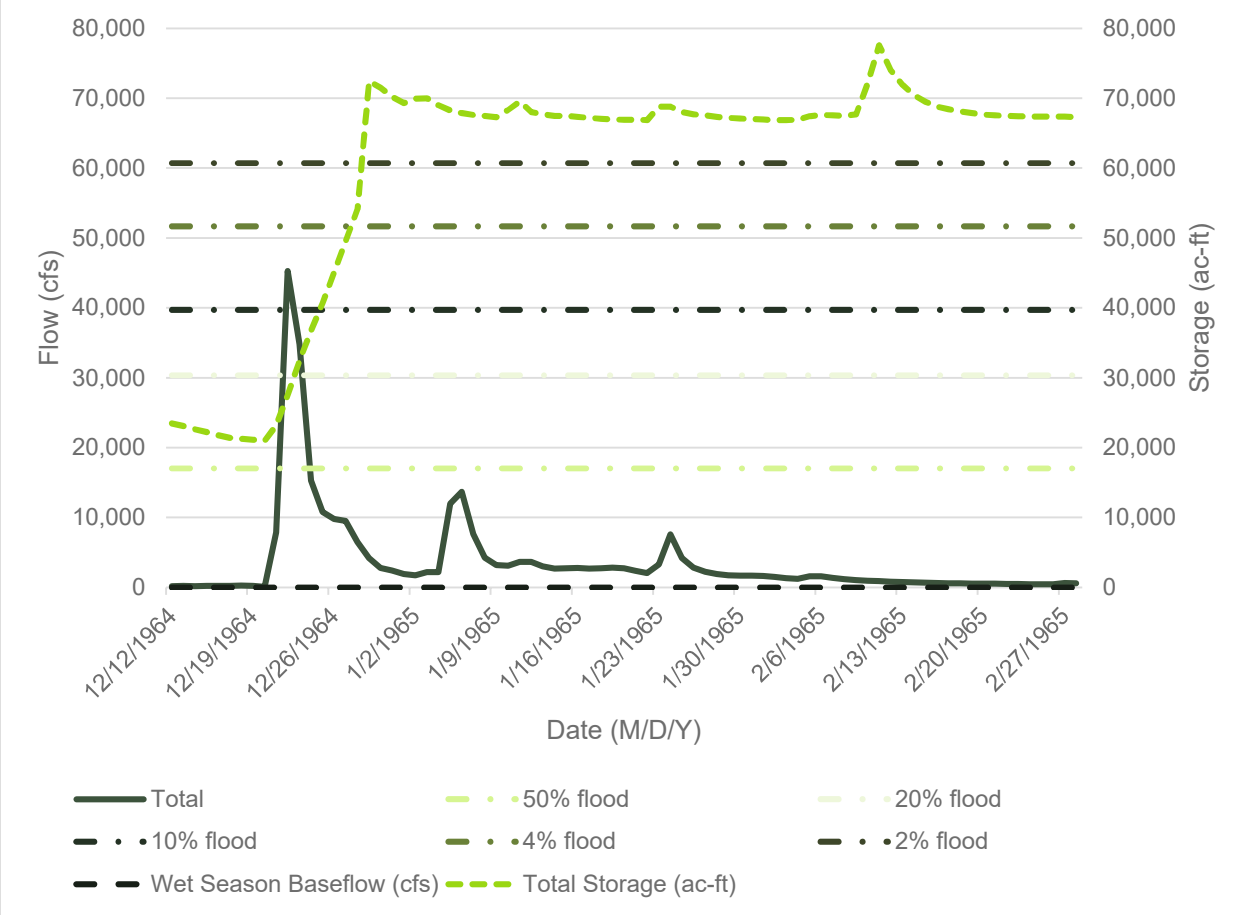
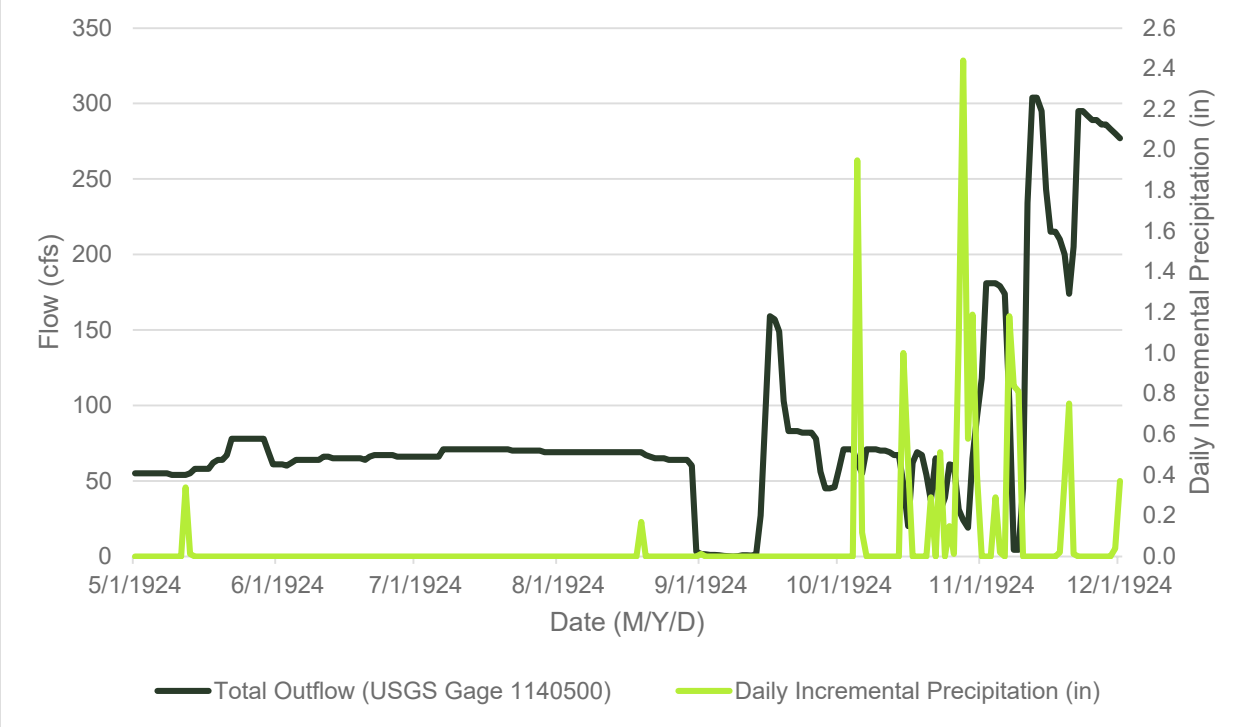


**Figure A3-8: Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 1924 dry season (period of record low outflow)**




SLR was not provided with any operational or maintenance data that has occurred since the inception of Scott Dam. There are notable low storage periods where maintenance activities would have required a conscious operational lowering of the dam WSE. Lower flows could have resulted from maintenance actions, changes in operation such as a drought year when limited to no outflows were required to maintain storage levels, or low inflow water years. The high flows are likely from precipitation events or high volumes of snowmelt.

# A4.0 Hydrologic and Hydraulic Data Analyses

The existing data (A2.0 & A3.0) was used in support of the hydrologic and hydraulic analyses completed for the Scott Dam scenario evaluation. Limited data was provided from previous studies and was recreated with limited precision. The hydrologic data included precipitation data, flood flows, drought conditions, flow contributions by watercourse and water balance recreation. The hydraulic data included stage-storage and rating curves for dam operations.

## A4.1 PG&E Water/Mass Balance Data

PG&E provided water data pertaining to WYs 2005-2024 which included Lake Pillsbury's storage volumes, WSEs measured from the PG&E specified datum, outflow from Lake Pillsbury (gage E-2), outflow from Eel River below Cape Horn Dam (gage E-11), and outflow from diversion to Russian River Basin (gage E-16) (Appendix A.B). PG&E also provided calculated net inflow into Lake Pillsbury values which were based on a mass balance approach per the hydrology and operations study memorandum (Cardno, 2019). The provided Lake Pillsbury net inflow values removed evaporation outflows from the lake water surface. Negative values for net inflow into Lake Pillsbury were reported which were possibly due to discrepancies in individual data points, inconsistencies in the time frames when measurements were recorded, changes to

the operation of the dam, and the nature of utilizing a mass balance approach. The data provided did not include all the required components to precisely reproduce the PG&E water/mass balance values. However, SLR was able to produce a water balance model utilizing available data for Scott Dam and Lake Pillsbury for the purposes of analyses of the potential mitigation scenarios. The water balance model produced by SLR is further discussed in Section A4.5.

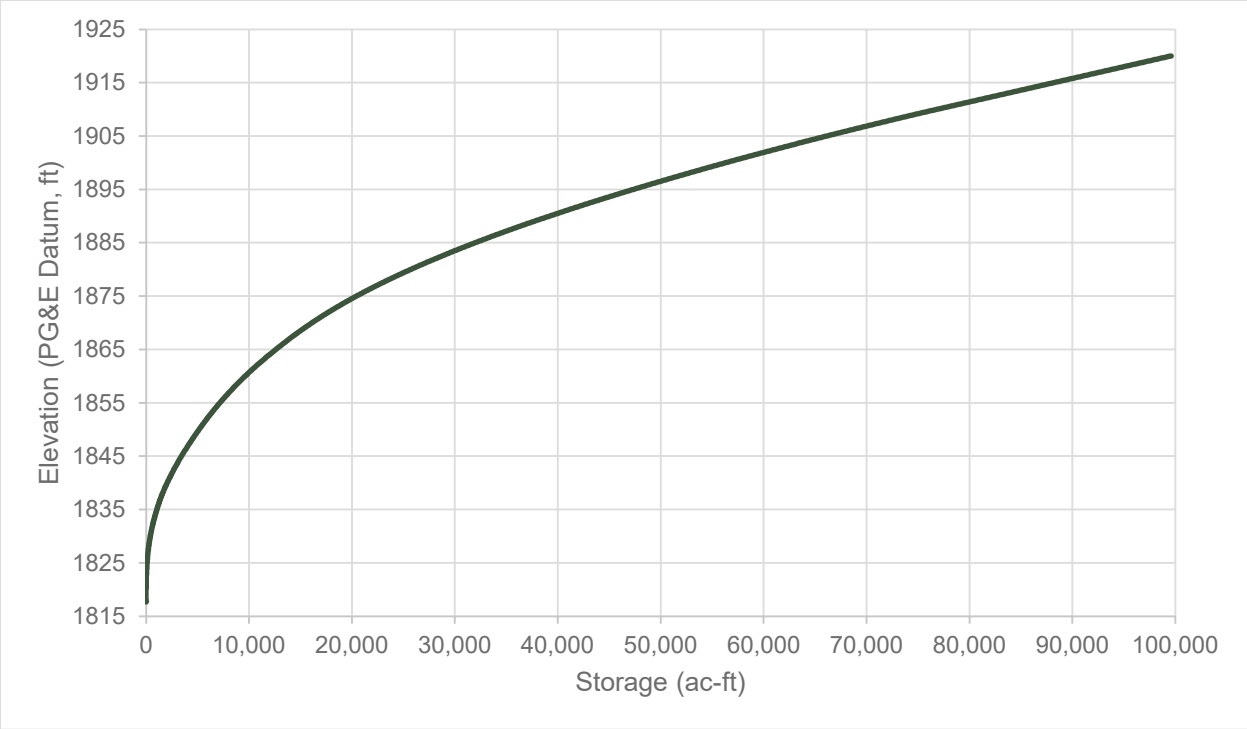
## A4.2 Stage-Storage Curve

Multiple bathymetry[12] surveys have historically been conducted for Lake Pillsbury in 1921, 1959, 1985, 2005, 2015-2016, and 2023 to estimate the water storage capacity impounded by Scott Dam. The original 1921 storage capacity for Lake Pillsbury was 94,400 ac-ft at WSE 1,910 feet using the PG&E datum (Porterfield, 1964). PG&E's bathymetry survey representing the 2015-2016 conditions, estimated the storage capacity for Lake Pillsbury to be 76,876 ac-ft at WSE 1,910 feet (PG&E, 2016). The change in storage capacity is due to sediment accumulation over time.

PG&E's bathymetry survey representing 2015-2016 conditions was used by SLR to generate a stage-storage curve that relates Lake Pillsbury's WSE to its water storage capacity as depicted in Figure A4-1 (PG&E, 2016). The 2015-2016 bathymetry survey was selected due to the availability of lake storage and water surface area values at each WSE on a 0.1 ft interval basis. SLR used this curve to evaluate water storage during each of the relevant scenarios and climate conditions. Since the bathymetry data range covered WSE up to 1,910 feet, SLR extrapolated values from 1,910 feet to 1,920 feet based on a best fit curve to evaluate the storage capacity. This stage-storage curve was used to estimate Lake Pillsbury's storage under each climate and mitigation scenario.

[12] The underwater equivalent to topography, the study of underwater depth in lakes, oceans and rivers.

**Figure A4-1: Recreated Stage-Storage Curve from PG&E 2016 Bathymetry Report**




## A4.3 Rating Curves

Scott Dam has two outflow structures: the needle valve and the spillway. Individual rating curves for the needle valve and the spillway were produced by Cardno (2019) and are presented in Figure A4-2 and Figure A4-3, respectively. Outflows from Lake Pillsbury through the needle valve are a function of the Lake Pillsbury WSE and the needle valve opening. The needle valve outflows can fluctuate over a wide range at each WSE due to the variability of the need valve opening size and flexibility in operations. Figure A4-2 also presents historical combinations of the needle valve opening and Lake Pillsbury WSEs with the needle valve rating curve. A plot of the WSE versus the needle valve outflow range representing minimum and maximum needle valve opening size extents is provided in Figure A4-4.

Figure A4-3 summarizes the variability in the outflows through the Scott Dam spillway. The spillway has five radial gates and 26 steel slide gates, with all but Gate 13 being manually operated. There are difficulties in defining an accurate rating curve as there is a lot of dependence on manual operations and no data to support which gates were operable or open during the period of record used to develop the rating curve. There are also no records of when the needle valve was installed as it is presumed the needle valve was not included in the original dam construction. The figure results show a combination of multiple analyses relating the Lake Pillsbury WSE to spillway outflow based on modeling, previous rating curve developments, and empirical analysis. The revised rating curve as reported by Cardno (2019) was selected as the most sensible for use in the analyses of the potential Scott Dam scenarios and is shown in Figure A4-5.

**Figure A4-2: Needle Valve Rating Curve with Historic Needle Valve Opening Values (Figure AQ 1-17, Cardno, 2019)**

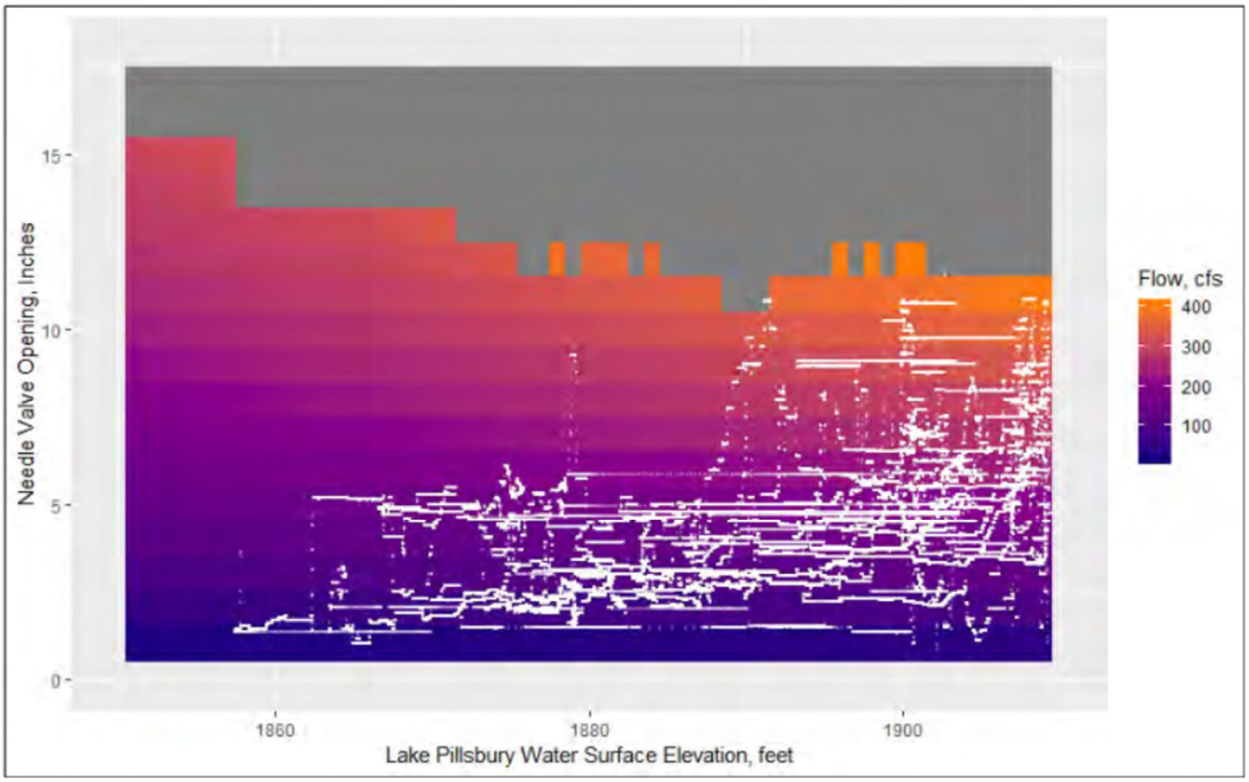


**Figure A4-3: Scott Dam Spillway Rating Curve (Figure AQ 1-18, Cardno, 2019)**

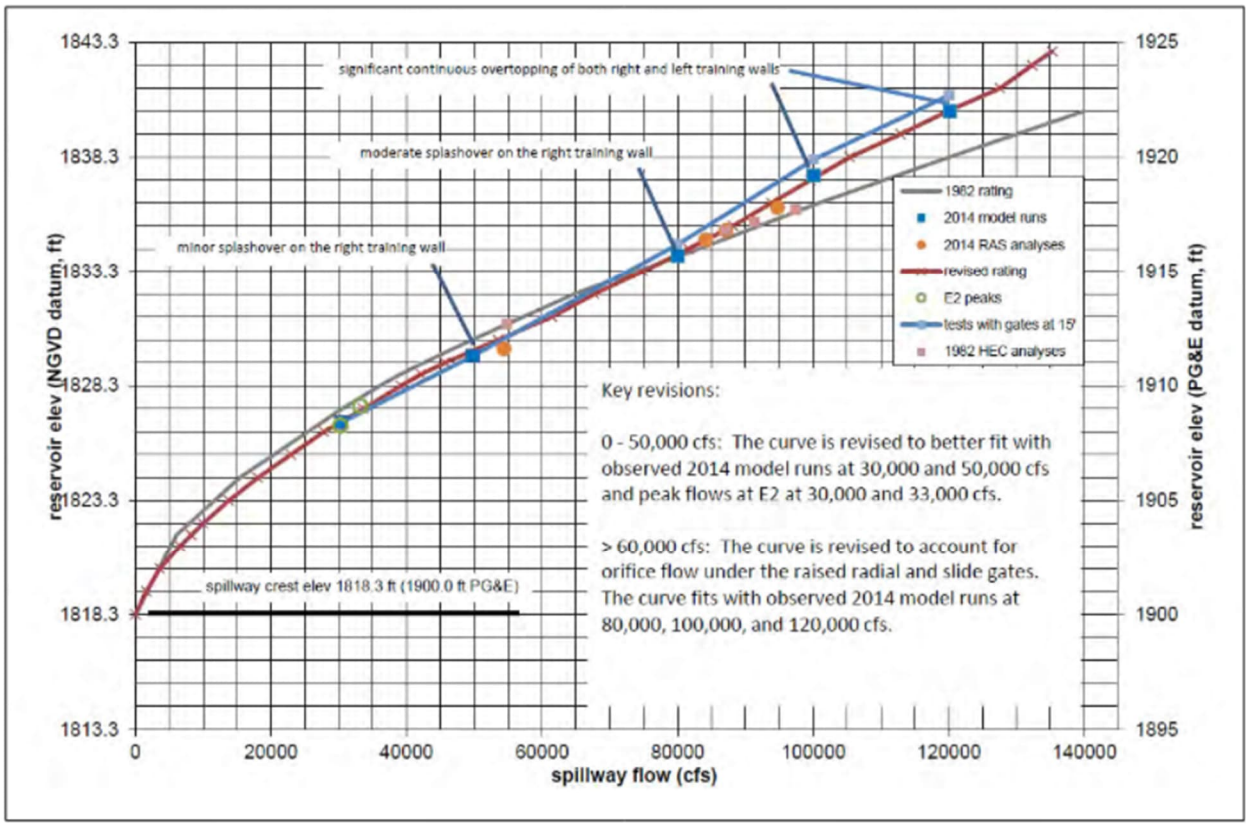


**Figure A4-4: Needle Valve Opening Outflow Range Estimates of the Minimum and Maximum (based on Figure AQ 1-17, Cardno, 2019)**

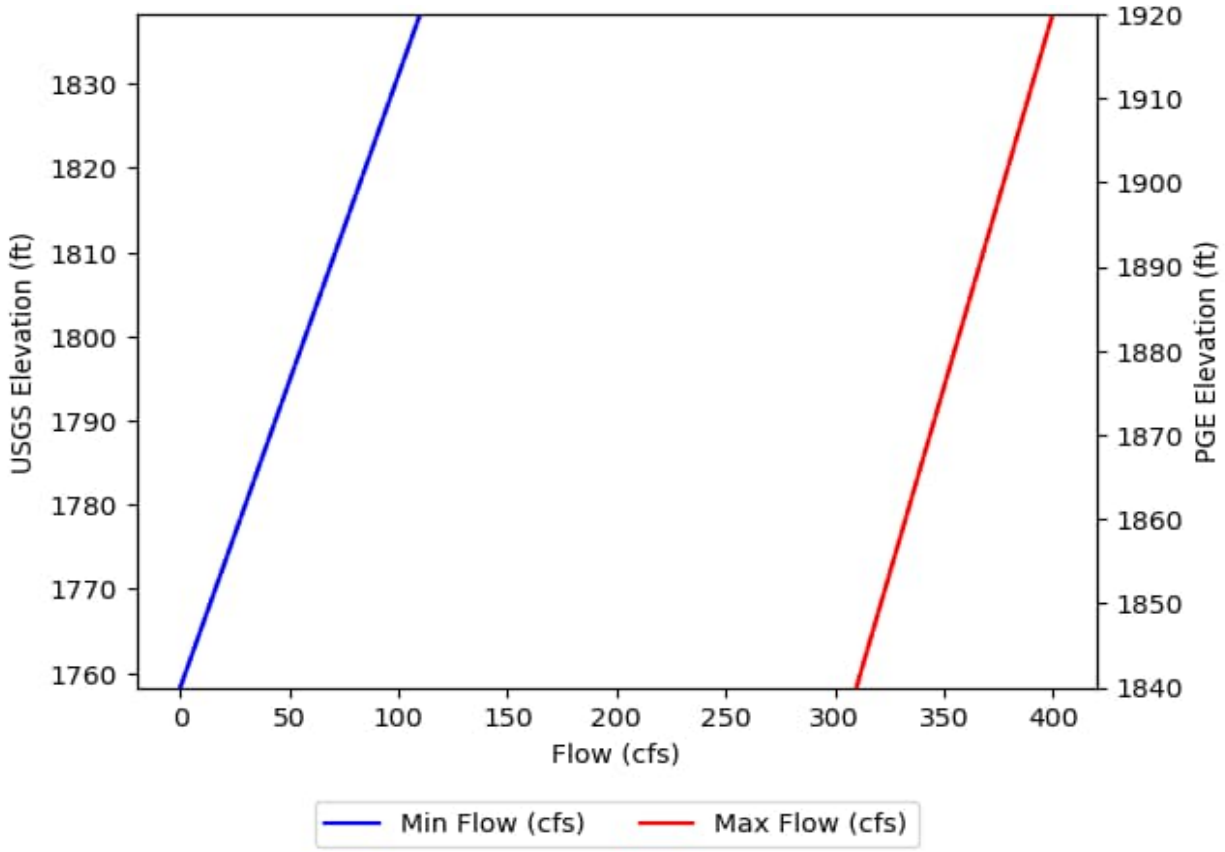
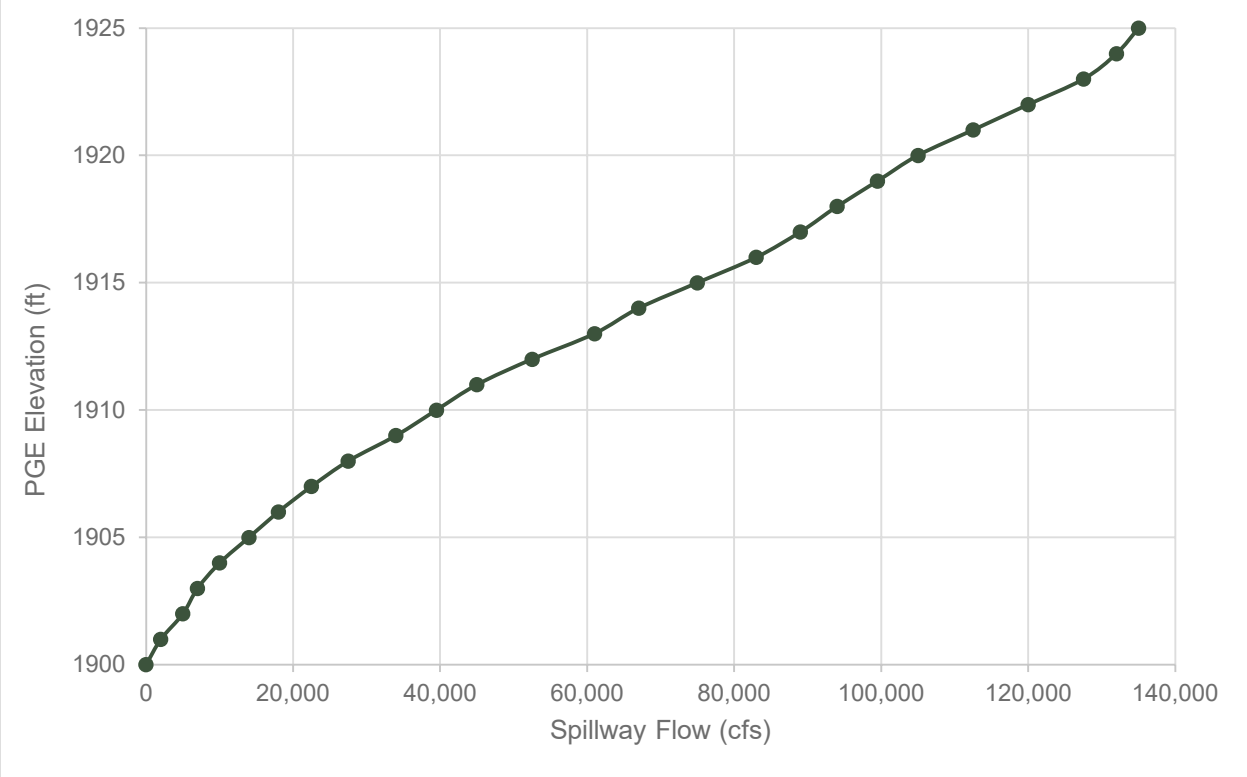


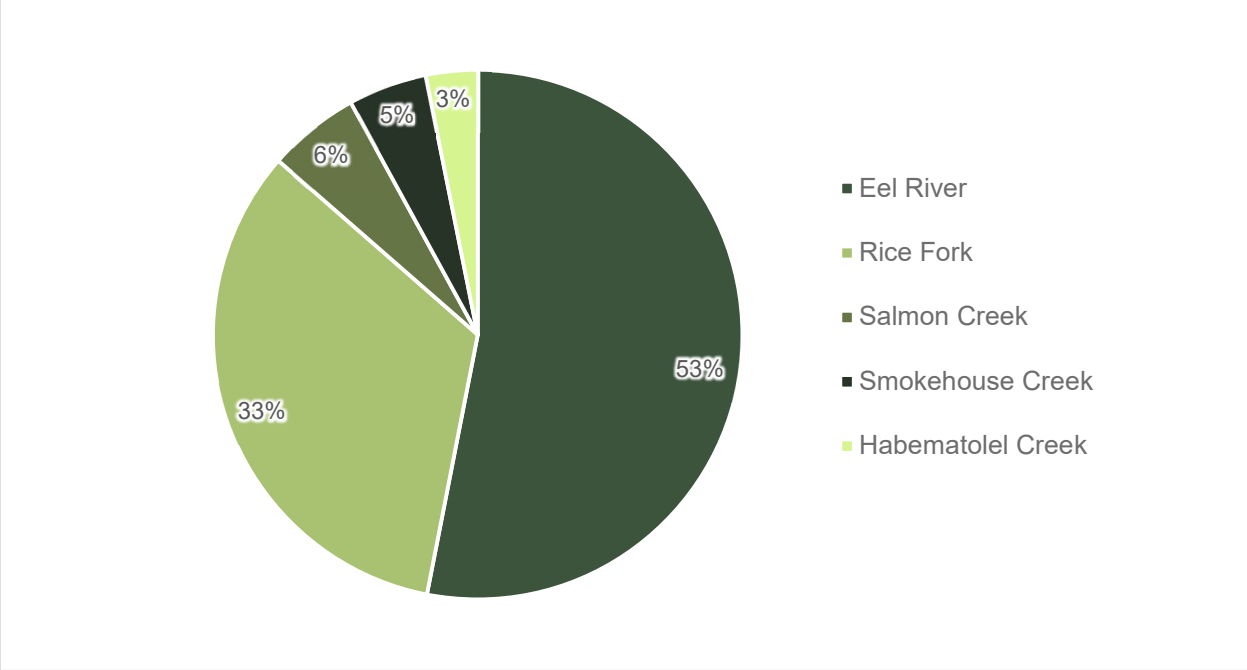
**Figure A4-5: Revised Rating Curve for the Scott Dam Spillway (based on Figure AQ 1-18, Cardno, 2019)**




## A4.4 Flow Contributions to Lake Pillsbury

SLR used StreamStats and USGS's hydrographic dataset to develop a basic understanding of the hydrology of the Eel River and its contributing watercourses under existing and post dam removal conditions. StreamStats reported that the main streams contributing to Lake Pillsbury are the Eel River, Rice Fork, Salmon Creek, Smokehouse Creek, and Habematolel Creek. The Eel River watershed has the highest contribution of flows by watershed area at approximately 53%, with the Rice Fork contributing 33%, and the Salmon, Smokehouse, and Habematolel Creeks combined contributing the other 14%. Figure A4-6 summarizes each major tributary flow contribution based on watershed area. The percentage contributions of each tributary were relatively consistent with the watershed area across all flood events calculated in StreamStats, from the 50% AEP (2-year recurrence) flood to the 0.02% AEP (500-year recurrence) flood. The Eel River ranged from 49-53% contribution, Rice Fork remained consistent at 33%, and the total contribution from the Salmon Creek, Smokehouse Creek, and Habematolel Creek combined ranged from 14-18%. SLR applied the flow contribution percentage based on watershed area to analyses of climate conditions and potential Scott Dam scenarios. These flow contributions are limited to area and flow comparison. Other contributing factors should be analyzed for further efficacy such as precipitation, slope, time of concentration, infiltration, and overland roughness based on the land cover.

**Figure A4-6: Flow Contributions by Watershed Area from Major Tributaries to the Eel River Watershed at Scott Dam**




### A4.4.1 Baseflows

All perennial watercourses have a baseflow[13] that fluctuates during the seasons but always retains some flow. The data available does not allow for any accurate projections in the potential baseflows. Analyzing the Scott Dam impaired Eel River outflow can provide very limited insights into the projected watercourse baseflows due to the storage and controls attenuating flows. The wet and dry season baseflows could roughly be estimated based upon the mass balance inflows provided by PG&E from 2006 to 2024 by removing evaporation and averaging the minimum average inflows, then correlating them to the percentage stream contributions. However, not all the streams are perennial, and the PG&E net inflow data had negative values that skewed the analysis so this method was deemed improbable and another hydrologic analysis using precipitation data would need to be completed for more reasonable results. This analysis did not report or predict baseflow levels and just reports stream typing.

## A4.5 Water Balance Model

A water balance model for Scott Dam and Lake Pillsbury was produced by SLR using Microsoft Excel-based spreadsheets. The water balance model results estimated Lake Pillsbury outflow in the Eel River below Scott Dam, Lake Pillsbury water storage, and Lake Pillsbury WSE based on estimated Lake Pillsbury unimpaired inflow on a daily model calculation timestep. Daily average data was available.

Unimpaired inflow to Lake Pillsbury values were projected for the Scott Dam system using the mass balance approach from Cardno (2019) which estimates Lake Pillsbury inflow by accounting for Scott Dam outflow, changes in Lake Pillsbury water storage, and evaporation

[13] The portion of the streamflow that is sustained between rainfalls, fed by groundwater or snowmelt and represents the normal flow within the watercourse.

losses from the Lake Pillsbury water surface area. Available data from the PG&E operated gages and PG&E mass balance data covering WY 2005 to 2024, USGS downloaded flow gage in the Eel River below Scott Dam and Lake Pillsbury storage gage covering WY 1957-2024, and the regional Coyote Dam (Lake Mendocino) pan evaporation gage covering WY 1994-2024 were used to recreate the mass balance. Since pan evaporation gage data represents a limited period of record, average monthly evaporation rates were calculated to produce a representative year that was repeated in the water balance model. The average annual evaporation is 42.9 inches.

Two sets for unimpaired inflow to Lake Pillsbury values were developed for the water balance model. A minimum daily Lake Pillsbury inflow value of 0 cfs was assumed to avoid negative values. One set utilized the PG&E data representing WY 2005-2024 and the other set utilized the USGS data representing WY 1957-2024. The Lake Pillsbury inflow values set using PG&E data was used for the purpose of calibrating the water balance model. Both data sets were deemed to have negligible value differences for the time periods that overlapped. Hence, the Lake Pillsbury inflow values set using USGS data was used for the evaluation of the potential Scott Dam scenarios since the set covered a longer record.

The stage-storage curve representing the 2015-2016 bathymetry survey discussed in Section A4.2 was used to estimate Lake Pillsbury water storage and WSE values. The rating curve discussed in Section A4.3 for the existing spillway was used to estimate outflow through the spillway when the Lake Pillsbury WSE is above the spillway invert elevation. A simplified rating curve discussed in Section A4.3 for the needle valve was used in the water balance model to estimate outflow through the needle valve. Needle valve outflow values ranged from minimum opening operating flow when the Lake Pillsbury WSE is low to maximum full capacity opening flow when the Lake Pillsbury WSE is high.

The water balance model was adapted based on the specific potential mitigation scenario analyzed. The overviews of the potential scenarios analyses and outcomes are presented in Section A7.0.

# A5.0 Fire Suppression Water Requirements

As discussed in Appendix B, Lake Pillsbury is currently an optimal resource for current support for fire suppression and operates as an extensive use water source for wildfire firefighting operations in the County of Lake. Appendix B information is summarized herein specifically to present the water storage needs with respect to optimal fire suppression within the County of lake.

When constructing a new reservoir which can support helicopter operations, specific size and maneuverability criteria should be met. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet at an average depth of 6 feet. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223-669 square feet. These considerations are summarized in Table  and listed by operational need for aerial firefighting.

**Table A5-1: General Aeiral Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Gallons [acre-feet] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Extensive Use | 100,000+ [0.31] | 6 feet | 2,230 sq. ft. | 47 ft × 47 ft or 30 ft × 75 ft |

| Operational Need | Required Gallons [acre-feet] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Limited Use (Low Volume) | 10,000 [0.031] | 6 feet | 223 sq. ft. | 15 ft × 15 ft or 10 ft × 23 ft |
| Limited Use (High Volume) | 30,000 [0.092] | 6 feet | 669 sq. ft. | 26 ft × 26 ft or 15 ft × 45 ft |

# A6.0 Data Limitations

While the amount of existing data is numerous regarding the proposed decommissioning of Scott Dam, in general the data is not comprehensive and relies on limited and incontiguous data that has been limitedly validated or calibrated. The SLR analyses projected results and assumptions are founded on public records, the limited data provided by PG&E, and hydrologic projections produced by other consultants. Data provided by others (PG&E, Two Basin Partnership, etc.), outside of publicly available data, did not provide the raw data or models used to develop and support the previous findings. The projections made were based upon reproducing the existing data by interpolating information from the various graphs and publicly available data.

The publicly available data, though abundant, was not always continuous and was limited in its viability to produce unimpaired flows or in projecting variability and/or extreme events. The projections and recommendations provided herein are based upon trends and ranges perceived using the data assessments completed by others with minor supplemental information to validate as available.

## A6.1 Station Data

There are various weather stations and gage stations that have recorded data within the Upper Eel River Data as far back as the early 1900's with varying degrees of record and data types reported. The weather station data was not clearly used in any of the previous analyses completed by others, as only the impaired or regulated stream outflow from Scott Dam and the Lake Pillsbury storage records were used for analysis, specifically within Lake County. This weather station data, particularly rainfall records coupled with additional modeling efforts, would further support the unimpaired climate condition projections.

The USGS gages 11470500 and 11470000 are maintained and operated by PG&E and record the average daily discharge from the Scott Dam control structures and the water storage within Lake Pillsbury based on the most recent lake bathymetry, respectively. Using the average daily flows and storage values have several limitations when projecting flow variations and responses to extreme (flood and drought) conditions.

Averaged data limitations can include:

1. Obscuring short-term variations in storage levels and flows, which are important for understanding flood events, ecological functions (fish passage), and/or rapid changes in reservoir levels.
2. Dampening or smoothing out important fluctuations such as evaporation, inflows, and outflows that would show the impact of high or low flow periods and/or changes in the reservoir levels.
3. An inaccurate representation of the actual system dynamics, especially during rapid changes in flow rates or storage levels.

4. Not capturing the full range or complexity of flows or reservoir operation nuances leading to less precise projections and assessments.

## A6.2 Water (Mass) Balance Method

A previous analysis by Cardno (2019) utilized a water or mass balance approach to project the system trends and inflows to Lake Pillsbury. The water balance approach approximated the inflows by accounting for the assumed outflows from the system such as operations and environmental releases and changes in storage. Cardno estimated these operational outflows by developing rating curves for the Scott Dam control structures (needle valve and spillway). The analysis also developed a flow pulse travel time relationship for each major stream reach to estimate flow routing or the travel time for water to move through the system and used statistical methods to estimate data gaps.

Cardno noted several specific concerns with the data interpretation and assessment limitations:

1. The spillway rating curve at Lake Pillsbury gives the spillway capacity at given WSEs with all the gates completely open. However, the reservoir rarely operated with all the gates completely open.
2. which can lead to discrepancies between recorded flows and the rating curve.
3. The needle valve rating curve relates the needle valve opening and Lake Pillsbury water surface elevation to the flow rate (Figure A4-3). There are instances where the calculated needle valve flows do not match the recorded stream gage flows, indicating potential inaccuracies. Additionally, the figure shows that both the needle valve opening, and outflow operate independently of WSE. This could indicate operator error, high variation in operator controls, and/or that a further look into the needle valve's relationship with storage and flow is needed.
4. High flow measurements at the Eel River stream gage just below the Scott Dam generally match the spillway capacity curve, but there are instances where recorded flows exceed the spillway capacity, suggesting the need for revision of the rating curve at high flows.

There are additional limitations with the mass/water balance approach used which include:

1. There are gaps in the data and even when using statistical methods to estimate the missing data, the estimates can introduce uncertainties. Wider and more prevalent gaps coupled with averaged data increases the uncertainty.
2. The travel time estimates and curves are developed using flow pulse data, but these estimates do not always reflect real-world conditions. Especially during extreme events and when there is not data that can be used to calibrate the estimates, such as within the watersheds above Scott Dam.
3. Evaporation rates were estimated using regression analysis, which may not capture all the variability in evaporation processes. Additionally, the reference source Cardno used were only recorded the average monthly evaporation between 1964 and 1970. Additional daily evaporation data is available that could be used to refine the regression and evaporation projections based on temperature and the Lake Pillsbury water surface area.
4. The mass balance flows averaged data using various techniques that can smooth and dampen important variations in flow data.

5. The stream gages are maintained and operated by PG&E and the accuracy of the data can be affected by factors such as operator error and gage bias.
6. There were no projections of the inflow and outflow contributions from groundwater, interception and evapotranspiration, subsurface flows, and snowmelt from Lake Pillsbury.

# A7.0 Scenario Overview

SLR utilized the previous analyses, other referenced reports, and assumptions to evaluate the hydrology and hydraulic conditions related to the baseline scenario and the seven (7) mitigation scenarios and have presented a response under four (4) climate conditions using statistical and analytical methods. These evaluations include projections of each scenario's outcomes for water supply, seasonality, flooding, and drought may affect availability. The baseline scenario reflects to historical conditions from 1922-2019, existing conditions refer to 2019-present, and scenarios A1, A2, B, C, D and E are all possible future pathways that the County of Lake could advocate. These scenarios outline a wide range of options, including partial or full dam removal, inclusion of a fish ladder, changes to the reservoir storage availability, and the creation of off-channel wetlands. These scenarios are based on PG&E's actions and intent as well as additional scenarios to support the County's interests.

## A7.1 Baseline Scenario: Historical Operational Conditions (WY 1922-2019)

The baseline scenario (historical operational conditions) scenario represents the period when Scott Dam and Lake Pillsbury maintained operations per the FERC license at full capacity storage. In 2018, McMillen Jacobs performed the Potter Valley Project Capital Modifications Feasibility Study, where they evaluated several alternatives for Scott Dam. These historical operational conditions scenario was referred to as the "Baseline" scenario in their Feasibility Study (McMillen, 2018).

Figure A7-1 summarized the historical operational conditions scenario and assumes a maximum capacity of 76,876 ac-ft and a usable storage capacity of 66,876 ac-ft as previously observed between 1922-2019 (PG&E, 2023). Over the 97-year period of record, the dam operations have changed along with the release requirements from the dam. Figure A7-2 shows how storage and outflow have changed since its construction in 1922 and indicates operational changes several times over the 100 years. Storage was decreased after each change in operations, reaching its lowest baseline after 2019 when the operational level was reduced by 10 ft. SLR did not observe any substantial changes in outflow correlating to operational changes from 1922-2019. These observed operational changes also correlate to bathymetric surveys completed and the updated reduction in storage capacity due to sediment accumulations as summarize in Table A7-1. Operations were assumed to maintain a consistent storage capacity in between surveys which were completely between 10-30 years since inception. Sediment accumulation typically happens each year with subsequent storage capacity loss. Accumulations and storage loss are not necessarily linear and are dependent upon variables such as sediment grain size, flows, flow velocity, slopes, shear stress, sediment concentration, vegetation and land cover, and anthropogenic activities. Lake Pillsbury on average loses approximately 210 to 230 ac-ft of storage each year.

**Table A7-1: Summary of PG&E Determined Lake Pillsbury Storage Capacities Based on Bathymetric Surveys and/or Changes in Operation**

| Bathymetric Survey Year | Lake Pillsbury Storage Capacity (ac-ft) |
|---|---|
| 1921 | 94,400 |
| 1959 | 86,700 |
| 1985 | 80,643 |
| 2005 | 74,993 |
| 2015 | 77,419 |
| 2016 | 76,876 |
| 2023 | 69,872 |

Notes:
1. Source for 1921 and 1959 from Porterfield (1694) (Porterfield, 1964)
2. Source for 1985 through 2023 from PG&E (2024) (PG&E, 2024)
3. 2015 to 2016 increased due to improved equipment & techniques (PG&E, 2017)

The historical operational conditions analysis primarily focused on the operational period from 2005-2019, however the historical conditions from 1922-2019 are also provided for comparison. Historically, these operational conditions have been able to meet the outflow obligations. However, there have been instances each year between 2013 and 2016 where PG&E has had to request a temporary flow variance due to drought conditions (PG&E, 2023).

**Figure A7-1: Historical Operational Conditions (WY 1922-2019) scenario: measured minimum, average, and maximum Lake Pillsbury WSE levels**

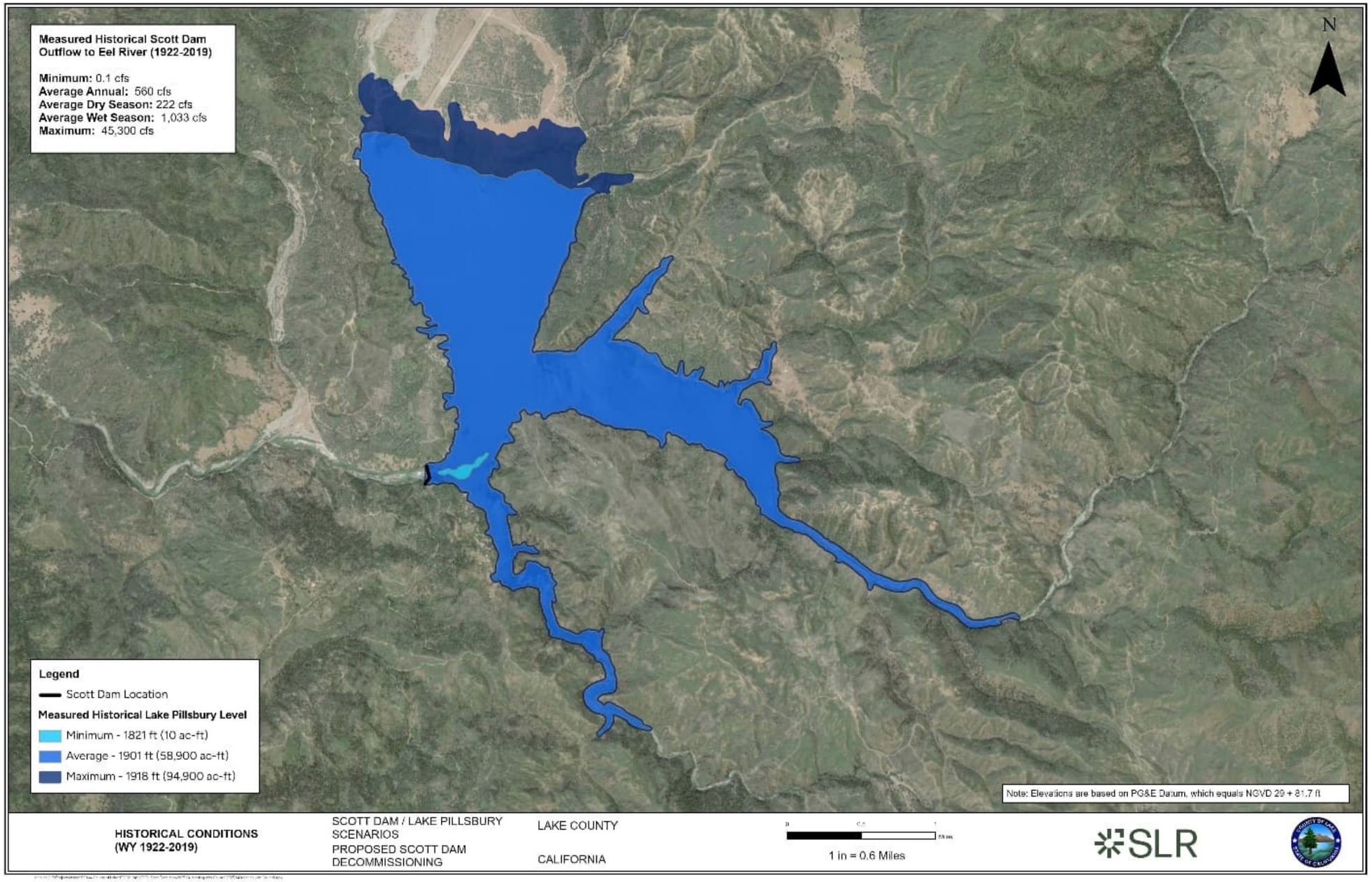
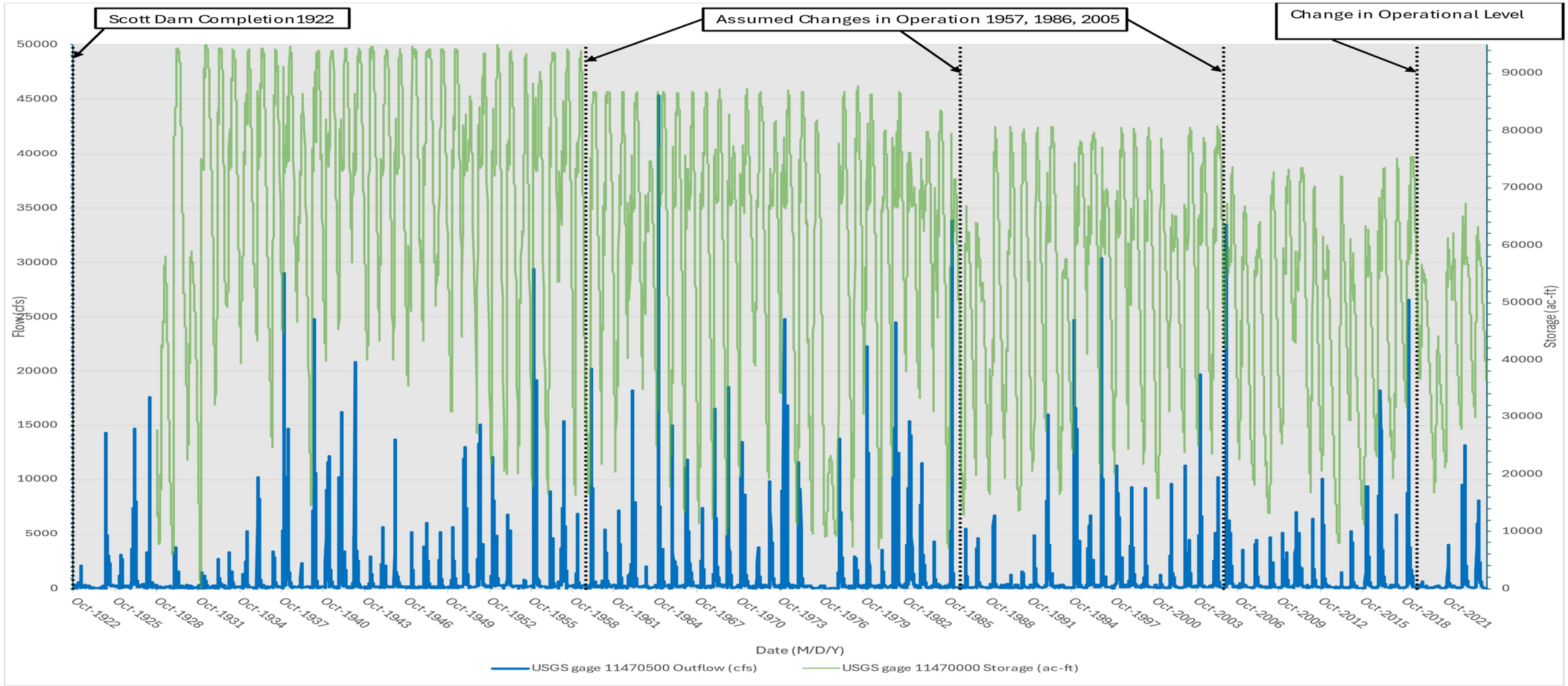


**Figure A7-2: Outflow and storage of Lake Pillsbury from 1922-2024 (dam construction and operational changes are specified)**



### A7.1.1 Hydrologic Analysis

The PG&E-provided data and available gage data were used to statistically analyze the observed outflows and storage for the historical operational historical conditions. Two water year timeframes were analyzed for historical conditions for the wet and dry season climate condition: 1922-2019 and 2005-2019. The 1922-2019 period was used to assess the flood and drought conditions along with the NIDIS.

#### A7.1.1.1 Wet and Dry Season Conditions

The minimum, median, average, and maximum values were provided for the average daily storage and outflow records for the wet and dry season over the WY period of record. Storage and outflow statistics were determined independently of each other, meaning the minimum outflow did not necessarily coincide with the minimum storage. Statistics for the wet and dry seasons are summarized in Table A7-2. These WSEs are provided for reference only as they were determined based upon the 2016 bathymetry and will lose accuracy the farther the storage deviates from the 2016 storage capacity. The USGS Porterfield (1964) report on Lake Pillsbury was developed to study the quantity, distribution, and properties of sediment deposited within Lake Pillsbury and recorded a maximum WSE of 1,829.1 feet Mean Seal Level (MSL/NGVD22) or 1910.8 feet PG&E on May 13, 1925. The maximum recorded by Porterfield was approximately 7.1 feet lower when compared to the 2016 bathymetric storage results. WYs 2005-2019 show a lower overall average in the storage and outflow observed as well as a lower maximum, and a higher minimum. This could be attributed to a longer period of record logging more data from extreme events as well as changes in operating procedures. The records for maintenance were not provided and could also influence the statistical validity.

**Table A7-2: Historical Operational Conditions – Statistics for Storage within Lake Pillsbury and Outflow Downstream of Scott Dam by Season**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| **WY 1922-2019** | | | | | | |
| Minimum | 10 | 1820.9 | 2 | 2,100 | 1840.1 | 0.1 |
| Median | 65,900 | 1904.8 | 375 | 59,700 | 1901.7 | 177 |
| Average | 59,810 | 1901.8 | 1,033 | 58,240 | 1901 | 222 |
| Maximum | 94,900 | 1917.9 | 45,300 | 94,900 | 1917.9 | 10,200 |
| **WY 2005-2019** | | | | | | |
| Minimum | 7,840 | 1856.4 | 37 | 11,300 | 1862.9 | 27 |
| Median | 55,400 | 1909.3 | 296 | 45,200 | 1879.4 | 147 |
| Average | 49,078 | 1896 | 820 | 45,911 | 1894.1 | 165 |
| Maximum | 80,200 | 1911.4 | 26,600 | 80,800 | 1911.7 | 2,400 |

### A7.1.1.2 Flood and Drought Conditions

Table A7-3 summarizes the observed maximum storage and outflow from the Scott Dam and Lake Pillsbury gages in response to extreme flood conditions for the historical operational conditions. As noted in Flood and Drought of Record Section 3.2.2, the flood of record occurred during WY 1982 (Metzler, 1983). The highest average daily outflow and storage observed during the flood of record were on January 26 and 27, 1983 respectively. The observed gage measurements were 76,260 ac-ft for storage and 15,400 cfs for outflow. In comparison, the maximum gage daily average storage measurement recorded within Lake Pillsbury was on April 24, 1932, and happened to be during one of the notable drought seasons within the last century. The highest gage recorded daily average outflow was observed on December 22, 1964, and does not correlate to any specific event.

**Table A7-3: Historical Operational Conditions – Observed Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | | | |
|---|---|---|---|---|---|
| | Date of Record Storage | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Date of Record Outflow | Average Daily Outflow (cfs) |
| Gage | 4/24/1932 | 94,900 | 1917.9 | 12/22/1964 | 45,300 |
| Historic Records | 1/26/1983 | 76,260 | 1909.7 | 1/27/1983 | 15,400 |

The Flood and Drought of Record Section A3.2.2 notes multiple drought occurrences over the past century. The lowest gage recorded average daily storage value of 10 ac-ft occurred on December 9, 1931, which coincides with the 1928-1934 drought. Presumably, this took place prior to the needle valve outflow structure installation at Scott Dam which would have incurred less precision in the outlet and storage controls. There is also a possibility of manual reservoir drawdown for maintenance or operational reasons that were made unavailable to SLR. The lowest gage recorded average daily outflow was 0.1 cfs which occurred on September 8, 1924, which does not correlate to a notable recorded event.

During drought periods, notable storage and outflow rates were observed when the average daily storage was below the recommended needle valve operating range of 10,000 ac-ft which occurred on January 20, 1977, and February 5, 2014. The lowest average daily storage recorded during the 1976-1977 drought was 9,018 ac-ft and the lowest average daily flow was 5 cfs. The lowest average daily storage recorded during the 2012-2016 drought was 7,840 ac-ft and the lowest average daily flow was 27 cfs. Table A7-4 summarizes the observed minimum storage and outflow from the Scott Dam and Lake Pillsbury gages occurring the full record period and during notable drought years.

**Table A7-4: Historical Operational Conditions – Observed Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | | | |
|---|---|---|---|---|---|
| | Date of Record | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Date of Record Outflow | Average Daily Outflow (cfs) |
| Gage | 12/9/1931 | 10 | 1820.9 | 9/8/1924 | 0.1 |
| Historic Records | **1928-1934** | | | | |
| | 12/9/1931 | 10 | 1820.9 | 4/28/1931 | 1.8 |
| | **1976-1977** | | | | |
| | 1/20/1977 | 9,018 | 1858.8 | 2/19/1977 | 5 |
| | **2012-2016** | | | | |
| | 2/5/2014 | 7,840 | 1856.4 | 10/30/2015 | 27 |

### A7.1.2 Baseline Scenario (Historical Operations) Outcomes

There main component being considered when analyzing the baseline historical conditions, is the scenario's historical ability to reliably supply water.

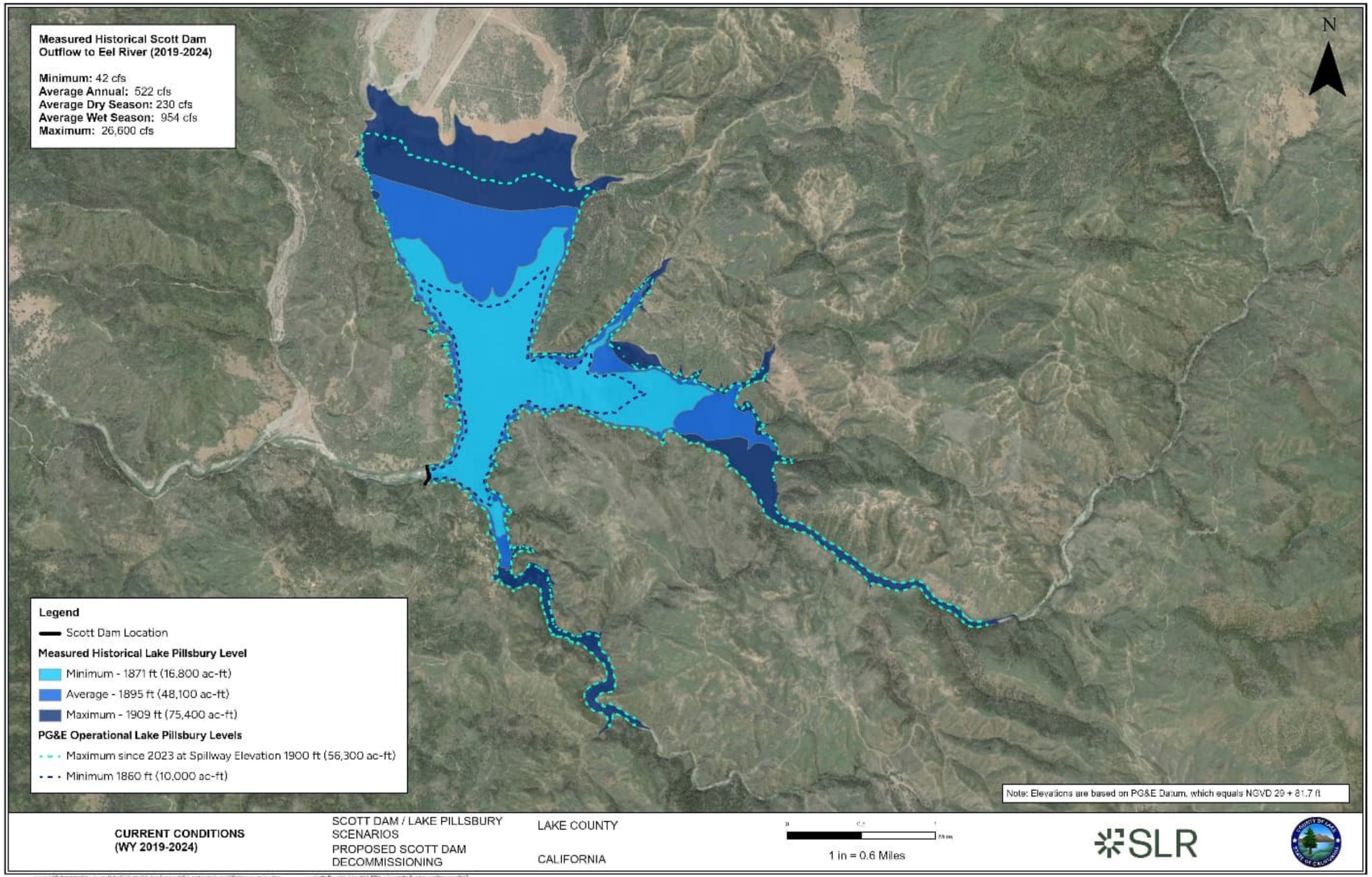
#### A7.1.2.1 Water Supply

This baseline historical scenario provides the most reliability on water supply in terms of flood control, potential groundwater recharge, downstream water diversion flow requirements, and operations due to the operability control provided by the needle valve and spillway gates for the purpose of downstream hydroelectric power generation. Lake Pillsbury has historically operated with an average storage volume of over 45,000 ac-ft (14 million gallons) of water availability. Floods have historically shown to be attenuated by the reservoir, reducing the flood risk to downstream infrastructure. This historical scenario has a long history of providing water supply for downstream uses other than periods of drought where flow variances were required. This scenario did not analyze future climate conditions but noted that the drought trends during the 21st century have been increasing in frequency, intensity, and duration. This historical scenario's water storage capacity has been reduced over time due to losses from sediment accumulation.

## A7.2 Existing Operational Conditions Scenario (WY 2019-Present)

The existing operational conditions scenario represents the Scott Dam and Lake Pillsbury operational period from 2019 to the present. Based on observed operational data, the maximum operational WSE for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 feet with a correspond water storage volume of 56,322 ac-ft as depicted in Figure A7-3. Historically, the maximum operational WSE for Lake Pillsbury was at top elevation of the spillway gates of 1,910 feet. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the WSE at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained. It is not known if there have been operational changes to the needle valve.

**Figure A7-3: Existing Operational Conditions (WY 2019-2024) scenario: measured minimum, average, maximum, and operational Lake Pillsbury WSE levels**



### A7.2.1 Hydrologic and Hydraulic Analysis

The PG&E-provided data and available gage data were used to statistically analyze the observed outflows and storage for the current operation conditions for WYs 2019-2024. An additional analysis was completed utilizing the water balance model to project the storage and outflows under the current operations as the observed values are over a limited timeframe.

#### A7.2.1.1 Wet and Dry Season Conditions

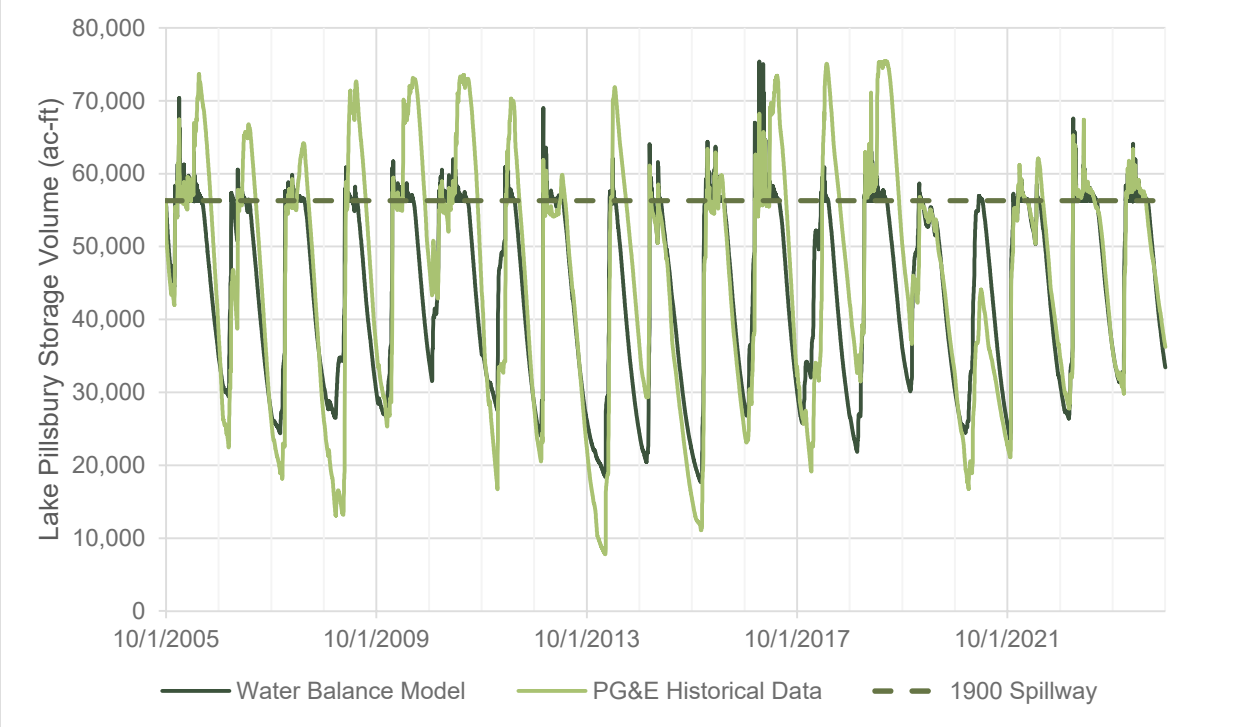
A summary of the observed wet and dry season climate conditions covering the five-year period is presented in Table A7-5. The minimum, median, average, and maximum values were provided for the average daily storage and outflow records for the wet and dry season over the WY period of record. Storage and outflow statistics were determined independently of each other, meaning the minimum outflow did not necessarily coincide with the minimum storage. The WSE that corresponds to the storage value was also provided for reference in the PG&E datum. The observed current operational conditions WY 2019-2024 shows a lower overall average in the storage and outflow when compared to observed historical operational conditions WY 1922-2019. This is attributed to the change in spillway operations resulting from the lowering of the maximum operational WSE for Lake Pillsbury.

**Table A7-5: Existing Operational Conditions – Statistics for Observed Storage within Lake Pillsbury and Observed Outflow Downstream of Scott Dam by Season (WY 2019-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 16,800 | 1870.9 | 67 | 21,100 | 1875.7 | 42 |
| Median | 53,200 | 1898.4 | 207 | 44,100 | 1893.1 | 115 |
| Average | 45,439 | 1893.8 | 768 | 43,789 | 1892.9 | 201 |
| Maximum | 67,300 | 1905.6 | 13,200 | 62,100 | 1903.0 | 2,150 |

Since the observed data record of the current operational conditions for Scott Dam and Lake Pillsbury is over a limited timeframe, the hydrologic analysis of the scenario was also conducted using the water balance model developed by SLR discussed in Section A4.5. The water balance inflow rates to Lake Pillsbury developed from PG&E data (WY 2005-2024) were modeled for current operational conditions for Scott Dam and Lake Pillsbury. The resulting water balance model storage values in Lake Pillsbury are presented in Figure A7-4. The observed historical storage values were included in the figure for comparison.

**Figure A7-4: Lake Pillsbury storage comparison of water balance model results and PG&E historical data**




The water balance inflow rates to Lake Pillsbury developed from the USGS data (WY 1957-2024) were modeled for current operational conditions for Scott Dam and Lake Pillsbury. The resulting water balance model storage and WES values in Lake Pillsbury are presented in Figure A7-5 and Figure A7-6. The resulting water balance model outflow values from Lake Pillsbury to Eel River below Scott Dam are presented in Figure A7-7. The results generally demonstrated a repeating water year cycle. During the dry season, storage within Lake Pillsbury and outflow from Scott Dam are decreasing. The Lake Pillsbury WSE is generally dropping below the dam spillway invert and thus outflow is through the needle valve. During the wet season, storage within Lake Pillsbury and outflow from Scott Dam are increasing. The Lake Pillsbury WSE rises to the dam spillway invert and outflow is through both the needle valve and spillway. Periodic increases in inflow during the wet season momentarily increase storage within Lake Pillsbury as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,900 ft. The long-term overall trend of water storage in Lake Pillsbury is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period is presented in Table A7-6.

**Figure A7-5: Lake Pillsbury storage water balance model results for current conditions**

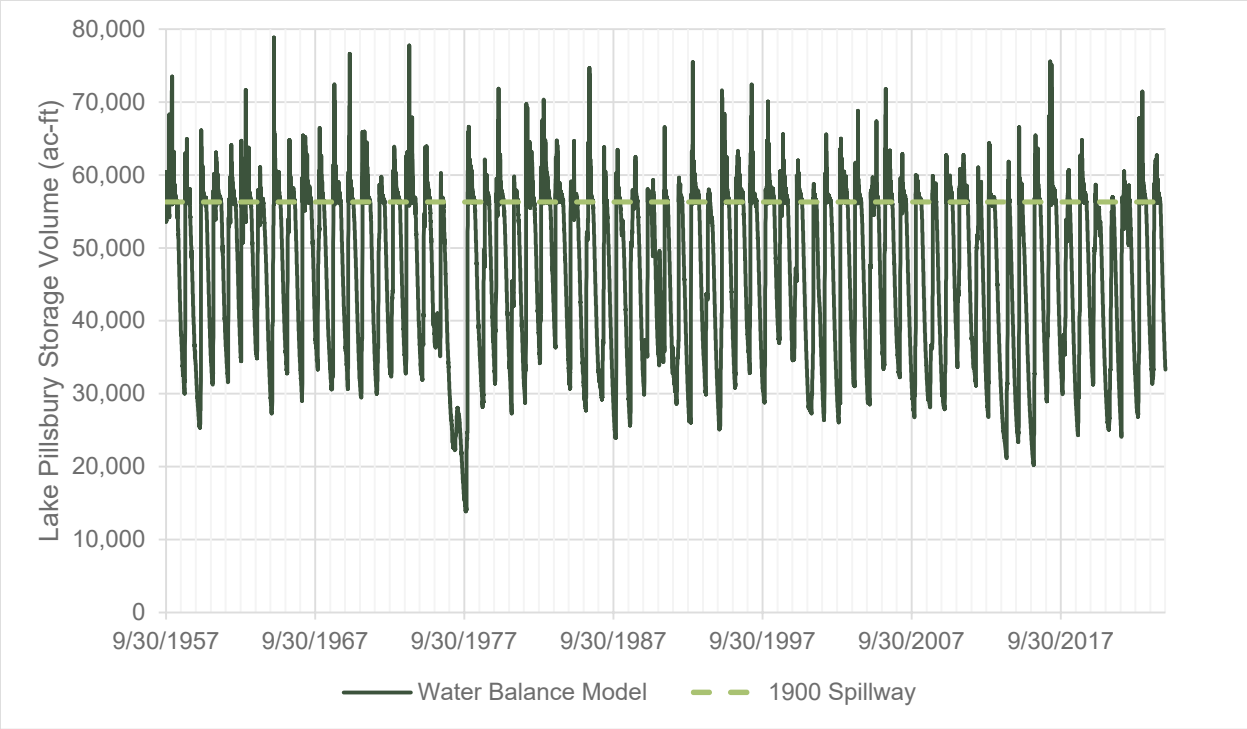



**Figure A7-6: Lake Pillsbury WSE water balance model results for current conditions**

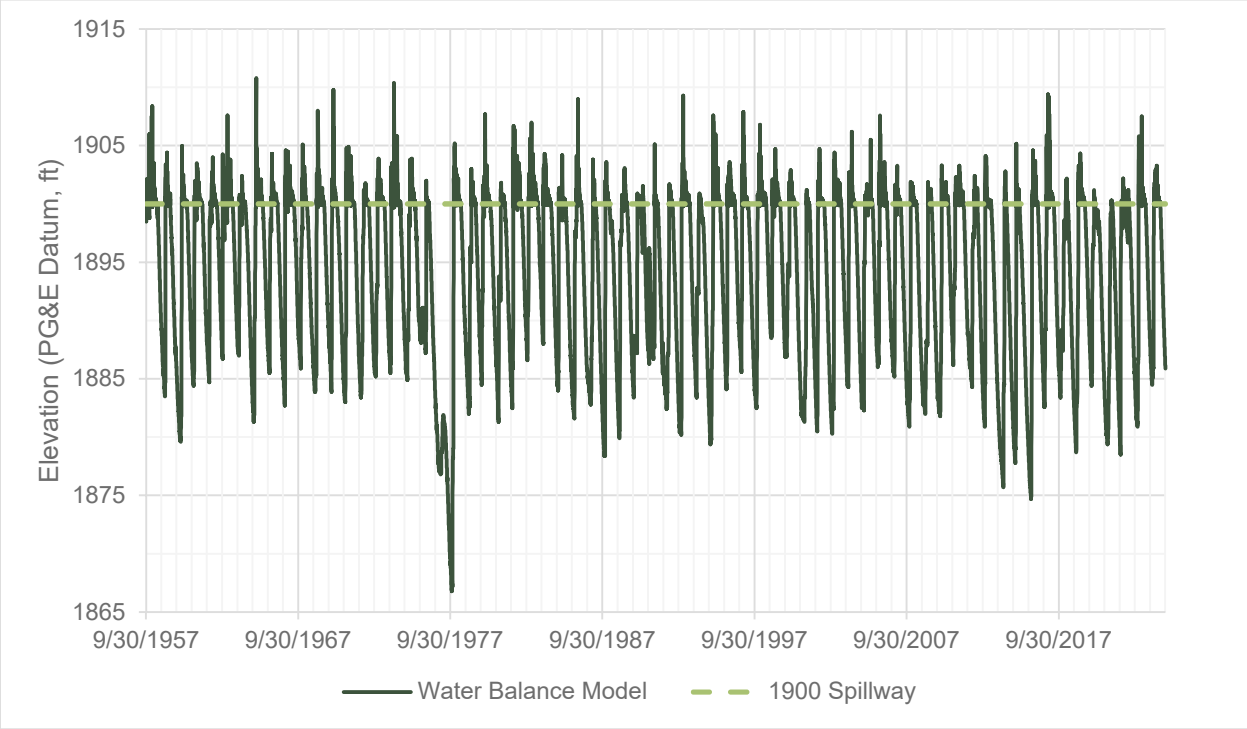


**Figure A7-7: Lake Pillsbury outflow water balance model results for current conditions**

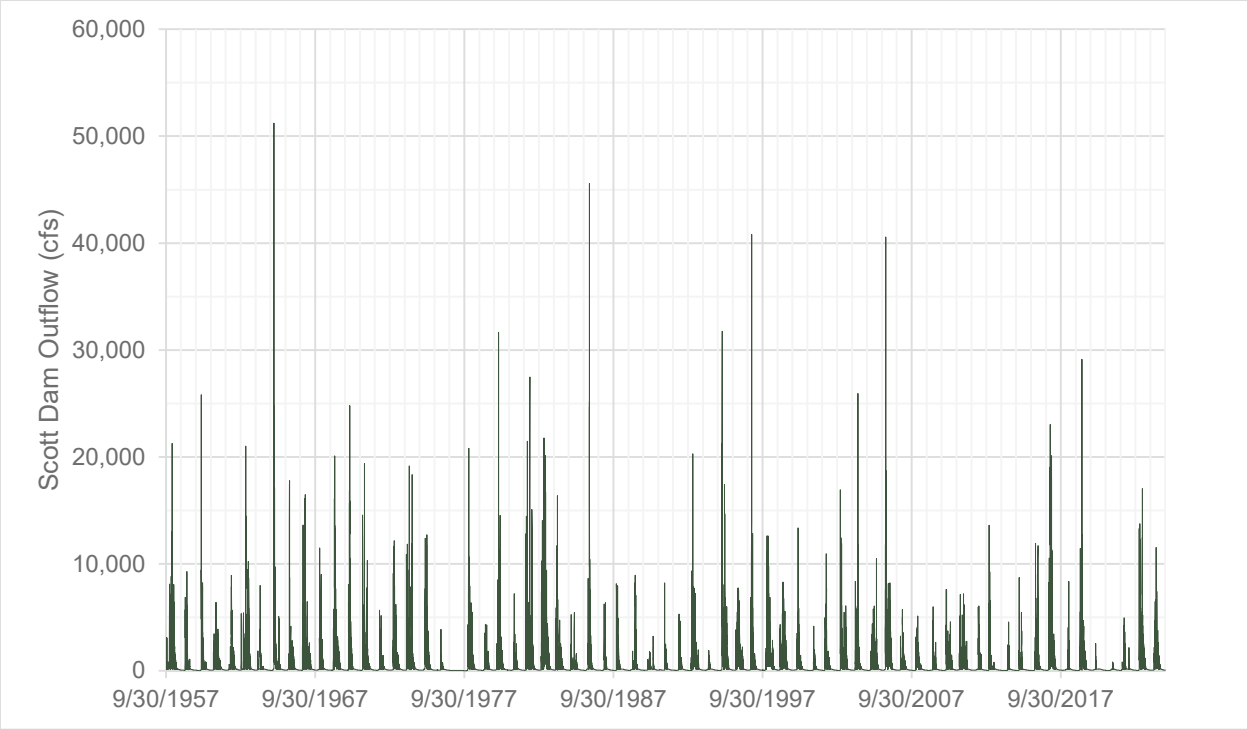



**Table A7-6: Existing Operational Conditions – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam by Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 13,870 | 1866.8 | 40 | 13,910 | 1866.9 | 40 |
| Average | 48,500 | 1895.0 | 1,052 | 45,070 | 1893.2 | 258 |
| Maximum | 78,910 | 1910.8 | 51,230 | 67,380 | 1905.5 | 15,110 |

### A7.2.1.2 Flood and Drought Conditions

This scenario's gage records do not coincide with any notable periods of flooding or drought. The observed minimum and maximum flow and storage can be used to approximate but is limited to the period of record. The minimum and maximum flow and storage values calculated per the water balance can also be used as a reference for projected flood and drought conditions. The gages and water balance flood and drought projections for storage within Lake Pillsbury and outflows from Scott Dam are summarized in Table A7-7 and Table A7-8, respectively. The gage records are lower for flooding and higher for drought conditions. In comparison to the baseline historic operations gaged condition (Table A7-3) the flood projections for storage are lower but the outflow is higher. Whereas the gaged projections under the historic conditions for drought are drastically lower than the current operations even when compared to the historic records per Table A7-4. The drought comparison is misleading when compared to baseline conditions as the circumstances for the gaged values are uncertain and

may have been forced due to operational maintenance. Analytically, reducing the storage capacity will reduce the drought tolerance and it is likely that under drought conditions the storage and outflow will be lower than the historic conditions observed values.

**Table A7-7: Existing Operational Conditions – Observed and Calculated Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Gage | 67,300 | 1905.6 | 13,200 |
| Water Balance | 78,910 | 1910.8 | 51,230 |

**Table A7-8: Existing Operational Conditions – Observed and Calculated Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Gage | 16,800 | 1870.9 | 42 |
| Water Balance | 13,870 | 1866.8 | 40 |

### A7.2.2 Projected Scenario Outcomes

The current operational conditions scenario analyzes the continual operations of Scott Dam at the current maximum operational WSE of 1,900 ft and the projected potential impacts on water supply.

#### A7.2.2.1 Water Supply

Flood control, downstream water diversion flow requirements, and operational reliability are not drastically difference from the baseline historical operational conditions scenario due to similar outflows from Scott Dam to the Eel River. The average WSE and storage in Lake Pillsbury for the current operational storage conditions are lower than the historical operational conditions due to the reductions of the maximum operational WSE and the passive operation of the spillway.

This scenario would generally increase flood risk as active operations of the spillway gates have ceased, which had previously allowed for greater control, attenuation, and retention of flood inflows. This scenario has slightly reduced drought tolerance due to a lower average WSE and storage in Lake Pillsbury than the historical operational conditions. Lake Pillsbury is expected to continue operating with an average storage volume of over 45,000 ac-ft (over 14 billion gallons) of water availability. However, the water supply capacity will continue to be reduced over time due to losses from continued sediment accumulation.

This scenario is not anticipated to drastically affect the potential groundwater recharge or downstream water diversion flows requirements, but could be affected under drought conditions which are difficult to adequately predict or quantify with the data available.

## A7.3 Scenario A1 – PG&E Proposed Removal

Under this scenario, PG&E proposes the full decommissioning of Scott Dam. The proposed approach is divided into three phases:

a) initial dewatering and removal of the upper portion of the dam,

b) initial sediment flushing, and

c) final dam removal.

As proposed by PG&E, the sediments that have accumulated within the impoundment will be managed passively. PG&E's current proposed action under the Final Draft Surrender Application (PG&E, 2025) includes the decommissioning and removal of Scott Dam, draining the Lake Pillsbury and removing associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. PG&E proposed a Rapid Dam Removal approach estimated to take 2 years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards) downstream of the Eel River (PG&E, 2025). PG&E anticipates temporary alteration of the Eel River channel and/or floodplain morphology from sediment deposition after deconstruction of the dam until subsequent high-flow events can resuspend the sediment and transport it farther downstream, until the river channel adapts to its natural condition. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. Other activities proposed by PG&E as part of decommissioning are documented in the Draft Application for Surrender of License (PG&E, 2025).

PG&E may propose additional responses at locations that are identified where natural fluvial processes are not mobilizing sediment to the volumes anticipated. These locations may have continued issues with bank instability and channel incision that may adversely affect habitat and fish passage.

### A7.3.1 Hydrologic and Hydraulic Analysis

The 2019 memorandum by Cardno projected unimpaired flows for the entire Eel River utilizing the mass balance method described in Sections A4.1 and A6.2 (Cardno, 2019). Cardno's data, analysis, and models were not provided for the unimpaired flow projections just downstream of Scott Dam. However, the Scott Dam and Cape Horn Dam Removal Alternatives references and provide a summary graph of the unimpaired monthly average discharge (McMillen, 2021). The graph was presented as part of the Results of Initial Water Supply Modeling for PVP and Russian River Alternatives, which are noted to be estimates from the Cardno unimpaired hydrology calculations (Addley et al, 2019). The unimpaired flow graph was reproduced and is presented in Figure A3-4. These unimpaired flows are used as the basis for the proposed removal climate conditions assessment for consistency and to compare previous report findings.

#### A7.3.1.1 Wet and Dry Season Conditions

The monthly average unimpaired flows from Figure A3-4 were analyzed by season, and statistics were calculated to support the projected outcomes for each season. The wet and dry season statistics are summarized in Table A7-9. The average monthly wet season flows range from 791 cfs in April to 1,535 cfs in February, and the average monthly dry season flows range from 17 cfs in August to 376 cfs in May.

The wet and dry season conditions under the dam removal scenario were roughly estimated for each major watercourse by associating the flow contribution summarized in Section A4.4.

The wet and dry season average monthly flows by watercourse are shown in Figure A7-8 and Figure A7-9, respectively.

**Table A7-9: Dam Removal Scenario A1 Wet and Dry Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 791 | 17 |
| Median | 1,237 | 59 |
| Average | 1,216 | 133 |
| Maximum | 1,535 | 376 |

### A7.3.1.2 Flood and Drought Conditions

The recommended 1% AEP flow just downstream of Scott Dam for this scenario, based on the current analyses, would be the 2025 USGS StreamStats flow of 60,100 cfs until a more in-depth analysis can be completed. A StreamStats assessment was also conducted to project the 1% AEP flows for each major watercourse contributing to flow just downstream of Scott Dam and are summarized in Table A7-10.

**Table A7-10: Projected StreamStats 1% Annual Exceedance Probability Flows for Watercourses Contributing to Flow Just Downstream of Scott Dam**

| Watercourse | Type | 1% AEP Flood [100-year Recurrence] (cfs) |
|---|---|---|
| Upper Eel River1 | Perennial | 34,600 |
| Rice Fork | Perennial | 22,900 |
| Salmon Creek | Intermittent | 5,170 |
| Smokehouse Creek | Intermittent | 4,600 |
| Habematolel Creek | Perennial | 3,110 |
| Eel River Downstream of Scott Dam | Perennial | 60,100 |

Notes:
1. The Upper Eel River flow represents the Eel River contribution just upstream of the Habematolel Creek confluence.

The drought conditions for this scenario are difficult to project without more detailed unimpaired flow data. The Eel River, Habematolel Creek, and Rice Fork are classified by the USGS as perennial streams, while the Smokehouse and Salmon Creeks are classified as intermittent. Smokehouse and Salmon Creek are likely not to have flows during a drought condition. The Eel River, Habematolel Creek, and Rice Fork may be more drought tolerant under specific conditions, however, the Eel River is likely to have the most reliable source of flow.

**Figure A7-8: Unimpaired Wet Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse**

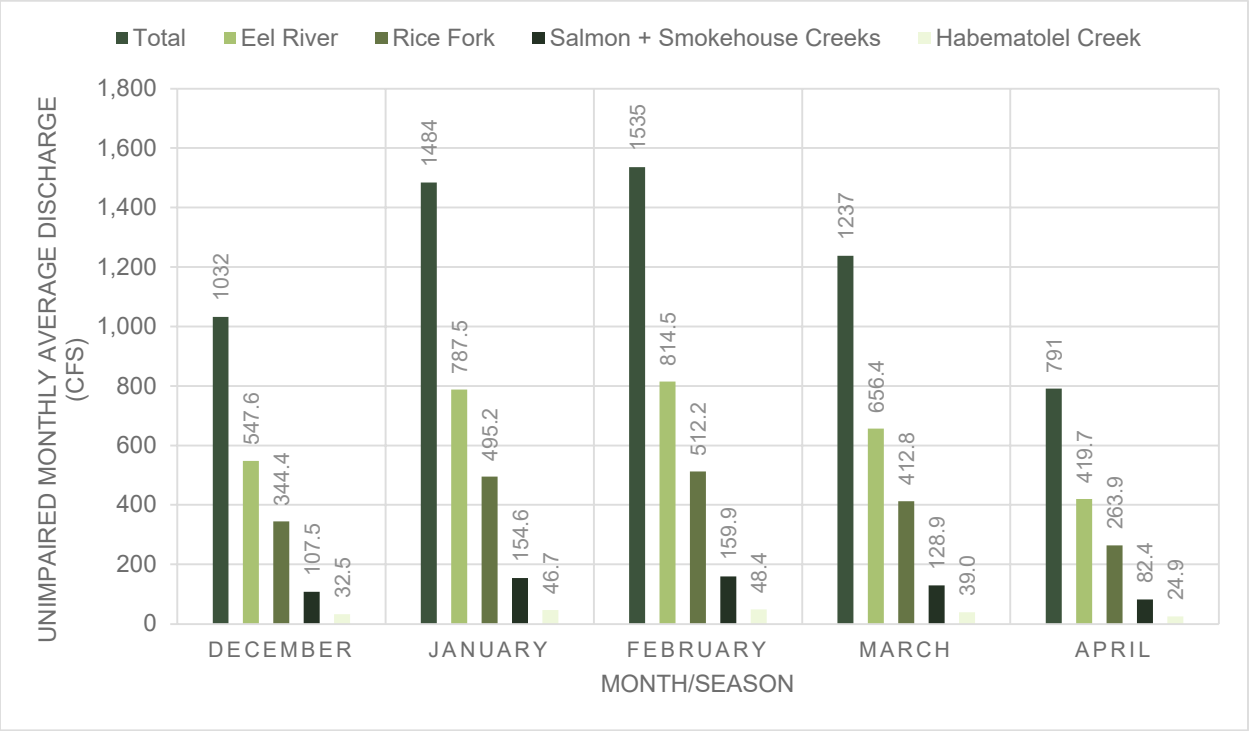
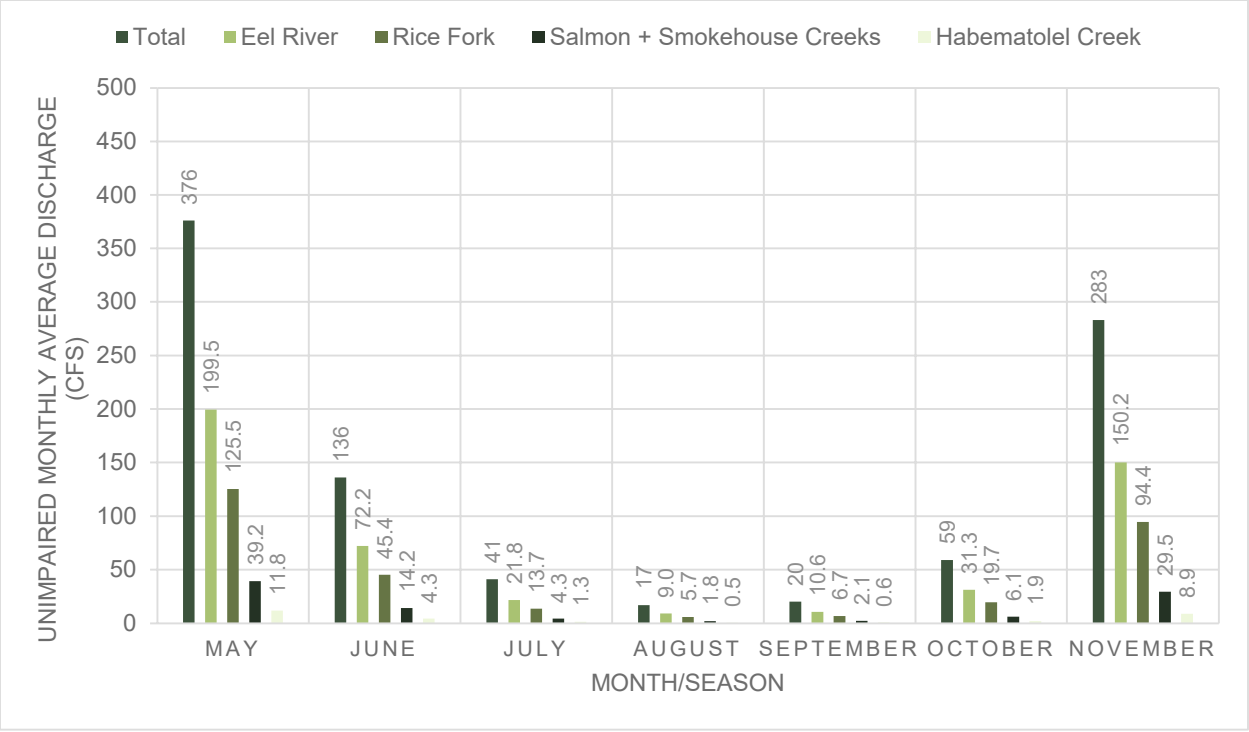



**Figure A7-9: Unimpaired Dry Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse**



### A7.3.2 Projected Scenario Outcomes

Scenario A1 analyzes PG&E's Scott Dam removal and the projected impacts on water supply. A plan view depiction of the Scott Dam and Lake Pillsbury area with projected response to Scenario A1 conditions is presented in Figure A7-10 and Figure A7-11. The Eel River and tributaries are assumed to return to their pre-dam condition as represented in the 1922 topographic and bathymetric mapping compiled by PG&E (2016).

#### A7.3.2.1 Water Supply

This scenario would eliminate all water storage, and any flood control. Water supply to the Russian River watershed would be limited to winter months and during wetter dry seasons where water is not diverted by irrigation. Under baseline existing conditions, the average monthly dry season flows are approximated between 147-222 cfs, and the wet season average flows range from 296-375 cfs. In comparison, the wet and dry season average flows would be much higher at 1,237 cfs (wet) and a bit lower at 133 cfs (dry).

This scenario would increase flood risk and likely increase the frequency, intensity, and duration of flood events seen under existing conditions of operations. This is attributed to the storage and dam controls attenuating the flows and retaining much of the flood waters. This scenario has very low drought tolerance when compared to the baseline scenario, where average monthly flows are projected to fluctuate between 17 and 376 cfs during the dry season. Flood and drought risk are difficult to quantify due to the limited unimpaired data analysis assumptions and would require more analysis to provide higher confidence in the projected response.

The passive sediment removal will cause flow to fluctuate and become unreliable to predict until the watercourses and sediments are semi-stabilized. Semi or quasi stabilization should be assumed when there are no major changes to the channel geometry and floodplain limits, as well as when vegetation has been re-established and unaffected during normal wet and dry season flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed, and recharging would be limited to the watercourse contributions. This could result in a reduction of the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

**Figure A7-10: Scenarios A1 and A2 PG&E proposed Scott Dam removal: Eel River and tributary watercourses originally feeding Lake Pillsbury**

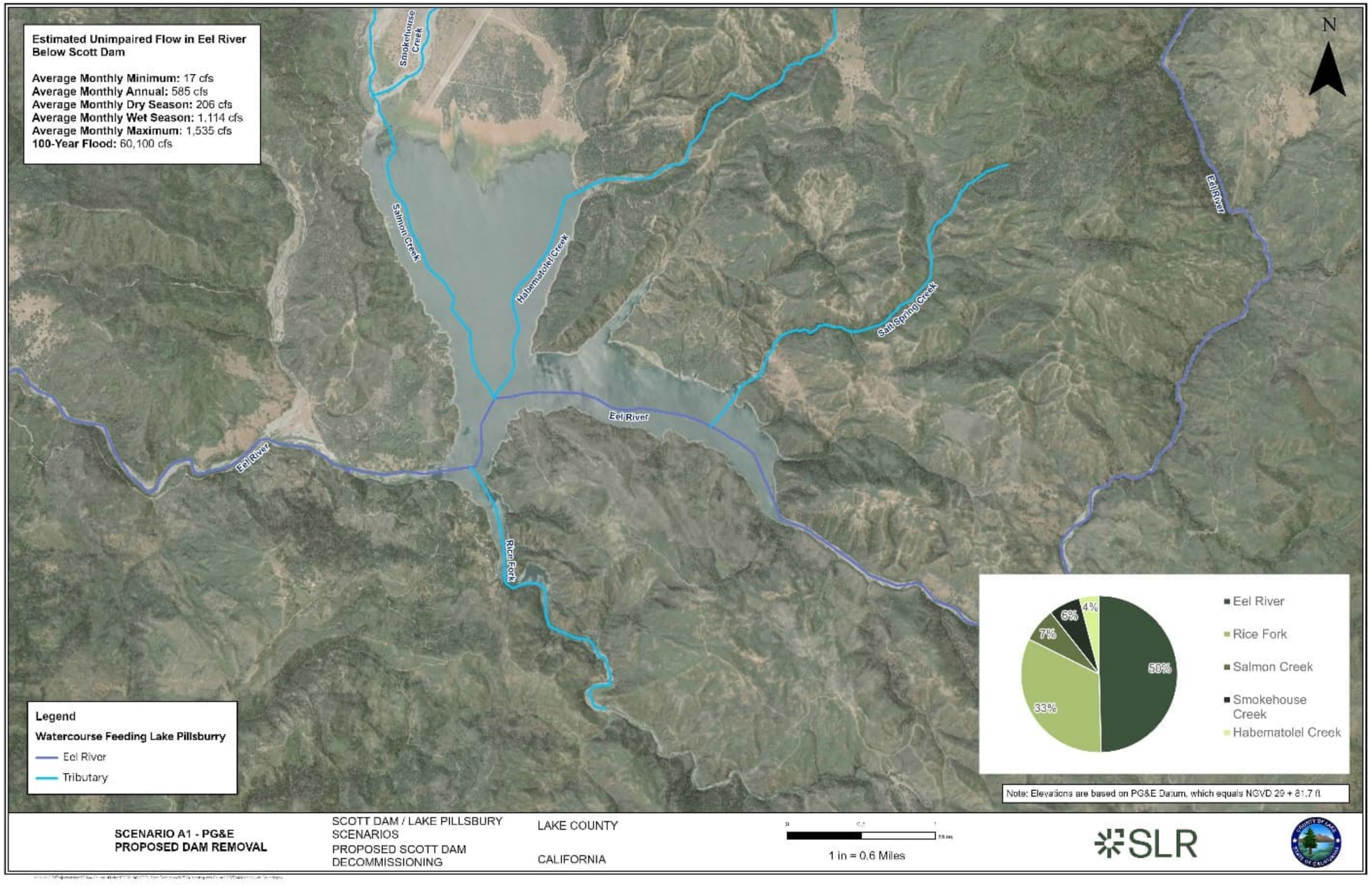
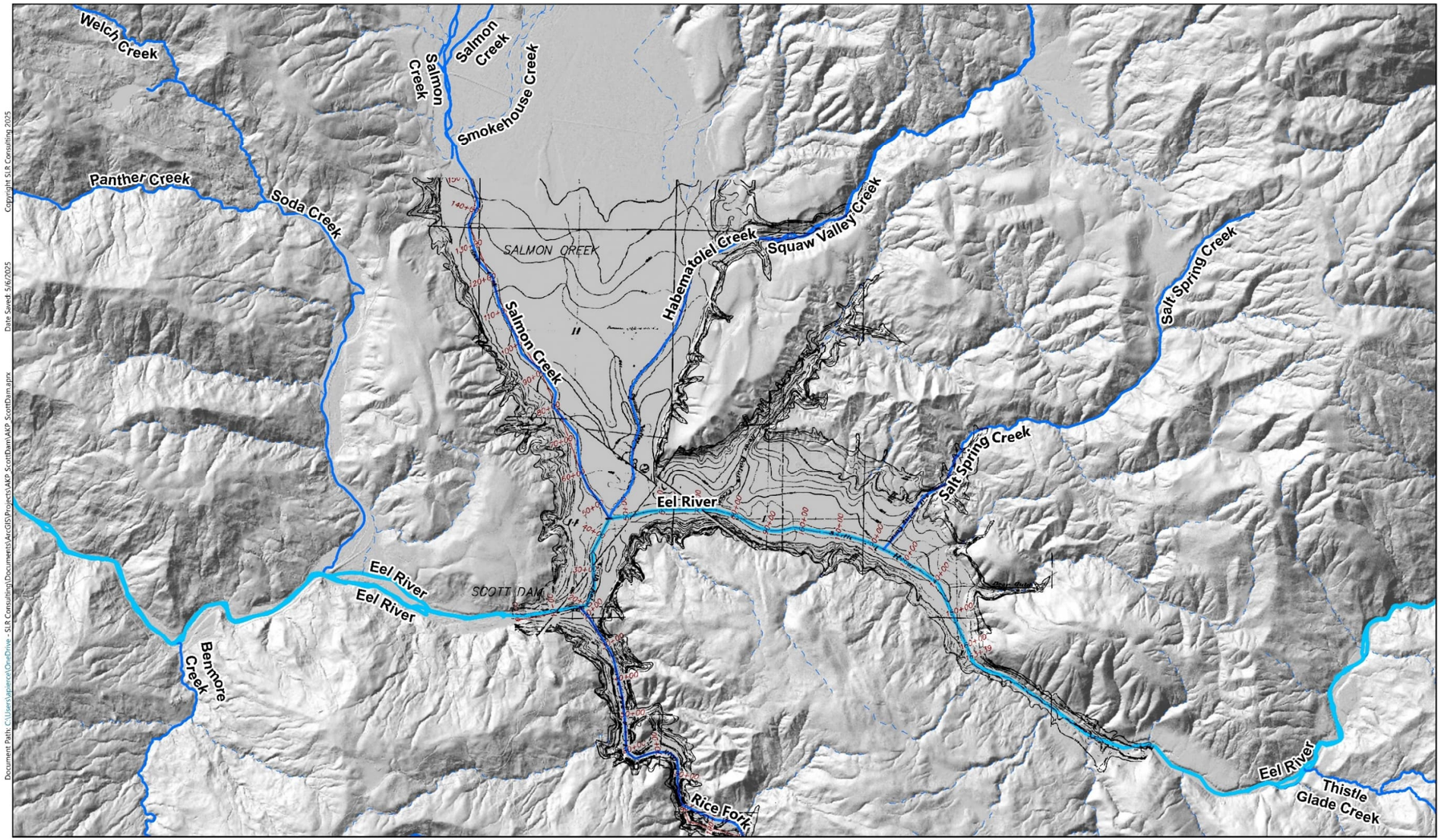


**Figure A7-11: Scenarios A1 and A2 Projected Response Post Dam Removal**




SCENARIO A1/A2 - SCOTT DAM DECOMMISSIONING

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

- Rivers
- Creeks
- Seasonal Creeks

Hillshade
High : 254
Low : 0

1922 Pillsbury Bathymetry
Red: Band_1
Green: Band_2
Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

SLR
22118 20TH AVE SE
STE. G202
BOTHELL, WA 98021
425.402.8800

## A7.4 Scenario A2 – Scenario A1 with Additional Mitigation

Under this scenario, PG&E proposes the full decommissioning of Scott Dam and the County of Lake proposes additional mitigation efforts implemented to address their concerns and those of the local community. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the County of Lake, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and would have higher ecosystem benefits to the community.

### A7.4.1 Hydrologic and Hydraulic Analysis

The changes in hydrology are expected to be the same as in Scenario A1, see Section A7.3.1. Refer to the unimpaired flows shown in Figure A3-4, Figure A7-8, and Figure A7-9 and the statistics summarized in Table A7-9 and Table A7-10. The only change would be the rate at which the channel and sediment would reach quasi-equilibrium, and flows would normalize. The Scenario A1 Eel River system prediction for reaching a semi-stable state is dependent upon flows and could take years to decades, whereas active restoration and sediment management could expedite this process to within 5 to 10 years post-dam removal.

### A7.4.2 Projected Scenario Outcomes

Scenario A2 projects the same outcomes as Scenario A1 for water supply with the main difference being the rate at which natural or pre-dam conditions flows will be attained. Figure A7-10 and Figure A7-11 will similarly apply when predicting the changes to the Eel River system upstream of Scott Dam upon scenario completion.

## A7.5 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

This scenario is similar to the current operational conditions described in Section A7.2, with the addition of a fish pathway, as previously evaluated by Mead and Hunt (Sullivan, 2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, a seismic retrofit would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures. The hydrologic and hydraulic evaluation is likely similar to the current condition's scenario, but specific hydrology analysis will need to be performed to accommodate a fish pathway to the existing Scott Dam.

## A7.6 Scenario C – Lowered Scott Dam with Fish Ladder

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 feet (PG&E datum) and the installation of a fish passage system,

such as a fish ladder structure. The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 feet would create a reservoir of approximately 40 percent of the current storage volume at WSE 1,900 feet. Potentially, the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury WSEs and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage as the current maximum operational WSE of 1,900 feet, the spillway invert may be further reduced in height by 25 feet below the existing spillway invert, which would result in a new lake footprint shown in Figure A7-16. At that reduced maximum operational WSE, the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam abutment area (where previous slides have occurred) by placing the concrete material on both or either of the downstream and upstream sides of the lowered dam. Further evaluation would be required to understand the effects on the adjacent land area and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation, and ecosystem benefits as compared to the full decommissioning.

### A7.6.1 Hydrologic and Hydraulic Analysis

Hydrologic analysis for the lowered Scott Dam with a fish ladder was conducted using the water balance model developed by SLR. This scenario assumed that the proposed lowered spillway would be constructed such that similar outflow rating curve performance could be able to be achieved compared to the current existing spillway. The lowered Scott Dam spillway would not be actively operated using gates, and instead outflow would be passive when the WSE is above the invert elevation. The assumed maximum operational WSE would be lowered to 1,875 ft, resulting in a storage capacity of 20,453 ac-ft. It is assumed that the lowered Scott Dam would possess an updated low-level outlet structure that would be able to operate at low WSEs. The updated low-level outlet structure is assumed to have similar outflow rating curve performance compared to the existing needle valve structure. The minimum low-level outlet outflow is assumed to be 20 cfs to maintain minimum ecological flow requirements. The new fish ladder structure is assumed to operate during two seasons in the water year. The first season is during November-December, and the second season is during April-May. The fish ladder is assumed to operate continuously during the season at a rate of 55 cfs.

The inflow rates to Lake Pillsbury developed from the USGS data mass balance (WY 1957-2024) were modeled for the operational conditions of this scenario for Scott Dam and Lake Pillsbury to project outcomes for outflow and storage. The resulting water balance model storage and WSE values in Lake Pillsbury are presented in Figure A7-12 and Figure A7-13. The resulting water balance model outflow values from Lake Pillsbury to the Eel River below Scott Dam are presented in Figure A7-14. A comparison of the modeled water balance storage values in Lake Pillsbury for the current operational conditions at spillway elevation 1,900 ft and the lowered Scott Dam with fish passage conditions at the lowered spillway elevation of 1,875 ft are presented in Figure A7-15.

**Figure A7-12: Lake Pillsbury storage water balance model results for lowered Scott Dam with fish ladder**

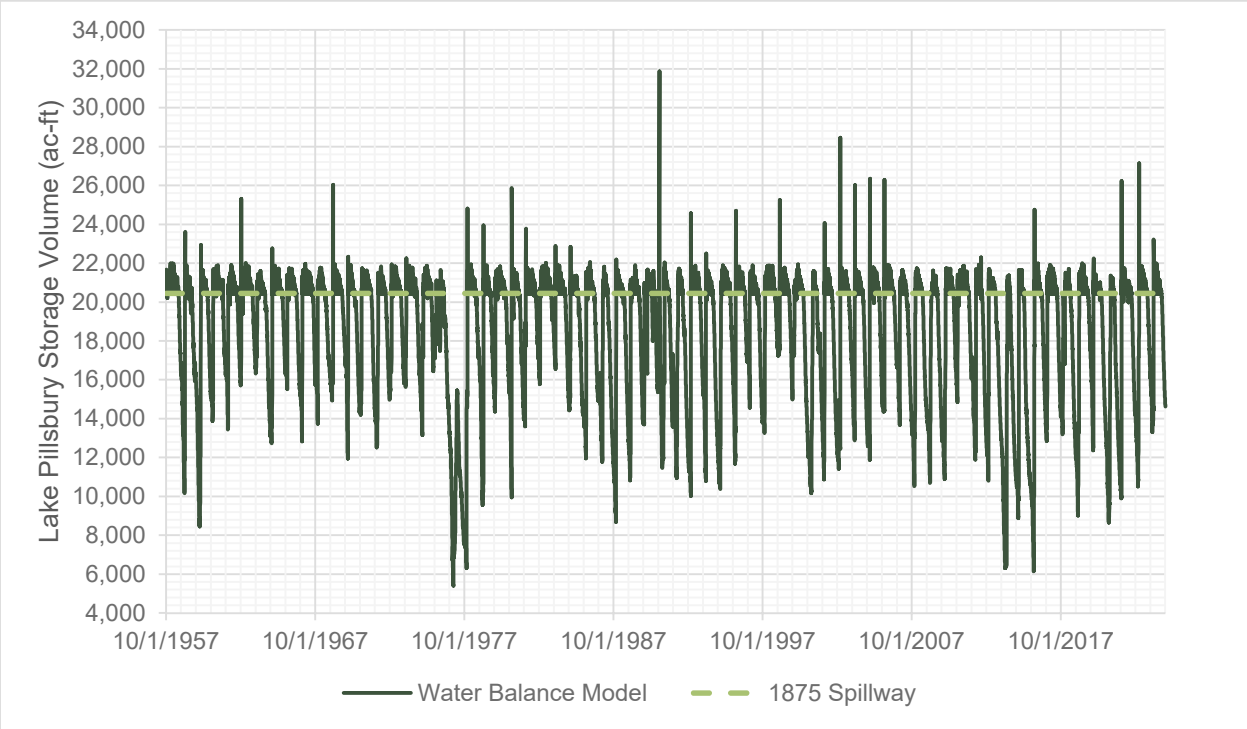



**Figure A7-13: Lake Pillsbury WSE water balance model results for lowered Scott Dam with fish ladder**

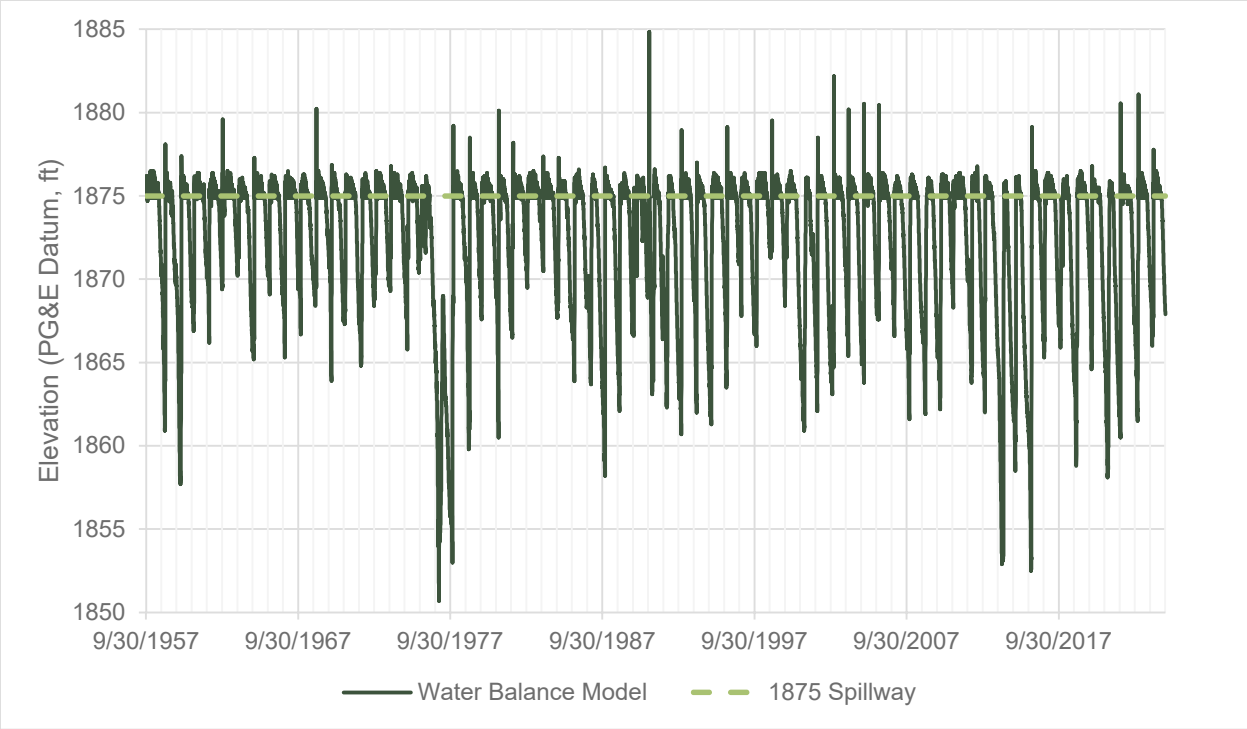


**Figure A7-14: Lake Pillsbury outflow water balance model results for lowered Scott Dam with fish ladder**

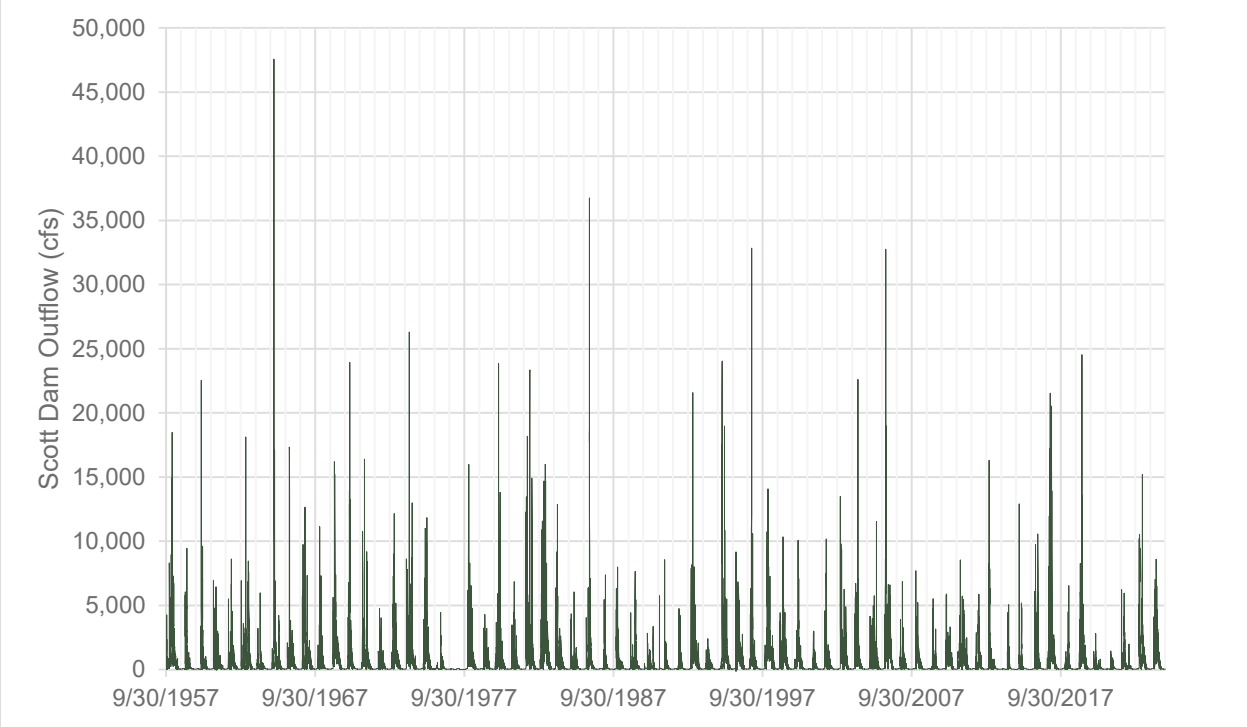
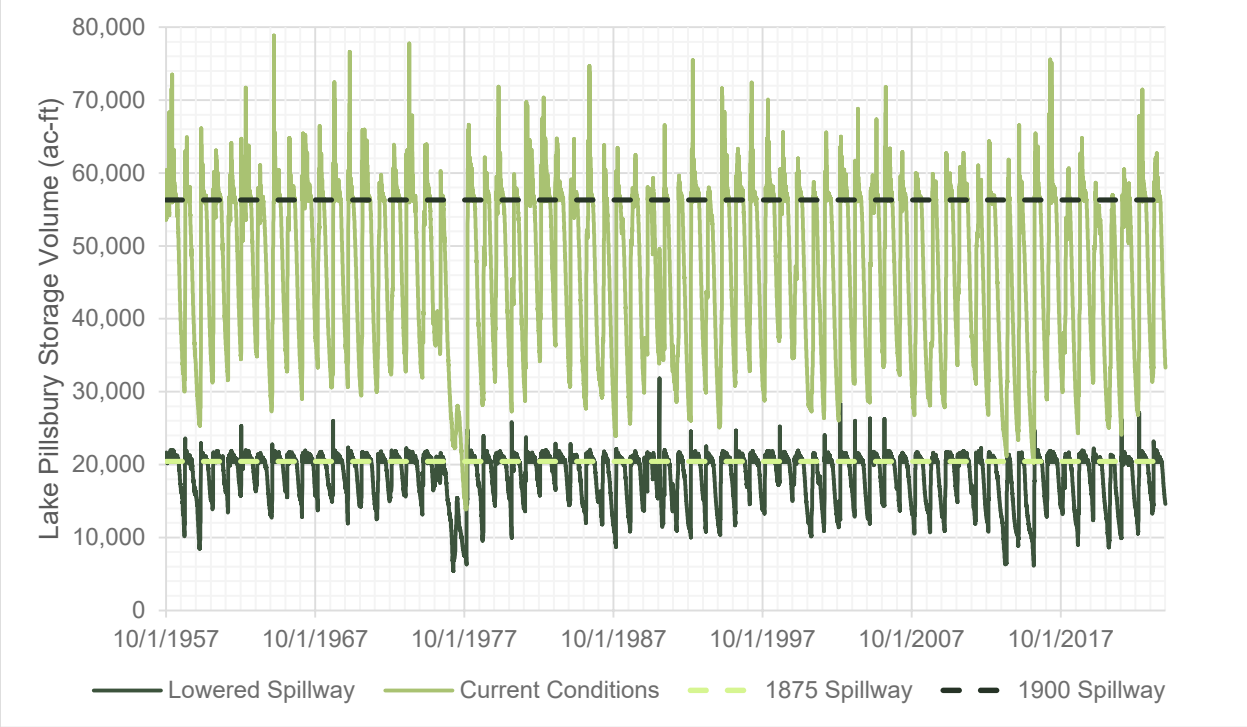



**Figure A7-15: Lake Pillsbury storage comparison of water balance model results for lowered Scott Dam with fish ladder and current conditions**



### A7.6.1.1 Wet and Dry Season Conditions

The mass balance results generally demonstrate a repeating water year cycle. During the dry season, storage within Lake Pillsbury and outflow from Scott Dam are decreasing. The Lake Pillsbury WSE is generally dropping below the lowered dam spillway invert, and thus outflow is through the low-level outlet. During the wet season, storage within Lake Pillsbury and outflow from Scott Dam are increasing. The Lake Pillsbury WSE rises to the lowered dam spillway invert and outflow is through both the low-level outlet and spillway. Periodic larger inflow during the wet season momentarily increase storage within Lake Pillsbury as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,875 ft. The long-term overall trend of water storage in Lake Pillsbury is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period is presented in Table A7-11 and visually shown in Figure A7-17.

**Table A7-11: Lowered Scott Dam with Fish Ladder – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam by Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 5,420 | 1850.7 | 20 | 7,340 | 1855.3 | 20 |
| Average | 18,760 | 1872.7 | 1,116 | 18,080 | 1872.1 | 220 |
| Maximum | 28,460 | 1882.2 | 47,470 | 31,840 | 1884.8 | 14,923 |

### A7.6.1.2 Flood and Drought Conditions

Projecting scenarios C's response to flood and drought conditions can only be estimated based on the water balance model using the gaged data and stage storage curve assumptions. The computed minimum and maximum flow and storage can be used to approximate the response, but further hydrologic and hydraulic analysis would be recommended to validate the assumptions. The water balance summaries for the flood and drought response are summarized in Table A7-12 and Table A7-13 based on the maximum (flood) and minimum (drought) calculated storage and outflows. The lowest storage is predicted to occur during the wet season and the lowest flows are purportedly the same during the wet and dry season. Though outflows are anticipated to be lower than 20 cfs during drought conditions.

**Table A7-12: Lowered Scott Dam with Fish Ladder – Calculated Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Water Balance | 31,840 | 1884.8 | 47,470 |

**Table A7-13: Lowered Scott Dam with Fish Ladder – Calculated Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Water Balance | 5,420 | 1850.7 | 20 |

## A7.6.2 Projected Scenario Outcomes

The lowered Scott Dam with a fish ladder scenario analyzes modified operations of Scott Dam at a maximum operational WSE of 1,875 ft and the projected potential impacts on water supply.

### A7.6.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury by 35,870 ac-ft by lowering the maximum operation WSE from 1,900 ft to 1,875 ft. Flood control, downstream water diversion flow requirements, and operational reliability would be impacted. Flood control and downstream water diversion flow requirements would be affected due to reduced dam operational control and the passive operation of the spillway. An updated low-level outlet structure that would be able to operate at low WSEs and maintain minimum ecological flow requirements to the Eel River.

This scenario would generally increase flood risk due to the stopping of active operations of the spillway gates, which allowed for greater control, attenuation, and retention of flood inflows. This scenario has reduced drought tolerance due to a lower average WSE and storage in Lake Pillsbury than the current operational conditions. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation. It is recommended that existing sediment be mitigated to increase the available water supply.

The Lake Pillsbury reservoir's ability to recharge the groundwater would likely be reduced. Additional evaluation of the slope stability of the existing slope would need to be considered. Currently, PG&E has maintained a minimum WSE due to slope safety concerns.

This scenario proposes that seasonal fish passage be provided during two seasons per year. The first season would be during November-December, and the second season would be during April-May to meet fish passage requirements. Lake Pillsbury is expected to be able to provide the required flow rates to properly allow fish passage.

**Figure A7-16: Scenario C lowered Scott Dam with Fish Ladder: Conceptual Lake Pillsbury Footprint**

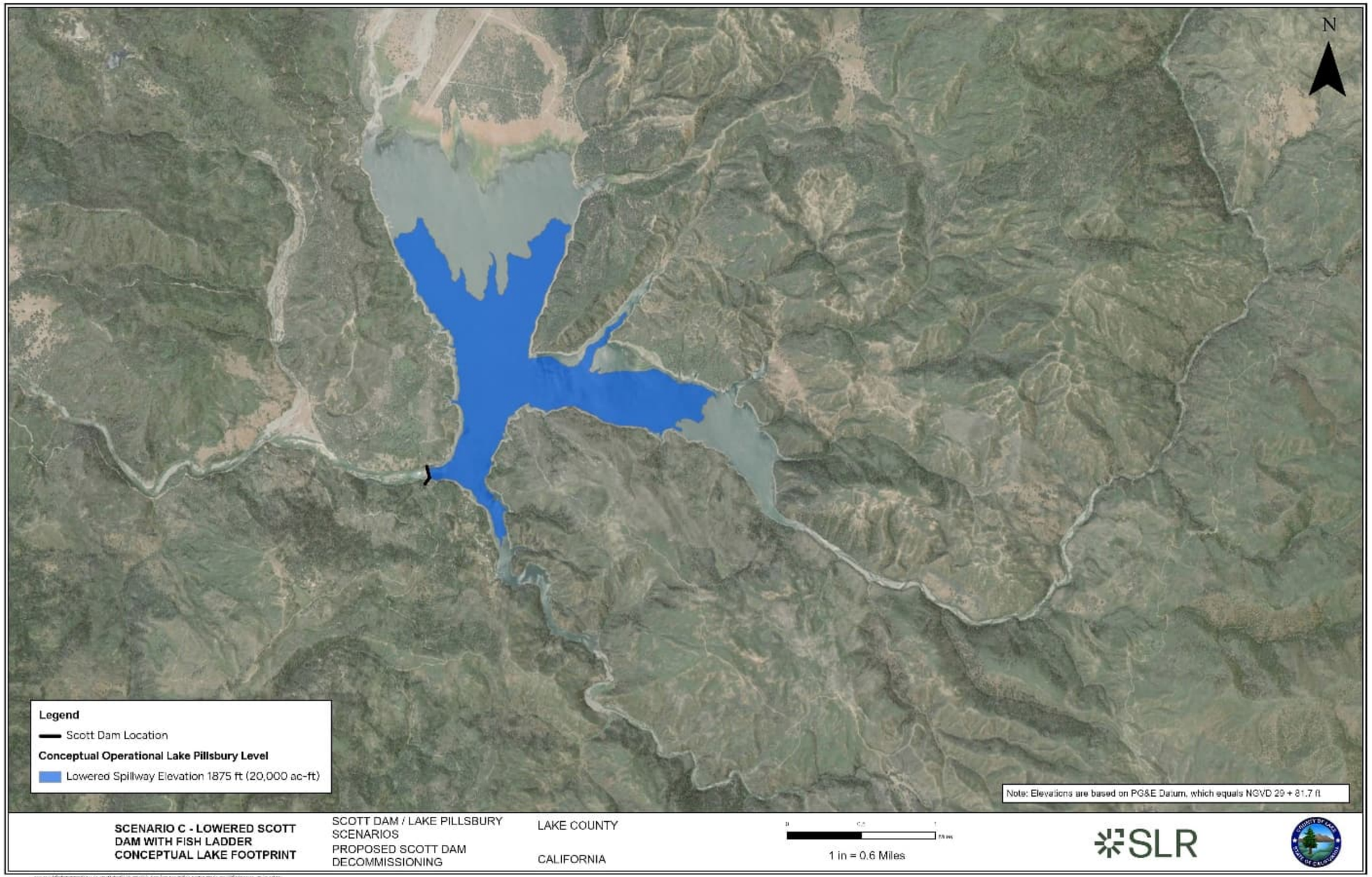
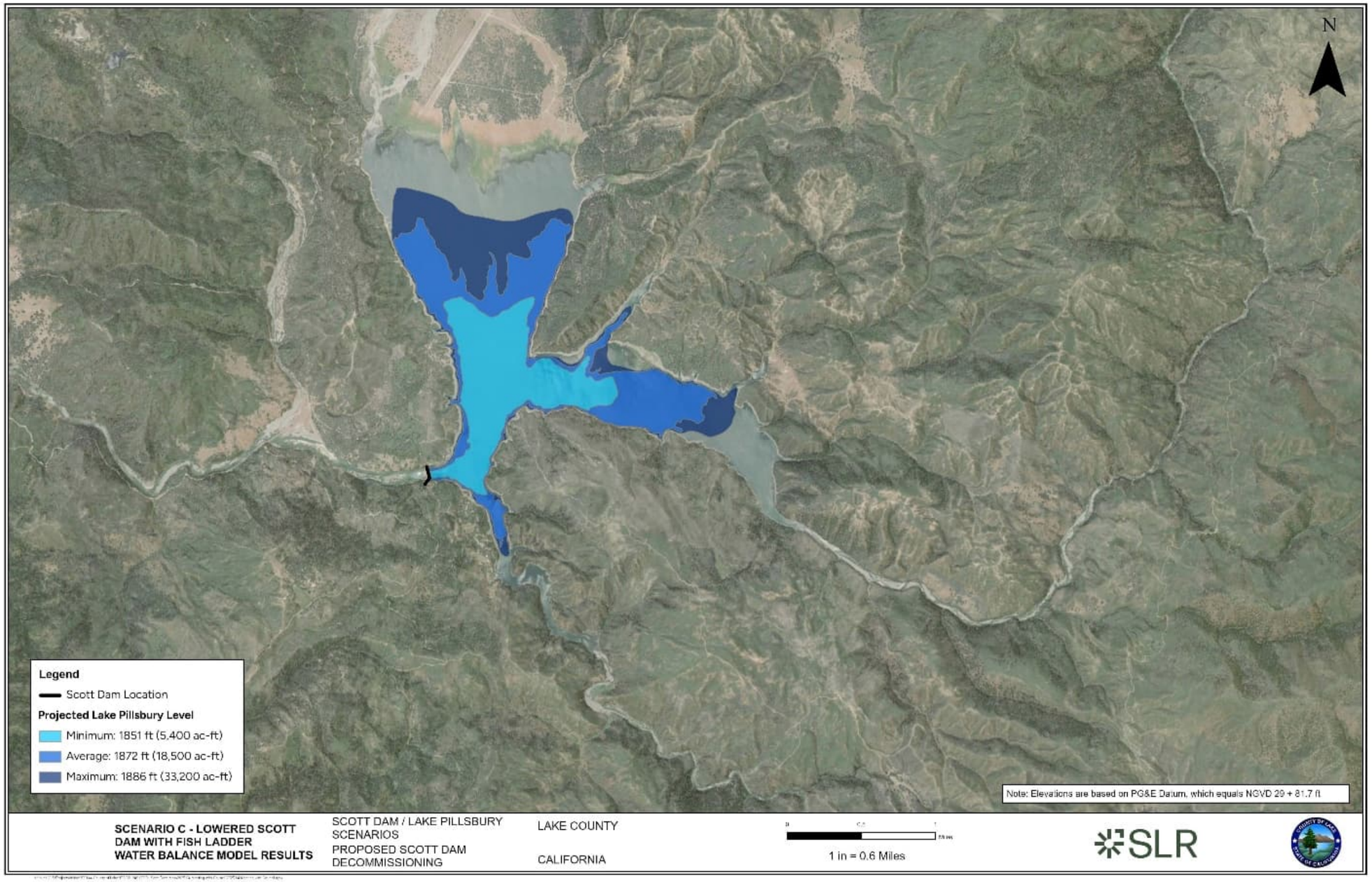


**Figure A7-17: Scenario C lowered Scott Dam with Fish Ladder: Projected Minimum, Average, and Maximum Lake Pillsbury WSE Levels**



## A7.7 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure. A new 3,100-foot long earthen dam would be established in the Salmon/Smokehouse and Habematolel Creek watershed areas to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir with contributions per Figure A7-18. The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction, with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within the Lake Pillsbury footprint can also be reused for the new impoundment construction if suitable. Low-permeability clay/silt sediments can be used in the core of the dam, and gravelly sediments can be used in the embankment/slopes of the new dam.

The water storage will be based on the Salmon/Smokehouse and Habematolel Creek watershed areas, and it is assumed the maximum capacity (approximately 9,990 ac-ft) would be provided at a maximum operational WSE of 1,885 feet as visually depicted in Figure A7-19. For this scenario, the approximate surface area would be 580 acres, approximately 30 percent of the current size (WSE 1,900 ft). The new dam would be constructed at the narrowest location possible that would not impair the flows through the Eel River. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational WSE. An active low-level outlet structure would not be included, though a passive outlet is considered for potential draining. This option will support continued use of recreation facilities, ecosystem benefits, fire suppression, and the upstream fish pathway along the Eel River, as Scott Dam will be removed as a fish barrier.

**Figure A7-18: Breakdown of percent contributions to the reduced reservoir under Scenario D (reduced reservoir with Salmon Creek as main tributary)**

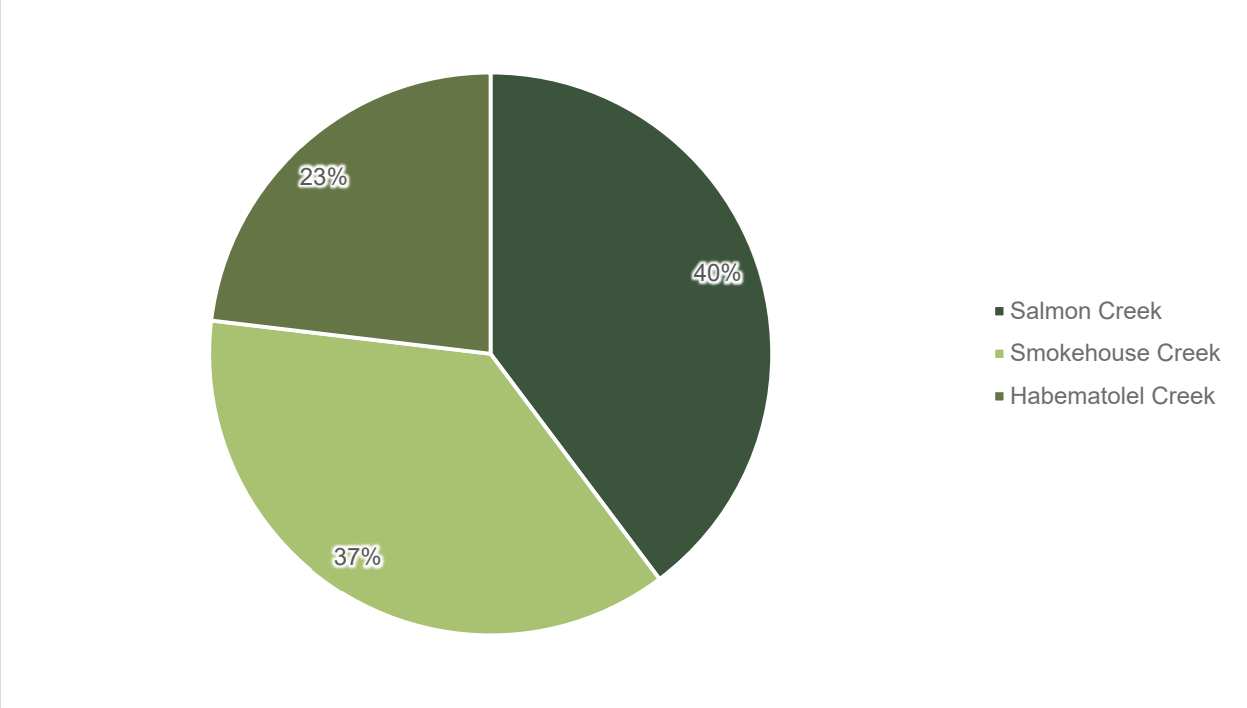
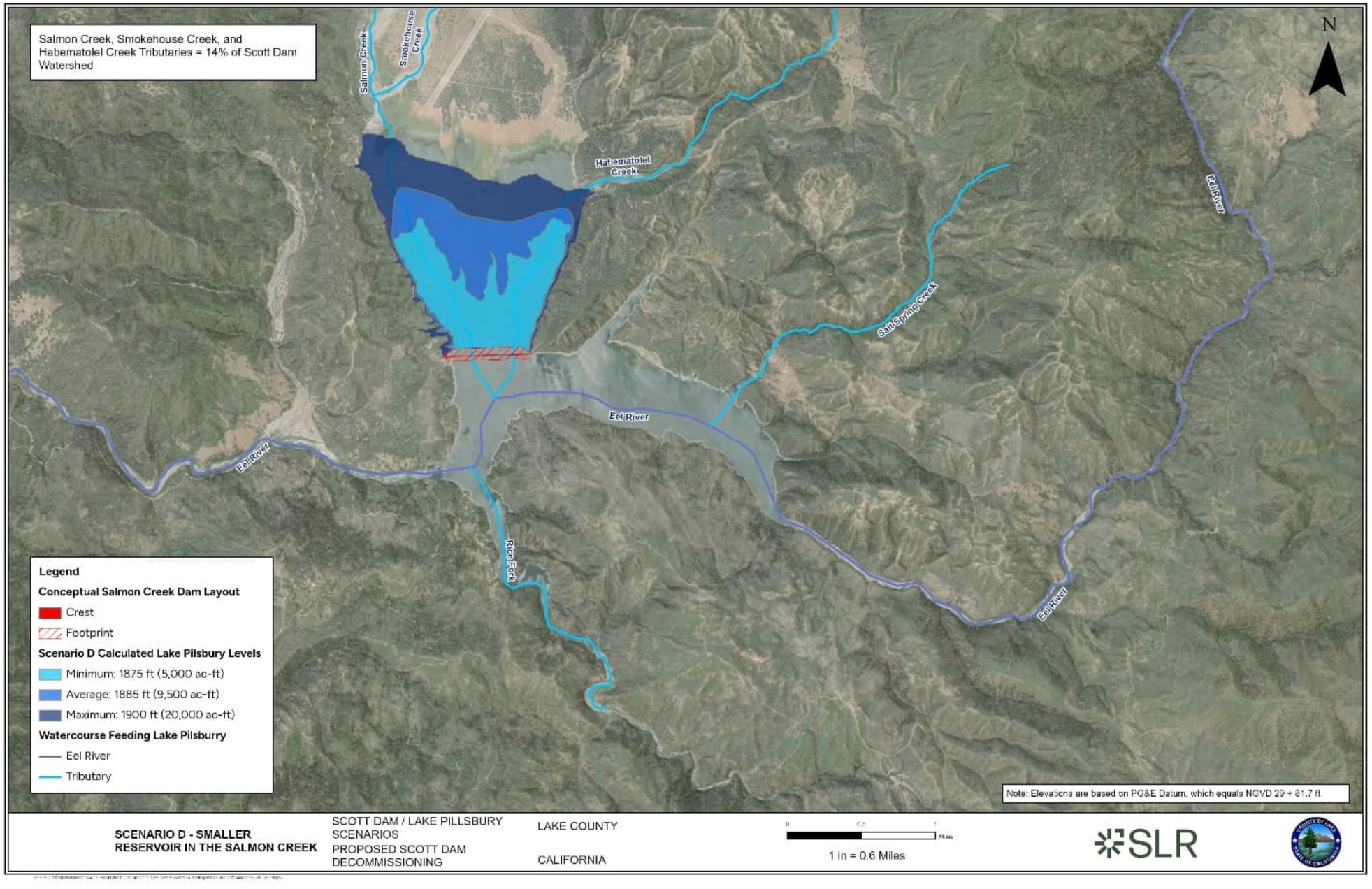


**Figure A7-19: Scenario D smaller reservoir in the Salmon Creek watershed: projected minimum, average, and maximum Lake Pillsbury WSE levels**



### A7.7.1 Hydrologic and Hydraulic Analysis

Hydrologic analysis for this scenario was conducted using the water balance model method originally develop by Cardno and repurposed for the new dam. SLR developed a stage-storage curve for the new dam to project how reducing the reservoir's storage capacity would impact WSE (Figure A7-20). This scenario assumed that only inflow into the reduced reservoir would be scaled from the current existing Lake Pillsbury watershed based on tributary area. Outflow from the reservoir would only occur passively through the spillway when the WSE is above the assumed spillway invert elevation of 1,885 ft. This scenario assumed that the proposed spillway would be constructed along a shorter length than the existing Scott Dam spillway. The analysis scaled the existing Scott Dam spillway rating curve to 25 percent of the outflow performance. No low-level outlet was assumed to be active.

The inflow rates developed from the USGS data (WY 1957-2024) were modeled for this scenario for the reduced reservoir. The resulting water balance model storage and WSE values in the reduced reservoir are presented in Figure A7-21 and Figure A7-22. The resulting water balance model outflow values from the reduced reservoir to the Eel River downstream are presented in Figure A7-23.

**Figure A7-20: Stage-storage capacity curve for the potential reduced reservoir under Scenario D**

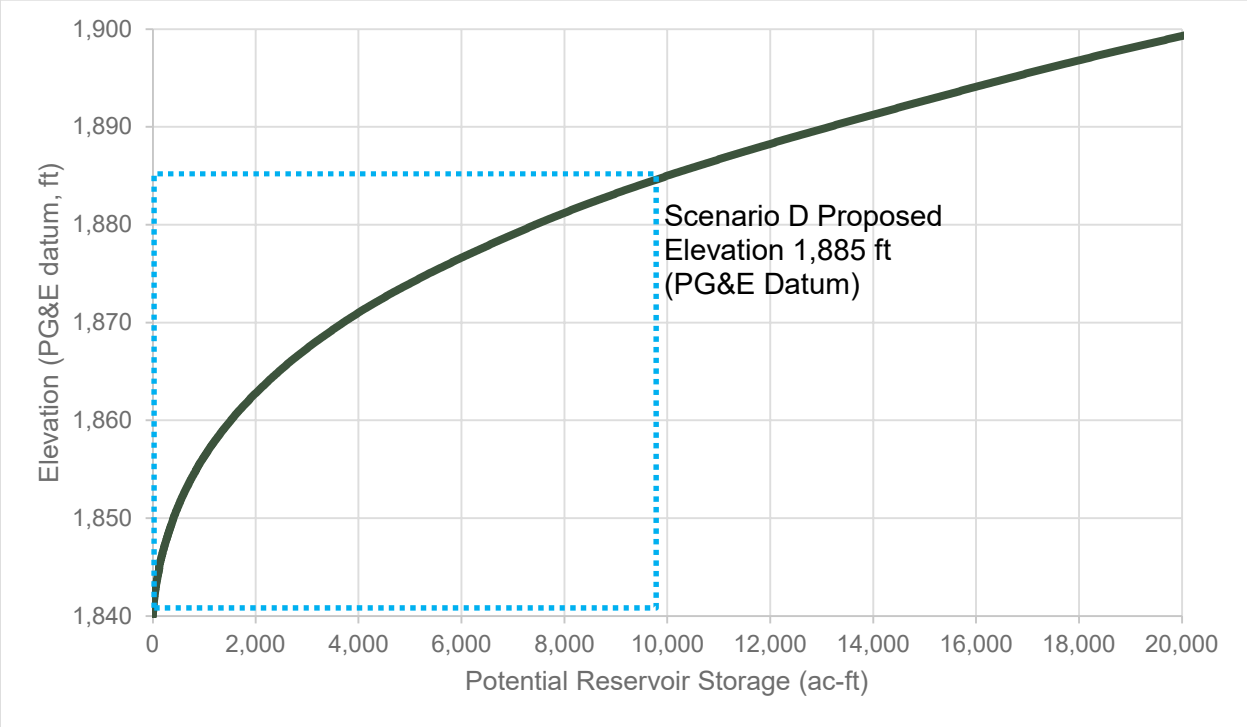


**Figure A7-21: Reduced reservoir with Salmon Creek storage water balance model results**

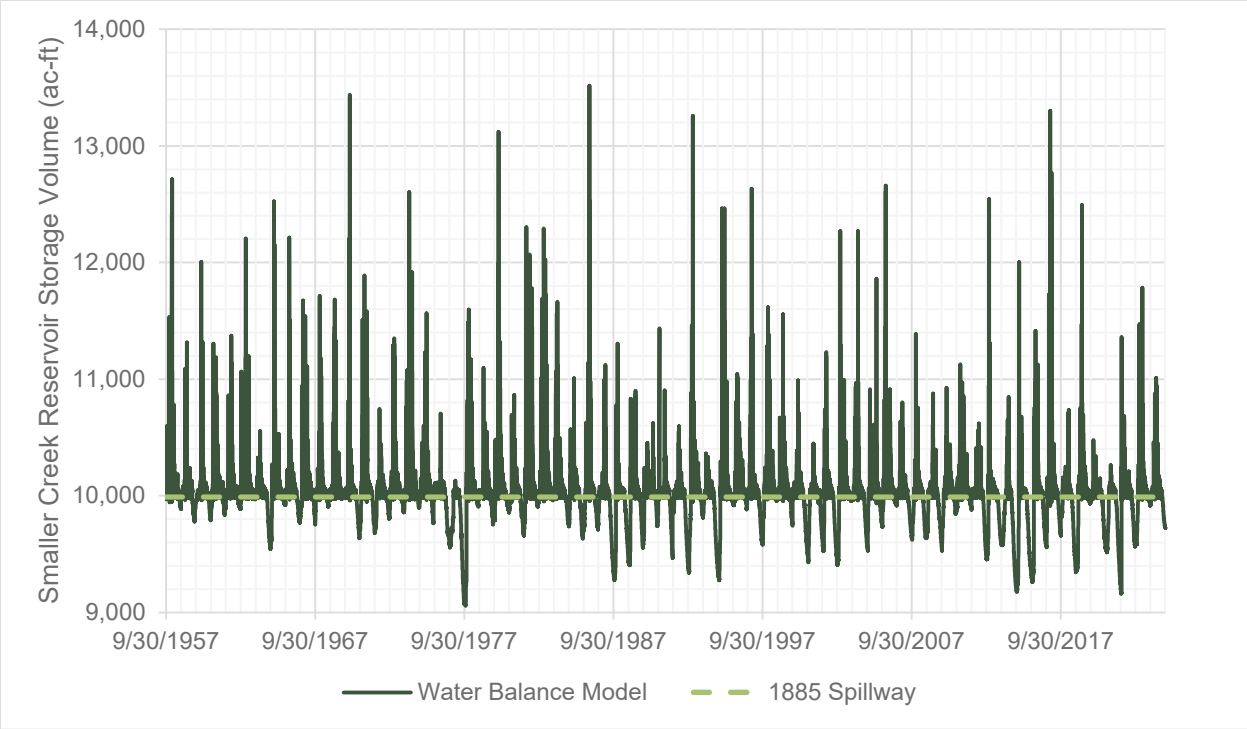



**Figure A7-22: Reduced reservoir within Salmon Creek WSE water balance model results**

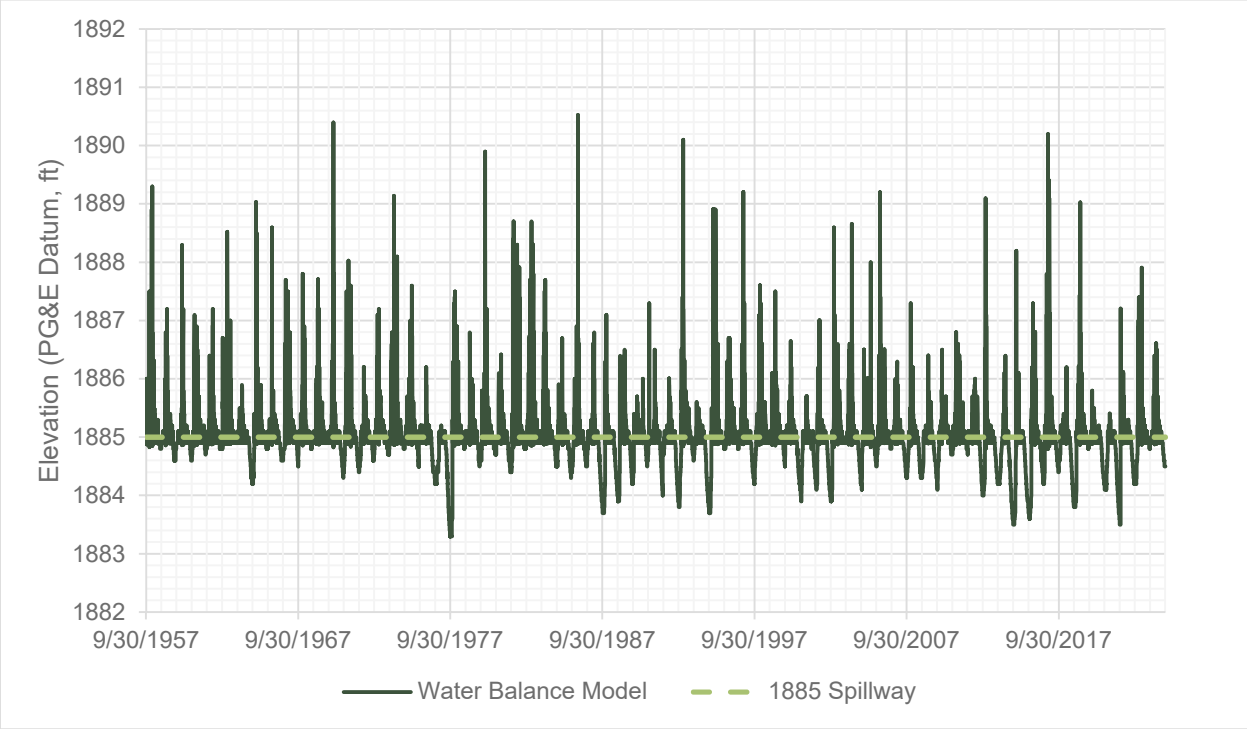
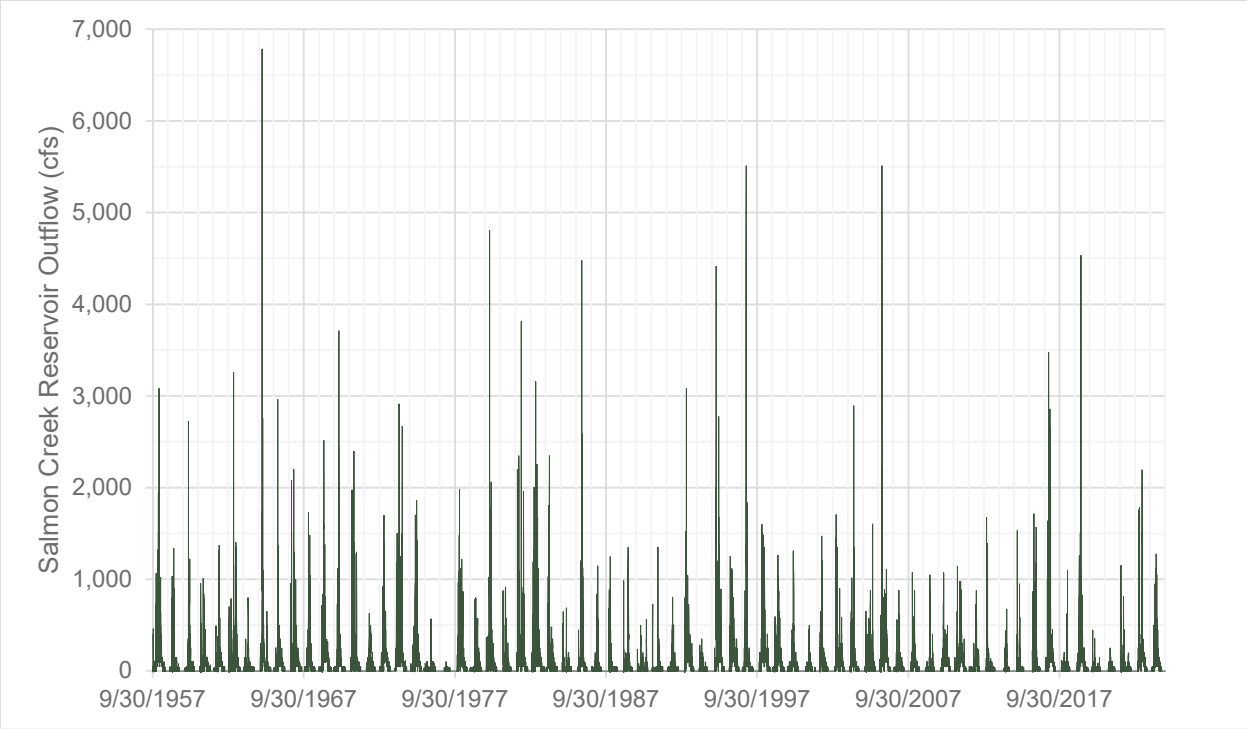


**Figure A7-23: Reduced reservoir within Salmon Creek outflow water balance model results**




### A7.7.1.1 Wet and Dry Season Conditions

The results generally demonstrated a repeating water year cycle. During the dry season, storage within the reduced reservoir will be below the spillway at the end of the dry season. During the wet season, the WSE rises to the dam spillway invert, and outflow occurs through the spillway. Periodic larger inflows during the wet season momentarily increase storage as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,885 ft. There are expected periods of no outflow from the reduced reservoir. The long-term overall trend of water storage in the reduced reservoir is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period for the reduced reservoir is presented in Table A7-14. The wet and dry season projected statistics for the Eel River immediately downstream of Scott Dam are summarized in Table A7-15 based on unimpaired flows. Table A7-15 does not account for outflows from the new earthen dam spillway.

**Table A7-14: Reduced Reservoir within Salmon Creek – Statistics for Projected Storage within the Reduced Lake Pillsbury Reservoir and Projected Outflow from the New Dam Spillway by Season**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 9,100 | 1883.3 | 0 | 9,060 | 1883.3 | 0 |
| Average | 10,110 | 1885.1 | 152 | 9,920 | 1884.8 | 25 |
| Maximum | 13,500 | 1890.5 | 6,750 | 11,860 | 1888.0 | 1,923 |

**Table A7-15: Scenario D Wet and Dry Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 684 | 15 |
| Median | 1,070 | 43 |
| Average | 1,051 | 80 |
| Maximum | 1,327 | 245 |

#### A7.7.1.2 Flood and Drought Conditions

The flood condition within the Eel River downstream of Scott Dam under this scenario could be roughly estimated based on flow contributions and the 1% AEP. The Eel River and Rice Fork contribute approximately 86% of the total unimpaired flows. This would give an approximate 1% AEP flood of 51,686 cfs when assuming zero contribution from the Salmon/Smokehouse and Habematolel Creeks. The storage will likely exceed the spillway WSE and contribute to the flows within the Eel River however the timing could be offset by the dam storage attenuating the flows. Further analysis of the contributions from the new dam spillway during an 1% AEP event and more in-depth hydrologic analysis would be recommended to refine this estimate.

Under a drought condition the storage within the reservoir has the potential to drop precipitously as there is not a consistent flow contribution and of much lower magnitude than the Eel River. The drought conditions for the Eel River downstream of the Scott Dam can only be assumed lower than the minimum projected values per Table A7-15 during the dry season of 15 cfs.

### A7.7.2 Projected Scenario Outcomes

The reduced reservoir scenario analyzes a new dam at a maximum operational WSE of 1,885 ft and the projected potential impacts on water supply.

#### A7.7.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury to only include inflow contributions from the Salmon/Smokehouse and Habematolel Creeks watershed areas to maintain a smaller reservoir. The maximum operational WSE at 1,885 ft would produce a storage volume of approximately 9,990 ac-ft which results in an approximate reduction of 80 percent of the current Lake Pillsbury storage volume at WSE 1,900 ft. The new dam is expected to require minimal operations as the outflow would occur passively through the spillway to the Eel River when the WSE is above the spillway invert. A low flow outlet is recommended to remain inactive unless drawdown is required. The drastic changes in the impoundment WSEs are expected to occur as the Habematolel Creek is the only perennial watercourse but has an overall lower contribution of flows when compared to the Salmon/Smokehouse Creeks, which are intermittent. Flood control would be reduced and limited to the smaller reservoir tributary. The storage feasibility needs to be evaluated, as the alluvial deposits in the north area are not well understood. Geotechnical feasibility needs to be evaluated due to the faults in the area.

Flood control and downstream flow diversion requirements would be unreliable and seasonally dependent. The Eel River and Rice Fork tributaries would be restored to unimpaired flow conditions as presented in Figure A7-19 and Table A7-15. Flood control and downstream water

diversion flow requirements would be affected due to the majority of the original Lake Pillsbury watershed being restored to unimpaired flow conditions. Water storage within the new impoundment will be dependent upon the sediment accumulations and further evaluation will be required to determine the accumulation rates from the Salmon/Smokehouse and Habematolel Creeks.

The Scenario D wet and dry seasons will have a reduced storage volume and sustainability when compared to the baseline scenario. There will be no controlled outlet flows but also less inflows. Outflows and inflows will vary depending on the season, flood, and drought conditions and will be affected significantly by evaporation and infiltration. The scenario's reaction to extreme climate conditions will also vary from the baseline. The flood flows will likely see an increase, but the drought flows will likely reduce on average, though a more detailed hydrologic analysis is recommended to validate these assumptions.

## A7.8 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2, but with specific mitigation added to enhance fish passage while maintaining fire suppression and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[14], the off-channel ponds would be constructed.

Two potential pond locations have been identified as shown in Figure A7-24 based upon the 1922 historic pre-dam topography and bathymetry (McBain Associates, 2018). The 1922 topography and bathymetry was georeferenced to the Lake County hillshade to predict how the site will look after restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table A7-16 summarizes the potential footprints and volumes of Ponds A and B assuming a 10-foot normal depth. The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design. The conceptual design would function similarly to the example provided in Figure A7-25, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table A7-16: Potential Pond Footprint and Storage Volume of Ponds A and B under Scenario E**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |

[14] Quasi-equilibrium in this case refers to the ability of a river to maintain its dimension, pattern and profile without either aggrading or degrading over time.

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

**Figure A7-24: Scenario E - Dam Removal with Off-Channel Pond Restoration**

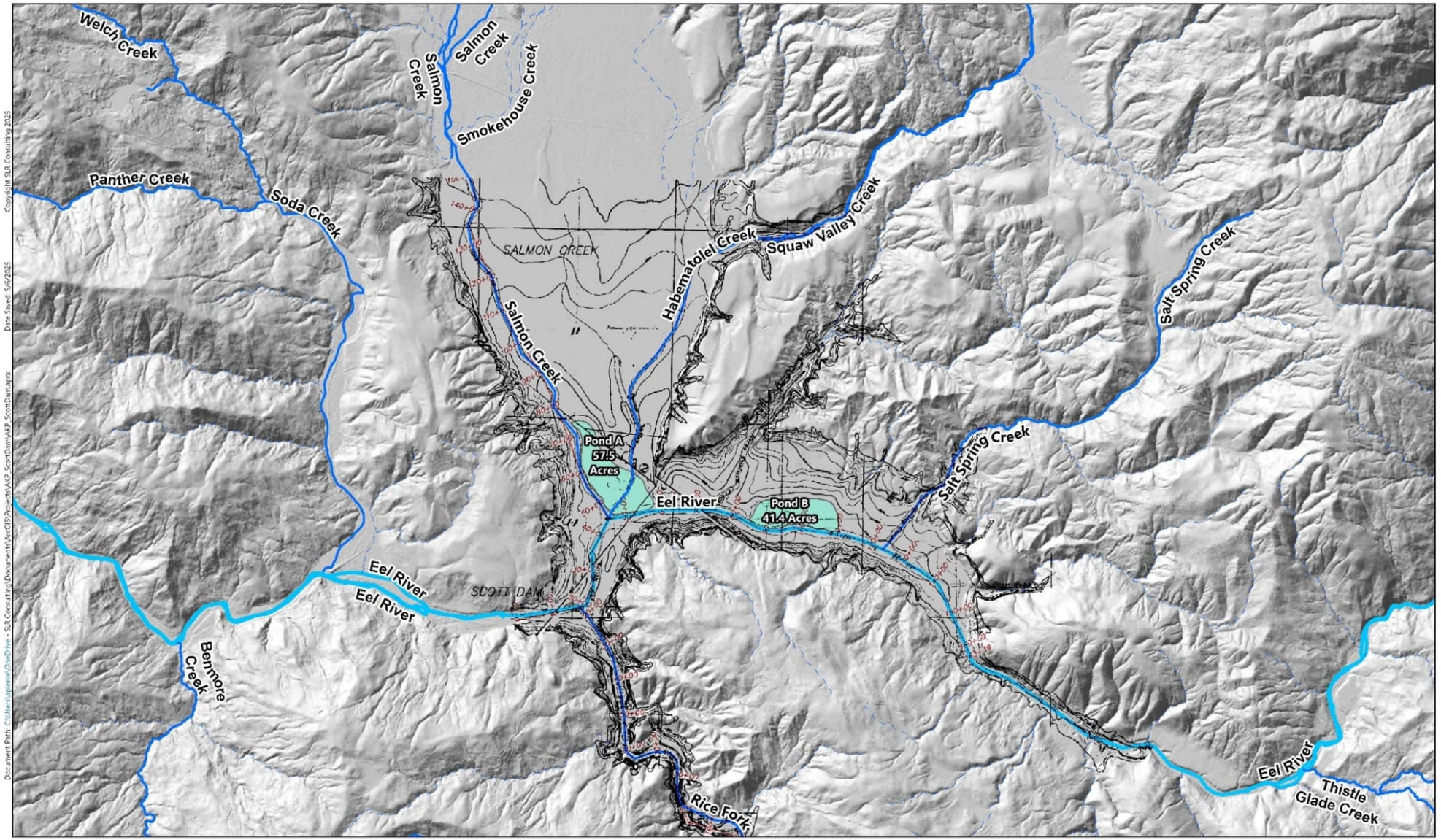
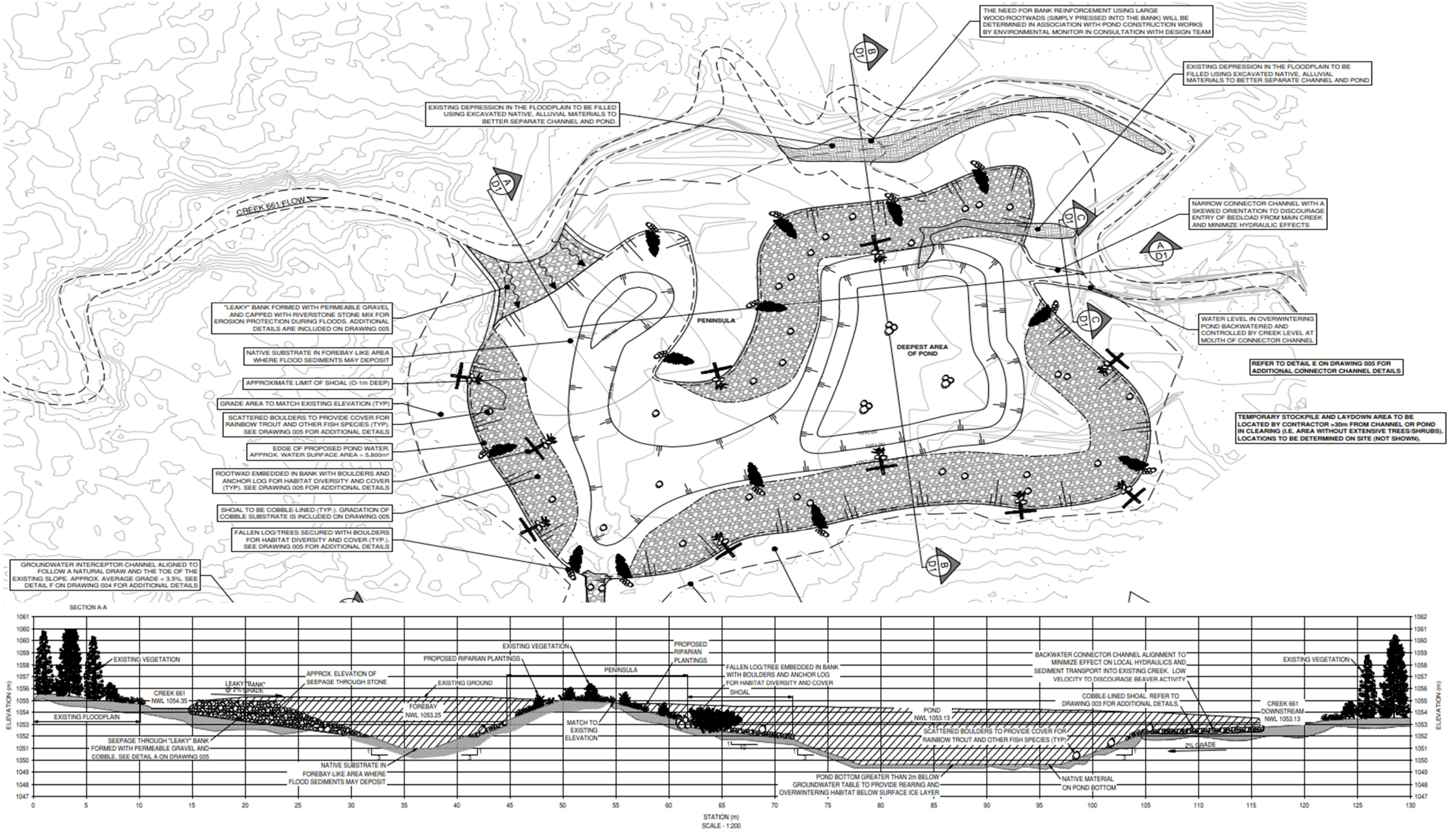


**Figure A7-25: Example Design of Off-Channel Pond**



### A7.8.1 Hydrologic and Hydraulic Analysis

The analysis for this scenario uses the analysis for the dam removal A1/A2 scenarios in terms of the climate conditions. The ponds will allow for water retention and will reduce outflows from the Eel River projected downstream of Scott Dam.

#### A7.8.1.1 Wet and Dry Season Conditions

Under this scenario, it is assumed that both Pond A and B will be constructed. Pond A will be fed by all inflow from the Habematolel Creek and up to 10% of the Salmon and Eel River during normal wet and dry seasons. Pond B assumes a contribution of an additional 10% of flows from the Eel River, meaning both ponds will take up to 10% of the Eel River flows each. Figure A7-26, Figure A7-27, and Table A7-17 summarize the adjusted average monthly predicted unimpaired flows downstream of Scott Dam for the wet and dry season by removing the assumed contributions from both Pond A and B. This scenario will reduce the total flow contribution from the Eel River downstream of Scott Dam by up to 14% when compared to Scenarios A1 and A2 during normal wet and dry seasons. This value vary depending upon the climate conditions and increase or decrease depending on the goals of the design, infiltration rates, groundwater tables, and pond usage.

**Table A7-17: Dam Removal Scenario E Wet and Dry Season Monthly Average Statistical Unimpaired Flows Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 682 | 15 |
| Median | 1,067 | 43 |
| Average | 1,048 | 80 |
| Maximum | 1,324 | 244 |

**Figure A7-26: Unimpaired Dry Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse under Scenario E**

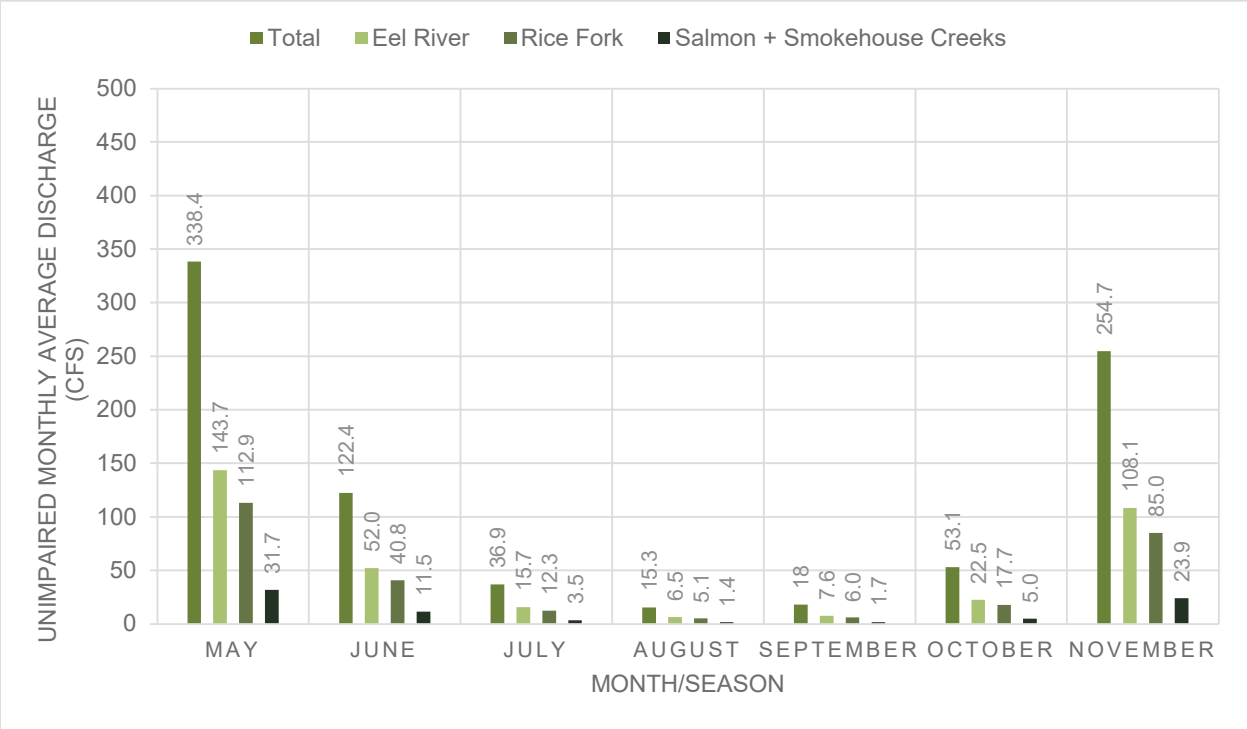
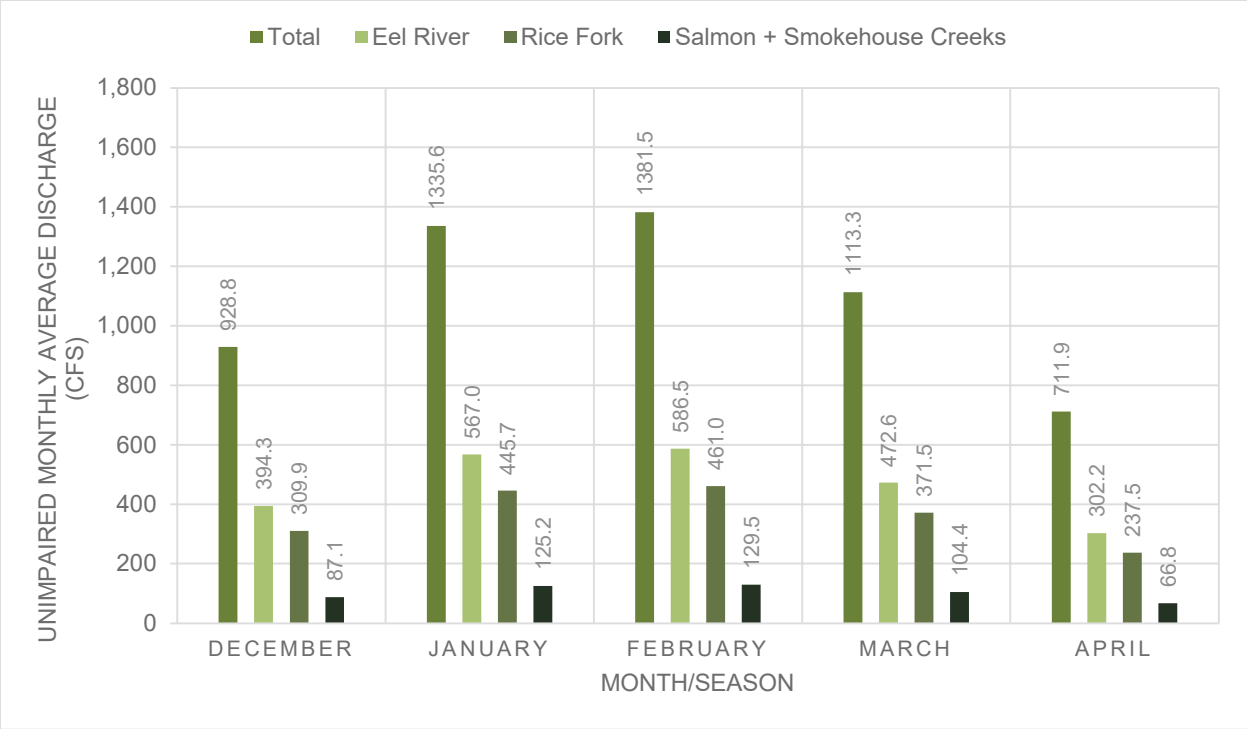



**Figure A7-27: Unimpaired Wet Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse under Scenario E**



#### A7.8.1.2 Flood and Drought Conditions

This scenario is not likely to drastically change flows during the extreme flood conditions summarized in Table A7-10 as the ponds will be located within the floodplain. The restoration and the off-channel ponds design is recommended to be resilient against the predicted design flows (1% AEP flows). Drought conditions would likely see a reduction from the Scenarios A1 and A2 predictions but are difficult to quantify or predict with the currently available data. The reduction or impacts to the extreme climate conditions would be dependent upon the infiltration rates, design goals, usage (fire suppression), and groundwater tables. Developing a resilient design to recover from extreme flood and drought conditions would be recommended and is viable from an ecological perspective. Resiliency in providing a consistent water supply can be more difficult to guarantee under this scenario. However, this option provides the highest potential benefit to water supply when compared to the other scenarios where the dam is being removed.

### A7.8.2 Projected Scenario Outcomes

The predicted scenario outcomes are similar to Scenario A2 but with additional benefits and outcomes with the inclusion of the ponds.

#### A7.8.2.1 Water Supply

The water supply downstream for this scenario is at a reduced benefit than Scenarios A1 and A2 due to a percentage of the flows being retained by the ponds. The storage availability is increased from the Scenarios A1 and A2 but significantly lower than the baseline. This scenario may reduce flood risk from storage retention but cannot be quantified with the information available. A flood model of the Eel River upstream and downstream of the Scott Dam location would need to be developed to adequately determine the potential flood risk. This scenario may provide water during drought conditions depending on the design of the pond and the severity of the drought. The ponds can also support groundwater recharge but would still cause a change in the groundwater table and flow patterns when compared to the baseline.

## A8.0 Conclusions

The purpose of this hydrologic and hydraulic analysis completed by SLR is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of mitigation strategies required under the Department of Water Resources (DWR) grant. Specifically, this Appendix outlines how potential decommissioning may affect the Eel River system and identifies potential restoration and alternate mitigation opportunities within the Lake Pillsbury area for different scenarios. All projected scenario response to the climate conditions are presented in Table A8-1.

This Appendix presents the initial analyses conducted to assess the feasibility of these alternatives, with a focus on the effects on the Eel River hydrology and hydraulics. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource.

This analysis does not represent a final decision or commitment for any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

The results present here provides an initial step in a broader, ongoing process. Further studies, analyses, and synthesis will be required, ultimately informing decisions by the County of Lake and other stakeholders.

**Table A8-1: Summary of Scenario Hydrology and Hydraulics under the Existing and Extreme Climate Conditions**

| Scenario | Existing Climate Conditions | | | | Extreme Climate Conditions | | | |
|---|---|---|---|---|---|---|---|---|
| | Wet Season (December - April) | | Dry Season (May - November) | | Flood | | Drought | |
| | Storage (ac-ft) | Outflow (cfs) | Storage (ac-ft) | Outflow (cfs) | Max Storage (ac-ft) [Date] | Max Outflow (cfs) [Date] | Min Storage (ac-ft) [Date] | Min Outflow (cfs) [Date] |
| Historical Conditions WY 1922-2019 & 2005-2019 | **1922-2019**<br>Min: 10<br>Median: 65,900<br>Average: 59,810<br>Max: 94,900<br><br>**2005-2019**<br>Min: 7,840<br>Median: 55,400<br>Average: 49,078<br>Max: 80,200 | **1922-2019**<br>Min: 2<br>Median: 375<br>Average: 1,033<br>Max: 45,300<br><br>**2005-2019**<br>Min: 37<br>Median: 296<br>Average: 820<br>Max: 26,600 | **1922-2019**<br>Min: 2,100<br>Median: 59,700<br>Average: 58,240<br>Max: 94,900<br><br>**2005-2019**<br>Min: 11,300<br>Median: 45,200<br>Average: 45,911<br>Max: 80,800 | **1922-2019**<br>Min: 0.1<br>Median: 177<br>Average: 222<br>Max: 10,200<br><br>**2005-2019**<br>Min: 27<br>Median: 147<br>Average: 165<br>Max: 2,400 | **1922-2019**<br>Flood of record:<br>76,260 [1/26/1983]<br>Max gage reading:<br>94,900 [4/24/1932]<br><br>**2005-2019**<br>Flood of record:<br>72,470 [12/31/2005]<br>Max gage reading:<br>80,200 [4/29/2005] | **1922-2019**<br>Flood of record:<br>15,400 [1/27/1983]<br>Max gage reading:<br>45,300 [12/22/1964]<br><br>**2005-2019**<br>Flood of record:<br>33,500 [12/31/2005]<br>Max gage reading:<br>33,500 [12/31/2005] | **1922-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>10 [12/9/1931]<br><br>**2005-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>7,840 [2/5/2014] | **1922-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>0.1 [9/8/1924]<br><br>**2005-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>27 [11/5/2015] |
| Current Conditions WY 2019-2024 & Projected | **2019-2024**<br>Min: 16,800<br>Median: 53,200<br>Average: 45,439<br>Max: 67,300<br><br>**Projected**<br>Min: 13,870<br>Average: 48,500<br>Max: 78,910 | **2019-2024**<br>Min: 67<br>Median: 207<br>Average: 768<br>Max: 13,200<br><br>**Projected**<br>Min: 40<br>Average: 1,052<br>Max: 51,230 | **2019-2024**<br>Min: 21,100<br>Median: 44,100<br>Average: 43,789<br>Max: 62,100<br><br>**Projected**<br>Min: 13,910<br>Average: 45,070<br>Max: 67,380 | **2019-2024**<br>Min: 42<br>Median: 115<br>Average: 201<br>Max: 2,150<br><br>**Projected**<br>Min: 40<br>Average: 258<br>Max: 15,110 | **Projected**<br>78,910 | **Projected**<br>51,230 | **Projected**<br>13,870 | Not enough data to adequately predict. Likely <40. |
| Scenario A1/A2 - PGE Proposed Removal | No storage | Min: 791<br>Average: 1,216<br>Max: 1,535 | No storage | Min: 17<br>Average: 134<br>Max: 376 | No storage | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | No storage | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <17. |
| Scenario C - Lowered Scott Dam with Fish Ladder | Min: 5,420<br>Average: 18,769<br>Max: 28,460 | Min: 20<br>Average: 1,116<br>Max: 47,470 | Min: 7,340<br>Average: 18,080<br>Max: 31,840 | Min: 20<br>Average: 220<br>Max: 14,923 | 31,840 | 47,470 | 5,420 | Not enough data to adequately predict. Likely <20. |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Min: 9,100<br>Average: 10,110<br>Max: 13,500 | Min: 684<br>Average: 1,051<br>Max: 1,327 | Min: 9,060<br>Average: 9,920<br>Max: 11,860 | Min: 15<br>Average: 43<br>Max: 245 | 13,500 | 51,686 | 9,060 | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <15. |
| Scenario E – Dam Removal + Off Channel Pond Restoration | ≥863 | Min: 682<br>Median: 1,067<br>Average: 1,048<br>Max: 1,324 | <863 | Min: 15<br>Median: 43<br>Average: 80<br>Max: 244 | ≥863 | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | <<863 | Not enough data to adequately predict. Restoration design can provide resiliency. Likely <15. |

Notes:
1. All outflow values predicted at the location just downstream of the Scott Dam within the Eel River.
2. All storage values are limited to predicted storage within Lake Pillsbury upstream of Scott Dam except scenario D where values are based within the reduced impoundment footprint.
3. Scenario B hydraulic and hydrologic parameters were not analyzed or reported in this table.

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# Appendix A.A Supporting Figures for Hydrologic and Hydraulic Analysis

## Appendix A - Hydrologic and Hydraulic Scenario Evaluation

Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

**Figure A.A9-1: Daily average outflow from Lake Pillsbury and daily average storage during the 1976-77 drought**

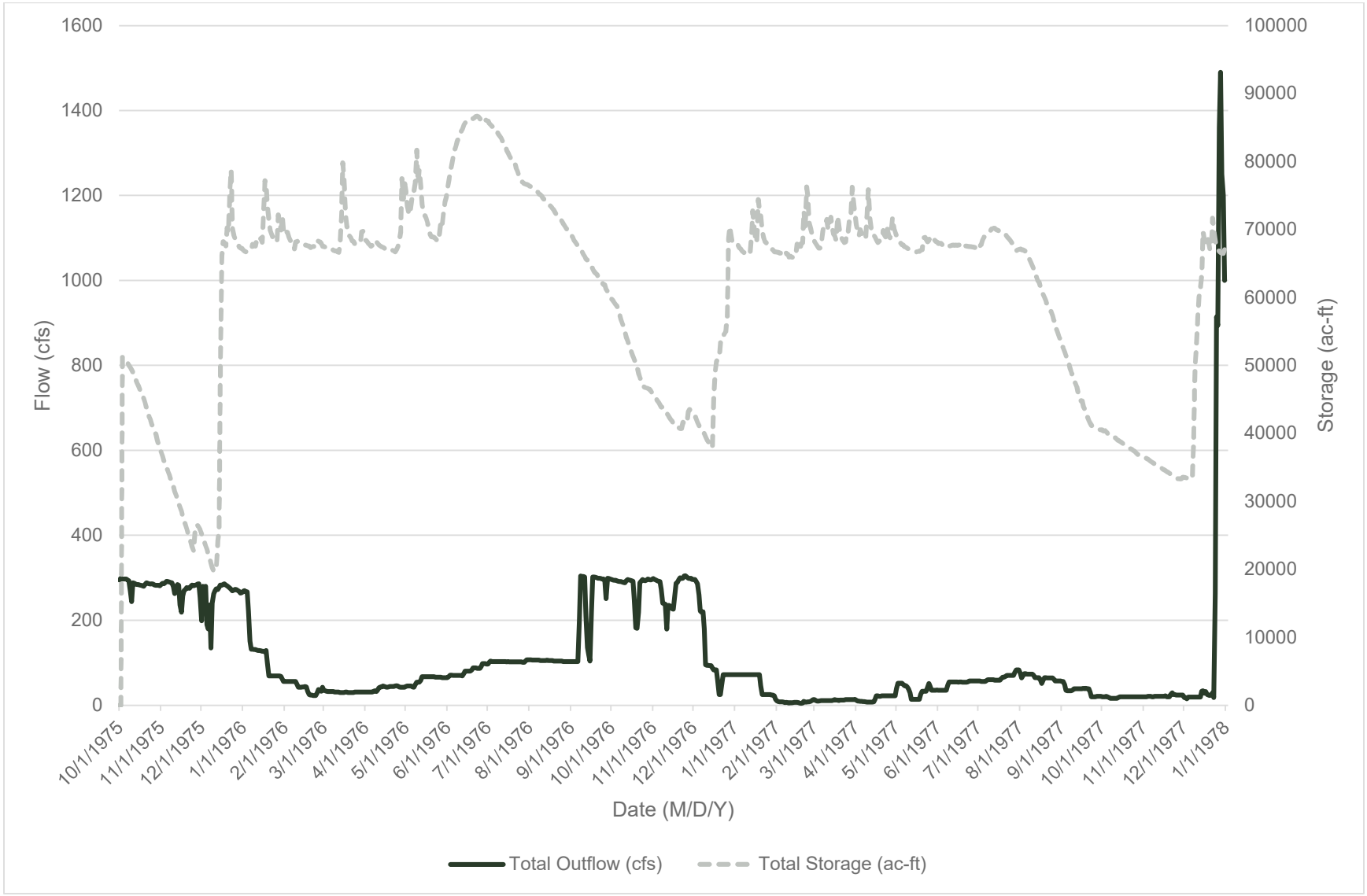


**Figure A.A 9-2: Daily average outflow from Lake Pillsbury and daily average storage during the 1982-83 flood (flood of record)**

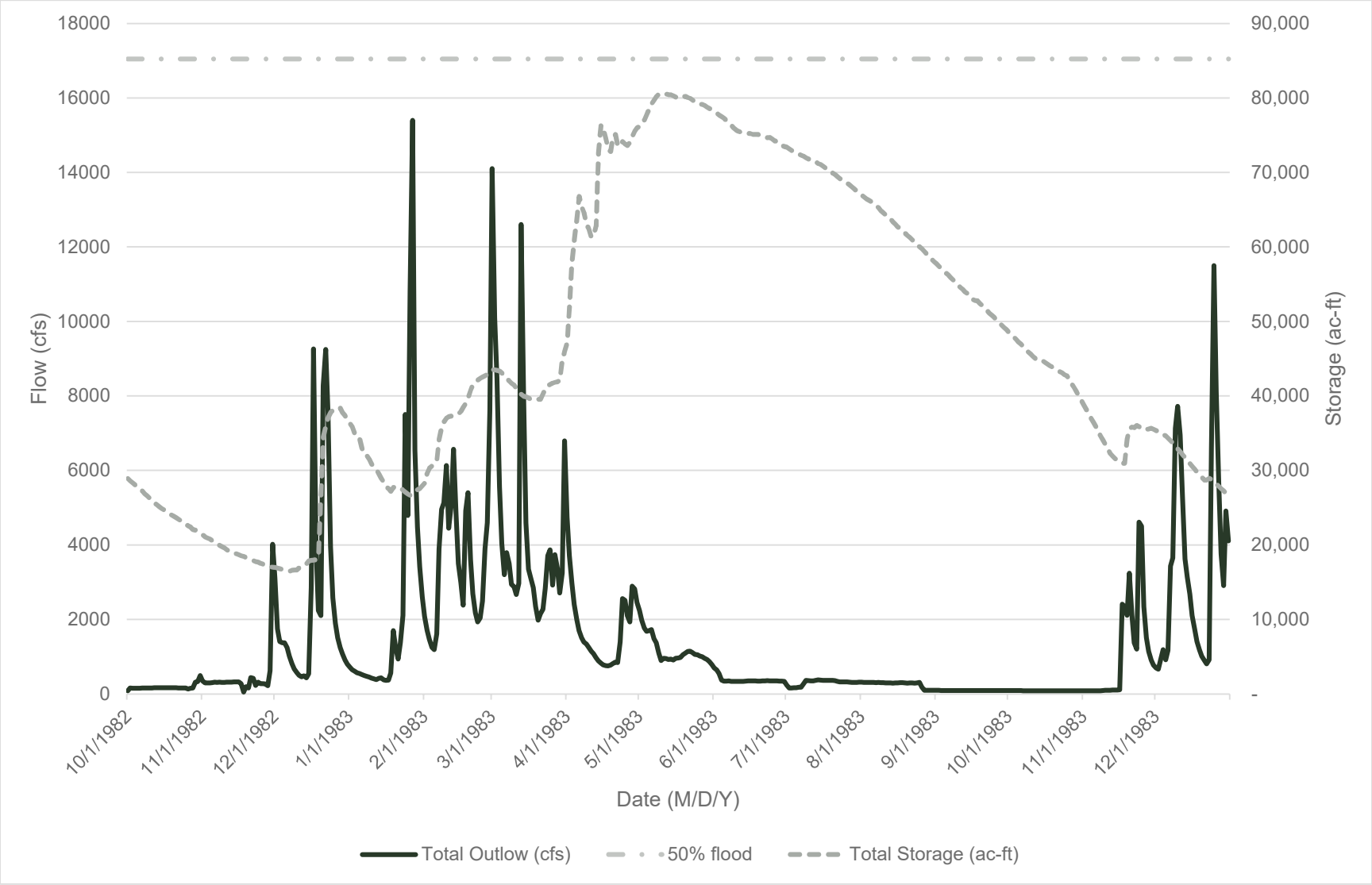


**Figure A.A 9-3: Daily average outflow from Lake Pillsbury and daily average storage during the 1995 flood**

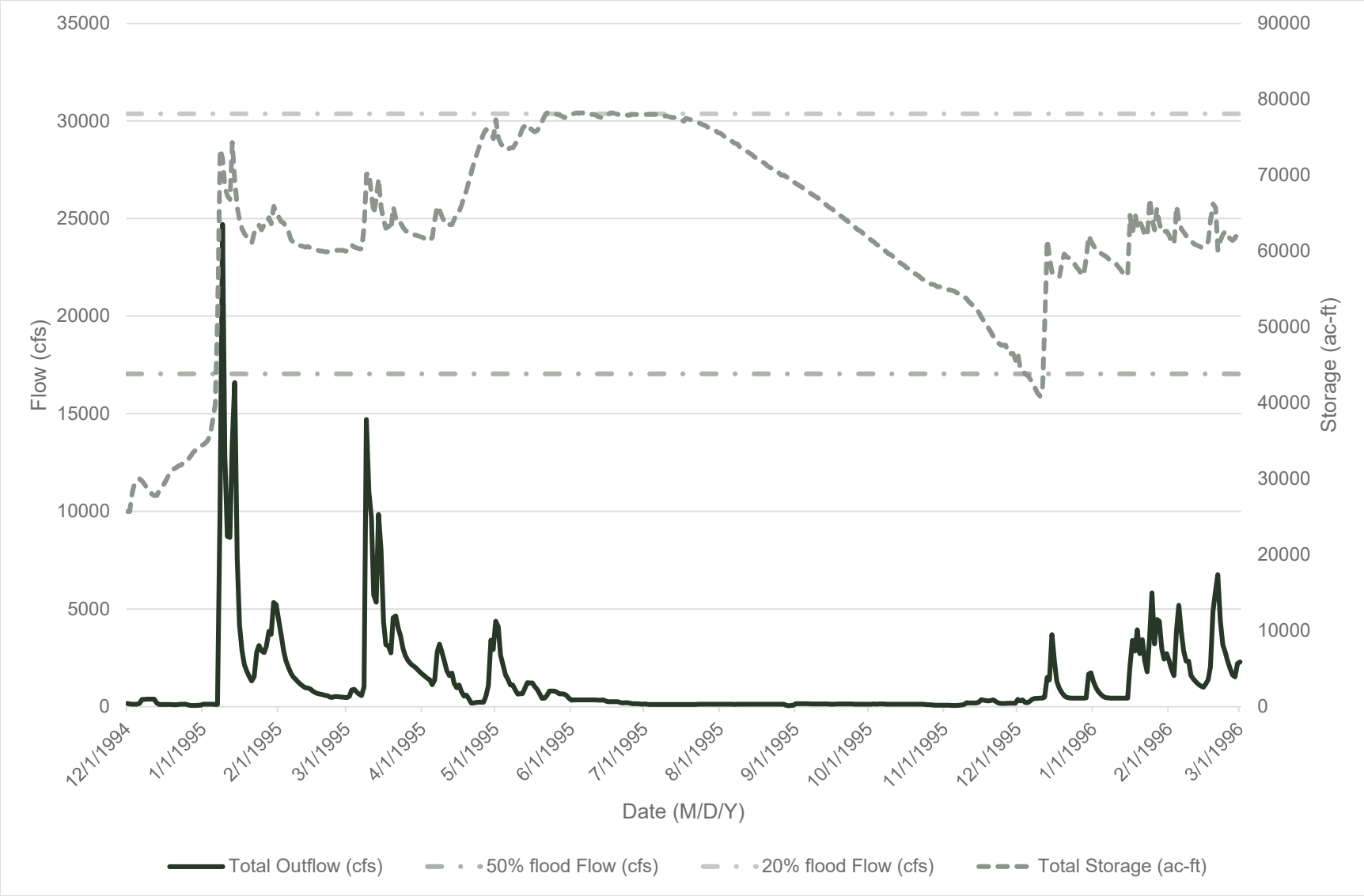


**Figure A.A 9-4: Daily average outflow from Lake Pillsbury and daily average storage during the 2020-22 drought**

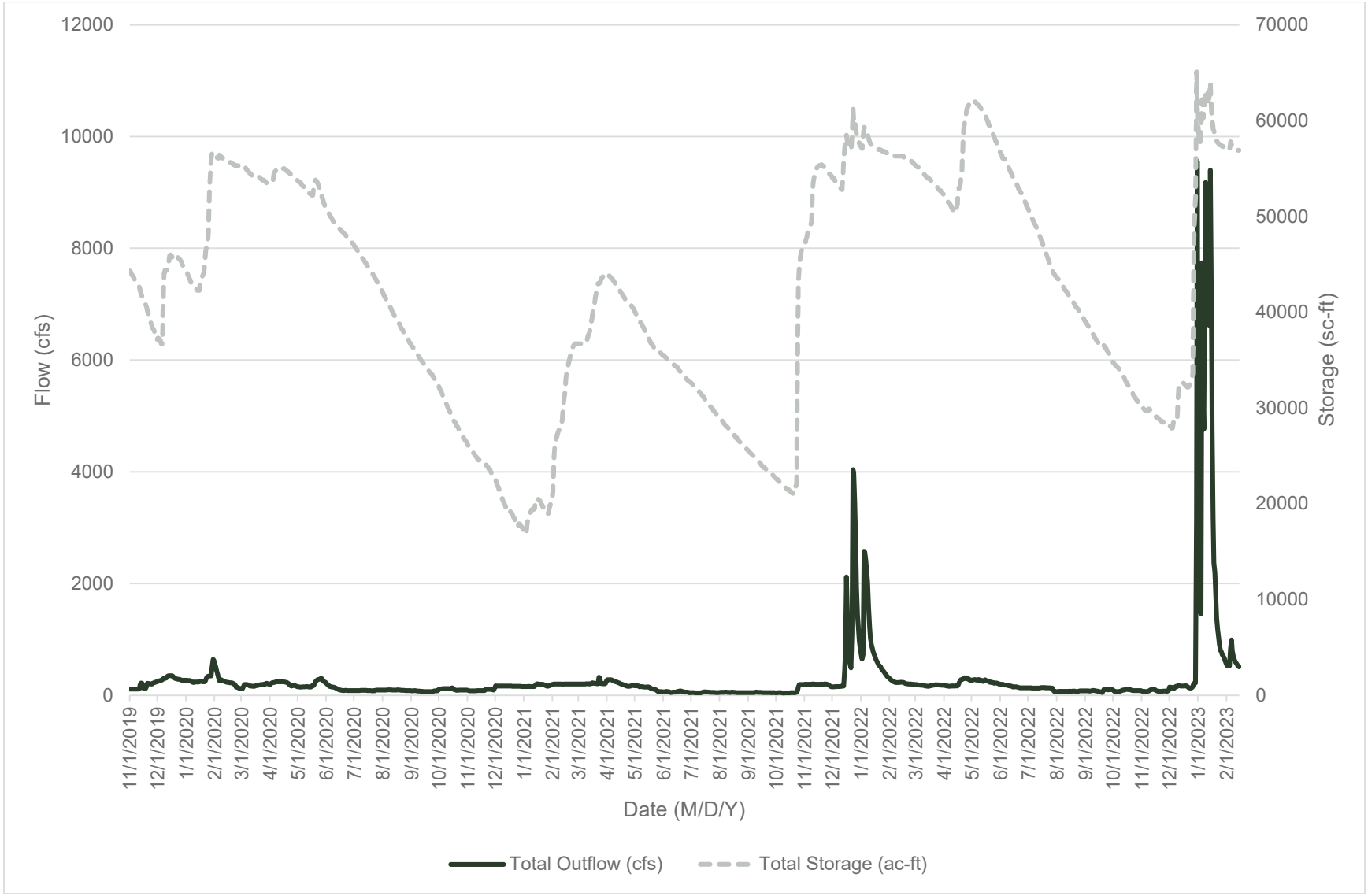


**Figure A.A 9-5:Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 2020-22 drought**

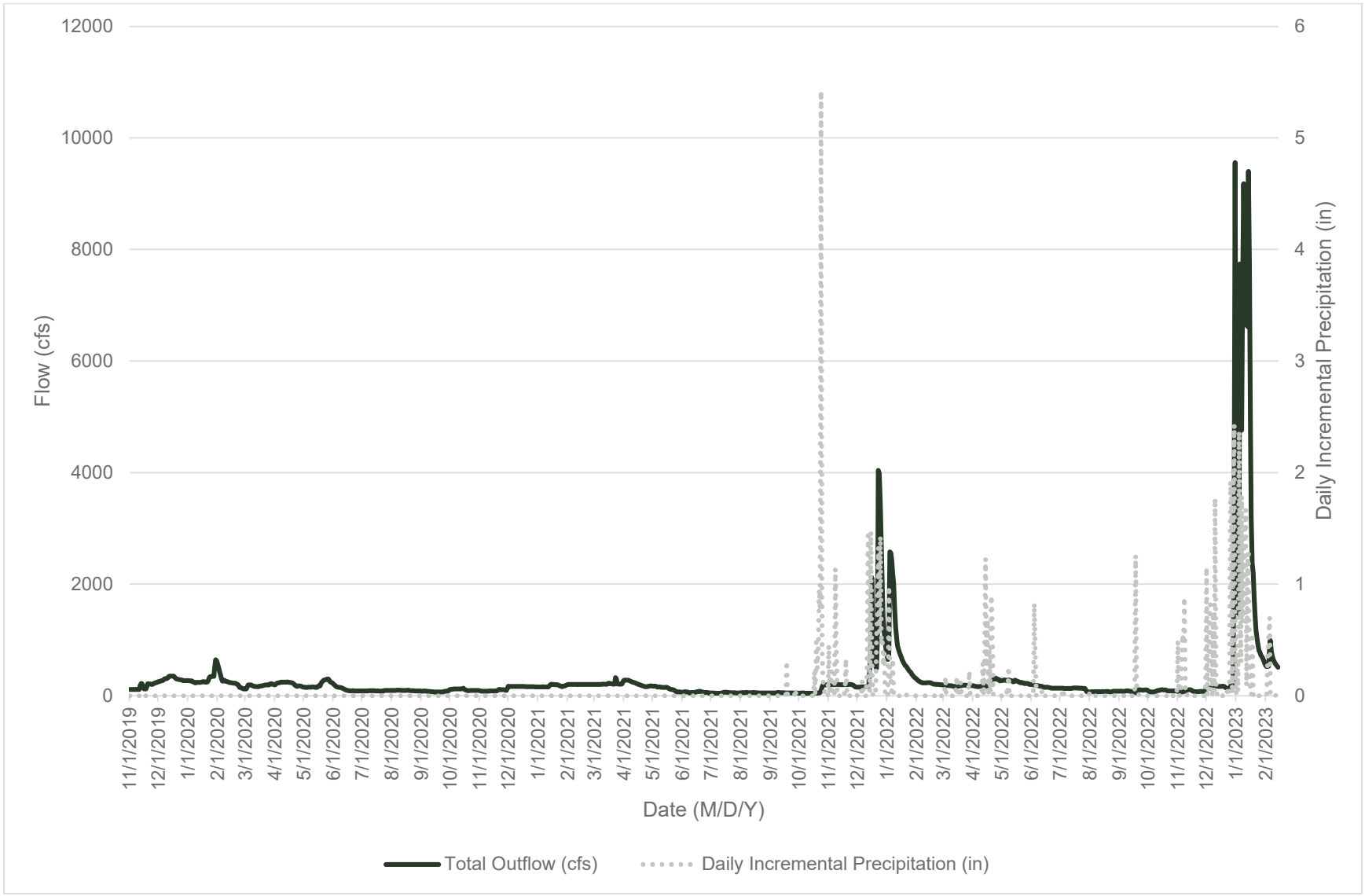


**Figure A.A 9-6: Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 1928-34 drought**

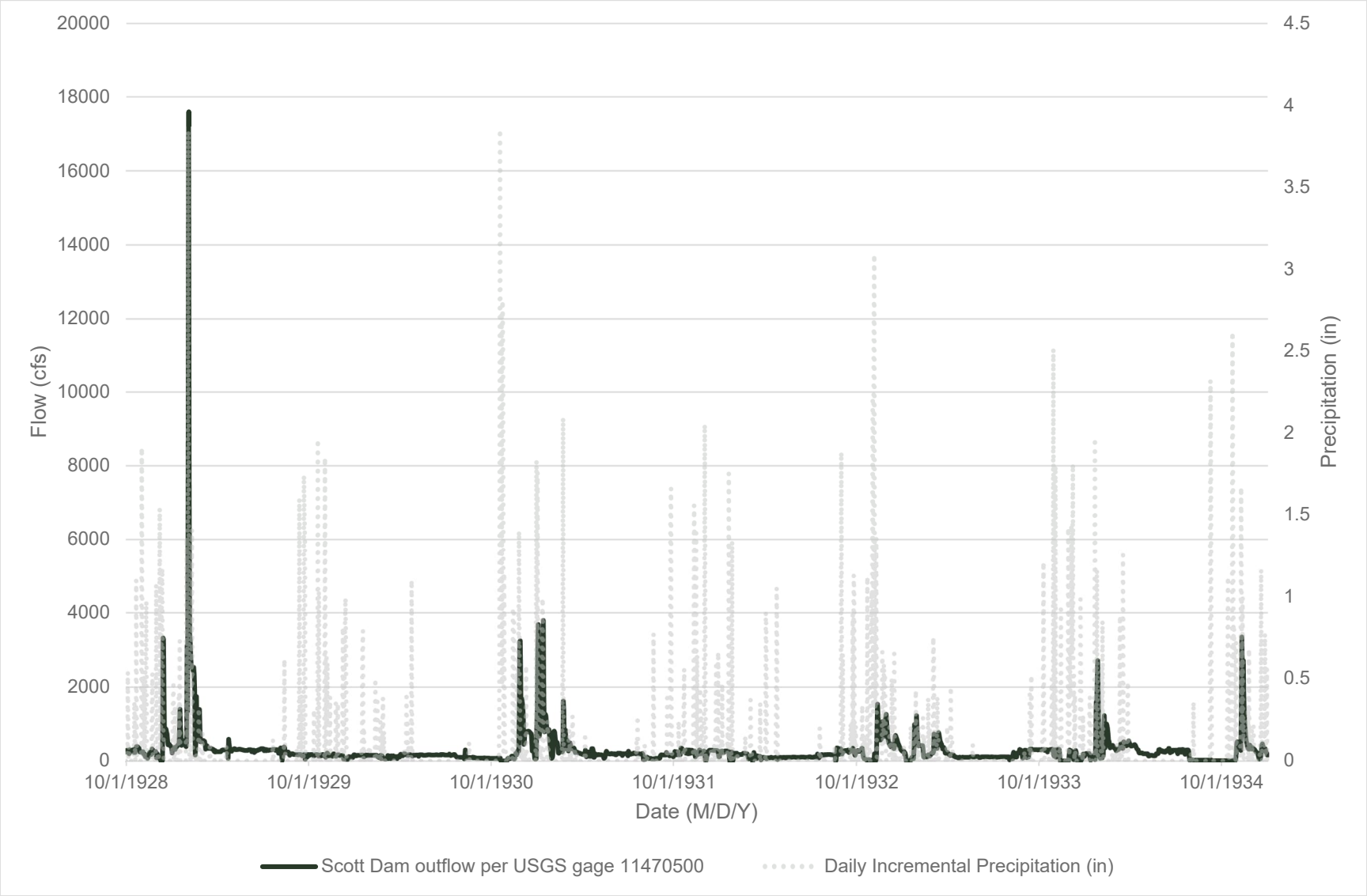


**Figure A.A 9-7: Daily average outflow from Lake Pillsbury and daily average storage during the 1928-34 drought**

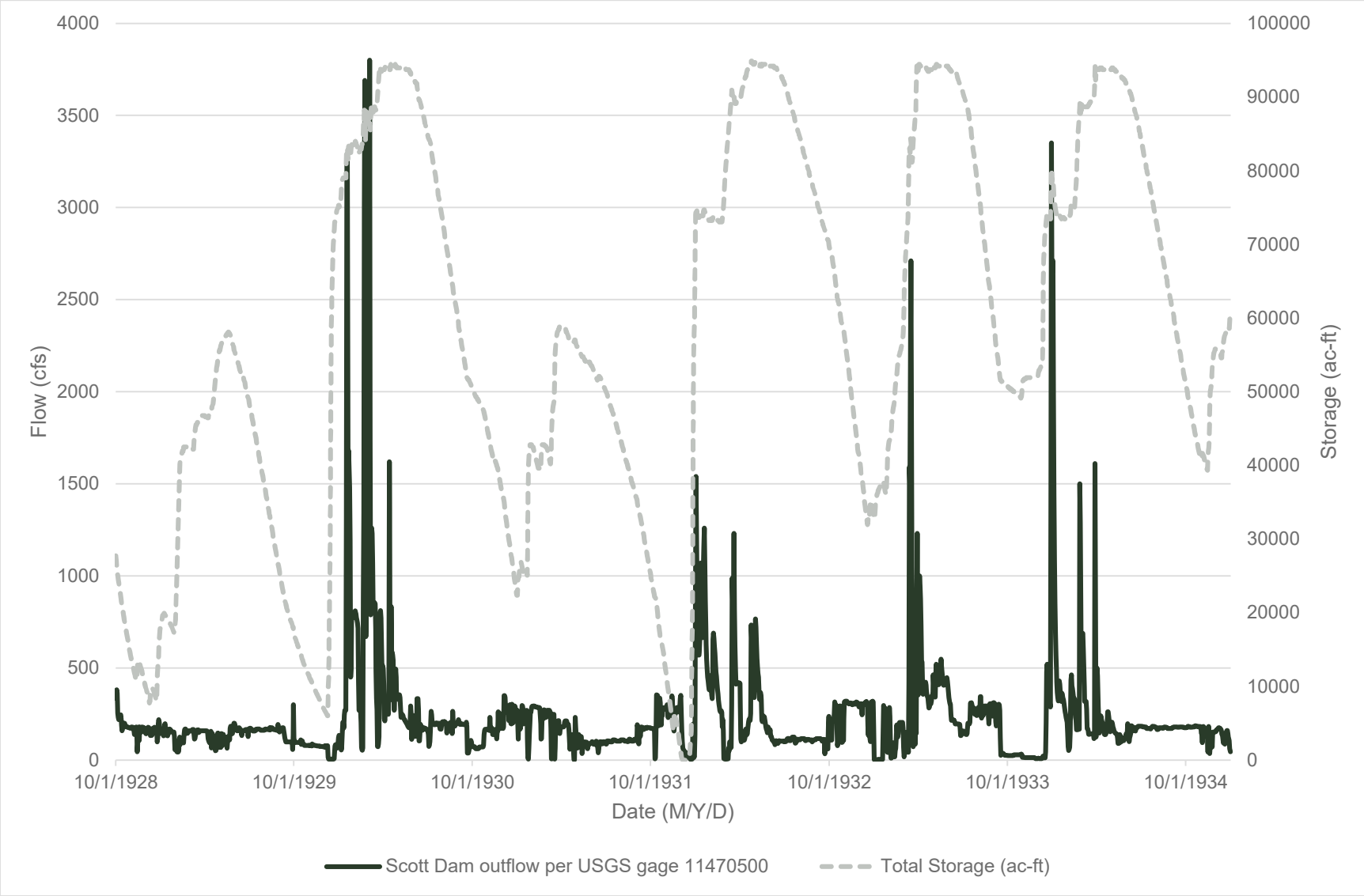


**Figure A.A9-8: Dry and wet conditions in Lake County, CA from 1910-40, including the 1937 flood**

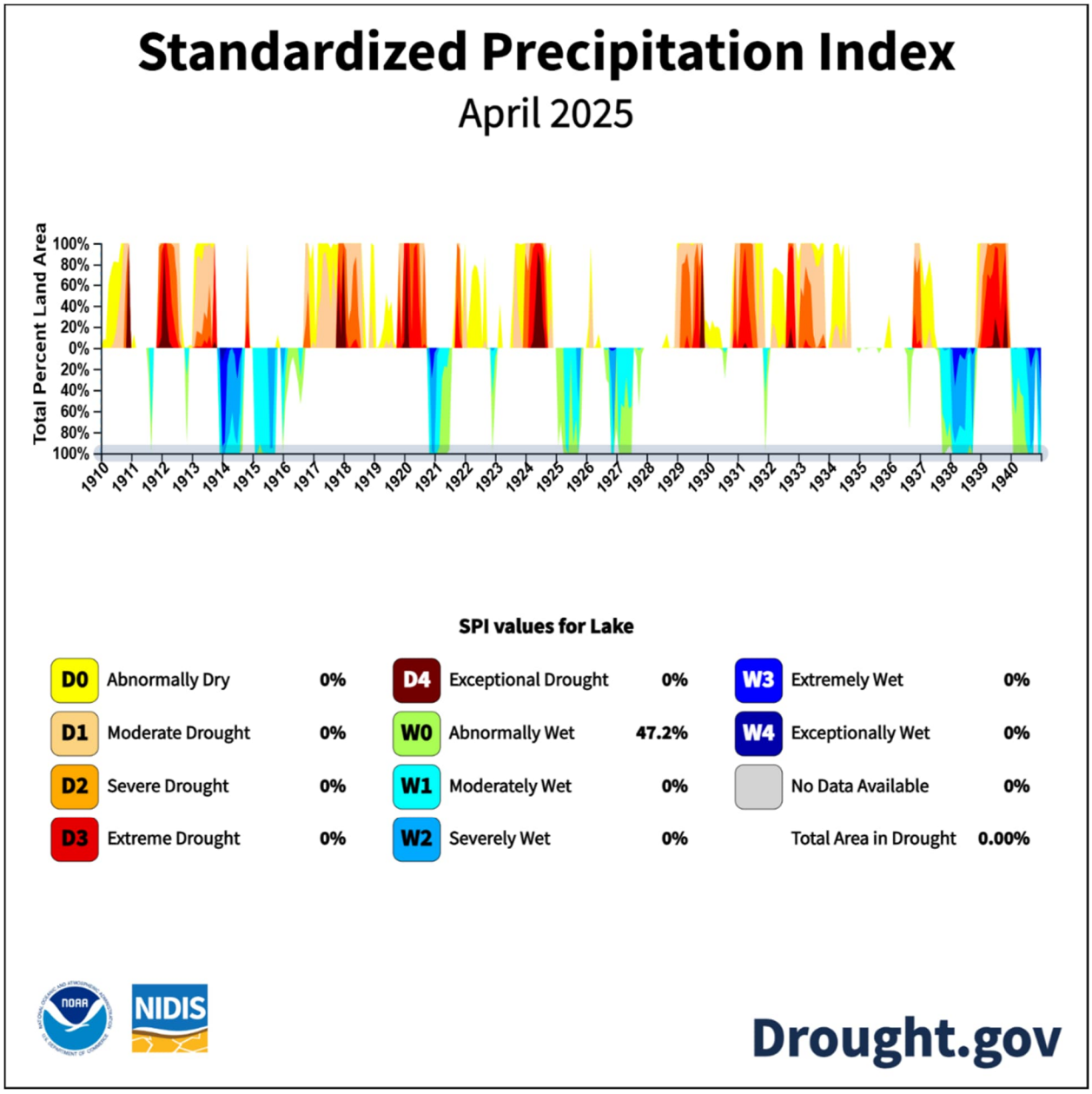


**Figure A.A9-9: Dry and wet conditions in Lake County, CA from 1980-85, including the exceptional 1983 flood**

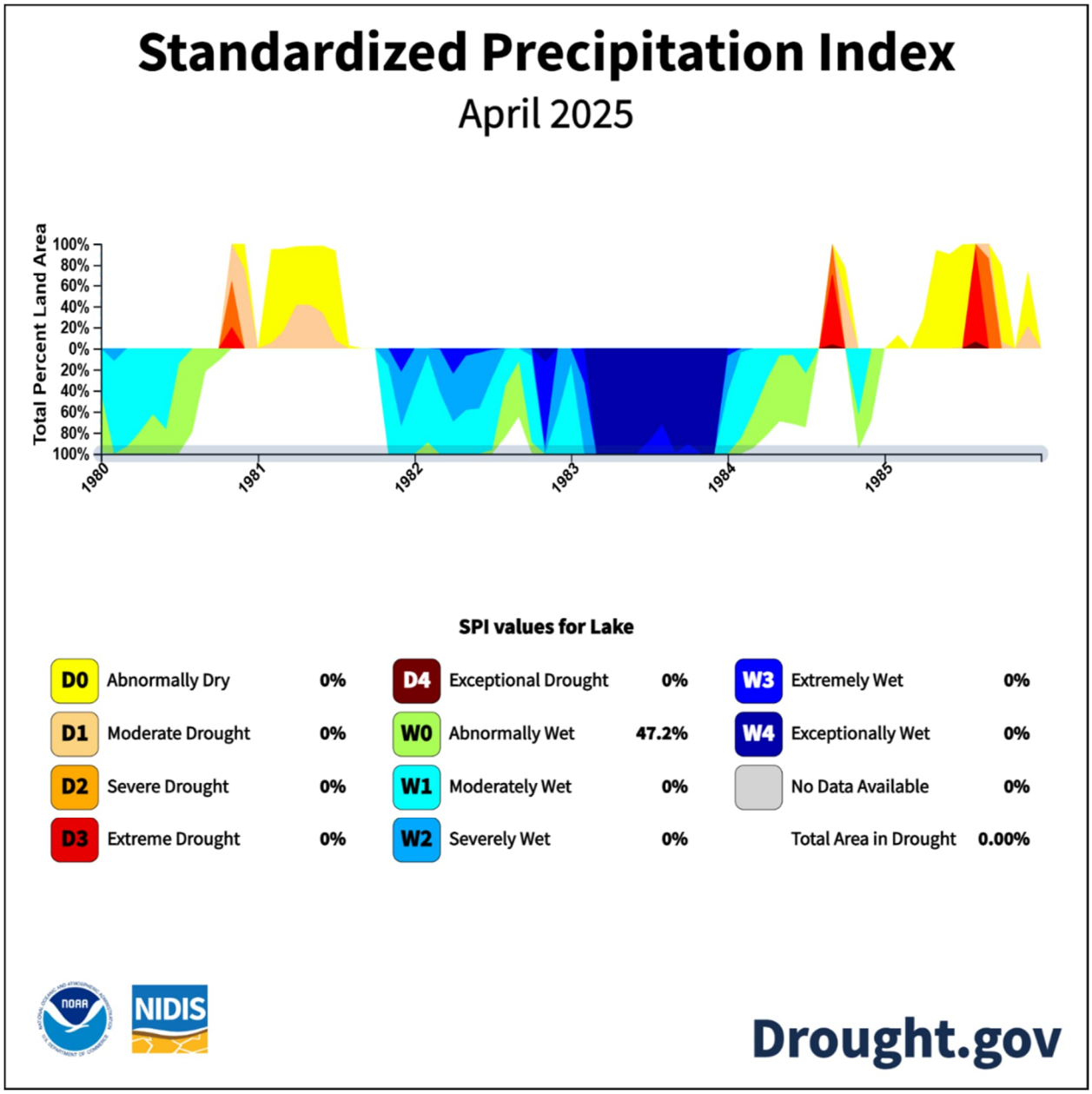


**Figure A.A9-10: Dry and wet conditions for the State of California from 2010-present, including the drought of record from 2012-16 and the recent 2020-23 drought**

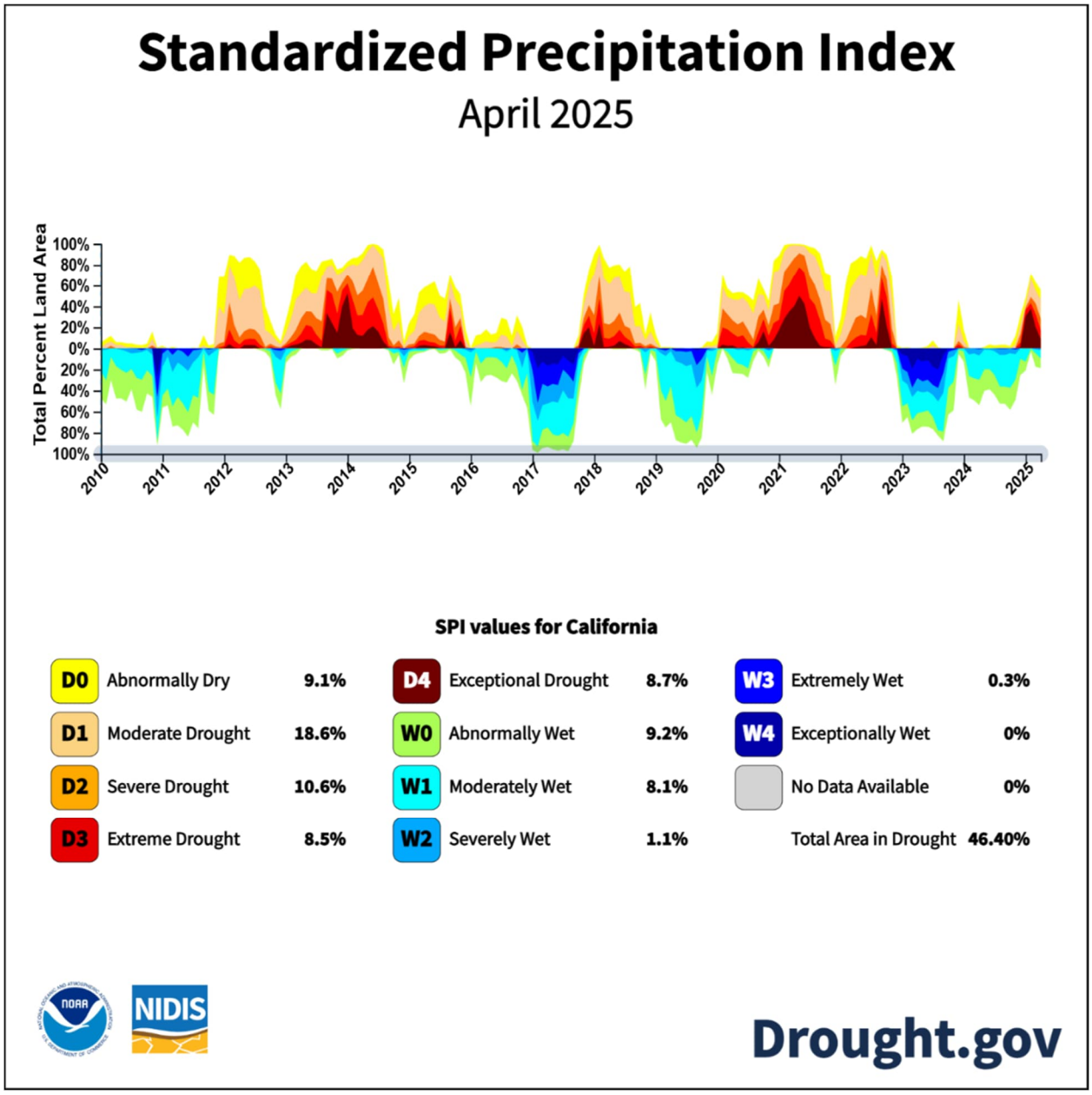


**Figure A.A9-11: Dry and wet conditions in Lake County, CA from 1970-79, including the severe 1976-77 drought**

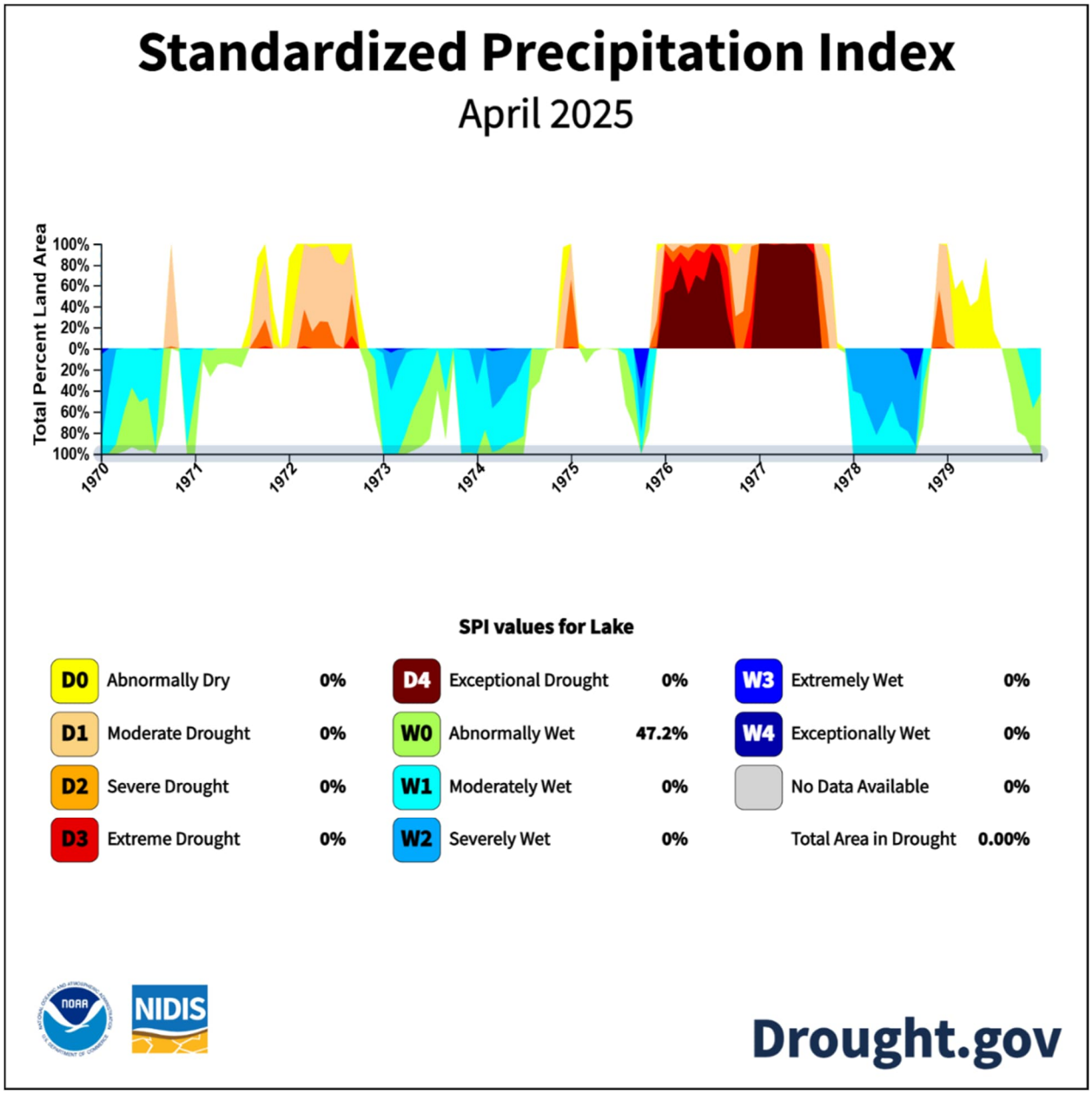


**Figure A.A9-12: Dry and wet conditions in Lake County, CA from 1928-34, including the dry period in 1924 (record low flow)**

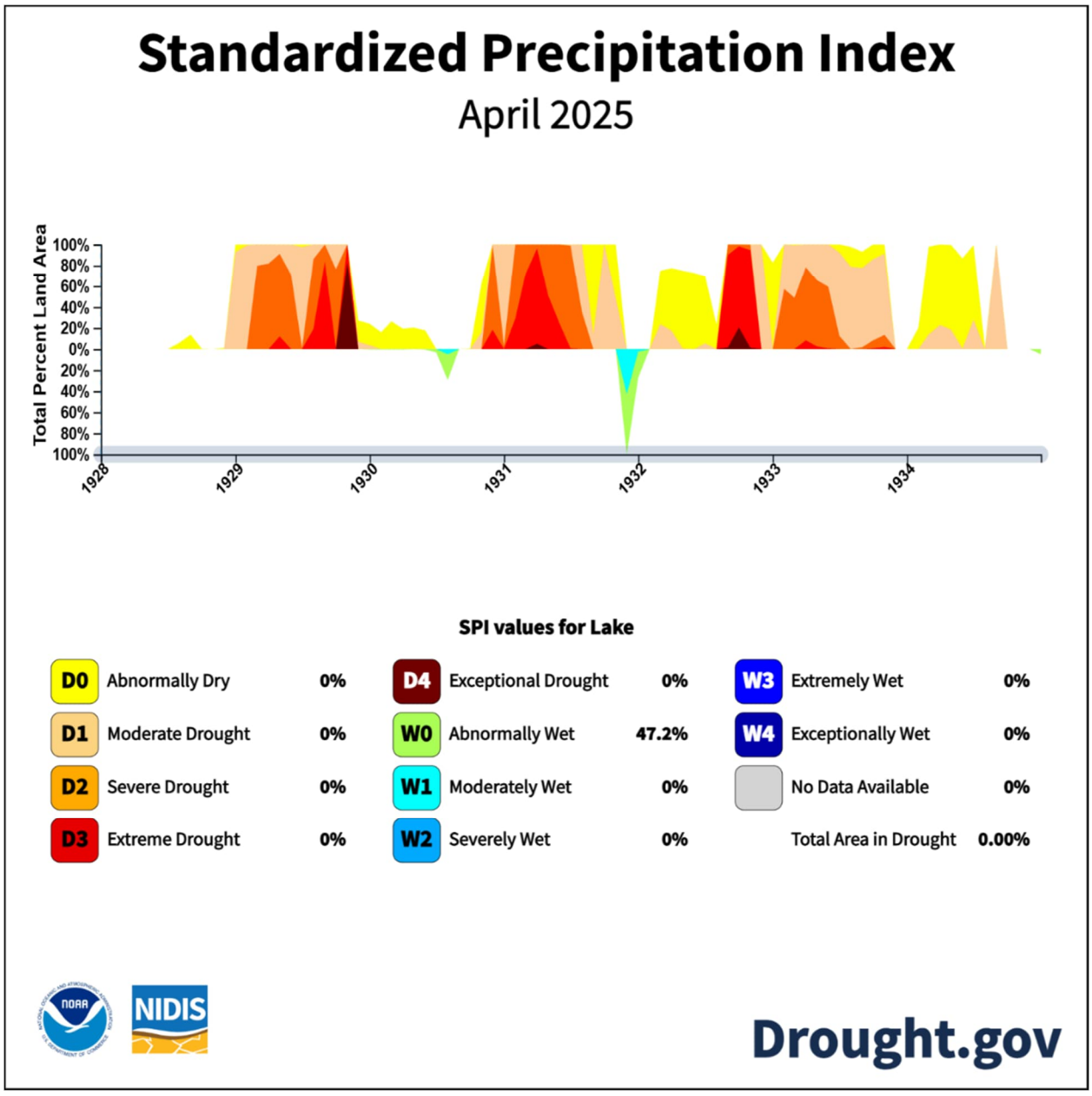
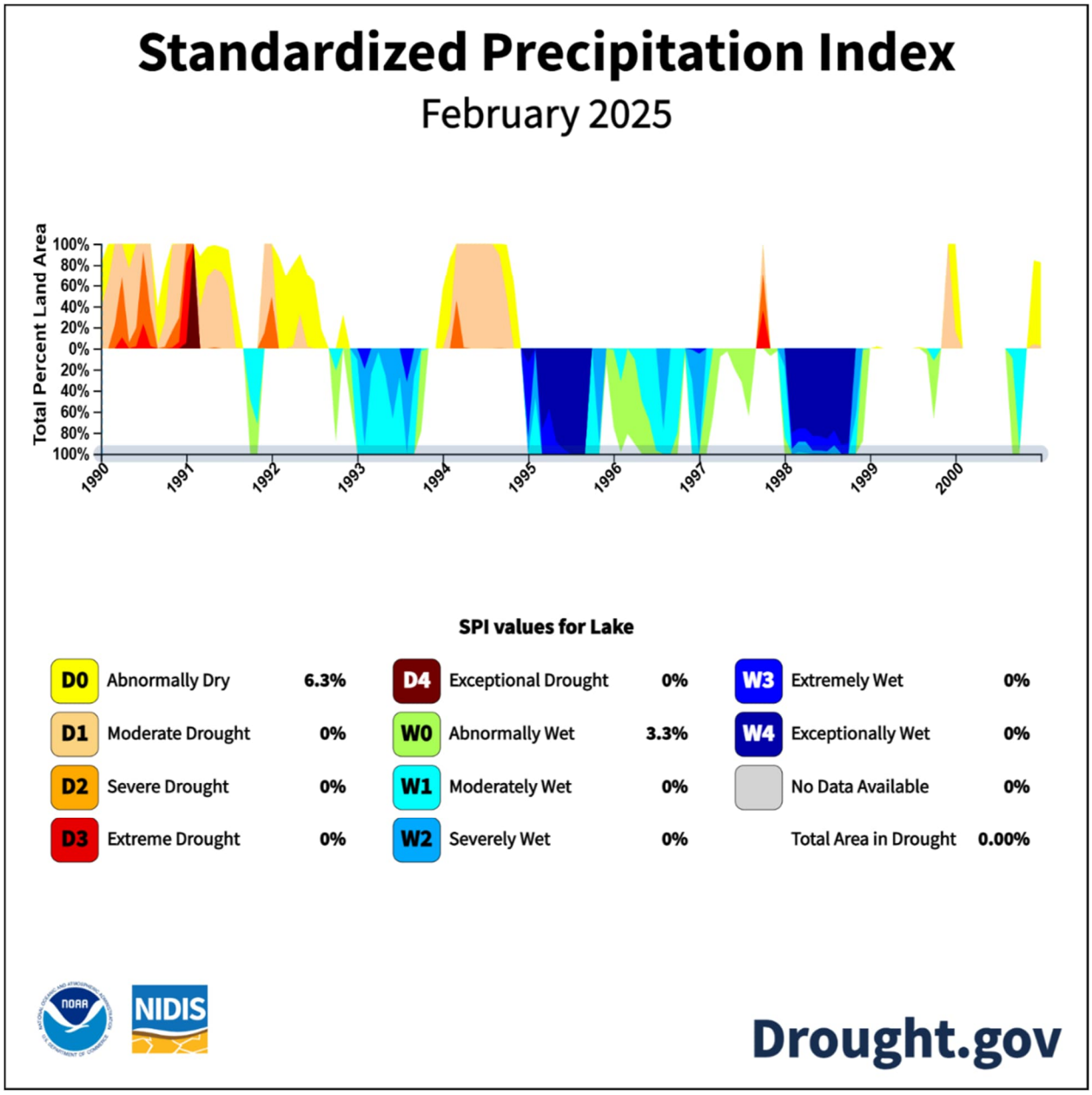


**Figure A.A9-13: Dry and wet conditions in Lake County, CA from 1990-2000 showing the 1995 flood**



# Appendix A.B PG&E Provided Data

## Appendix A - Hydrologic and Hydraulic Scenario Evaluation

Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/1/2005 | 56,469 | 1900.00 | -38 | 136 | 26 | 130 |
| 10/2/2005 | 51,200 | 1897.10 | 0 | 135 | 28 | 129 |
| 10/3/2005 | 51,000 | 1897.00 | 27 | 135 | 28 | 130 |
| 10/4/2005 | 50,700 | 1896.90 | -23 | 135 | 28 | 130 |
| 10/5/2005 | 50,500 | 1896.80 | 27 | 135 | 27 | 129 |
| 10/6/2005 | 50,200 | 1896.60 | -24 | 134 | 28 | 129 |
| 10/7/2005 | 50,000 | 1896.50 | 26 | 134 | 29 | 129 |
| 10/8/2005 | 49,700 | 1896.30 | -24 | 134 | 29 | 129 |
| 10/9/2005 | 49,500 | 1896.20 | 26 | 134 | 30 | 129 |
| 10/10/2005 | 49,200 | 1896.00 | -25 | 133 | 30 | 129 |
| 10/11/2005 | 48,900 | 1895.80 | -25 | 133 | 31 | 129 |
| 10/12/2005 | 48,667 | 1895.70 | 9 | 133 | 32 | 129 |
| 10/13/2005 | 48,433 | 1895.60 | 9 | 133 | 32 | 128 |
| 10/14/2005 | 48,200 | 1895.40 | 9 | 133 | 35 | 130 |
| 10/15/2005 | 47,967 | 1895.30 | 9 | 133 | 36 | 130 |
| 10/16/2005 | 47,733 | 1895.20 | 9 | 133 | 38 | 128 |
| 10/17/2005 | 47,500 | 1895.00 | 8 | 132 | 49 | 131 |
| 10/18/2005 | 47,200 | 1894.90 | -26 | 132 | 61 | 131 |
| 10/19/2005 | 47,000 | 1894.70 | 25 | 132 | 60 | 128 |
| 10/20/2005 | 46,800 | 1894.60 | 25 | 132 | 49 | 104 |
| 10/21/2005 | 46,500 | 1894.40 | -26 | 132 | 50 | 104 |
| 10/22/2005 | 46,300 | 1894.30 | 25 | 132 | 49 | 104 |
| 10/23/2005 | 46,000 | 1894.10 | -26 | 132 | 47 | 107 |
| 10/24/2005 | 45,800 | 1894.00 | 25 | 132 | 45 | 109 |
| 10/25/2005 | 45,600 | 1893.90 | 25 | 132 | 47 | 109 |
| 10/26/2005 | 45,400 | 1893.80 | 26 | 133 | 45 | 117 |
| 10/27/2005 | 45,200 | 1893.70 | 25 | 132 | 44 | 114 |
| 10/28/2005 | 45,100 | 1893.60 | 76 | 133 | 45 | 119 |
| 10/29/2005 | 44,900 | 1893.50 | 25 | 132 | 43 | 114 |
| 10/30/2005 | 44,700 | 1893.40 | 24 | 131 | 45 | 109 |
| 10/31/2005 | 44,500 | 1893.30 | 24 | 131 | 46 | 108 |
| 11/1/2005 | 44,200 | 1893.10 | -27 | 131 | 49 | 104 |
| 11/2/2005 | 44,000 | 1893.00 | 24 | 131 | 50 | 103 |
| 11/3/2005 | 43,900 | 1892.90 | 76 | 133 | 49 | 116 |
| 11/4/2005 | 43,800 | 1892.80 | 76 | 133 | 48 | 127 |
| 11/5/2005 | 43,600 | 1892.70 | 24 | 131 | 47 | 115 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/6/2005 | 43,400 | 1892.60 | 25 | 132 | 50 | 114 |
| 11/7/2005 | 44,300 | 1893.10 | 587 | 140 | 89 | 177 |
| 11/8/2005 | 44,800 | 1893.40 | 380 | 135 | 63 | 186 |
| 11/9/2005 | 44,800 | 1893.40 | 125 | 132 | 69 | 102 |
| 11/10/2005 | 44,700 | 1893.40 | 75 | 132 | 82 | 74 |
| 11/11/2005 | 44,600 | 1893.30 | 75 | 132 | 83 | 68 |
| 11/12/2005 | 44,400 | 1893.20 | 24 | 131 | 83 | 67 |
| 11/13/2005 | 44,200 | 1893.10 | 24 | 131 | 84 | 65 |
| 11/14/2005 | 44,100 | 1893.00 | 74 | 131 | 83 | 65 |
| 11/15/2005 | 43,900 | 1892.90 | 24 | 131 | 83 | 64 |
| 11/16/2005 | 43,700 | 1892.80 | 24 | 131 | 87 | 57 |
| 11/17/2005 | 43,500 | 1892.60 | 24 | 131 | 87 | 58 |
| 11/18/2005 | 43,300 | 1892.50 | 23 | 130 | 85 | 61 |
| 11/19/2005 | 43,100 | 1892.40 | 23 | 130 | 80 | 68 |
| 11/20/2005 | 43,000 | 1892.30 | 73 | 130 | 77 | 72 |
| 11/21/2005 | 42,800 | 1892.20 | 23 | 130 | 75 | 75 |
| 11/22/2005 | 42,600 | 1892.10 | 23 | 130 | 71 | 81 |
| 11/23/2005 | 42,400 | 1892.00 | 23 | 130 | 69 | 84 |
| 11/24/2005 | 42,200 | 1891.80 | 23 | 130 | 65 | 88 |
| 11/25/2005 | 42,200 | 1891.80 | 128 | 135 | 64 | 115 |
| 11/26/2005 | 42,100 | 1891.80 | 73 | 130 | 64 | 107 |
| 11/27/2005 | 42,000 | 1891.70 | 73 | 130 | 67 | 95 |
| 11/28/2005 | 42,900 | 1892.30 | 587 | 140 | 127 | 99 |
| 11/29/2005 | 43,900 | 1892.90 | 636 | 139 | 278 | 107 |
| 11/30/2005 | 44,300 | 1893.10 | 329 | 134 | 103 | 104 |
| 12/1/2005 | 53,500 | 1898.40 | 4,786 | 155 | 1,110 | 88 |
| 12/2/2005 | 55,500 | 1899.50 | 1,355 | 365 | 640 | 74 |
| 12/3/2005 | 55,400 | 1899.50 | 666 | 754 | 721 | 88 |
| 12/4/2005 | 55,100 | 1899.30 | 374 | 553 | 568 | 88 |
| 12/5/2005 | 54,900 | 1899.20 | 245 | 364 | 398 | 95 |
| 12/6/2005 | 54,800 | 1899.10 | 204 | 268 | 273 | 99 |
| 12/7/2005 | 54,700 | 1899.10 | 158 | 219 | 197 | 98 |
| 12/8/2005 | 54,600 | 1899.00 | 138 | 198 | 160 | 99 |
| 12/9/2005 | 54,600 | 1899.00 | 169 | 178 | 121 | 102 |
| 12/10/2005 | 54,500 | 1899.00 | 103 | 162 | 100 | 94 |
| 12/11/2005 | 54,400 | 1898.90 | 92 | 150 | 103 | 75 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/12/2005 | 54,400 | 1898.90 | 136 | 143 | 103 | 64 |
| 12/13/2005 | 54,300 | 1898.90 | 82 | 139 | 107 | 53 |
| 12/14/2005 | 54,200 | 1898.80 | 79 | 136 | 107 | 46 |
| 12/15/2005 | 54,100 | 1898.70 | 78 | 135 | 107 | 41 |
| 12/16/2005 | 54,000 | 1898.70 | 77 | 134 | 107 | 40 |
| 12/17/2005 | 54,000 | 1898.70 | 128 | 135 | 107 | 40 |
| 12/18/2005 | 56,200 | 1899.90 | 1,481 | 391 | 300 | 107 |
| 12/19/2005 | 59,000 | 1901.40 | 5,435 | 4,235 | 4,670 | 71 |
| 12/20/2005 | 58,500 | 1901.10 | 3,484 | 3,932 | 4,260 | 40 |
| 12/21/2005 | 60,300 | 1902.00 | 4,164 | 3,428 | 4,100 | 40 |
| 12/22/2005 | 61,400 | 1902.60 | 8,218 | 8,067 | 8,890 | 15 |
| 12/23/2005 | 58,300 | 1901.00 | 3,466 | 5,294 | 6,570 | 24 |
| 12/24/2005 | 57,000 | 1900.30 | 1,548 | 2,319 | 2,940 | 41 |
| 12/25/2005 | 56,900 | 1900.30 | 1,386 | 1,512 | 1,910 | 41 |
| 12/26/2005 | 58,400 | 1901.00 | 3,869 | 3,277 | 3,820 | 41 |
| 12/27/2005 | 62,300 | 1903.00 | 6,277 | 4,537 | 5,370 | 33 |
| 12/28/2005 | 64,100 | 1903.90 | 17,192 | 17,141 | 18,500 | 0 |
| 12/29/2005 | 59,700 | 1901.70 | 4,966 | 7,562 | 9,450 | 0 |
| 12/30/2005 | 67,300 | 1905.50 | 14,129 | 10,839 | 11,700 | 10 |
| 12/31/2005 | 65,800 | 1904.80 | 29,897 | 32,266 | 39,800 | 0 |
| 1/1/2006 | 63,300 | 1903.50 | 10,474 | 12,352 | 16,100 | 0 |
| 1/2/2006 | 60,300 | 1902.00 | 6,869 | 8,823 | 11,100 | 0 |
| 1/3/2006 | 59,700 | 1901.70 | 4,870 | 5,445 | 7,000 | 22 |
| 1/4/2006 | 59,300 | 1901.50 | 4,348 | 4,790 | 6,020 | 45 |
| 1/5/2006 | 57,900 | 1900.80 | 2,407 | 3,277 | 4,140 | 42 |
| 1/6/2006 | 57,400 | 1900.50 | 1,951 | 2,319 | 2,950 | 41 |
| 1/7/2006 | 57,000 | 1900.30 | 1,523 | 1,815 | 2,250 | 40 |
| 1/8/2006 | 56,700 | 1900.20 | 1,283 | 1,510 | 1,750 | 41 |
| 1/9/2006 | 56,400 | 1900.00 | 989 | 1,200 | 1,390 | 40 |
| 1/10/2006 | 56,300 | 1899.90 | 909 | 1,010 | 1,150 | 51 |
| 1/11/2006 | 56,500 | 1900.00 | 1,231 | 1,190 | 1,350 | 70 |
| 1/12/2006 | 56,300 | 1899.90 | 1,039 | 1,200 | 1,260 | 165 |
| 1/13/2006 | 56,200 | 1899.90 | 1,137 | 1,250 | 1,220 | 265 |
| 1/14/2006 | 56,600 | 1900.10 | 1,437 | 1,300 | 1,280 | 265 |
| 1/15/2006 | 56,300 | 1899.90 | 1,027 | 1,240 | 1,220 | 265 |
| 1/16/2006 | 56,200 | 1899.90 | 1,080 | 1,190 | 1,160 | 265 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/17/2006 | 57,300 | 1900.50 | 1,913 | 1,430 | 1,440 | 265 |
| 1/18/2006 | 58,200 | 1900.90 | 3,711 | 3,428 | 3,910 | 264 |
| 1/19/2006 | 57,400 | 1900.50 | 1,877 | 2,400 | 2,600 | 264 |
| 1/20/2006 | 57,100 | 1900.40 | 1,483 | 1,720 | 1,780 | 264 |
| 1/21/2006 | 56,800 | 1900.20 | 1,758 | 2,010 | 2,120 | 263 |
| 1/22/2006 | 56,500 | 1900.00 | 1,749 | 2,000 | 2,120 | 263 |
| 1/23/2006 | 56,300 | 1899.90 | 1,771 | 1,970 | 2,080 | 264 |
| 1/24/2006 | 56,200 | 1899.90 | 1,821 | 1,970 | 2,090 | 264 |
| 1/25/2006 | 56,100 | 1899.80 | 1,888 | 2,040 | 2,160 | 265 |
| 1/26/2006 | 56,000 | 1899.80 | 1,935 | 2,090 | 2,220 | 265 |
| 1/27/2006 | 55,900 | 1899.70 | 1,935 | 2,090 | 2,240 | 246 |
| 1/28/2006 | 56,400 | 1900.00 | 2,257 | 2,110 | 2,250 | 262 |
| 1/29/2006 | 56,400 | 1900.00 | 2,071 | 2,180 | 2,340 | 262 |
| 1/30/2006 | 57,500 | 1900.60 | 2,683 | 2,240 | 2,410 | 261 |
| 1/31/2006 | 57,000 | 1900.30 | 2,047 | 2,420 | 2,620 | 260 |
| 2/1/2006 | 58,900 | 1901.30 | 3,827 | 3,020 | 3,330 | 260 |
| 2/2/2006 | 58,900 | 1901.30 | 4,437 | 4,670 | 5,940 | 127 |
| 2/3/2006 | 58,200 | 1900.90 | 2,953 | 3,480 | 4,390 | 178 |
| 2/4/2006 | 57,700 | 1900.70 | 2,446 | 2,840 | 3,500 | 265 |
| 2/5/2006 | 57,200 | 1900.40 | 1,971 | 2,340 | 2,960 | 265 |
| 2/6/2006 | 56,800 | 1900.20 | 1,622 | 1,920 | 2,370 | 263 |
| 2/7/2006 | 56,500 | 1900.00 | 1,416 | 1,650 | 2,020 | 263 |
| 2/8/2006 | 56,300 | 1899.90 | 1,296 | 1,470 | 1,930 | 262 |
| 2/9/2006 | 56,100 | 1899.80 | 1,182 | 1,350 | 1,810 | 262 |
| 2/10/2006 | 56,000 | 1899.80 | 1,118 | 1,230 | 1,600 | 262 |
| 2/11/2006 | 55,900 | 1899.70 | 1,014 | 1,120 | 1,410 | 262 |
| 2/12/2006 | 55,700 | 1899.60 | 878 | 1,030 | 1,260 | 262 |
| 2/13/2006 | 55,600 | 1899.60 | 857 | 955 | 1,170 | 262 |
| 2/14/2006 | 55,600 | 1899.60 | 841 | 885 | 1,140 | 262 |
| 2/15/2006 | 55,500 | 1899.50 | 725 | 816 | 1,110 | 262 |
| 2/16/2006 | 55,400 | 1899.50 | 673 | 761 | 1,080 | 262 |
| 2/17/2006 | 55,400 | 1899.50 | 686 | 722 | 1,060 | 263 |
| 2/18/2006 | 55,400 | 1899.50 | 671 | 706 | 1,030 | 263 |
| 2/19/2006 | 55,300 | 1899.40 | 570 | 653 | 1,250 | 66 |
| 2/20/2006 | 55,300 | 1899.40 | 571 | 601 | 1,160 | 96 |
| 2/21/2006 | 55,200 | 1899.30 | 482 | 560 | 1,030 | 163 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/22/2006 | 55,200 | 1899.30 | 494 | 520 | 1,050 | 202 |
| 2/23/2006 | 55,200 | 1899.30 | 463 | 487 | 1,040 | 265 |
| 2/24/2006 | 55,100 | 1899.30 | 386 | 459 | 1,050 | 265 |
| 2/25/2006 | 55,100 | 1899.30 | 411 | 433 | 1,170 | 266 |
| 2/26/2006 | 55,200 | 1899.30 | 455 | 426 | 1,370 | 266 |
| 2/27/2006 | 59,600 | 1901.70 | 2,937 | 756 | 1,840 | 46 |
| 2/28/2006 | 59,200 | 1901.50 | 2,672 | 3,025 | 3,860 | 78 |
| 3/1/2006 | 58,200 | 1900.90 | 1,651 | 2,269 | 2,870 | 162 |
| 3/2/2006 | 57,600 | 1900.60 | 1,374 | 1,765 | 2,120 | 216 |
| 3/3/2006 | 57,300 | 1900.50 | 1,286 | 1,512 | 1,960 | 265 |
| 3/4/2006 | 56,800 | 1900.20 | 1,041 | 1,361 | 1,590 | 265 |
| 3/5/2006 | 59,400 | 1901.60 | 2,748 | 1,512 | 1,930 | 201 |
| 3/6/2006 | 58,900 | 1901.30 | 2,861 | 3,277 | 3,880 | 30 |
| 3/7/2006 | 58,300 | 1901.00 | 2,092 | 2,521 | 3,850 | 108 |
| 3/8/2006 | 57,500 | 1900.60 | 2,470 | 3,025 | 3,910 | 265 |
| 3/9/2006 | 57,200 | 1900.40 | 1,765 | 2,017 | 2,560 | 264 |
| 3/10/2006 | 56,800 | 1900.20 | 1,331 | 1,613 | 2,080 | 264 |
| 3/11/2006 | 56,600 | 1900.10 | 1,336 | 1,512 | 1,710 | 263 |
| 3/12/2006 | 56,400 | 1900.00 | 1,192 | 1,361 | 1,590 | 97 |
| 3/13/2006 | 56,400 | 1900.00 | 1,173 | 1,235 | 1,470 | 0 |
| 3/14/2006 | 56,800 | 1900.20 | 1,567 | 1,437 | 1,790 | 0 |
| 3/15/2006 | 56,600 | 1900.10 | 1,336 | 1,512 | 1,880 | 0 |
| 3/16/2006 | 58,000 | 1900.80 | 2,382 | 1,765 | 2,360 | 0 |
| 3/17/2006 | 57,600 | 1900.60 | 2,241 | 2,571 | 3,560 | 0 |
| 3/18/2006 | 57,000 | 1900.30 | 1,565 | 1,966 | 2,610 | 20 |
| 3/19/2006 | 56,700 | 1900.20 | 1,405 | 1,639 | 1,950 | 167 |
| 3/20/2006 | 56,600 | 1900.10 | 1,386 | 1,512 | 1,750 | 167 |
| 3/21/2006 | 56,400 | 1900.00 | 1,240 | 1,412 | 1,570 | 165 |
| 3/22/2006 | 56,300 | 1899.90 | 1,147 | 1,260 | 1,340 | 165 |
| 3/23/2006 | 56,300 | 1899.90 | 1,102 | 1,160 | 1,220 | 165 |
| 3/24/2006 | 57,300 | 1900.50 | 2,037 | 1,613 | 1,740 | 165 |
| 3/25/2006 | 58,300 | 1901.00 | 2,995 | 2,622 | 3,070 | 165 |
| 3/26/2006 | 57,600 | 1900.60 | 2,090 | 2,571 | 3,130 | 164 |
| 3/27/2006 | 57,300 | 1900.50 | 1,765 | 2,017 | 2,350 | 165 |
| 3/28/2006 | 57,300 | 1900.50 | 1,820 | 1,916 | 2,190 | 164 |
| 3/29/2006 | 57,200 | 1900.40 | 1,770 | 1,916 | 2,180 | 164 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/30/2006 | 57,000 | 1900.30 | 1,623 | 1,815 | 1,960 | 165 |
| 3/31/2006 | 57,800 | 1900.70 | 2,223 | 1,916 | 2,170 | 215 |
| 4/1/2006 | 58,000 | 1900.80 | 2,496 | 2,521 | 2,930 | 266 |
| 4/2/2006 | 58,000 | 1900.80 | 2,395 | 2,521 | 2,920 | 265 |
| 4/3/2006 | 59,400 | 1901.60 | 4,537 | 4,033 | 4,910 | 114 |
| 4/4/2006 | 58,400 | 1901.00 | 3,088 | 3,781 | 4,620 | 28 |
| 4/5/2006 | 57,800 | 1900.70 | 2,380 | 2,823 | 3,450 | 47 |
| 4/6/2006 | 57,400 | 1900.50 | 1,954 | 2,269 | 2,620 | 47 |
| 4/7/2006 | 57,200 | 1900.40 | 1,719 | 1,916 | 2,190 | 47 |
| 4/8/2006 | 57,100 | 1900.40 | 1,674 | 1,815 | 2,030 | 107 |
| 4/9/2006 | 57,100 | 1900.40 | 1,676 | 1,765 | 1,860 | 266 |
| 4/10/2006 | 58,400 | 1901.00 | 2,044 | 1,462 | 1,490 | 265 |
| 4/11/2006 | 60,500 | 1902.10 | 2,160 | 1,160 | 1,160 | 265 |
| 4/12/2006 | 67,000 | 1905.40 | 6,151 | 3,025 | 3,520 | 200 |
| 4/13/2006 | 66,000 | 1904.90 | 3,327 | 4,033 | 4,770 | 144 |
| 4/14/2006 | 64,300 | 1904.00 | 2,256 | 3,277 | 3,800 | 263 |
| 4/15/2006 | 63,900 | 1903.80 | 2,433 | 2,773 | 3,150 | 222 |
| 4/16/2006 | 64,100 | 1903.90 | 2,975 | 3,025 | 3,620 | 100 |
| 4/17/2006 | 63,800 | 1903.80 | 2,244 | 2,521 | 2,780 | 214 |
| 4/18/2006 | 63,900 | 1903.80 | 1,775 | 1,815 | 1,940 | 188 |
| 4/19/2006 | 64,400 | 1904.10 | 1,689 | 1,512 | 1,500 | 215 |
| 4/20/2006 | 65,500 | 1904.60 | 1,704 | 1,210 | 1,060 | 266 |
| 4/21/2006 | 66,400 | 1905.10 | 1,747 | 1,361 | 1,190 | 266 |
| 4/22/2006 | 66,500 | 1905.10 | 1,727 | 1,765 | 1,590 | 266 |
| 4/23/2006 | 66,000 | 1904.90 | 1,520 | 1,865 | 1,720 | 266 |
| 4/24/2006 | 65,800 | 1904.80 | 1,480 | 1,664 | 1,520 | 266 |
| 4/25/2006 | 65,900 | 1904.80 | 1,391 | 1,412 | 1,310 | 265 |
| 4/26/2006 | 66,000 | 1904.90 | 1,368 | 1,386 | 1,330 | 258 |
| 4/27/2006 | 66,300 | 1905.00 | 1,492 | 1,412 | 1,430 | 264 |
| 4/28/2006 | 67,000 | 1905.40 | 1,790 | 1,512 | 1,520 | 263 |
| 4/29/2006 | 67,600 | 1905.60 | 1,835 | 1,613 | 1,670 | 264 |
| 4/30/2006 | 67,600 | 1905.60 | 1,676 | 1,765 | 1,830 | 264 |
| 5/1/2006 | 67,300 | 1905.50 | 1,477 | 1,714 | 1,800 | 264 |
| 5/2/2006 | 67,800 | 1905.70 | 1,689 | 1,512 | 1,600 | 264 |
| 5/3/2006 | 68,800 | 1906.20 | 1,797 | 1,361 | 1,460 | 263 |
| 5/4/2006 | 69,500 | 1906.60 | 1,359 | 1,059 | 1,100 | 262 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/5/2006 | 70,100 | 1906.80 | 1,165 | 907 | 779 | 266 |
| 5/6/2006 | 70,500 | 1907.00 | 1,064 | 907 | 788 | 267 |
| 5/7/2006 | 70,500 | 1907.00 | 934 | 983 | 854 | 266 |
| 5/8/2006 | 70,300 | 1906.90 | 905 | 1,059 | 909 | 266 |
| 5/9/2006 | 70,200 | 1906.90 | 907 | 1,008 | 857 | 249 |
| 5/10/2006 | 70,300 | 1906.90 | 913 | 907 | 722 | 268 |
| 5/11/2006 | 70,800 | 1907.20 | 875 | 655 | 518 | 256 |
| 5/12/2006 | 71,500 | 1907.50 | 785 | 455 | 370 | 192 |
| 5/13/2006 | 72,100 | 1907.80 | 744 | 465 | 386 | 177 |
| 5/14/2006 | 72,700 | 1908.10 | 729 | 449 | 378 | 177 |
| 5/15/2006 | 73,200 | 1908.30 | 705 | 477 | 370 | 177 |
| 5/16/2006 | 73,600 | 1908.50 | 729 | 555 | 453 | 190 |
| 5/17/2006 | 73,700 | 1908.50 | 625 | 605 | 478 | 207 |
| 5/18/2006 | 73,600 | 1908.50 | 596 | 681 | 535 | 220 |
| 5/19/2006 | 73,300 | 1908.30 | 567 | 756 | 571 | 249 |
| 5/20/2006 | 73,000 | 1908.20 | 615 | 807 | 633 | 266 |
| 5/21/2006 | 72,700 | 1908.10 | 567 | 756 | 608 | 241 |
| 5/22/2006 | 72,700 | 1908.10 | 623 | 655 | 488 | 254 |
| 5/23/2006 | 72,600 | 1908.00 | 524 | 605 | 467 | 229 |
| 5/24/2006 | 72,500 | 1908.00 | 462 | 539 | 372 | 242 |
| 5/25/2006 | 72,500 | 1908.00 | 450 | 474 | 375 | 175 |
| 5/26/2006 | 72,400 | 1907.90 | 399 | 473 | 371 | 177 |
| 5/27/2006 | 72,300 | 1907.90 | 397 | 471 | 367 | 177 |
| 5/28/2006 | 72,200 | 1907.80 | 396 | 470 | 364 | 177 |
| 5/29/2006 | 72,000 | 1907.70 | 344 | 468 | 362 | 177 |
| 5/30/2006 | 71,800 | 1907.60 | 342 | 466 | 359 | 177 |
| 5/31/2006 | 71,500 | 1907.50 | 289 | 463 | 358 | 175 |
| 6/1/2006 | 71,400 | 1907.40 | 387 | 460 | 357 | 174 |
| 6/2/2006 | 71,100 | 1907.30 | 282 | 456 | 334 | 191 |
| 6/3/2006 | 71,000 | 1907.30 | 315 | 385 | 285 | 190 |
| 6/4/2006 | 70,900 | 1907.20 | 269 | 336 | 298 | 134 |
| 6/5/2006 | 70,800 | 1907.20 | 216 | 280 | 230 | 133 |
| 6/6/2006 | 70,600 | 1907.10 | 163 | 278 | 222 | 133 |
| 6/7/2006 | 70,300 | 1906.90 | 113 | 278 | 218 | 134 |
| 6/8/2006 | 70,200 | 1906.90 | 213 | 277 | 216 | 134 |
| 6/9/2006 | 70,000 | 1906.80 | 161 | 276 | 214 | 133 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/10/2006 | 69,900 | 1906.70 | 211 | 275 | 216 | 132 |
| 6/11/2006 | 69,800 | 1906.70 | 210 | 274 | 201 | 142 |
| 6/12/2006 | 69,600 | 1906.60 | 159 | 273 | 182 | 155 |
| 6/13/2006 | 69,400 | 1906.50 | 140 | 254 | 167 | 155 |
| 6/14/2006 | 69,300 | 1906.50 | 176 | 238 | 137 | 155 |
| 6/15/2006 | 69,200 | 1906.40 | 141 | 201 | 125 | 148 |
| 6/16/2006 | 69,100 | 1906.40 | 131 | 191 | 99 | 128 |
| 6/17/2006 | 69,000 | 1906.30 | 134 | 194 | 107 | 132 |
| 6/18/2006 | 68,900 | 1906.30 | 131 | 191 | 95 | 134 |
| 6/19/2006 | 68,800 | 1906.20 | 130 | 190 | 92 | 134 |
| 6/20/2006 | 68,700 | 1906.20 | 128 | 188 | 89 | 134 |
| 6/21/2006 | 68,500 | 1906.10 | 76 | 186 | 84 | 134 |
| 6/22/2006 | 68,300 | 1906.00 | 92 | 203 | 92 | 134 |
| 6/23/2006 | 68,100 | 1905.90 | 102 | 213 | 118 | 134 |
| 6/24/2006 | 68,000 | 1905.80 | 148 | 209 | 112 | 134 |
| 6/25/2006 | 67,700 | 1905.70 | 37 | 198 | 97 | 134 |
| 6/26/2006 | 67,500 | 1905.60 | 86 | 197 | 91 | 134 |
| 6/27/2006 | 67,200 | 1905.40 | 33 | 194 | 88 | 134 |
| 6/28/2006 | 67,000 | 1905.40 | 81 | 191 | 85 | 134 |
| 6/29/2006 | 66,800 | 1905.30 | 79 | 189 | 74 | 133 |
| 6/30/2006 | 66,600 | 1905.20 | 86 | 197 | 76 | 133 |
| 7/1/2006 | 66,300 | 1905.00 | 22 | 182 | 52 | 138 |
| 7/2/2006 | 66,100 | 1904.90 | 70 | 180 | 43 | 141 |
| 7/3/2006 | 65,900 | 1904.80 | 70 | 180 | 37 | 142 |
| 7/4/2006 | 65,700 | 1904.70 | 70 | 180 | 36 | 143 |
| 7/5/2006 | 65,300 | 1904.50 | -31 | 180 | 36 | 142 |
| 7/6/2006 | 65,100 | 1904.40 | 70 | 180 | 37 | 142 |
| 7/7/2006 | 64,800 | 1904.30 | 19 | 179 | 39 | 139 |
| 7/8/2006 | 64,600 | 1904.20 | 69 | 179 | 38 | 140 |
| 7/9/2006 | 64,400 | 1904.10 | 69 | 179 | 37 | 141 |
| 7/10/2006 | 64,200 | 1904.00 | 69 | 179 | 35 | 141 |
| 7/11/2006 | 63,900 | 1903.80 | 19 | 179 | 36 | 136 |
| 7/12/2006 | 63,600 | 1903.70 | 19 | 179 | 36 | 137 |
| 7/13/2006 | 63,300 | 1903.50 | 19 | 179 | 34 | 140 |
| 7/14/2006 | 63,100 | 1903.40 | 69 | 179 | 32 | 141 |
| 7/15/2006 | 62,700 | 1903.20 | -32 | 179 | 30 | 140 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/16/2006 | 62,600 | 1903.20 | 120 | 179 | 33 | 139 |
| 7/17/2006 | 62,200 | 1903.00 | -32 | 179 | 37 | 133 |
| 7/18/2006 | 61,900 | 1902.80 | 19 | 179 | 39 | 133 |
| 7/19/2006 | 61,600 | 1902.70 | 20 | 180 | 39 | 133 |
| 7/20/2006 | 61,400 | 1902.60 | 70 | 180 | 40 | 133 |
| 7/21/2006 | 61,100 | 1902.40 | 20 | 180 | 40 | 135 |
| 7/22/2006 | 60,800 | 1902.30 | 20 | 180 | 39 | 133 |
| 7/23/2006 | 60,500 | 1902.10 | 19 | 179 | 35 | 133 |
| 7/24/2006 | 60,200 | 1902.00 | 19 | 179 | 36 | 132 |
| 7/25/2006 | 59,900 | 1901.80 | 19 | 179 | 36 | 129 |
| 7/26/2006 | 59,900 | 1901.80 | 171 | 180 | 35 | 130 |
| 7/27/2006 | 59,600 | 1901.70 | 18 | 178 | 35 | 132 |
| 7/28/2006 | 59,200 | 1901.50 | -33 | 178 | 36 | 132 |
| 7/29/2006 | 58,900 | 1901.30 | 17 | 177 | 38 | 131 |
| 7/30/2006 | 58,600 | 1901.10 | 17 | 177 | 39 | 132 |
| 7/31/2006 | 58,300 | 1901.00 | 17 | 177 | 40 | 133 |
| 8/1/2006 | 58,000 | 1900.80 | 17 | 177 | 41 | 132 |
| 8/2/2006 | 57,700 | 1900.70 | 19 | 179 | 41 | 131 |
| 8/3/2006 | 57,400 | 1900.50 | 21 | 181 | 42 | 132 |
| 8/4/2006 | 57,000 | 1900.30 | -31 | 180 | 44 | 132 |
| 8/5/2006 | 56,700 | 1900.20 | 20 | 180 | 44 | 132 |
| 8/6/2006 | 56,400 | 1900.00 | 20 | 180 | 43 | 131 |
| 8/7/2006 | 56,100 | 1899.80 | 20 | 180 | 43 | 132 |
| 8/8/2006 | 55,800 | 1899.70 | 19 | 179 | 44 | 131 |
| 8/9/2006 | 55,500 | 1899.50 | 19 | 179 | 43 | 132 |
| 8/10/2006 | 55,100 | 1899.30 | -29 | 182 | 43 | 131 |
| 8/11/2006 | 54,800 | 1899.10 | 25 | 185 | 43 | 133 |
| 8/12/2006 | 54,500 | 1899.00 | 25 | 185 | 43 | 134 |
| 8/13/2006 | 54,200 | 1898.80 | 25 | 185 | 42 | 134 |
| 8/14/2006 | 53,900 | 1898.60 | 25 | 185 | 42 | 135 |
| 8/15/2006 | 53,500 | 1898.40 | -27 | 184 | 41 | 135 |
| 8/16/2006 | 53,200 | 1898.30 | 24 | 184 | 40 | 134 |
| 8/17/2006 | 52,900 | 1898.10 | 23 | 183 | 40 | 135 |
| 8/18/2006 | 52,600 | 1897.90 | 23 | 183 | 40 | 134 |
| 8/19/2006 | 52,200 | 1897.70 | -29 | 182 | 40 | 134 |
| 8/20/2006 | 51,900 | 1897.50 | 21 | 181 | 40 | 134 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/21/2006 | 51,600 | 1897.40 | 20 | 180 | 40 | 134 |
| 8/22/2006 | 51,300 | 1897.20 | 20 | 180 | 39 | 134 |
| 8/23/2006 | 51,000 | 1897.00 | 19 | 179 | 39 | 134 |
| 8/24/2006 | 50,600 | 1896.80 | -32 | 179 | 39 | 134 |
| 8/25/2006 | 50,300 | 1896.60 | 18 | 178 | 38 | 134 |
| 8/26/2006 | 50,000 | 1896.50 | 17 | 177 | 40 | 134 |
| 8/27/2006 | 49,700 | 1896.30 | 17 | 177 | 41 | 134 |
| 8/28/2006 | 49,300 | 1896.10 | -34 | 176 | 39 | 134 |
| 8/29/2006 | 49,000 | 1895.90 | 18 | 178 | 39 | 133 |
| 8/30/2006 | 48,700 | 1895.70 | 21 | 181 | 38 | 140 |
| 8/31/2006 | 48,400 | 1895.60 | 21 | 181 | 37 | 136 |
| 9/1/2006 | 48,000 | 1895.30 | -31 | 180 | 39 | 134 |
| 9/2/2006 | 47,700 | 1895.10 | 21 | 181 | 38 | 136 |
| 9/3/2006 | 47,400 | 1895.00 | 21 | 181 | 38 | 135 |
| 9/4/2006 | 47,000 | 1894.70 | -30 | 181 | 40 | 134 |
| 9/5/2006 | 46,700 | 1894.60 | 21 | 181 | 39 | 126 |
| 9/6/2006 | 46,400 | 1894.40 | 21 | 181 | 40 | 129 |
| 9/7/2006 | 46,100 | 1894.20 | 22 | 182 | 41 | 132 |
| 9/8/2006 | 45,700 | 1894.00 | -30 | 181 | 41 | 135 |
| 9/9/2006 | 45,400 | 1893.80 | 25 | 186 | 42 | 135 |
| 9/10/2006 | 45,100 | 1893.60 | 26 | 187 | 41 | 135 |
| 9/11/2006 | 44,800 | 1893.40 | 28 | 189 | 40 | 135 |
| 9/12/2006 | 44,400 | 1893.20 | -21 | 190 | 41 | 136 |
| 9/13/2006 | 44,100 | 1893.00 | 25 | 185 | 41 | 137 |
| 9/14/2006 | 43,700 | 1892.80 | -22 | 189 | 39 | 136 |
| 9/15/2006 | 43,400 | 1892.60 | 28 | 189 | 40 | 137 |
| 9/16/2006 | 43,100 | 1892.40 | 1 | 160 | 43 | 115 |
| 9/17/2006 | 42,900 | 1892.30 | 24 | 131 | 40 | 99 |
| 9/18/2006 | 42,700 | 1892.20 | 23 | 130 | 39 | 98 |
| 9/19/2006 | 42,400 | 1892.00 | -26 | 132 | 39 | 92 |
| 9/20/2006 | 42,100 | 1891.80 | -5 | 154 | 39 | 94 |
| 9/21/2006 | 41,800 | 1891.60 | 22 | 182 | 42 | 115 |
| 9/22/2006 | 41,400 | 1891.30 | -11 | 201 | 41 | 137 |
| 9/23/2006 | 41,000 | 1891.10 | -11 | 200 | 42 | 148 |
| 9/24/2006 | 40,600 | 1890.80 | -12 | 200 | 41 | 168 |
| 9/25/2006 | 40,200 | 1890.60 | -13 | 199 | 41 | 171 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/26/2006 | 39,800 | 1890.30 | -13 | 198 | 40 | 207 |
| 9/27/2006 | 39,300 | 1890.00 | -64 | 198 | 40 | 204 |
| 9/28/2006 | 38,900 | 1889.70 | -15 | 197 | 41 | 202 |
| 9/29/2006 | 38,500 | 1889.50 | -15 | 196 | 42 | 169 |
| 9/30/2006 | 38,100 | 1889.20 | -16 | 196 | 42 | 161 |
| 10/1/2006 | 37,807 | 1889.00 | 38 | 195 | 40 | 154 |
| 10/2/2006 | 37,462 | 1888.80 | 14 | 198 | 40 | 153 |
| 10/3/2006 | 37,125 | 1888.60 | 22 | 202 | 43 | 172 |
| 10/4/2006 | 36,783 | 1888.30 | 19 | 201 | 43 | 198 |
| 10/5/2006 | 36,454 | 1888.10 | 26 | 202 | 43 | 171 |
| 10/6/2006 | 36,117 | 1887.90 | 21 | 201 | 43 | 157 |
| 10/7/2006 | 35,770 | 1887.70 | 17 | 202 | 43 | 159 |
| 10/8/2006 | 35,452 | 1887.40 | 32 | 202 | 43 | 159 |
| 10/9/2006 | 35,092 | 1887.20 | 16 | 208 | 36 | 158 |
| 10/10/2006 | 34,717 | 1886.90 | 15 | 214 | 29 | 163 |
| 10/11/2006 | 34,318 | 1886.60 | 8 | 221 | 34 | 185 |
| 10/12/2006 | 33,883 | 1886.30 | -4 | 227 | 42 | 202 |
| 10/13/2006 | 33,478 | 1886.00 | 12 | 228 | 40 | 202 |
| 10/14/2006 | 33,044 | 1885.70 | -2 | 229 | 38 | 201 |
| 10/15/2006 | 32,619 | 1885.40 | 4 | 230 | 37 | 201 |
| 10/16/2006 | 32,251 | 1885.10 | 33 | 230 | 37 | 203 |
| 10/17/2006 | 31,775 | 1884.80 | -20 | 231 | 39 | 198 |
| 10/18/2006 | 31,361 | 1884.50 | 12 | 232 | 36 | 201 |
| 10/19/2006 | 30,960 | 1884.20 | 19 | 233 | 37 | 198 |
| 10/20/2006 | 30,512 | 1883.80 | -3 | 234 | 39 | 206 |
| 10/21/2006 | 30,109 | 1883.50 | 20 | 235 | 42 | 204 |
| 10/22/2006 | 29,679 | 1883.20 | 7 | 236 | 45 | 206 |
| 10/23/2006 | 29,269 | 1882.90 | 18 | 237 | 43 | 207 |
| 10/24/2006 | 28,813 | 1882.50 | -4 | 238 | 46 | 201 |
| 10/25/2006 | 28,426 | 1882.20 | 32 | 239 | 47 | 205 |
| 10/26/2006 | 28,005 | 1881.90 | 15 | 239 | 51 | 197 |
| 10/27/2006 | 27,707 | 1881.60 | -7 | 151 | 71 | 148 |
| 10/28/2006 | 27,380 | 1881.30 | 16 | 190 | 44 | 151 |
| 10/29/2006 | 27,069 | 1881.10 | 23 | 189 | 45 | 150 |
| 10/30/2006 | 26,747 | 1880.80 | 17 | 189 | 45 | 151 |
| 10/31/2006 | 26,400 | 1880.50 | -46 | 136 | 44 | 125 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/1/2006 | 26,308 | 1880.40 | 73 | 126 | 45 | 46 |
| 11/2/2006 | 26,331 | 1880.50 | 95 | 88 | 43 | 44 |
| 11/3/2006 | 26,263 | 1880.40 | 40 | 78 | 41 | 43 |
| 11/4/2006 | 26,158 | 1880.30 | 31 | 88 | 47 | 43 |
| 11/5/2006 | 26,054 | 1880.20 | 27 | 84 | 52 | 45 |
| 11/6/2006 | 25,958 | 1880.10 | 30 | 82 | 44 | 44 |
| 11/7/2006 | 25,867 | 1880.10 | 32 | 82 | 41 | 46 |
| 11/8/2006 | 25,749 | 1880.00 | 18 | 82 | 41 | 45 |
| 11/9/2006 | 25,647 | 1879.90 | 26 | 82 | 41 | 45 |
| 11/10/2006 | 25,556 | 1879.80 | 30 | 80 | 40 | 45 |
| 11/11/2006 | 25,532 | 1879.80 | 66 | 82 | 41 | 45 |
| 11/12/2006 | 25,521 | 1879.80 | 73 | 83 | 41 | 45 |
| 11/13/2006 | 25,649 | 1879.90 | 144 | 84 | 48 | 45 |
| 11/14/2006 | 25,729 | 1879.90 | 119 | 83 | 76 | 45 |
| 11/15/2006 | 25,728 | 1879.90 | 84 | 89 | 96 | 45 |
| 11/16/2006 | 25,668 | 1879.90 | 69 | 104 | 116 | 45 |
| 11/17/2006 | 25,557 | 1879.80 | 54 | 116 | 125 | 45 |
| 11/18/2006 | 25,374 | 1879.60 | 45 | 144 | 166 | 44 |
| 11/19/2006 | 25,246 | 1879.50 | 84 | 156 | 140 | 45 |
| 11/20/2006 | 24,964 | 1879.30 | 6 | 156 | 141 | 45 |
| 11/21/2006 | 24,798 | 1879.10 | 50 | 141 | 137 | 45 |
| 11/22/2006 | 24,645 | 1879.00 | 43 | 126 | 103 | 43 |
| 11/23/2006 | 24,511 | 1878.90 | 51 | 125 | 110 | 45 |
| 11/24/2006 | 24,400 | 1878.80 | 53 | 115 | 97 | 46 |
| 11/25/2006 | 24,289 | 1878.70 | 45 | 106 | 85 | 46 |
| 11/26/2006 | 24,298 | 1878.70 | 104 | 105 | 103 | 45 |
| 11/27/2006 | 24,357 | 1878.70 | 110 | 85 | 96 | 45 |
| 11/28/2006 | 24,331 | 1878.70 | 68 | 85 | 49 | 45 |
| 11/29/2006 | 24,246 | 1878.60 | 57 | 105 | 79 | 45 |
| 11/30/2006 | 24,105 | 1878.50 | 42 | 119 | 102 | 45 |
| 12/1/2006 | 23,920 | 1878.30 | 50 | 151 | 124 | 45 |
| 12/2/2006 | 23,769 | 1878.20 | 41 | 123 | 143 | 17 |
| 12/3/2006 | 23,611 | 1878.10 | 42 | 128 | 153 | 1 |
| 12/4/2006 | 23,423 | 1877.90 | 28 | 129 | 153 | 1 |
| 12/5/2006 | 23,251 | 1877.70 | 36 | 129 | 152 | 0 |
| 12/6/2006 | 23,103 | 1877.60 | 49 | 130 | 153 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/7/2006 | 22,905 | 1877.40 | 26 | 132 | 149 | 1 |
| 12/8/2006 | 22,613 | 1877.10 | 44 | 202 | 178 | 29 |
| 12/9/2006 | 22,480 | 1877.00 | 142 | 220 | 166 | 72 |
| 12/10/2006 | 22,841 | 1877.30 | 392 | 221 | 241 | 82 |
| 12/11/2006 | 23,405 | 1877.90 | 500 | 227 | 219 | 93 |
| 12/12/2006 | 25,712 | 1879.90 | 1,383 | 232 | 397 | 148 |
| 12/13/2006 | 26,561 | 1880.70 | 651 | 235 | 225 | 200 |
| 12/14/2006 | 27,485 | 1881.40 | 697 | 243 | 186 | 262 |
| 12/15/2006 | 28,435 | 1882.20 | 766 | 302 | 219 | 262 |
| 12/16/2006 | 29,409 | 1883.00 | 834 | 361 | 172 | 226 |
| 12/17/2006 | 30,407 | 1883.80 | 812 | 325 | 236 | 114 |
| 12/18/2006 | 31,069 | 1884.30 | 603 | 283 | 247 | 58 |
| 12/19/2006 | 30,867 | 1884.10 | 162 | 277 | 184 | 125 |
| 12/20/2006 | 30,558 | 1883.90 | 156 | 328 | 191 | 156 |
| 12/21/2006 | 32,293 | 1885.20 | 1,234 | 378 | 350 | 236 |
| 12/22/2006 | 33,069 | 1885.70 | 750 | 378 | 250 | 264 |
| 12/23/2006 | 33,158 | 1885.80 | 404 | 378 | 199 | 235 |
| 12/24/2006 | 33,077 | 1885.80 | 318 | 378 | 182 | 228 |
| 12/25/2006 | 32,896 | 1885.60 | 268 | 378 | 180 | 220 |
| 12/26/2006 | 38,473 | 1889.50 | 3,219 | 429 | 703 | 209 |
| 12/27/2006 | 43,058 | 1892.40 | 2,696 | 405 | 626 | 246 |
| 12/28/2006 | 44,223 | 1893.10 | 950 | 382 | 309 | 170 |
| 12/29/2006 | 44,745 | 1893.40 | 604 | 358 | 236 | 130 |
| 12/30/2006 | 45,002 | 1893.60 | 448 | 335 | 186 | 136 |
| 12/31/2006 | 45,066 | 1893.60 | 328 | 311 | 168 | 146 |
| 1/1/2007 | 45,028 | 1893.60 | 218 | 288 | 144 | 152 |
| 1/2/2007 | 44,969 | 1893.50 | 191 | 265 | 126 | 152 |
| 1/3/2007 | 45,086 | 1893.60 | 278 | 263 | 148 | 153 |
| 1/4/2007 | 46,005 | 1894.20 | 682 | 262 | 302 | 158 |
| 1/5/2007 | 46,433 | 1894.40 | 433 | 260 | 212 | 160 |
| 1/6/2007 | 46,678 | 1894.60 | 366 | 296 | 205 | 164 |
| 1/7/2007 | 46,817 | 1894.60 | 325 | 314 | 202 | 163 |
| 1/8/2007 | 46,850 | 1894.70 | 270 | 312 | 216 | 155 |
| 1/9/2007 | 46,840 | 1894.60 | 247 | 310 | 211 | 156 |
| 1/10/2007 | 46,764 | 1894.60 | 212 | 308 | 206 | 161 |
| 1/11/2007 | 46,698 | 1894.60 | 216 | 306 | 185 | 160 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/12/2007 | 46,493 | 1894.40 | 145 | 304 | 183 | 160 |
| 1/13/2007 | 46,148 | 1894.20 | 72 | 302 | 185 | 159 |
| 1/14/2007 | 45,802 | 1894.00 | 71 | 300 | 181 | 159 |
| 1/15/2007 | 45,534 | 1893.90 | 108 | 297 | 185 | 159 |
| 1/16/2007 | 45,251 | 1893.70 | 99 | 295 | 184 | 159 |
| 1/17/2007 | 44,953 | 1893.50 | 90 | 293 | 186 | 158 |
| 1/18/2007 | 44,652 | 1893.30 | 87 | 291 | 188 | 158 |
| 1/19/2007 | 44,316 | 1893.10 | 68 | 289 | 182 | 161 |
| 1/20/2007 | 43,993 | 1892.90 | 73 | 287 | 178 | 161 |
| 1/21/2007 | 43,667 | 1892.70 | 71 | 285 | 172 | 161 |
| 1/22/2007 | 43,360 | 1892.60 | 79 | 283 | 164 | 157 |
| 1/23/2007 | 43,026 | 1892.40 | 64 | 281 | 159 | 160 |
| 1/24/2007 | 42,714 | 1892.20 | 73 | 279 | 160 | 157 |
| 1/25/2007 | 42,381 | 1892.00 | 61 | 277 | 160 | 156 |
| 1/26/2007 | 42,038 | 1891.70 | 55 | 275 | 157 | 155 |
| 1/27/2007 | 41,721 | 1891.50 | 66 | 273 | 150 | 154 |
| 1/28/2007 | 41,444 | 1891.40 | 85 | 271 | 129 | 159 |
| 1/29/2007 | 41,157 | 1891.20 | 79 | 269 | 115 | 155 |
| 1/30/2007 | 40,874 | 1891.00 | 79 | 267 | 115 | 159 |
| 1/31/2007 | 40,601 | 1890.80 | 83 | 264 | 106 | 160 |
| 2/1/2007 | 40,306 | 1890.70 | 70 | 262 | 102 | 159 |
| 2/2/2007 | 40,014 | 1890.50 | 70 | 260 | 103 | 158 |
| 2/3/2007 | 39,726 | 1890.30 | 71 | 258 | 102 | 159 |
| 2/4/2007 | 39,434 | 1890.10 | 67 | 256 | 102 | 160 |
| 2/5/2007 | 39,113 | 1889.90 | 51 | 254 | 102 | 160 |
| 2/6/2007 | 38,824 | 1889.70 | 66 | 252 | 102 | 160 |
| 2/7/2007 | 38,761 | 1889.70 | 158 | 220 | 102 | 116 |
| 2/8/2007 | 40,023 | 1890.50 | 790 | 169 | 105 | 49 |
| 2/9/2007 | 45,251 | 1893.70 | 2,770 | 142 | 112 | 49 |
| 2/10/2007 | 56,791 | 1900.20 | 5,994 | 202 | 1,008 | 49 |
| 2/11/2007 | 57,811 | 1900.70 | 2,314 | 2,521 | 4,033 | 23 |
| 2/12/2007 | 57,003 | 1900.30 | 1,476 | 2,640 | 3,025 | 61 |
| 2/13/2007 | 56,648 | 1900.10 | 1,179 | 1,890 | 2,269 | 119 |
| 2/14/2007 | 56,173 | 1899.90 | 776 | 1,400 | 1,512 | 154 |
| 2/15/2007 | 55,796 | 1899.70 | 551 | 1,008 | 1,109 | 152 |
| 2/16/2007 | 55,438 | 1899.50 | 490 | 907 | 856 | 153 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/17/2007 | 55,270 | 1899.40 | 480 | 756 | 635 | 157 |
| 2/18/2007 | 55,082 | 1899.30 | 382 | 630 | 508 | 159 |
| 2/19/2007 | 54,989 | 1899.20 | 377 | 555 | 424 | 158 |
| 2/20/2007 | 54,877 | 1899.20 | 332 | 504 | 361 | 158 |
| 2/21/2007 | 54,989 | 1899.20 | 445 | 504 | 359 | 157 |
| 2/22/2007 | 56,363 | 1900.00 | 1,610 | 1,260 | 1,210 | 153 |
| 2/23/2007 | 55,947 | 1899.70 | 806 | 1,400 | 1,400 | 150 |
| 2/24/2007 | 56,648 | 1900.10 | 1,264 | 1,250 | 1,330 | 119 |
| 2/25/2007 | 57,761 | 1900.70 | 2,626 | 2,900 | 3,580 | 45 |
| 2/26/2007 | 57,126 | 1900.40 | 1,437 | 2,460 | 3,050 | 45 |
| 2/27/2007 | 56,648 | 1900.10 | 1,138 | 1,920 | 2,380 | 56 |
| 2/28/2007 | 56,192 | 1899.90 | 869 | 1,520 | 1,720 | 107 |
| 3/1/2007 | 55,890 | 1899.70 | 793 | 1,300 | 1,280 | 154 |
| 3/2/2007 | 55,758 | 1899.60 | 781 | 1,160 | 1,060 | 152 |
| 3/3/2007 | 55,928 | 1899.70 | 940 | 1,170 | 1,060 | 155 |
| 3/4/2007 | 56,022 | 1899.80 | 986 | 1,290 | 1,210 | 154 |
| 3/5/2007 | 56,022 | 1899.80 | 959 | 1,320 | 1,250 | 154 |
| 3/6/2007 | 56,003 | 1899.80 | 943 | 1,310 | 1,230 | 154 |
| 3/7/2007 | 55,965 | 1899.80 | 912 | 1,280 | 1,220 | 118 |
| 3/8/2007 | 55,871 | 1899.70 | 856 | 1,240 | 1,160 | 151 |
| 3/9/2007 | 55,796 | 1899.70 | 809 | 1,160 | 1,109 | 82 |
| 3/10/2007 | 55,645 | 1899.60 | 701 | 1,060 | 1,059 | 42 |
| 3/11/2007 | 55,683 | 1899.60 | 782 | 1,040 | 1,008 | 42 |
| 3/12/2007 | 56,477 | 1900.00 | 993 | 797 | 857 | 42 |
| 3/13/2007 | 57,703 | 1900.70 | 973 | 457 | 504 | 42 |
| 3/14/2007 | 58,674 | 1901.20 | 864 | 484 | 479 | 43 |
| 3/15/2007 | 59,400 | 1901.60 | 722 | 458 | 454 | 43 |
| 3/16/2007 | 60,153 | 1902.00 | 651 | 337 | 345 | 42 |
| 3/17/2007 | 60,952 | 1902.40 | 599 | 230 | 242 | 42 |
| 3/18/2007 | 61,656 | 1902.70 | 556 | 237 | 240 | 43 |
| 3/19/2007 | 62,223 | 1903.00 | 491 | 242 | 232 | 42 |
| 3/20/2007 | 62,781 | 1903.30 | 491 | 249 | 233 | 40 |
| 3/21/2007 | 63,266 | 1903.50 | 449 | 241 | 225 | 44 |
| 3/22/2007 | 63,754 | 1903.80 | 436 | 221 | 204 | 42 |
| 3/23/2007 | 64,243 | 1904.00 | 433 | 216 | 192 | 42 |
| 3/24/2007 | 64,735 | 1904.20 | 437 | 220 | 192 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/25/2007 | 65,230 | 1904.50 | 440 | 222 | 192 | 43 |
| 3/26/2007 | 65,214 | 1904.50 | 185 | 226 | 212 | 43 |
| 3/27/2007 | 65,263 | 1904.50 | 222 | 232 | 210 | 43 |
| 3/28/2007 | 65,296 | 1904.50 | 214 | 231 | 172 | 75 |
| 3/29/2007 | 65,231 | 1904.50 | 164 | 231 | 157 | 87 |
| 3/30/2007 | 65,304 | 1904.50 | 233 | 230 | 166 | 71 |
| 3/31/2007 | 65,427 | 1904.60 | 258 | 229 | 178 | 50 |
| 4/1/2007 | 65,531 | 1904.60 | 248 | 229 | 173 | 42 |
| 4/2/2007 | 65,714 | 1904.70 | 287 | 228 | 192 | 42 |
| 4/3/2007 | 65,739 | 1904.70 | 207 | 227 | 195 | 41 |
| 4/4/2007 | 65,755 | 1904.70 | 202 | 227 | 199 | 43 |
| 4/5/2007 | 65,722 | 1904.70 | 177 | 226 | 198 | 43 |
| 4/6/2007 | 65,689 | 1904.70 | 176 | 225 | 198 | 43 |
| 4/7/2007 | 65,649 | 1904.70 | 172 | 225 | 193 | 42 |
| 4/8/2007 | 65,649 | 1904.70 | 192 | 224 | 179 | 42 |
| 4/9/2007 | 65,714 | 1904.70 | 224 | 223 | 178 | 42 |
| 4/10/2007 | 65,567 | 1904.70 | 117 | 222 | 195 | 42 |
| 4/11/2007 | 65,608 | 1904.70 | 211 | 222 | 201 | 74 |
| 4/12/2007 | 65,403 | 1904.60 | 87 | 221 | 208 | 74 |
| 4/13/2007 | 65,313 | 1904.50 | 144 | 220 | 171 | 66 |
| 4/14/2007 | 65,255 | 1904.50 | 160 | 220 | 169 | 69 |
| 4/15/2007 | 65,190 | 1904.50 | 156 | 219 | 171 | 58 |
| 4/16/2007 | 65,108 | 1904.40 | 147 | 218 | 168 | 53 |
| 4/17/2007 | 65,084 | 1904.40 | 176 | 218 | 163 | 53 |
| 4/18/2007 | 64,978 | 1904.40 | 134 | 217 | 158 | 53 |
| 4/19/2007 | 64,937 | 1904.30 | 166 | 216 | 158 | 52 |
| 4/20/2007 | 64,896 | 1904.30 | 166 | 216 | 157 | 53 |
| 4/21/2007 | 65,313 | 1904.50 | 396 | 215 | 183 | 53 |
| 4/22/2007 | 66,306 | 1905.00 | 671 | 193 | 309 | 60 |
| 4/23/2007 | 66,553 | 1905.10 | 311 | 216 | 159 | 87 |
| 4/24/2007 | 66,727 | 1905.20 | 299 | 252 | 180 | 88 |
| 4/25/2007 | 66,768 | 1905.20 | 232 | 251 | 198 | 88 |
| 4/26/2007 | 66,793 | 1905.30 | 223 | 250 | 193 | 88 |
| 4/27/2007 | 66,727 | 1905.20 | 177 | 249 | 189 | 88 |
| 4/28/2007 | 66,686 | 1905.20 | 188 | 248 | 185 | 87 |
| 4/29/2007 | 66,603 | 1905.20 | 167 | 247 | 183 | 87 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/30/2007 | 66,471 | 1905.10 | 141 | 247 | 180 | 88 |
| 5/1/2007 | 66,405 | 1905.10 | 174 | 246 | 193 | 88 |
| 5/2/2007 | 66,306 | 1905.00 | 157 | 245 | 213 | 88 |
| 5/3/2007 | 66,281 | 1905.00 | 193 | 244 | 206 | 88 |
| 5/4/2007 | 66,281 | 1905.00 | 205 | 243 | 221 | 88 |
| 5/5/2007 | 66,215 | 1905.00 | 171 | 242 | 203 | 88 |
| 5/6/2007 | 66,092 | 1904.90 | 142 | 241 | 190 | 88 |
| 5/7/2007 | 65,968 | 1904.90 | 141 | 240 | 190 | 85 |
| 5/8/2007 | 65,821 | 1904.80 | 129 | 239 | 181 | 88 |
| 5/9/2007 | 65,632 | 1904.70 | 107 | 238 | 176 | 89 |
| 5/10/2007 | 65,419 | 1904.60 | 94 | 237 | 174 | 88 |
| 5/11/2007 | 65,108 | 1904.40 | 44 | 236 | 224 | 99 |
| 5/12/2007 | 64,880 | 1904.30 | 85 | 235 | 172 | 89 |
| 5/13/2007 | 64,693 | 1904.20 | 105 | 234 | 159 | 90 |
| 5/14/2007 | 64,482 | 1904.10 | 92 | 234 | 158 | 89 |
| 5/15/2007 | 64,255 | 1904.00 | 84 | 233 | 140 | 109 |
| 5/16/2007 | 63,988 | 1903.90 | 63 | 232 | 128 | 131 |
| 5/17/2007 | 63,715 | 1903.70 | 59 | 231 | 127 | 131 |
| 5/18/2007 | 63,449 | 1903.60 | 62 | 230 | 125 | 130 |
| 5/19/2007 | 63,201 | 1903.50 | 70 | 229 | 116 | 130 |
| 5/20/2007 | 62,912 | 1903.30 | 49 | 228 | 102 | 130 |
| 5/21/2007 | 62,689 | 1903.20 | 82 | 227 | 109 | 125 |
| 5/22/2007 | 62,466 | 1903.10 | 69 | 209 | 117 | 131 |
| 5/23/2007 | 62,220 | 1903.00 | 58 | 209 | 97 | 127 |
| 5/24/2007 | 61,982 | 1902.90 | 67 | 217 | 86 | 117 |
| 5/25/2007 | 61,738 | 1902.80 | 64 | 217 | 85 | 118 |
| 5/26/2007 | 61,478 | 1902.60 | 50 | 208 | 83 | 117 |
| 5/27/2007 | 61,234 | 1902.50 | 51 | 198 | 74 | 118 |
| 5/28/2007 | 61,029 | 1902.40 | 64 | 188 | 70 | 117 |
| 5/29/2007 | 60,747 | 1902.30 | 30 | 195 | 69 | 117 |
| 5/30/2007 | 60,490 | 1902.10 | 49 | 205 | 65 | 126 |
| 5/31/2007 | 60,233 | 1902.00 | 46 | 200 | 62 | 131 |
| 6/1/2007 | 59,977 | 1901.90 | 41 | 192 | 58 | 130 |
| 6/2/2007 | 59,714 | 1901.70 | 36 | 190 | 58 | 129 |
| 6/3/2007 | 59,459 | 1901.60 | 40 | 190 | 58 | 129 |
| 6/4/2007 | 59,205 | 1901.50 | 40 | 190 | 60 | 128 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/5/2007 | 58,929 | 1901.30 | 25 | 184 | 57 | 129 |
| 6/6/2007 | 58,616 | 1901.20 | 2 | 178 | 53 | 128 |
| 6/7/2007 | 58,380 | 1901.00 | 41 | 178 | 48 | 129 |
| 6/8/2007 | 58,129 | 1900.90 | 33 | 178 | 47 | 130 |
| 6/9/2007 | 57,879 | 1900.80 | 34 | 178 | 47 | 129 |
| 6/10/2007 | 57,622 | 1900.60 | 30 | 178 | 46 | 129 |
| 6/11/2007 | 57,374 | 1900.50 | 34 | 177 | 46 | 130 |
| 6/12/2007 | 57,104 | 1900.40 | 23 | 177 | 45 | 130 |
| 6/13/2007 | 56,858 | 1900.20 | 32 | 173 | 42 | 130 |
| 6/14/2007 | 56,612 | 1900.10 | 27 | 166 | 35 | 130 |
| 6/15/2007 | 56,359 | 1900.00 | 23 | 164 | 34 | 130 |
| 6/16/2007 | 56,100 | 1899.80 | 20 | 164 | 33 | 129 |
| 6/17/2007 | 55,850 | 1899.70 | 24 | 164 | 32 | 130 |
| 6/18/2007 | 55,570 | 1899.50 | 7 | 161 | 31 | 130 |
| 6/19/2007 | 55,306 | 1899.40 | 12 | 156 | 29 | 130 |
| 6/20/2007 | 55,028 | 1899.30 | 4 | 156 | 29 | 127 |
| 6/21/2007 | 54,744 | 1899.10 | 1 | 156 | 29 | 127 |
| 6/22/2007 | 54,483 | 1899.00 | 13 | 156 | 28 | 129 |
| 6/23/2007 | 54,230 | 1898.80 | 17 | 156 | 26 | 129 |
| 6/24/2007 | 53,949 | 1898.70 | 3 | 156 | 25 | 128 |
| 6/25/2007 | 53,711 | 1898.50 | 24 | 156 | 25 | 127 |
| 6/26/2007 | 53,395 | 1898.40 | -15 | 156 | 25 | 130 |
| 6/27/2007 | 53,145 | 1898.20 | 18 | 156 | 24 | 130 |
| 6/28/2007 | 52,867 | 1898.10 | 4 | 156 | 23 | 130 |
| 6/29/2007 | 52,597 | 1897.90 | 8 | 156 | 23 | 129 |
| 6/30/2007 | 52,307 | 1897.80 | -2 | 156 | 23 | 129 |
| 7/1/2007 | 52,018 | 1897.60 | -1 | 156 | 24 | 130 |
| 7/2/2007 | 51,743 | 1897.40 | 6 | 156 | 23 | 130 |
| 7/3/2007 | 51,462 | 1897.30 | 2 | 155 | 22 | 131 |
| 7/4/2007 | 51,189 | 1897.10 | 6 | 155 | 22 | 127 |
| 7/5/2007 | 50,923 | 1897.00 | 10 | 155 | 22 | 130 |
| 7/6/2007 | 50,639 | 1896.80 | 1 | 155 | 22 | 131 |
| 7/7/2007 | 50,334 | 1896.70 | -10 | 155 | 22 | 131 |
| 7/8/2007 | 50,051 | 1896.50 | 1 | 155 | 22 | 130 |
| 7/9/2007 | 49,754 | 1896.30 | -6 | 155 | 22 | 130 |
| 7/10/2007 | 49,473 | 1896.20 | 2 | 155 | 22 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/11/2007 | 49,192 | 1896.00 | 2 | 155 | 22 | 130 |
| 7/12/2007 | 48,893 | 1895.80 | -7 | 155 | 22 | 129 |
| 7/13/2007 | 48,601 | 1895.70 | -3 | 155 | 22 | 130 |
| 7/14/2007 | 48,303 | 1895.50 | -7 | 155 | 22 | 130 |
| 7/15/2007 | 48,013 | 1895.30 | -3 | 155 | 22 | 130 |
| 7/16/2007 | 47,717 | 1895.20 | -6 | 155 | 22 | 130 |
| 7/17/2007 | 47,409 | 1895.00 | -12 | 155 | 22 | 130 |
| 7/18/2007 | 47,156 | 1894.80 | 16 | 155 | 22 | 130 |
| 7/19/2007 | 46,883 | 1894.70 | 6 | 155 | 22 | 130 |
| 7/20/2007 | 46,592 | 1894.50 | -3 | 155 | 22 | 130 |
| 7/21/2007 | 46,288 | 1894.30 | -10 | 154 | 22 | 129 |
| 7/22/2007 | 46,005 | 1894.20 | 1 | 154 | 22 | 129 |
| 7/23/2007 | 45,749 | 1894.00 | 14 | 154 | 22 | 130 |
| 7/24/2007 | 45,417 | 1893.80 | -24 | 154 | 22 | 129 |
| 7/25/2007 | 45,138 | 1893.60 | 3 | 154 | 22 | 129 |
| 7/26/2007 | 44,833 | 1893.50 | -11 | 154 | 22 | 129 |
| 7/27/2007 | 44,535 | 1893.30 | -7 | 154 | 22 | 129 |
| 7/28/2007 | 44,227 | 1893.10 | -12 | 154 | 22 | 129 |
| 7/29/2007 | 43,932 | 1892.90 | -6 | 154 | 22 | 130 |
| 7/30/2007 | 43,620 | 1892.70 | -14 | 154 | 22 | 130 |
| 7/31/2007 | 43,328 | 1892.50 | -4 | 154 | 23 | 130 |
| 8/1/2007 | 43,017 | 1892.40 | -14 | 154 | 23 | 129 |
| 8/2/2007 | 42,714 | 1892.20 | -10 | 154 | 22 | 129 |
| 8/3/2007 | 42,387 | 1892.00 | -22 | 154 | 23 | 129 |
| 8/4/2007 | 42,088 | 1891.80 | -8 | 154 | 23 | 130 |
| 8/5/2007 | 41,771 | 1891.60 | -17 | 154 | 22 | 131 |
| 8/6/2007 | 41,466 | 1891.40 | -11 | 154 | 23 | 130 |
| 8/7/2007 | 41,163 | 1891.20 | -10 | 153 | 22 | 130 |
| 8/8/2007 | 40,849 | 1891.00 | -16 | 153 | 23 | 131 |
| 8/9/2007 | 40,555 | 1890.80 | -6 | 153 | 23 | 131 |
| 8/10/2007 | 40,239 | 1890.60 | -17 | 153 | 23 | 130 |
| 8/11/2007 | 39,929 | 1890.40 | -14 | 153 | 23 | 130 |
| 8/12/2007 | 39,620 | 1890.20 | -13 | 153 | 23 | 130 |
| 8/13/2007 | 39,326 | 1890.00 | -6 | 153 | 22 | 130 |
| 8/14/2007 | 39,008 | 1889.80 | -18 | 153 | 22 | 129 |
| 8/15/2007 | 38,717 | 1889.60 | -4 | 153 | 22 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/16/2007 | 38,414 | 1889.40 | -10 | 153 | 23 | 129 |
| 8/17/2007 | 38,106 | 1889.20 | -13 | 153 | 23 | 128 |
| 8/18/2007 | 37,807 | 1889.00 | -8 | 153 | 23 | 130 |
| 8/19/2007 | 37,508 | 1888.80 | -8 | 153 | 23 | 130 |
| 8/20/2007 | 37,224 | 1888.60 | -1 | 153 | 22 | 130 |
| 8/21/2007 | 36,927 | 1888.40 | -8 | 153 | 21 | 129 |
| 8/22/2007 | 36,633 | 1888.20 | -6 | 153 | 22 | 130 |
| 8/23/2007 | 36,340 | 1888.00 | -6 | 153 | 21 | 130 |
| 8/24/2007 | 36,031 | 1887.80 | -14 | 153 | 21 | 130 |
| 8/25/2007 | 35,724 | 1887.60 | -13 | 152 | 22 | 129 |
| 8/26/2007 | 35,424 | 1887.40 | -9 | 152 | 22 | 130 |
| 8/27/2007 | 35,120 | 1887.20 | -11 | 152 | 22 | 129 |
| 8/28/2007 | 34,835 | 1887.00 | -2 | 152 | 21 | 129 |
| 8/29/2007 | 34,517 | 1886.80 | -18 | 152 | 21 | 129 |
| 8/30/2007 | 34,224 | 1886.60 | -6 | 152 | 22 | 129 |
| 8/31/2007 | 33,937 | 1886.40 | -3 | 152 | 22 | 131 |
| 9/1/2007 | 33,614 | 1886.10 | -21 | 152 | 22 | 131 |
| 9/2/2007 | 33,326 | 1885.90 | -4 | 152 | 22 | 131 |
| 9/3/2007 | 33,006 | 1885.70 | -20 | 152 | 22 | 131 |
| 9/4/2007 | 32,689 | 1885.50 | -18 | 152 | 22 | 131 |
| 9/5/2007 | 32,373 | 1885.20 | -18 | 152 | 22 | 130 |
| 9/6/2007 | 32,086 | 1885.00 | -3 | 152 | 21 | 130 |
| 9/7/2007 | 31,790 | 1884.80 | -8 | 152 | 23 | 131 |
| 9/8/2007 | 31,497 | 1884.60 | -6 | 152 | 23 | 131 |
| 9/9/2007 | 31,188 | 1884.40 | -14 | 152 | 23 | 130 |
| 9/10/2007 | 30,893 | 1884.10 | -7 | 152 | 21 | 131 |
| 9/11/2007 | 30,604 | 1883.90 | -4 | 151 | 23 | 130 |
| 9/12/2007 | 30,302 | 1883.70 | -11 | 151 | 22 | 130 |
| 9/13/2007 | 30,017 | 1883.50 | -2 | 151 | 22 | 130 |
| 9/14/2007 | 29,734 | 1883.20 | -3 | 151 | 22 | 130 |
| 9/15/2007 | 29,438 | 1883.00 | -3 | 151 | 23 | 131 |
| 9/16/2007 | 29,194 | 1882.80 | -3 | 125 | 23 | 112 |
| 9/17/2007 | 28,981 | 1882.60 | -3 | 114 | 23 | 91 |
| 9/18/2007 | 28,764 | 1882.50 | -3 | 110 | 22 | 89 |
| 9/19/2007 | 28,553 | 1882.30 | -3 | 111 | 22 | 89 |
| 9/20/2007 | 28,353 | 1882.10 | -3 | 103 | 22 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/21/2007 | 28,155 | 1882.00 | -3 | 111 | 23 | 89 |
| 9/22/2007 | 27,957 | 1881.80 | -3 | 110 | 22 | 89 |
| 9/23/2007 | 27,746 | 1881.60 | -3 | 110 | 23 | 89 |
| 9/24/2007 | 27,550 | 1881.50 | -3 | 110 | 22 | 87 |
| 9/25/2007 | 27,346 | 1881.30 | -3 | 110 | 22 | 88 |
| 9/26/2007 | 27,139 | 1881.10 | -3 | 112 | 21 | 87 |
| 9/27/2007 | 26,938 | 1881.00 | -3 | 112 | 23 | 88 |
| 9/28/2007 | 26,723 | 1880.80 | -3 | 112 | 24 | 87 |
| 9/29/2007 | 26,520 | 1880.60 | -3 | 113 | 23 | 88 |
| 9/30/2007 | 26,318 | 1880.50 | -3 | 113 | 23 | 88 |
| 10/1/2007 | 26,100 | 1880.30 | -3 | 113 | 22 | 86 |
| 10/2/2007 | 25,900 | 1880.10 | -3 | 113 | 22 | 87 |
| 10/3/2007 | 25,700 | 1879.90 | -3 | 115 | 22 | 87 |
| 10/4/2007 | 25,500 | 1879.80 | -3 | 115 | 23 | 88 |
| 10/5/2007 | 25,300 | 1879.60 | -3 | 117 | 24 | 89 |
| 10/6/2007 | 25,100 | 1879.40 | -3 | 112 | 23 | 89 |
| 10/7/2007 | 24,900 | 1879.20 | -3 | 110 | 24 | 88 |
| 10/8/2007 | 24,725 | 1879.10 | 27 | 114 | 23 | 88 |
| 10/9/2007 | 24,550 | 1878.90 | 28 | 115 | 26 | 87 |
| 10/10/2007 | 24,375 | 1878.80 | 28 | 116 | 26 | 88 |
| 10/11/2007 | 24,200 | 1878.60 | 28 | 115 | 28 | 87 |
| 10/12/2007 | 24,033 | 1878.40 | 34 | 118 | 26 | 88 |
| 10/13/2007 | 23,867 | 1878.30 | 34 | 118 | 25 | 89 |
| 10/14/2007 | 23,700 | 1878.10 | 30 | 113 | 26 | 89 |
| 10/15/2007 | 23,600 | 1878.00 | 62 | 110 | 27 | 88 |
| 10/16/2007 | 23,500 | 1877.90 | 38 | 76 | 27 | 59 |
| 10/17/2007 | 23,400 | 1877.90 | 37 | 75 | 27 | 46 |
| 10/18/2007 | 23,400 | 1877.90 | 92 | 81 | 27 | 45 |
| 10/19/2007 | 23,400 | 1877.90 | 91 | 79 | 69 | 43 |
| 10/20/2007 | 23,500 | 1877.90 | 139 | 76 | 54 | 44 |
| 10/21/2007 | 23,400 | 1877.90 | 43 | 83 | 41 | 44 |
| 10/22/2007 | 23,300 | 1877.80 | 52 | 96 | 61 | 44 |
| 10/23/2007 | 23,167 | 1877.60 | 39 | 101 | 54 | 44 |
| 10/24/2007 | 23,033 | 1877.50 | 39 | 101 | 58 | 44 |
| 10/25/2007 | 22,900 | 1877.40 | 39 | 102 | 58 | 44 |
| 10/26/2007 | 22,767 | 1877.30 | 41 | 104 | 59 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/27/2007 | 22,633 | 1877.10 | 37 | 99 | 53 | 46 |
| 10/28/2007 | 22,500 | 1877.00 | 29 | 87 | 44 | 46 |
| 10/29/2007 | 22,400 | 1876.90 | 41 | 80 | 33 | 44 |
| 10/30/2007 | 22,300 | 1876.80 | 40 | 79 | 30 | 44 |
| 10/31/2007 | 22,200 | 1876.70 | 40 | 79 | 30 | 44 |
| 11/1/2007 | 22,100 | 1876.60 | 40 | 79 | 30 | 44 |
| 11/2/2007 | 22,000 | 1876.50 | 40 | 79 | 30 | 44 |
| 11/3/2007 | 21,900 | 1876.40 | 40 | 79 | 29 | 44 |
| 11/4/2007 | 21,800 | 1876.30 | 40 | 79 | 29 | 44 |
| 11/5/2007 | 21,700 | 1876.20 | 40 | 79 | 29 | 44 |
| 11/6/2007 | 21,600 | 1876.10 | 40 | 79 | 29 | 29 |
| 11/7/2007 | 21,500 | 1876.00 | 40 | 79 | 28 | 45 |
| 11/8/2007 | 21,400 | 1875.90 | 40 | 79 | 28 | 44 |
| 11/9/2007 | 21,300 | 1875.80 | 40 | 79 | 28 | 44 |
| 11/10/2007 | 21,300 | 1875.80 | 76 | 80 | 28 | 46 |
| 11/11/2007 | 21,225 | 1875.70 | 38 | 80 | 29 | 47 |
| 11/12/2007 | 21,150 | 1875.70 | 37 | 79 | 29 | 38 |
| 11/13/2007 | 21,075 | 1875.60 | 37 | 79 | 29 | 32 |
| 11/14/2007 | 21,000 | 1875.50 | 37 | 79 | 30 | 48 |
| 11/15/2007 | 20,900 | 1875.40 | 27 | 81 | 29 | 45 |
| 11/16/2007 | 20,820 | 1875.30 | 38 | 82 | 30 | 45 |
| 11/17/2007 | 20,740 | 1875.20 | 39 | 83 | 30 | 45 |
| 11/18/2007 | 20,660 | 1875.20 | 39 | 83 | 31 | 44 |
| 11/19/2007 | 20,580 | 1875.10 | 39 | 83 | 31 | 44 |
| 11/20/2007 | 20,500 | 1875.00 | 39 | 83 | 30 | 45 |
| 11/21/2007 | 20,400 | 1874.90 | 28 | 83 | 29 | 45 |
| 11/22/2007 | 20,300 | 1874.80 | 28 | 83 | 29 | 46 |
| 11/23/2007 | 20,200 | 1874.70 | 28 | 83 | 29 | 46 |
| 11/24/2007 | 20,100 | 1874.60 | 28 | 83 | 29 | 46 |
| 11/25/2007 | 20,000 | 1874.50 | 28 | 83 | 29 | 46 |
| 11/26/2007 | 19,900 | 1874.40 | 28 | 83 | 29 | 46 |
| 11/27/2007 | 19,800 | 1874.30 | 28 | 83 | 29 | 46 |
| 11/28/2007 | 19,600 | 1874.00 | 56 | 165 | 29 | 45 |
| 11/29/2007 | 19,500 | 1873.90 | 27 | 82 | 29 | 43 |
| 11/30/2007 | 19,400 | 1873.80 | 27 | 81 | 29 | 43 |
| 12/1/2007 | 19,200 | 1873.60 | 12 | 119 | 29 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/2/2007 | 19,000 | 1873.40 | 50 | 159 | 74 | 43 |
| 12/3/2007 | 18,900 | 1873.30 | 108 | 167 | 103 | 43 |
| 12/4/2007 | 20,000 | 1874.50 | 729 | 184 | 280 | 44 |
| 12/5/2007 | 20,000 | 1874.50 | 166 | 175 | 135 | 44 |
| 12/6/2007 | 20,000 | 1874.50 | 200 | 211 | 148 | 44 |
| 12/7/2007 | 20,300 | 1874.80 | 364 | 224 | 206 | 44 |
| 12/8/2007 | 20,200 | 1874.70 | 156 | 217 | 156 | 43 |
| 12/9/2007 | 20,100 | 1874.60 | 154 | 215 | 146 | 44 |
| 12/10/2007 | 19,840 | 1874.30 | 76 | 218 | 145 | 43 |
| 12/11/2007 | 19,580 | 1874.00 | 65 | 206 | 135 | 43 |
| 12/12/2007 | 19,320 | 1873.70 | 74 | 216 | 141 | 41 |
| 12/13/2007 | 19,060 | 1873.40 | 87 | 230 | 149 | 43 |
| 12/14/2007 | 18,800 | 1873.20 | 65 | 206 | 133 | 43 |
| 12/15/2007 | 18,600 | 1872.90 | 83 | 193 | 126 | 43 |
| 12/16/2007 | 18,400 | 1872.70 | 76 | 186 | 123 | 44 |
| 12/17/2007 | 18,200 | 1872.50 | 72 | 182 | 125 | 45 |
| 12/18/2007 | 19,400 | 1873.80 | 749 | 152 | 314 | 45 |
| 12/19/2007 | 20,200 | 1874.70 | 571 | 176 | 212 | 44 |
| 12/20/2007 | 22,700 | 1877.20 | 1,453 | 203 | 603 | 43 |
| 12/21/2007 | 23,000 | 1877.50 | 355 | 215 | 230 | 44 |
| 12/22/2007 | 23,000 | 1877.50 | 203 | 214 | 178 | 43 |
| 12/23/2007 | 22,900 | 1877.40 | 157 | 218 | 163 | 43 |
| 12/24/2007 | 22,900 | 1877.40 | 215 | 226 | 162 | 42 |
| 12/25/2007 | 22,900 | 1877.40 | 210 | 221 | 156 | 43 |
| 12/26/2007 | 22,700 | 1877.20 | 105 | 217 | 151 | 43 |
| 12/27/2007 | 22,600 | 1877.10 | 157 | 218 | 150 | 43 |
| 12/28/2007 | 22,500 | 1877.00 | 158 | 219 | 151 | 43 |
| 12/29/2007 | 22,500 | 1877.00 | 196 | 206 | 161 | 44 |
| 12/30/2007 | 22,700 | 1877.20 | 278 | 187 | 170 | 44 |
| 12/31/2007 | 22,800 | 1877.30 | 228 | 187 | 148 | 45 |
| 1/1/2008 | 22,700 | 1877.20 | 132 | 192 | 146 | 45 |
| 1/2/2008 | 22,600 | 1877.10 | 132 | 192 | 143 | 43 |
| 1/3/2008 | 23,500 | 1877.90 | 639 | 195 | 213 | 45 |
| 1/4/2008 | 39,300 | 1890.00 | 8,204 | 251 | 1,940 | 44 |
| 1/5/2008 | 44,100 | 1893.00 | 2,649 | 241 | 922 | 44 |
| 1/6/2008 | 46,200 | 1894.30 | 1,285 | 238 | 548 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/7/2008 | 47,300 | 1894.90 | 839 | 299 | 322 | 129 |
| 1/8/2008 | 49,400 | 1896.10 | 1,385 | 343 | 448 | 180 |
| 1/9/2008 | 51,100 | 1897.10 | 1,174 | 334 | 454 | 183 |
| 1/10/2008 | 53,400 | 1898.40 | 1,476 | 333 | 482 | 184 |
| 1/11/2008 | 54,900 | 1899.20 | 1,214 | 482 | 428 | 186 |
| 1/12/2008 | 55,600 | 1899.60 | 1,350 | 1,050 | 786 | 186 |
| 1/13/2008 | 55,600 | 1899.60 | 1,178 | 1,240 | 1,070 | 186 |
| 1/14/2008 | 55,500 | 1899.50 | 1,052 | 1,160 | 957 | 185 |
| 1/15/2008 | 55,300 | 1899.40 | 887 | 1,040 | 778 | 184 |
| 1/16/2008 | 55,100 | 1899.30 | 797 | 945 | 634 | 184 |
| 1/17/2008 | 55,000 | 1899.20 | 736 | 828 | 510 | 185 |
| 1/18/2008 | 54,900 | 1899.20 | 641 | 728 | 413 | 186 |
| 1/19/2008 | 54,800 | 1899.10 | 570 | 653 | 347 | 186 |
| 1/20/2008 | 54,800 | 1899.10 | 570 | 600 | 306 | 186 |
| 1/21/2008 | 54,700 | 1899.10 | 471 | 549 | 266 | 186 |
| 1/22/2008 | 54,700 | 1899.10 | 486 | 512 | 235 | 186 |
| 1/23/2008 | 54,600 | 1899.00 | 392 | 466 | 209 | 176 |
| 1/24/2008 | 54,500 | 1899.00 | 369 | 442 | 196 | 170 |
| 1/25/2008 | 55,300 | 1899.40 | 857 | 478 | 305 | 171 |
| 1/26/2008 | 57,000 | 1900.30 | 2,862 | 2,110 | 2,700 | 170 |
| 1/27/2008 | 57,200 | 1900.40 | 2,371 | 2,390 | 2,680 | 169 |
| 1/28/2008 | 56,600 | 1900.10 | 2,101 | 2,530 | 2,820 | 169 |
| 1/29/2008 | 56,000 | 1899.80 | 1,341 | 1,730 | 1,840 | 169 |
| 1/30/2008 | 55,600 | 1899.60 | 1,071 | 1,340 | 1,310 | 116 |
| 1/31/2008 | 57,800 | 1900.70 | 2,667 | 1,640 | 2,370 | 119 |
| 2/1/2008 | 56,700 | 1900.20 | 2,172 | 2,870 | 3,490 | 170 |
| 2/2/2008 | 57,200 | 1900.40 | 1,981 | 1,820 | 2,290 | 169 |
| 2/3/2008 | 57,900 | 1900.80 | 4,248 | 4,100 | 5,070 | 99 |
| 2/4/2008 | 56,700 | 1900.20 | 2,017 | 2,760 | 3,310 | 74 |
| 2/5/2008 | 56,100 | 1899.80 | 1,322 | 1,710 | 2,000 | 90 |
| 2/6/2008 | 55,800 | 1899.70 | 1,160 | 1,380 | 1,390 | 172 |
| 2/7/2008 | 55,700 | 1899.60 | 1,118 | 1,230 | 1,110 | 171 |
| 2/8/2008 | 55,600 | 1899.60 | 1,061 | 1,170 | 988 | 172 |
| 2/9/2008 | 55,600 | 1899.60 | 1,083 | 1,140 | 921 | 173 |
| 2/10/2008 | 55,600 | 1899.60 | 1,102 | 1,160 | 947 | 173 |
| 2/11/2008 | 55,700 | 1899.60 | 1,190 | 1,200 | 1,050 | 110 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/12/2008 | 55,800 | 1899.70 | 1,219 | 1,230 | 1,100 | 91 |
| 2/13/2008 | 55,900 | 1899.70 | 1,304 | 1,320 | 1,200 | 91 |
| 2/14/2008 | 55,700 | 1899.60 | 1,106 | 1,270 | 1,110 | 91 |
| 2/15/2008 | 55,500 | 1899.50 | 973 | 1,130 | 917 | 91 |
| 2/16/2008 | 55,400 | 1899.50 | 928 | 1,030 | 781 | 91 |
| 2/17/2008 | 55,400 | 1899.50 | 959 | 1,010 | 738 | 91 |
| 2/18/2008 | 55,400 | 1899.50 | 969 | 1,020 | 739 | 91 |
| 2/19/2008 | 55,400 | 1899.50 | 969 | 1,020 | 733 | 91 |
| 2/20/2008 | 55,300 | 1899.40 | 900 | 1,000 | 708 | 90 |
| 2/21/2008 | 55,300 | 1899.40 | 918 | 966 | 667 | 90 |
| 2/22/2008 | 55,300 | 1899.40 | 923 | 972 | 699 | 88 |
| 2/23/2008 | 55,400 | 1899.50 | 942 | 938 | 663 | 92 |
| 2/24/2008 | 59,300 | 1901.50 | 4,731 | 2,910 | 3,240 | 89 |
| 2/25/2008 | 57,500 | 1900.60 | 3,358 | 4,490 | 4,530 | 145 |
| 2/26/2008 | 56,700 | 1900.20 | 1,877 | 2,400 | 2,490 | 248 |
| 2/27/2008 | 56,200 | 1899.90 | 1,382 | 1,720 | 1,720 | 247 |
| 2/28/2008 | 56,100 | 1899.80 | 1,375 | 1,500 | 1,350 | 246 |
| 2/29/2008 | 56,000 | 1899.80 | 1,299 | 1,420 | 1,180 | 246 |
| 3/1/2008 | 55,900 | 1899.70 | 1,242 | 1,360 | 1,060 | 245 |
| 3/2/2008 | 55,700 | 1899.60 | 1,087 | 1,250 | 913 | 245 |
| 3/3/2008 | 55,500 | 1899.50 | 973 | 1,130 | 741 | 246 |
| 3/4/2008 | 55,400 | 1899.50 | 928 | 1,030 | 755 | 114 |
| 3/5/2008 | 55,400 | 1899.50 | 911 | 959 | 724 | 44 |
| 3/6/2008 | 55,400 | 1899.50 | 884 | 931 | 689 | 45 |
| 3/7/2008 | 55,300 | 1899.40 | 787 | 882 | 636 | 47 |
| 3/8/2008 | 55,300 | 1899.40 | 795 | 837 | 596 | 45 |
| 3/9/2008 | 55,300 | 1899.40 | 776 | 817 | 569 | 44 |
| 3/10/2008 | 55,300 | 1899.40 | 795 | 837 | 574 | 44 |
| 3/11/2008 | 55,400 | 1899.50 | 925 | 921 | 635 | 46 |
| 3/12/2008 | 55,400 | 1899.50 | 889 | 936 | 651 | 46 |
| 3/13/2008 | 55,700 | 1899.60 | 1,139 | 1,040 | 730 | 46 |
| 3/14/2008 | 55,700 | 1899.60 | 1,083 | 1,140 | 881 | 46 |
| 3/15/2008 | 55,500 | 1899.50 | 906 | 1,060 | 818 | 46 |
| 3/16/2008 | 55,300 | 1899.40 | 781 | 928 | 667 | 45 |
| 3/17/2008 | 55,200 | 1899.30 | 718 | 809 | 554 | 45 |
| 3/18/2008 | 55,100 | 1899.30 | 655 | 743 | 497 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/19/2008 | 55,200 | 1899.30 | 754 | 741 | 490 | 47 |
| 3/20/2008 | 55,100 | 1899.30 | 654 | 742 | 466 | 72 |
| 3/21/2008 | 55,100 | 1899.30 | 664 | 699 | 444 | 66 |
| 3/22/2008 | 55,000 | 1899.20 | 572 | 655 | 426 | 50 |
| 3/23/2008 | 55,000 | 1899.20 | 601 | 633 | 403 | 50 |
| 3/24/2008 | 55,300 | 1899.40 | 599 | 471 | 352 | 50 |
| 3/25/2008 | 55,900 | 1899.70 | 531 | 241 | 169 | 47 |
| 3/26/2008 | 56,400 | 1900.00 | 501 | 262 | 178 | 45 |
| 3/27/2008 | 56,800 | 1900.20 | 471 | 283 | 178 | 59 |
| 3/28/2008 | 57,200 | 1900.40 | 488 | 301 | 182 | 66 |
| 3/29/2008 | 57,500 | 1900.60 | 454 | 319 | 201 | 59 |
| 3/30/2008 | 57,700 | 1900.70 | 417 | 333 | 196 | 70 |
| 3/31/2008 | 57,900 | 1900.80 | 396 | 311 | 184 | 76 |
| 4/1/2008 | 58,100 | 1900.90 | 372 | 285 | 194 | 76 |
| 4/2/2008 | 58,300 | 1901.00 | 368 | 281 | 193 | 71 |
| 4/3/2008 | 58,500 | 1901.10 | 361 | 274 | 196 | 60 |
| 4/4/2008 | 58,700 | 1901.20 | 360 | 273 | 194 | 60 |
| 4/5/2008 | 58,900 | 1901.30 | 358 | 271 | 192 | 60 |
| 4/6/2008 | 59,100 | 1901.40 | 358 | 271 | 191 | 60 |
| 4/7/2008 | 59,200 | 1901.50 | 315 | 278 | 192 | 60 |
| 4/8/2008 | 59,300 | 1901.50 | 327 | 291 | 193 | 69 |
| 4/9/2008 | 59,400 | 1901.60 | 330 | 294 | 190 | 73 |
| 4/10/2008 | 59,500 | 1901.60 | 330 | 294 | 189 | 73 |
| 4/11/2008 | 59,700 | 1901.70 | 382 | 296 | 188 | 73 |
| 4/12/2008 | 59,900 | 1901.80 | 374 | 288 | 194 | 63 |
| 4/13/2008 | 60,300 | 1902.00 | 467 | 279 | 192 | 58 |
| 4/14/2008 | 60,700 | 1902.20 | 468 | 280 | 191 | 58 |
| 4/15/2008 | 61,100 | 1902.40 | 467 | 279 | 187 | 65 |
| 4/16/2008 | 61,300 | 1902.50 | 383 | 297 | 184 | 75 |
| 4/17/2008 | 61,400 | 1902.60 | 346 | 311 | 182 | 87 |
| 4/18/2008 | 61,600 | 1902.70 | 392 | 307 | 184 | 85 |
| 4/19/2008 | 61,900 | 1902.80 | 437 | 301 | 184 | 80 |
| 4/20/2008 | 62,000 | 1902.90 | 334 | 299 | 182 | 80 |
| 4/21/2008 | 62,100 | 1902.90 | 333 | 297 | 181 | 79 |
| 4/22/2008 | 62,300 | 1903.00 | 385 | 299 | 187 | 79 |
| 4/23/2008 | 62,400 | 1903.10 | 334 | 299 | 194 | 79 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/24/2008 | 62,400 | 1903.10 | 284 | 299 | 188 | 78 |
| 4/25/2008 | 62,400 | 1903.10 | 286 | 301 | 183 | 80 |
| 4/26/2008 | 62,500 | 1903.10 | 338 | 303 | 184 | 80 |
| 4/27/2008 | 62,600 | 1903.20 | 339 | 304 | 183 | 79 |
| 4/28/2008 | 62,900 | 1903.30 | 431 | 295 | 182 | 75 |
| 4/29/2008 | 63,100 | 1903.40 | 377 | 291 | 182 | 70 |
| 4/30/2008 | 63,300 | 1903.50 | 382 | 296 | 182 | 72 |
| 5/1/2008 | 63,300 | 1903.50 | 290 | 305 | 185 | 77 |
| 5/2/2008 | 63,400 | 1903.60 | 336 | 301 | 183 | 79 |
| 5/3/2008 | 63,400 | 1903.60 | 283 | 298 | 184 | 79 |
| 5/4/2008 | 63,500 | 1903.60 | 327 | 291 | 184 | 79 |
| 5/5/2008 | 63,600 | 1903.70 | 319 | 283 | 184 | 76 |
| 5/6/2008 | 63,700 | 1903.70 | 313 | 276 | 184 | 72 |
| 5/7/2008 | 63,900 | 1903.80 | 358 | 271 | 184 | 72 |
| 5/8/2008 | 64,000 | 1903.90 | 305 | 268 | 187 | 72 |
| 5/9/2008 | 64,100 | 1903.90 | 299 | 262 | 188 | 72 |
| 5/10/2008 | 64,100 | 1903.90 | 241 | 254 | 185 | 73 |
| 5/11/2008 | 64,200 | 1904.00 | 288 | 250 | 186 | 73 |
| 5/12/2008 | 64,200 | 1904.00 | 234 | 246 | 186 | 72 |
| 5/13/2008 | 64,100 | 1903.90 | 179 | 241 | 186 | 72 |
| 5/14/2008 | 64,100 | 1903.90 | 236 | 248 | 187 | 72 |
| 5/15/2008 | 64,100 | 1903.90 | 277 | 292 | 184 | 109 |
| 5/16/2008 | 64,100 | 1903.90 | 278 | 293 | 190 | 111 |
| 5/17/2008 | 64,100 | 1903.90 | 273 | 287 | 187 | 113 |
| 5/18/2008 | 64,000 | 1903.90 | 220 | 285 | 188 | 113 |
| 5/19/2008 | 63,800 | 1903.80 | 177 | 292 | 188 | 121 |
| 5/20/2008 | 63,600 | 1903.70 | 180 | 296 | 183 | 130 |
| 5/21/2008 | 63,300 | 1903.50 | 147 | 314 | 186 | 130 |
| 5/22/2008 | 63,000 | 1903.40 | 181 | 350 | 212 | 129 |
| 5/23/2008 | 62,600 | 1903.20 | 135 | 354 | 215 | 130 |
| 5/24/2008 | 62,300 | 1903.00 | 164 | 332 | 199 | 132 |
| 5/25/2008 | 62,000 | 1902.90 | 140 | 307 | 186 | 132 |
| 5/26/2008 | 61,733 | 1902.80 | 117 | 265 | 157 | 131 |
| 5/27/2008 | 61,467 | 1902.60 | 104 | 251 | 147 | 130 |
| 5/28/2008 | 61,200 | 1902.50 | 112 | 259 | 140 | 127 |
| 5/29/2008 | 61,000 | 1902.40 | 169 | 284 | 140 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/30/2008 | 60,800 | 1902.30 | 174 | 289 | 129 | 130 |
| 5/31/2008 | 60,500 | 1902.10 | 134 | 300 | 108 | 130 |
| 6/1/2008 | 60,200 | 1902.00 | 135 | 301 | 106 | 130 |
| 6/2/2008 | 59,900 | 1901.80 | 129 | 295 | 103 | 131 |
| 6/3/2008 | 59,600 | 1901.70 | 113 | 278 | 99 | 131 |
| 6/4/2008 | 59,300 | 1901.50 | 94 | 258 | 88 | 129 |
| 6/5/2008 | 59,000 | 1901.40 | 81 | 244 | 81 | 130 |
| 6/6/2008 | 58,733 | 1901.20 | 82 | 228 | 72 | 132 |
| 6/7/2008 | 58,467 | 1901.10 | 72 | 217 | 67 | 131 |
| 6/8/2008 | 58,200 | 1900.90 | 59 | 204 | 59 | 131 |
| 6/9/2008 | 57,950 | 1900.80 | 65 | 201 | 56 | 131 |
| 6/10/2008 | 57,700 | 1900.70 | 61 | 197 | 55 | 131 |
| 6/11/2008 | 57,450 | 1900.50 | 54 | 189 | 51 | 131 |
| 6/12/2008 | 57,200 | 1900.40 | 53 | 188 | 49 | 132 |
| 6/13/2008 | 56,950 | 1900.30 | 57 | 193 | 44 | 131 |
| 6/14/2008 | 56,700 | 1900.20 | 54 | 189 | 41 | 131 |
| 6/15/2008 | 56,400 | 1900.00 | 25 | 185 | 39 | 131 |
| 6/16/2008 | 56,100 | 1899.80 | 56 | 218 | 52 | 130 |
| 6/17/2008 | 55,750 | 1899.60 | 60 | 249 | 110 | 136 |
| 6/18/2008 | 55,500 | 1899.50 | 39 | 174 | 40 | 130 |
| 6/19/2008 | 55,250 | 1899.40 | 35 | 170 | 29 | 130 |
| 6/20/2008 | 55,000 | 1899.20 | 31 | 165 | 27 | 130 |
| 6/21/2008 | 54,700 | 1899.10 | 3 | 162 | 25 | 129 |
| 6/22/2008 | 54,450 | 1898.90 | 24 | 158 | 23 | 129 |
| 6/23/2008 | 54,200 | 1898.80 | 20 | 154 | 20 | 130 |
| 6/24/2008 | 53,950 | 1898.70 | 19 | 153 | 19 | 130 |
| 6/25/2008 | 53,700 | 1898.50 | 18 | 152 | 18 | 130 |
| 6/26/2008 | 53,450 | 1898.40 | 17 | 151 | 18 | 130 |
| 6/27/2008 | 53,200 | 1898.30 | 16 | 150 | 18 | 130 |
| 6/28/2008 | 52,950 | 1898.10 | 16 | 149 | 18 | 130 |
| 6/29/2008 | 52,700 | 1898.00 | 14 | 147 | 17 | 130 |
| 6/30/2008 | 52,400 | 1897.80 | -11 | 148 | 14 | 131 |
| 7/1/2008 | 52,200 | 1897.70 | 38 | 146 | 14 | 130 |
| 7/2/2008 | 51,900 | 1897.50 | -13 | 145 | 14 | 131 |
| 7/3/2008 | 51,600 | 1897.40 | -13 | 146 | 13 | 130 |
| 7/4/2008 | 51,400 | 1897.30 | 39 | 147 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/5/2008 | 51,100 | 1897.10 | -11 | 148 | 13 | 130 |
| 7/6/2008 | 50,800 | 1896.90 | -12 | 147 | 13 | 130 |
| 7/7/2008 | 50,600 | 1896.80 | 37 | 145 | 13 | 130 |
| 7/8/2008 | 50,350 | 1896.70 | 14 | 147 | 13 | 130 |
| 7/9/2008 | 50,100 | 1896.50 | 14 | 147 | 13 | 130 |
| 7/10/2008 | 49,850 | 1896.40 | 11 | 144 | 13 | 130 |
| 7/11/2008 | 49,600 | 1896.20 | 10 | 143 | 13 | 130 |
| 7/12/2008 | 49,300 | 1896.10 | -15 | 143 | 13 | 130 |
| 7/13/2008 | 49,000 | 1895.90 | -15 | 143 | 12 | 130 |
| 7/14/2008 | 48,700 | 1895.70 | -14 | 144 | 12 | 130 |
| 7/15/2008 | 48,400 | 1895.60 | -12 | 147 | 12 | 130 |
| 7/16/2008 | 48,100 | 1895.40 | -10 | 149 | 12 | 130 |
| 7/17/2008 | 47,800 | 1895.20 | -10 | 149 | 13 | 130 |
| 7/18/2008 | 47,500 | 1895.00 | -10 | 149 | 12 | 130 |
| 7/19/2008 | 47,200 | 1894.90 | -10 | 149 | 12 | 130 |
| 7/20/2008 | 46,900 | 1894.70 | -8 | 151 | 12 | 130 |
| 7/21/2008 | 46,600 | 1894.50 | -8 | 151 | 12 | 131 |
| 7/22/2008 | 46,300 | 1894.30 | -9 | 150 | 12 | 130 |
| 7/23/2008 | 46,100 | 1894.20 | 42 | 150 | 12 | 130 |
| 7/24/2008 | 45,800 | 1894.00 | -9 | 150 | 13 | 129 |
| 7/25/2008 | 45,500 | 1893.90 | -9 | 150 | 12 | 130 |
| 7/26/2008 | 45,200 | 1893.70 | -11 | 148 | 12 | 130 |
| 7/27/2008 | 44,900 | 1893.50 | -7 | 152 | 12 | 130 |
| 7/28/2008 | 44,600 | 1893.30 | -8 | 151 | 12 | 130 |
| 7/29/2008 | 44,300 | 1893.10 | -8 | 151 | 12 | 130 |
| 7/30/2008 | 44,000 | 1893.00 | 8 | 168 | 12 | 130 |
| 7/31/2008 | 43,600 | 1892.70 | -27 | 184 | 12 | 133 |
| 8/1/2008 | 43,300 | 1892.50 | -7 | 152 | 12 | 130 |
| 8/2/2008 | 43,000 | 1892.30 | -7 | 152 | 12 | 130 |
| 8/3/2008 | 42,700 | 1892.20 | -8 | 151 | 12 | 130 |
| 8/4/2008 | 42,400 | 1892.00 | -8 | 151 | 12 | 130 |
| 8/5/2008 | 42,100 | 1891.80 | -8 | 151 | 12 | 130 |
| 8/6/2008 | 41,800 | 1891.60 | -9 | 150 | 12 | 129 |
| 8/7/2008 | 41,500 | 1891.40 | -9 | 150 | 12 | 129 |
| 8/8/2008 | 41,200 | 1891.20 | -9 | 150 | 12 | 129 |
| 8/9/2008 | 40,900 | 1891.00 | -9 | 150 | 12 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/10/2008 | 40,600 | 1890.80 | -9 | 150 | 12 | 130 |
| 8/11/2008 | 40,300 | 1890.60 | -9 | 150 | 12 | 130 |
| 8/12/2008 | 40,000 | 1890.50 | -7 | 152 | 12 | 129 |
| 8/13/2008 | 39,700 | 1890.30 | -7 | 152 | 12 | 130 |
| 8/14/2008 | 39,400 | 1890.10 | -6 | 153 | 12 | 129 |
| 8/15/2008 | 39,100 | 1889.90 | -5 | 154 | 12 | 130 |
| 8/16/2008 | 38,800 | 1889.70 | 0 | 159 | 14 | 129 |
| 8/17/2008 | 38,500 | 1889.50 | 9 | 169 | 17 | 130 |
| 8/18/2008 | 38,200 | 1889.30 | 7 | 167 | 16 | 131 |
| 8/19/2008 | 37,900 | 1889.10 | 6 | 166 | 17 | 130 |
| 8/20/2008 | 37,600 | 1888.90 | 7 | 167 | 17 | 130 |
| 8/21/2008 | 37,300 | 1888.70 | 7 | 167 | 17 | 130 |
| 8/22/2008 | 37,000 | 1888.50 | 6 | 166 | 16 | 130 |
| 8/23/2008 | 36,700 | 1888.30 | 6 | 165 | 16 | 130 |
| 8/24/2008 | 36,400 | 1888.10 | 6 | 165 | 16 | 130 |
| 8/25/2008 | 36,100 | 1887.90 | 6 | 165 | 15 | 130 |
| 8/26/2008 | 35,800 | 1887.70 | 6 | 165 | 15 | 130 |
| 8/27/2008 | 35,400 | 1887.40 | -45 | 165 | 15 | 130 |
| 8/28/2008 | 35,100 | 1887.20 | 6 | 165 | 14 | 130 |
| 8/29/2008 | 34,800 | 1887.00 | 6 | 165 | 14 | 130 |
| 8/30/2008 | 34,500 | 1886.80 | 6 | 166 | 14 | 131 |
| 8/31/2008 | 34,200 | 1886.60 | 8 | 168 | 15 | 130 |
| 9/1/2008 | 33,900 | 1886.30 | 6 | 166 | 14 | 130 |
| 9/2/2008 | 33,600 | 1886.10 | 7 | 167 | 14 | 130 |
| 9/3/2008 | 33,300 | 1885.90 | 25 | 185 | 15 | 130 |
| 9/4/2008 | 32,900 | 1885.60 | 0 | 212 | 44 | 135 |
| 9/5/2008 | 32,600 | 1885.40 | 9 | 169 | 16 | 130 |
| 9/6/2008 | 32,300 | 1885.20 | 10 | 170 | 14 | 130 |
| 9/7/2008 | 32,000 | 1885.00 | 11 | 171 | 14 | 130 |
| 9/8/2008 | 31,650 | 1884.70 | 2 | 188 | 15 | 129 |
| 9/9/2008 | 31,300 | 1884.40 | 23 | 210 | 46 | 132 |
| 9/10/2008 | 31,100 | 1884.30 | 58 | 167 | 15 | 130 |
| 9/11/2008 | 30,800 | 1884.10 | 7 | 167 | 13 | 130 |
| 9/12/2008 | 30,500 | 1883.80 | 7 | 167 | 13 | 130 |
| 9/13/2008 | 30,200 | 1883.60 | 7 | 167 | 13 | 130 |
| 9/14/2008 | 29,900 | 1883.40 | 9 | 169 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/15/2008 | 29,600 | 1883.10 | 9 | 169 | 13 | 130 |
| 9/16/2008 | 29,300 | 1882.90 | -2 | 157 | 31 | 103 |
| 9/17/2008 | 29,100 | 1882.70 | 48 | 157 | 35 | 99 |
| 9/18/2008 | 28,800 | 1882.50 | -11 | 148 | 35 | 97 |
| 9/19/2008 | 28,600 | 1882.30 | 22 | 129 | 23 | 91 |
| 9/20/2008 | 28,400 | 1882.20 | 16 | 123 | 17 | 90 |
| 9/21/2008 | 28,200 | 1882.00 | 14 | 121 | 14 | 90 |
| 9/22/2008 | 28,000 | 1881.90 | 14 | 121 | 14 | 90 |
| 9/23/2008 | 27,800 | 1881.70 | 13 | 120 | 13 | 90 |
| 9/24/2008 | 27,600 | 1881.50 | 13 | 120 | 13 | 90 |
| 9/25/2008 | 27,300 | 1881.30 | -37 | 120 | 13 | 90 |
| 9/26/2008 | 27,100 | 1881.10 | 12 | 119 | 13 | 90 |
| 9/27/2008 | 26,900 | 1880.90 | 13 | 120 | 13 | 90 |
| 9/28/2008 | 26,700 | 1880.80 | 13 | 120 | 12 | 90 |
| 9/29/2008 | 26,500 | 1880.60 | 12 | 119 | 12 | 90 |
| 9/30/2008 | 26,300 | 1880.40 | 12 | 119 | 12 | 90 |
| 10/1/2008 | 26,100 | 1880.30 | 8 | 115 | 14 | 90 |
| 10/2/2008 | 25,900 | 1880.10 | 11 | 118 | 15 | 90 |
| 10/3/2008 | 25,800 | 1880.00 | 65 | 122 | 32 | 91 |
| 10/4/2008 | 25,700 | 1879.90 | 60 | 116 | 30 | 91 |
| 10/5/2008 | 25,600 | 1879.80 | 60 | 116 | 18 | 90 |
| 10/6/2008 | 25,400 | 1879.70 | 10 | 117 | 20 | 90 |
| 10/7/2008 | 25,200 | 1879.50 | 13 | 120 | 22 | 90 |
| 10/8/2008 | 25,000 | 1879.30 | 16 | 123 | 25 | 90 |
| 10/9/2008 | 24,767 | 1879.10 | 3 | 127 | 29 | 90 |
| 10/10/2008 | 24,533 | 1878.90 | 7 | 131 | 31 | 90 |
| 10/11/2008 | 24,300 | 1878.70 | 4 | 128 | 35 | 91 |
| 10/12/2008 | 24,100 | 1878.50 | 17 | 124 | 32 | 90 |
| 10/13/2008 | 23,867 | 1878.30 | -1 | 123 | 30 | 90 |
| 10/14/2008 | 23,633 | 1878.10 | -2 | 122 | 30 | 90 |
| 10/15/2008 | 23,400 | 1877.90 | -2 | 122 | 30 | 90 |
| 10/16/2008 | 23,300 | 1877.80 | 47 | 103 | 52 | 60 |
| 10/17/2008 | 23,100 | 1877.60 | -15 | 90 | 38 | 45 |
| 10/18/2008 | 23,000 | 1877.50 | 32 | 87 | 33 | 45 |
| 10/19/2008 | 22,900 | 1877.40 | 31 | 86 | 30 | 45 |
| 10/20/2008 | 22,700 | 1877.20 | -19 | 86 | 30 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/21/2008 | 22,600 | 1877.10 | 31 | 86 | 30 | 45 |
| 10/22/2008 | 22,500 | 1877.00 | 31 | 86 | 30 | 45 |
| 10/23/2008 | 22,300 | 1876.80 | -19 | 86 | 30 | 45 |
| 10/24/2008 | 22,200 | 1876.70 | 32 | 87 | 32 | 45 |
| 10/25/2008 | 22,100 | 1876.60 | 32 | 87 | 33 | 45 |
| 10/26/2008 | 22,000 | 1876.50 | 31 | 86 | 33 | 45 |
| 10/27/2008 | 21,800 | 1876.30 | -19 | 86 | 33 | 45 |
| 10/28/2008 | 21,700 | 1876.20 | 31 | 86 | 33 | 45 |
| 10/29/2008 | 21,600 | 1876.10 | 32 | 87 | 34 | 45 |
| 10/30/2008 | 21,400 | 1875.90 | -18 | 87 | 35 | 44 |
| 10/31/2008 | 21,300 | 1875.80 | 33 | 88 | 38 | 45 |
| 11/1/2008 | 21,400 | 1875.90 | 135 | 89 | 45 | 45 |
| 11/2/2008 | 21,600 | 1876.10 | 205 | 110 | 69 | 45 |
| 11/3/2008 | 21,800 | 1876.30 | 221 | 126 | 131 | 49 |
| 11/4/2008 | 22,100 | 1876.60 | 261 | 116 | 126 | 46 |
| 11/5/2008 | 22,000 | 1876.50 | 77 | 134 | 109 | 45 |
| 11/6/2008 | 21,767 | 1876.30 | 46 | 172 | 134 | 44 |
| 11/7/2008 | 21,533 | 1876.10 | 63 | 190 | 150 | 44 |
| 11/8/2008 | 21,300 | 1875.80 | 73 | 201 | 158 | 44 |
| 11/9/2008 | 21,000 | 1875.50 | 42 | 203 | 156 | 45 |
| 11/10/2008 | 20,700 | 1875.20 | 33 | 194 | 150 | 45 |
| 11/11/2008 | 20,500 | 1875.00 | 73 | 183 | 140 | 45 |
| 11/12/2008 | 20,300 | 1874.80 | 52 | 161 | 127 | 45 |
| 11/13/2008 | 20,100 | 1874.60 | 30 | 138 | 104 | 45 |
| 11/14/2008 | 19,900 | 1874.40 | 15 | 122 | 89 | 45 |
| 11/15/2008 | 19,800 | 1874.30 | 58 | 114 | 73 | 45 |
| 11/16/2008 | 19,600 | 1874.00 | 27 | 135 | 73 | 45 |
| 11/17/2008 | 19,400 | 1873.80 | 36 | 144 | 113 | 52 |
| 11/18/2008 | 19,300 | 1873.70 | 49 | 105 | 59 | 46 |
| 11/19/2008 | 19,100 | 1873.50 | -7 | 99 | 47 | 46 |
| 11/20/2008 | 19,000 | 1873.40 | 41 | 96 | 43 | 45 |
| 11/21/2008 | 18,900 | 1873.30 | 40 | 95 | 37 | 46 |
| 11/22/2008 | 18,800 | 1873.20 | 40 | 95 | 37 | 46 |
| 11/23/2008 | 18,700 | 1873.00 | 38 | 93 | 36 | 45 |
| 11/24/2008 | 18,600 | 1872.90 | 35 | 90 | 32 | 46 |
| 11/25/2008 | 18,500 | 1872.80 | 34 | 89 | 29 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/26/2008 | 18,400 | 1872.70 | 34 | 89 | 28 | 45 |
| 11/27/2008 | 18,300 | 1872.60 | 34 | 89 | 28 | 45 |
| 11/28/2008 | 18,300 | 1872.60 | 85 | 89 | 28 | 46 |
| 11/29/2008 | 18,200 | 1872.50 | 34 | 89 | 28 | 45 |
| 11/30/2008 | 18,100 | 1872.40 | 34 | 89 | 28 | 46 |
| 12/1/2008 | 17,900 | 1872.10 | 28 | 136 | 57 | 46 |
| 12/2/2008 | 17,600 | 1871.80 | 16 | 176 | 130 | 44 |
| 12/3/2008 | 17,300 | 1871.40 | 15 | 175 | 130 | 45 |
| 12/4/2008 | 17,000 | 1871.00 | 14 | 174 | 130 | 45 |
| 12/5/2008 | 16,800 | 1870.80 | 62 | 171 | 128 | 45 |
| 12/6/2008 | 16,500 | 1870.40 | 10 | 170 | 126 | 44 |
| 12/7/2008 | 16,300 | 1870.20 | 60 | 169 | 126 | 44 |
| 12/8/2008 | 16,000 | 1869.80 | 9 | 169 | 125 | 44 |
| 12/9/2008 | 15,800 | 1869.50 | 59 | 168 | 124 | 44 |
| 12/10/2008 | 15,500 | 1869.10 | 7 | 167 | 124 | 44 |
| 12/11/2008 | 15,300 | 1868.80 | 57 | 166 | 124 | 44 |
| 12/12/2008 | 15,000 | 1868.40 | 6 | 166 | 124 | 44 |
| 12/13/2008 | 14,800 | 1868.20 | 59 | 168 | 124 | 44 |
| 12/14/2008 | 14,500 | 1867.70 | 9 | 169 | 126 | 45 |
| 12/15/2008 | 14,300 | 1867.50 | 60 | 169 | 130 | 45 |
| 12/16/2008 | 14,100 | 1867.20 | 59 | 168 | 125 | 45 |
| 12/17/2008 | 13,800 | 1866.70 | 6 | 166 | 124 | 44 |
| 12/18/2008 | 13,600 | 1866.40 | 57 | 166 | 127 | 44 |
| 12/19/2008 | 13,400 | 1866.10 | 56 | 165 | 140 | 44 |
| 12/20/2008 | 13,200 | 1865.80 | 55 | 164 | 130 | 44 |
| 12/21/2008 | 13,100 | 1865.70 | 106 | 165 | 154 | 45 |
| 12/22/2008 | 13,200 | 1865.80 | 207 | 165 | 187 | 45 |
| 12/23/2008 | 13,100 | 1865.70 | 105 | 164 | 145 | 45 |
| 12/24/2008 | 13,300 | 1866.00 | 262 | 170 | 181 | 45 |
| 12/25/2008 | 14,700 | 1868.00 | 870 | 173 | 449 | 45 |
| 12/26/2008 | 14,800 | 1868.20 | 222 | 181 | 190 | 45 |
| 12/27/2008 | 14,800 | 1868.20 | 177 | 186 | 167 | 45 |
| 12/28/2008 | 15,000 | 1868.40 | 278 | 187 | 241 | 45 |
| 12/29/2008 | 15,400 | 1869.00 | 380 | 188 | 244 | 44 |
| 12/30/2008 | 15,700 | 1869.40 | 331 | 189 | 207 | 44 |
| 12/31/2008 | 15,700 | 1869.40 | 179 | 188 | 175 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/1/2009 | 15,700 | 1869.40 | 179 | 188 | 165 | 44 |
| 1/2/2009 | 16,000 | 1869.80 | 331 | 189 | 230 | 44 |
| 1/3/2009 | 16,300 | 1870.20 | 332 | 190 | 203 | 45 |
| 1/4/2009 | 16,300 | 1870.20 | 180 | 190 | 173 | 45 |
| 1/5/2009 | 16,400 | 1870.30 | 231 | 190 | 184 | 45 |
| 1/6/2009 | 16,500 | 1870.40 | 231 | 190 | 184 | 44 |
| 1/7/2009 | 16,600 | 1870.50 | 231 | 190 | 173 | 44 |
| 1/8/2009 | 16,600 | 1870.50 | 183 | 193 | 168 | 44 |
| 1/9/2009 | 16,600 | 1870.50 | 189 | 199 | 167 | 44 |
| 1/10/2009 | 16,500 | 1870.40 | 138 | 198 | 161 | 44 |
| 1/11/2009 | 16,400 | 1870.30 | 137 | 197 | 158 | 44 |
| 1/12/2009 | 16,300 | 1870.20 | 137 | 197 | 156 | 44 |
| 1/13/2009 | 16,300 | 1870.20 | 187 | 197 | 155 | 44 |
| 1/14/2009 | 16,200 | 1870.00 | 137 | 197 | 153 | 44 |
| 1/15/2009 | 16,100 | 1869.90 | 133 | 193 | 150 | 44 |
| 1/16/2009 | 16,000 | 1869.80 | 123 | 183 | 143 | 44 |
| 1/17/2009 | 15,900 | 1869.60 | 110 | 169 | 133 | 44 |
| 1/18/2009 | 15,800 | 1869.50 | 102 | 160 | 125 | 44 |
| 1/19/2009 | 15,600 | 1869.20 | 45 | 154 | 119 | 44 |
| 1/20/2009 | 15,500 | 1869.10 | 92 | 150 | 115 | 44 |
| 1/21/2009 | 15,400 | 1869.00 | 93 | 151 | 114 | 44 |
| 1/22/2009 | 15,300 | 1868.80 | 93 | 151 | 117 | 44 |
| 1/23/2009 | 15,200 | 1868.70 | 93 | 151 | 116 | 44 |
| 1/24/2009 | 15,100 | 1868.60 | 95 | 153 | 115 | 44 |
| 1/25/2009 | 15,000 | 1868.40 | 95 | 153 | 117 | 44 |
| 1/26/2009 | 14,800 | 1868.20 | 45 | 153 | 116 | 44 |
| 1/27/2009 | 14,700 | 1868.00 | 95 | 153 | 116 | 44 |
| 1/28/2009 | 14,500 | 1867.70 | 45 | 153 | 115 | 44 |
| 1/29/2009 | 14,400 | 1867.60 | 96 | 154 | 115 | 44 |
| 1/30/2009 | 14,200 | 1867.30 | 47 | 156 | 117 | 44 |
| 1/31/2009 | 14,100 | 1867.20 | 74 | 131 | 114 | 17 |
| 2/1/2009 | 13,900 | 1866.90 | 29 | 137 | 120 | 0 |
| 2/2/2009 | 13,800 | 1866.70 | 80 | 137 | 118 | 0 |
| 2/3/2009 | 13,600 | 1866.40 | 28 | 136 | 117 | 0 |
| 2/4/2009 | 13,500 | 1866.30 | 79 | 136 | 117 | 0 |
| 2/5/2009 | 13,400 | 1866.10 | 79 | 136 | 123 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/6/2009 | 13,500 | 1866.30 | 181 | 137 | 143 | 0 |
| 2/7/2009 | 13,500 | 1866.30 | 130 | 137 | 139 | 0 |
| 2/8/2009 | 13,400 | 1866.10 | 80 | 137 | 134 | 0 |
| 2/9/2009 | 13,400 | 1866.10 | 130 | 137 | 133 | 0 |
| 2/10/2009 | 13,300 | 1866.00 | 81 | 138 | 129 | 0 |
| 2/11/2009 | 13,300 | 1866.00 | 156 | 164 | 147 | 26 |
| 2/12/2009 | 13,200 | 1865.80 | 103 | 161 | 150 | 45 |
| 2/13/2009 | 13,500 | 1866.30 | 304 | 161 | 183 | 44 |
| 2/14/2009 | 13,700 | 1866.60 | 254 | 161 | 183 | 44 |
| 2/15/2009 | 14,800 | 1868.20 | 720 | 174 | 361 | 44 |
| 2/16/2009 | 16,300 | 1870.20 | 934 | 187 | 461 | 44 |
| 2/17/2009 | 17,400 | 1871.50 | 735 | 190 | 437 | 43 |
| 2/18/2009 | 18,000 | 1872.20 | 469 | 175 | 327 | 43 |
| 2/19/2009 | 18,400 | 1872.70 | 343 | 149 | 237 | 43 |
| 2/20/2009 | 18,800 | 1873.20 | 333 | 138 | 205 | 43 |
| 2/21/2009 | 19,200 | 1873.60 | 329 | 134 | 199 | 44 |
| 2/22/2009 | 25,800 | 1880.00 | 3,443 | 122 | 752 | 45 |
| 2/23/2009 | 35,300 | 1887.30 | 4,887 | 103 | 1,220 | 44 |
| 2/24/2009 | 41,000 | 1891.10 | 2,954 | 84 | 887 | 43 |
| 2/25/2009 | 43,700 | 1892.80 | 1,435 | 78 | 398 | 43 |
| 2/26/2009 | 46,400 | 1894.40 | 1,451 | 95 | 446 | 43 |
| 2/27/2009 | 48,100 | 1895.40 | 989 | 139 | 382 | 43 |
| 2/28/2009 | 49,300 | 1896.10 | 737 | 139 | 303 | 43 |
| 3/1/2009 | 52,600 | 1897.90 | 1,783 | 126 | 315 | 43 |
| 3/2/2009 | 58,800 | 1901.30 | 5,605 | 2,610 | 2,650 | 43 |
| 3/3/2009 | 58,200 | 1900.90 | 4,144 | 4,680 | 5,240 | 142 |
| 3/4/2009 | 57,000 | 1900.30 | 2,492 | 3,260 | 3,630 | 184 |
| 3/5/2009 | 56,300 | 1899.90 | 1,766 | 2,230 | 2,160 | 189 |
| 3/6/2009 | 55,800 | 1899.70 | 1,334 | 1,670 | 1,410 | 183 |
| 3/7/2009 | 55,600 | 1899.60 | 1,191 | 1,360 | 974 | 184 |
| 3/8/2009 | 55,400 | 1899.50 | 1,011 | 1,170 | 859 | 101 |
| 3/9/2009 | 55,200 | 1899.30 | 868 | 1,020 | 761 | 46 |
| 3/10/2009 | 55,100 | 1899.30 | 773 | 867 | 635 | 46 |
| 3/11/2009 | 55,100 | 1899.30 | 643 | 677 | 541 | 45 |
| 3/12/2009 | 55,300 | 1899.40 | 466 | 384 | 379 | 44 |
| 3/13/2009 | 55,700 | 1899.60 | 385 | 193 | 205 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/14/2009 | 56,100 | 1899.80 | 384 | 192 | 196 | 43 |
| 3/15/2009 | 56,700 | 1900.20 | 487 | 194 | 221 | 43 |
| 3/16/2009 | 58,700 | 1901.20 | 1,196 | 198 | 433 | 44 |
| 3/17/2009 | 60,600 | 1902.20 | 1,147 | 199 | 426 | 45 |
| 3/18/2009 | 62,000 | 1902.90 | 889 | 193 | 354 | 44 |
| 3/19/2009 | 63,100 | 1903.40 | 739 | 194 | 321 | 44 |
| 3/20/2009 | 64,000 | 1903.90 | 639 | 195 | 299 | 44 |
| 3/21/2009 | 65,000 | 1904.40 | 675 | 180 | 281 | 45 |
| 3/22/2009 | 66,000 | 1904.90 | 649 | 152 | 302 | 46 |
| 3/23/2009 | 66,800 | 1905.30 | 541 | 145 | 251 | 46 |
| 3/24/2009 | 67,400 | 1905.50 | 458 | 164 | 255 | 46 |
| 3/25/2009 | 68,000 | 1905.80 | 466 | 172 | 214 | 81 |
| 3/26/2009 | 68,500 | 1906.10 | 432 | 189 | 191 | 100 |
| 3/27/2009 | 69,000 | 1906.30 | 446 | 204 | 208 | 88 |
| 3/28/2009 | 69,600 | 1906.60 | 492 | 200 | 189 | 100 |
| 3/29/2009 | 70,000 | 1906.80 | 393 | 201 | 178 | 100 |
| 3/30/2009 | 70,500 | 1907.00 | 444 | 202 | 175 | 99 |
| 3/31/2009 | 70,800 | 1907.20 | 348 | 207 | 173 | 96 |
| 4/1/2009 | 71,200 | 1907.40 | 427 | 237 | 186 | 96 |
| 4/2/2009 | 71,400 | 1907.40 | 341 | 253 | 243 | 67 |
| 4/3/2009 | 71,200 | 1907.40 | 293 | 415 | 323 | 46 |
| 4/4/2009 | 70,400 | 1907.00 | 538 | 991 | 636 | 75 |
| 4/5/2009 | 69,800 | 1906.70 | 469 | 812 | 621 | 89 |
| 4/6/2009 | 69,600 | 1906.60 | 235 | 353 | 301 | 90 |
| 4/7/2009 | 69,600 | 1906.60 | 282 | 297 | 243 | 91 |
| 4/8/2009 | 69,600 | 1906.60 | 267 | 281 | 247 | 77 |
| 4/9/2009 | 69,600 | 1906.60 | 259 | 273 | 253 | 66 |
| 4/10/2009 | 69,600 | 1906.60 | 259 | 273 | 255 | 65 |
| 4/11/2009 | 69,500 | 1906.60 | 215 | 279 | 249 | 66 |
| 4/12/2009 | 69,400 | 1906.50 | 209 | 273 | 255 | 57 |
| 4/13/2009 | 69,300 | 1906.50 | 210 | 274 | 246 | 51 |
| 4/14/2009 | 69,000 | 1906.30 | 234 | 406 | 318 | 77 |
| 4/15/2009 | 68,800 | 1906.20 | 150 | 264 | 255 | 64 |
| 4/16/2009 | 68,700 | 1906.20 | 188 | 251 | 225 | 64 |
| 4/17/2009 | 68,600 | 1906.10 | 181 | 244 | 215 | 65 |
| 4/18/2009 | 68,500 | 1906.10 | 176 | 238 | 210 | 64 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/19/2009 | 68,400 | 1906.00 | 163 | 225 | 201 | 64 |
| 4/20/2009 | 68,300 | 1906.00 | 153 | 214 | 188 | 64 |
| 4/21/2009 | 68,300 | 1906.00 | 195 | 205 | 179 | 64 |
| 4/22/2009 | 68,400 | 1906.00 | 238 | 197 | 171 | 64 |
| 4/23/2009 | 68,400 | 1906.00 | 192 | 202 | 169 | 64 |
| 4/24/2009 | 68,300 | 1906.00 | 151 | 212 | 172 | 71 |
| 4/25/2009 | 68,200 | 1905.90 | 157 | 218 | 174 | 74 |
| 4/26/2009 | 68,100 | 1905.90 | 155 | 216 | 173 | 74 |
| 4/27/2009 | 67,900 | 1905.80 | 104 | 216 | 173 | 74 |
| 4/28/2009 | 67,700 | 1905.70 | 118 | 230 | 173 | 81 |
| 4/29/2009 | 67,500 | 1905.60 | 121 | 233 | 171 | 90 |
| 4/30/2009 | 67,400 | 1905.50 | 165 | 227 | 167 | 89 |
| 5/1/2009 | 67,400 | 1905.50 | 209 | 220 | 170 | 90 |
| 5/2/2009 | 68,200 | 1905.90 | 613 | 221 | 207 | 90 |
| 5/3/2009 | 69,100 | 1906.40 | 670 | 228 | 250 | 90 |
| 5/4/2009 | 70,700 | 1907.10 | 1,053 | 259 | 281 | 89 |
| 5/5/2009 | 71,500 | 1907.50 | 2,683 | 2,400 | 2,460 | 89 |
| 5/6/2009 | 71,100 | 1907.30 | 1,299 | 1,580 | 1,880 | 90 |
| 5/7/2009 | 71,700 | 1907.60 | 799 | 523 | 486 | 90 |
| 5/8/2009 | 72,100 | 1907.80 | 648 | 470 | 446 | 90 |
| 5/9/2009 | 72,400 | 1907.90 | 459 | 324 | 320 | 89 |
| 5/10/2009 | 72,500 | 1908.00 | 372 | 339 | 315 | 90 |
| 5/11/2009 | 72,600 | 1908.00 | 342 | 307 | 289 | 91 |
| 5/12/2009 | 72,700 | 1908.10 | 329 | 293 | 269 | 91 |
| 5/13/2009 | 72,700 | 1908.10 | 275 | 289 | 267 | 82 |
| 5/14/2009 | 72,600 | 1908.00 | 229 | 294 | 254 | 91 |
| 5/15/2009 | 72,500 | 1908.00 | 255 | 322 | 249 | 108 |
| 5/16/2009 | 72,300 | 1907.90 | 227 | 345 | 246 | 130 |
| 5/17/2009 | 72,100 | 1907.80 | 226 | 344 | 242 | 129 |
| 5/18/2009 | 71,800 | 1907.60 | 174 | 342 | 237 | 130 |
| 5/19/2009 | 71,500 | 1907.50 | 168 | 336 | 229 | 129 |
| 5/20/2009 | 71,200 | 1907.40 | 162 | 330 | 225 | 129 |
| 5/21/2009 | 70,800 | 1907.20 | 217 | 441 | 281 | 135 |
| 5/22/2009 | 70,500 | 1907.00 | 136 | 302 | 214 | 130 |
| 5/23/2009 | 70,200 | 1906.90 | 126 | 292 | 188 | 130 |
| 5/24/2009 | 69,900 | 1906.70 | 111 | 276 | 175 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/25/2009 | 69,600 | 1906.60 | 101 | 265 | 165 | 130 |
| 5/26/2009 | 69,400 | 1906.50 | 143 | 257 | 157 | 130 |
| 5/27/2009 | 69,100 | 1906.40 | 90 | 254 | 153 | 130 |
| 5/28/2009 | 68,800 | 1906.20 | 80 | 243 | 147 | 129 |
| 5/29/2009 | 68,500 | 1906.10 | 73 | 236 | 139 | 129 |
| 5/30/2009 | 68,200 | 1905.90 | 70 | 233 | 135 | 129 |
| 5/31/2009 | 67,900 | 1905.80 | 61 | 223 | 129 | 130 |
| 6/1/2009 | 67,600 | 1905.60 | 55 | 217 | 121 | 130 |
| 6/2/2009 | 67,300 | 1905.50 | 54 | 216 | 121 | 130 |
| 6/3/2009 | 67,100 | 1905.40 | 99 | 210 | 117 | 130 |
| 6/4/2009 | 67,000 | 1905.40 | 149 | 210 | 117 | 130 |
| 6/5/2009 | 66,800 | 1905.30 | 99 | 210 | 115 | 131 |
| 6/6/2009 | 66,500 | 1905.10 | 49 | 211 | 113 | 131 |
| 6/7/2009 | 66,300 | 1905.00 | 95 | 206 | 109 | 130 |
| 6/8/2009 | 66,000 | 1904.90 | 37 | 198 | 102 | 130 |
| 6/9/2009 | 65,800 | 1904.80 | 81 | 191 | 95 | 130 |
| 6/10/2009 | 65,500 | 1904.60 | 24 | 184 | 88 | 130 |
| 6/11/2009 | 65,300 | 1904.50 | 67 | 177 | 79 | 130 |
| 6/12/2009 | 65,000 | 1904.40 | 11 | 171 | 72 | 130 |
| 6/13/2009 | 64,800 | 1904.30 | 59 | 168 | 64 | 130 |
| 6/14/2009 | 64,600 | 1904.20 | 53 | 162 | 52 | 130 |
| 6/15/2009 | 64,600 | 1904.20 | 153 | 161 | 50 | 131 |
| 6/16/2009 | 64,400 | 1904.10 | 48 | 157 | 46 | 130 |
| 6/17/2009 | 64,200 | 1904.00 | 42 | 150 | 37 | 130 |
| 6/18/2009 | 63,900 | 1903.80 | -7 | 152 | 31 | 130 |
| 6/19/2009 | 63,800 | 1903.80 | 91 | 149 | 27 | 131 |
| 6/20/2009 | 63,500 | 1903.60 | -12 | 147 | 26 | 130 |
| 6/21/2009 | 63,300 | 1903.50 | 39 | 147 | 26 | 130 |
| 6/22/2009 | 63,100 | 1903.40 | 40 | 148 | 26 | 130 |
| 6/23/2009 | 62,900 | 1903.30 | 35 | 143 | 23 | 131 |
| 6/24/2009 | 62,600 | 1903.20 | -17 | 141 | 19 | 130 |
| 6/25/2009 | 62,400 | 1903.10 | 32 | 140 | 19 | 130 |
| 6/26/2009 | 62,200 | 1903.00 | 31 | 139 | 16 | 130 |
| 6/27/2009 | 62,000 | 1902.90 | 28 | 136 | 14 | 130 |
| 6/28/2009 | 61,700 | 1902.70 | -23 | 135 | 12 | 130 |
| 6/29/2009 | 61,500 | 1902.60 | 27 | 135 | 12 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/30/2009 | 61,300 | 1902.50 | 30 | 138 | 12 | 129 |
| 7/1/2009 | 61,000 | 1902.40 | -16 | 142 | 15 | 129 |
| 7/2/2009 | 60,700 | 1902.20 | -14 | 144 | 15 | 130 |
| 7/3/2009 | 60,500 | 1902.10 | 32 | 140 | 15 | 129 |
| 7/4/2009 | 60,200 | 1902.00 | -18 | 140 | 13 | 129 |
| 7/5/2009 | 59,900 | 1901.80 | -18 | 140 | 13 | 129 |
| 7/6/2009 | 59,700 | 1901.70 | 32 | 140 | 12 | 130 |
| 7/7/2009 | 59,400 | 1901.60 | -17 | 141 | 12 | 130 |
| 7/8/2009 | 59,100 | 1901.40 | -16 | 142 | 13 | 130 |
| 7/9/2009 | 58,900 | 1901.30 | 34 | 142 | 13 | 130 |
| 7/10/2009 | 58,600 | 1901.10 | -15 | 143 | 13 | 130 |
| 7/11/2009 | 58,300 | 1901.00 | -16 | 142 | 13 | 130 |
| 7/12/2009 | 58,000 | 1900.80 | -16 | 142 | 13 | 129 |
| 7/13/2009 | 57,800 | 1900.70 | 32 | 140 | 13 | 129 |
| 7/14/2009 | 57,500 | 1900.60 | -18 | 140 | 11 | 129 |
| 7/15/2009 | 57,200 | 1900.40 | -15 | 143 | 11 | 129 |
| 7/16/2009 | 57,000 | 1900.30 | 37 | 145 | 13 | 130 |
| 7/17/2009 | 56,700 | 1900.20 | -13 | 145 | 12 | 130 |
| 7/18/2009 | 56,400 | 1900.00 | -11 | 148 | 12 | 131 |
| 7/19/2009 | 56,100 | 1899.80 | -10 | 149 | 12 | 130 |
| 7/20/2009 | 55,800 | 1899.70 | -11 | 148 | 11 | 130 |
| 7/21/2009 | 55,500 | 1899.50 | -9 | 150 | 11 | 130 |
| 7/22/2009 | 55,200 | 1899.30 | -7 | 152 | 13 | 130 |
| 7/23/2009 | 54,900 | 1899.20 | -7 | 152 | 14 | 131 |
| 7/24/2009 | 54,600 | 1899.00 | -7 | 152 | 14 | 131 |
| 7/25/2009 | 54,300 | 1898.90 | -6 | 153 | 14 | 131 |
| 7/26/2009 | 54,100 | 1898.70 | 46 | 155 | 13 | 130 |
| 7/27/2009 | 53,800 | 1898.60 | -3 | 156 | 13 | 130 |
| 7/28/2009 | 53,500 | 1898.40 | -5 | 154 | 13 | 130 |
| 7/29/2009 | 53,200 | 1898.30 | -4 | 155 | 13 | 130 |
| 7/30/2009 | 52,900 | 1898.10 | -3 | 156 | 13 | 131 |
| 7/31/2009 | 52,600 | 1897.90 | -1 | 158 | 13 | 130 |
| 8/1/2009 | 52,300 | 1897.80 | -1 | 158 | 13 | 129 |
| 8/2/2009 | 52,000 | 1897.60 | -1 | 158 | 13 | 130 |
| 8/3/2009 | 51,700 | 1897.40 | 2 | 161 | 13 | 130 |
| 8/4/2009 | 51,300 | 1897.20 | -48 | 162 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/5/2009 | 51,100 | 1897.10 | 53 | 162 | 13 | 130 |
| 8/6/2009 | 50,800 | 1896.90 | 4 | 163 | 14 | 130 |
| 8/7/2009 | 50,500 | 1896.80 | 5 | 164 | 14 | 130 |
| 8/8/2009 | 50,200 | 1896.60 | 5 | 164 | 13 | 130 |
| 8/9/2009 | 49,900 | 1896.40 | 5 | 164 | 12 | 129 |
| 8/10/2009 | 49,600 | 1896.20 | 6 | 165 | 12 | 129 |
| 8/11/2009 | 49,300 | 1896.10 | 6 | 166 | 12 | 129 |
| 8/12/2009 | 49,000 | 1895.90 | 7 | 167 | 12 | 129 |
| 8/13/2009 | 48,700 | 1895.70 | 6 | 165 | 11 | 130 |
| 8/14/2009 | 48,400 | 1895.60 | 6 | 166 | 10 | 130 |
| 8/15/2009 | 48,100 | 1895.40 | 8 | 168 | 11 | 130 |
| 8/16/2009 | 47,800 | 1895.20 | 11 | 171 | 10 | 130 |
| 8/17/2009 | 47,500 | 1895.00 | 12 | 172 | 10 | 130 |
| 8/18/2009 | 47,300 | 1894.90 | 64 | 173 | 9 | 130 |
| 8/19/2009 | 47,000 | 1894.70 | 2 | 161 | 9 | 130 |
| 8/20/2009 | 46,700 | 1894.60 | -17 | 141 | 9 | 130 |
| 8/21/2009 | 46,400 | 1894.40 | -16 | 142 | 9 | 130 |
| 8/22/2009 | 46,100 | 1894.20 | -17 | 141 | 9 | 130 |
| 8/23/2009 | 45,800 | 1894.00 | -16 | 142 | 10 | 130 |
| 8/24/2009 | 45,500 | 1893.90 | -15 | 143 | 11 | 130 |
| 8/25/2009 | 45,200 | 1893.70 | -13 | 145 | 11 | 130 |
| 8/26/2009 | 44,900 | 1893.50 | -13 | 146 | 12 | 130 |
| 8/27/2009 | 44,600 | 1893.30 | -10 | 149 | 10 | 130 |
| 8/28/2009 | 44,400 | 1893.20 | 42 | 150 | 10 | 130 |
| 8/29/2009 | 44,100 | 1893.00 | -13 | 146 | 11 | 130 |
| 8/30/2009 | 43,800 | 1892.80 | -13 | 145 | 10 | 130 |
| 8/31/2009 | 43,500 | 1892.60 | -13 | 146 | 10 | 131 |
| 9/1/2009 | 43,200 | 1892.50 | -12 | 147 | 11 | 130 |
| 9/2/2009 | 42,900 | 1892.30 | -13 | 146 | 12 | 129 |
| 9/3/2009 | 42,600 | 1892.10 | -12 | 147 | 10 | 131 |
| 9/4/2009 | 42,300 | 1891.90 | -11 | 148 | 10 | 130 |
| 9/5/2009 | 42,000 | 1891.70 | -11 | 148 | 11 | 130 |
| 9/6/2009 | 41,700 | 1891.50 | -12 | 147 | 11 | 130 |
| 9/7/2009 | 41,400 | 1891.30 | -11 | 148 | 11 | 130 |
| 9/8/2009 | 41,100 | 1891.20 | -11 | 148 | 11 | 130 |
| 9/9/2009 | 40,800 | 1891.00 | -12 | 147 | 10 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/10/2009 | 40,500 | 1890.80 | -12 | 147 | 10 | 130 |
| 9/11/2009 | 40,200 | 1890.60 | -15 | 143 | 10 | 130 |
| 9/12/2009 | 40,000 | 1890.50 | 34 | 142 | 10 | 130 |
| 9/13/2009 | 39,700 | 1890.30 | -15 | 143 | 10 | 130 |
| 9/14/2009 | 39,400 | 1890.10 | -14 | 144 | 11 | 130 |
| 9/15/2009 | 39,100 | 1889.90 | -13 | 145 | 10 | 130 |
| 9/16/2009 | 38,900 | 1889.70 | 18 | 125 | 22 | 104 |
| 9/17/2009 | 38,700 | 1889.60 | 6 | 112 | 16 | 92 |
| 9/18/2009 | 38,500 | 1889.50 | -2 | 104 | 12 | 92 |
| 9/19/2009 | 38,300 | 1889.40 | -3 | 103 | 10 | 92 |
| 9/20/2009 | 38,100 | 1889.20 | -2 | 104 | 11 | 92 |
| 9/21/2009 | 37,900 | 1889.10 | -3 | 103 | 11 | 92 |
| 9/22/2009 | 37,600 | 1888.90 | -54 | 102 | 11 | 91 |
| 9/23/2009 | 37,400 | 1888.80 | -4 | 102 | 11 | 92 |
| 9/24/2009 | 37,200 | 1888.60 | -5 | 101 | 11 | 92 |
| 9/25/2009 | 37,000 | 1888.50 | -4 | 102 | 10 | 92 |
| 9/26/2009 | 36,800 | 1888.40 | -4 | 102 | 10 | 91 |
| 9/27/2009 | 36,600 | 1888.20 | -4 | 102 | 10 | 91 |
| 9/28/2009 | 36,400 | 1888.10 | -4 | 102 | 10 | 91 |
| 9/29/2009 | 36,200 | 1887.90 | -4 | 102 | 10 | 92 |
| 9/30/2009 | 36,000 | 1887.80 | -4 | 102 | 11 | 92 |
| 10/1/2009 | 35,755 | 1887.60 | -2 | 101 | 8 | 92 |
| 10/2/2009 | 35,538 | 1887.50 | -6 | 102 | 9 | 92 |
| 10/3/2009 | 35,321 | 1887.30 | -11 | 102 | 9 | 90 |
| 10/4/2009 | 35,105 | 1887.20 | -9 | 102 | 11 | 89 |
| 10/5/2009 | 34,890 | 1887.00 | 1 | 108 | 13 | 89 |
| 10/6/2009 | 34,661 | 1886.90 | -2 | 113 | 20 | 89 |
| 10/7/2009 | 34,448 | 1886.70 | 5 | 114 | 20 | 91 |
| 10/8/2009 | 34,236 | 1886.60 | 4 | 114 | 18 | 92 |
| 10/9/2009 | 34,024 | 1886.40 | 5 | 113 | 19 | 91 |
| 10/10/2009 | 33,799 | 1886.30 | 8 | 118 | 19 | 91 |
| 10/11/2009 | 33,589 | 1886.10 | 9 | 118 | 20 | 90 |
| 10/12/2009 | 33,393 | 1886.00 | 15 | 119 | 20 | 90 |
| 10/13/2009 | 33,533 | 1886.10 | 188 | 122 | 42 | 90 |
| 10/14/2009 | 33,813 | 1886.30 | 257 | 122 | 31 | 90 |
| 10/15/2009 | 33,813 | 1886.30 | 140 | 131 | 28 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/16/2009 | 33,701 | 1886.20 | 72 | 128 | 75 | 59 |
| 10/17/2009 | 33,547 | 1886.10 | 47 | 125 | 73 | 46 |
| 10/18/2009 | 33,352 | 1886.00 | 38 | 138 | 97 | 44 |
| 10/19/2009 | 33,185 | 1885.80 | 55 | 140 | 102 | 44 |
| 10/20/2009 | 32,978 | 1885.70 | 33 | 143 | 102 | 45 |
| 10/21/2009 | 32,744 | 1885.50 | 31 | 146 | 113 | 46 |
| 10/22/2009 | 32,565 | 1885.40 | 34 | 128 | 92 | 45 |
| 10/23/2009 | 32,373 | 1885.20 | 27 | 117 | 80 | 45 |
| 10/24/2009 | 32,237 | 1885.10 | 33 | 104 | 67 | 45 |
| 10/25/2009 | 32,115 | 1885.00 | 30 | 96 | 44 | 43 |
| 10/26/2009 | 31,952 | 1884.90 | 28 | 97 | 42 | 44 |
| 10/27/2009 | 31,831 | 1884.80 | 23 | 95 | 39 | 46 |
| 10/28/2009 | 31,710 | 1884.70 | 28 | 92 | 34 | 45 |
| 10/29/2009 | 31,562 | 1884.60 | 20 | 96 | 34 | 45 |
| 10/30/2009 | 31,455 | 1884.60 | 26 | 90 | 41 | 45 |
| 10/31/2009 | 31,334 | 1884.50 | 23 | 91 | 32 | 44 |
| 11/1/2009 | 31,214 | 1884.40 | 27 | 92 | 36 | 44 |
| 11/2/2009 | 31,107 | 1884.30 | 25 | 92 | 36 | 44 |
| 11/3/2009 | 30,988 | 1884.20 | 27 | 93 | 35 | 45 |
| 11/4/2009 | 30,855 | 1884.10 | 23 | 95 | 35 | 45 |
| 11/5/2009 | 30,737 | 1884.00 | 42 | 107 | 62 | 45 |
| 11/6/2009 | 30,657 | 1884.00 | 38 | 92 | 43 | 45 |
| 11/7/2009 | 30,565 | 1883.90 | 39 | 92 | 35 | 45 |
| 11/8/2009 | 30,474 | 1883.80 | 38 | 92 | 35 | 45 |
| 11/9/2009 | 30,369 | 1883.70 | 35 | 92 | 36 | 45 |
| 11/10/2009 | 30,278 | 1883.70 | 36 | 92 | 36 | 45 |
| 11/11/2009 | 30,173 | 1883.60 | 34 | 93 | 36 | 45 |
| 11/12/2009 | 30,056 | 1883.50 | 29 | 93 | 36 | 45 |
| 11/13/2009 | 29,952 | 1883.40 | 30 | 93 | 36 | 45 |
| 11/14/2009 | 29,849 | 1883.30 | 31 | 93 | 37 | 44 |
| 11/15/2009 | 29,732 | 1883.20 | 30 | 93 | 36 | 44 |
| 11/16/2009 | 29,629 | 1883.20 | 30 | 92 | 36 | 44 |
| 11/17/2009 | 29,539 | 1883.10 | 45 | 93 | 39 | 44 |
| 11/18/2009 | 29,437 | 1883.00 | 32 | 93 | 39 | 44 |
| 11/19/2009 | 29,335 | 1882.90 | 35 | 92 | 38 | 44 |
| 11/20/2009 | 29,424 | 1883.00 | 126 | 94 | 59 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/21/2009 | 29,488 | 1883.00 | 116 | 93 | 58 | 44 |
| 11/22/2009 | 29,462 | 1883.00 | 80 | 97 | 55 | 44 |
| 11/23/2009 | 29,411 | 1883.00 | 73 | 107 | 76 | 45 |
| 11/24/2009 | 29,322 | 1882.90 | 54 | 111 | 77 | 45 |
| 11/25/2009 | 29,194 | 1882.80 | 49 | 119 | 82 | 44 |
| 11/26/2009 | 29,067 | 1882.70 | 46 | 119 | 89 | 44 |
| 11/27/2009 | 28,928 | 1882.60 | 49 | 125 | 95 | 43 |
| 11/28/2009 | 28,764 | 1882.50 | 34 | 134 | 109 | 44 |
| 11/29/2009 | 28,601 | 1882.30 | 40 | 134 | 108 | 45 |
| 11/30/2009 | 28,438 | 1882.20 | 37 | 126 | 106 | 45 |
| 12/1/2009 | 28,128 | 1882.00 | 38 | 142 | 88 | 45 |
| 12/2/2009 | 27,869 | 1881.70 | 40 | 189 | 157 | 45 |
| 12/3/2009 | 27,600 | 1881.50 | 37 | 178 | 137 | 45 |
| 12/4/2009 | 27,333 | 1881.30 | 38 | 178 | 136 | 45 |
| 12/5/2009 | 27,068 | 1881.10 | 37 | 176 | 135 | 45 |
| 12/6/2009 | 26,805 | 1880.90 | 39 | 173 | 132 | 45 |
| 12/7/2009 | 26,543 | 1880.60 | 37 | 173 | 132 | 45 |
| 12/8/2009 | 26,273 | 1880.40 | 29 | 172 | 131 | 45 |
| 12/9/2009 | 26,005 | 1880.20 | 34 | 172 | 133 | 44 |
| 12/10/2009 | 25,751 | 1880.00 | 37 | 171 | 132 | 43 |
| 12/11/2009 | 25,544 | 1879.80 | 61 | 172 | 134 | 43 |
| 12/12/2009 | 25,452 | 1879.70 | 120 | 173 | 141 | 43 |
| 12/13/2009 | 25,498 | 1879.70 | 191 | 172 | 157 | 43 |
| 12/14/2009 | 25,361 | 1879.60 | 100 | 172 | 142 | 44 |
| 12/15/2009 | 25,315 | 1879.60 | 133 | 173 | 142 | 44 |
| 12/16/2009 | 27,140 | 1881.10 | 1,086 | 178 | 327 | 44 |
| 12/17/2009 | 27,466 | 1881.40 | 354 | 185 | 177 | 44 |
| 12/18/2009 | 27,442 | 1881.40 | 196 | 211 | 174 | 45 |
| 12/19/2009 | 27,357 | 1881.30 | 154 | 210 | 165 | 46 |
| 12/20/2009 | 27,260 | 1881.20 | 159 | 211 | 166 | 46 |
| 12/21/2009 | 27,442 | 1881.40 | 283 | 211 | 205 | 46 |
| 12/22/2009 | 27,636 | 1881.60 | 306 | 211 | 207 | 46 |
| 12/23/2009 | 27,624 | 1881.50 | 199 | 211 | 176 | 46 |
| 12/24/2009 | 27,527 | 1881.50 | 155 | 210 | 170 | 43 |
| 12/25/2009 | 27,381 | 1881.30 | 131 | 211 | 162 | 47 |
| 12/26/2009 | 27,236 | 1881.20 | 131 | 211 | 160 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/27/2009 | 27,200 | 1881.20 | 182 | 210 | 174 | 46 |
| 12/28/2009 | 27,092 | 1881.10 | 146 | 209 | 169 | 46 |
| 12/29/2009 | 26,996 | 1881.00 | 152 | 210 | 166 | 46 |
| 12/30/2009 | 26,888 | 1880.90 | 140 | 204 | 169 | 46 |
| 12/31/2009 | 26,828 | 1880.90 | 158 | 195 | 161 | 46 |
| 1/1/2010 | 28,091 | 1881.90 | 795 | 194 | 264 | 46 |
| 1/2/2010 | 28,890 | 1882.60 | 612 | 195 | 229 | 46 |
| 1/3/2010 | 29,156 | 1882.80 | 324 | 195 | 180 | 46 |
| 1/4/2010 | 29,233 | 1882.80 | 237 | 196 | 165 | 46 |
| 1/5/2010 | 29,245 | 1882.90 | 190 | 195 | 159 | 46 |
| 1/6/2010 | 29,194 | 1882.80 | 163 | 194 | 155 | 46 |
| 1/7/2010 | 29,118 | 1882.80 | 150 | 194 | 152 | 46 |
| 1/8/2010 | 29,029 | 1882.70 | 140 | 193 | 150 | 46 |
| 1/9/2010 | 28,915 | 1882.60 | 129 | 192 | 148 | 46 |
| 1/10/2010 | 28,789 | 1882.50 | 120 | 191 | 147 | 46 |
| 1/11/2010 | 28,676 | 1882.40 | 120 | 190 | 147 | 46 |
| 1/12/2010 | 30,343 | 1883.70 | 978 | 195 | 302 | 46 |
| 1/13/2010 | 33,603 | 1886.10 | 1,869 | 203 | 463 | 45 |
| 1/14/2010 | 34,647 | 1886.90 | 730 | 200 | 262 | 45 |
| 1/15/2010 | 35,148 | 1887.20 | 454 | 201 | 209 | 46 |
| 1/16/2010 | 35,451 | 1887.40 | 346 | 201 | 189 | 46 |
| 1/17/2010 | 35,988 | 1887.80 | 444 | 203 | 200 | 50 |
| 1/18/2010 | 42,007 | 1891.70 | 3,125 | 222 | 793 | 96 |
| 1/19/2010 | 51,836 | 1897.50 | 5,204 | 245 | 1,500 | 132 |
| 1/20/2010 | 57,568 | 1900.60 | 4,244 | 1,530 | 2,140 | 114 |
| 1/21/2010 | 57,433 | 1900.50 | 3,500 | 3,900 | 4,660 | 47 |
| 1/22/2010 | 56,306 | 1899.90 | 1,757 | 2,670 | 3,020 | 80 |
| 1/23/2010 | 55,928 | 1899.70 | 1,284 | 1,880 | 1,770 | 174 |
| 1/24/2010 | 56,022 | 1899.80 | 1,267 | 1,630 | 1,380 | 224 |
| 1/25/2010 | 59,460 | 1901.60 | 4,746 | 3,620 | 4,030 | 204 |
| 1/26/2010 | 58,032 | 1900.90 | 4,279 | 5,170 | 6,000 | 74 |
| 1/27/2010 | 56,839 | 1900.20 | 2,209 | 2,920 | 3,370 | 211 |
| 1/28/2010 | 56,174 | 1899.90 | 1,492 | 1,920 | 1,980 | 249 |
| 1/29/2010 | 55,890 | 1899.70 | 1,190 | 1,460 | 1,360 | 250 |
| 1/30/2010 | 55,815 | 1899.70 | 1,190 | 1,350 | 1,200 | 251 |
| 1/31/2010 | 55,645 | 1899.60 | 985 | 1,170 | 1,030 | 249 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/1/2010 | 55,495 | 1899.50 | 819 | 995 | 849 | 249 |
| 2/2/2010 | 55,382 | 1899.40 | 735 | 921 | 739 | 249 |
| 2/3/2010 | 55,420 | 1899.50 | 772 | 894 | 693 | 248 |
| 2/4/2010 | 56,117 | 1899.80 | 1,080 | 952 | 744 | 248 |
| 2/5/2010 | 57,145 | 1900.40 | 2,994 | 2,500 | 2,720 | 244 |
| 2/6/2010 | 56,667 | 1900.10 | 1,872 | 2,150 | 2,450 | 241 |
| 2/7/2010 | 56,249 | 1899.90 | 1,492 | 1,730 | 1,910 | 241 |
| 2/8/2010 | 55,909 | 1899.70 | 1,162 | 1,380 | 1,430 | 241 |
| 2/9/2010 | 55,702 | 1899.60 | 996 | 1,180 | 1,140 | 240 |
| 2/10/2010 | 55,513 | 1899.50 | 832 | 1,030 | 898 | 241 |
| 2/11/2010 | 55,401 | 1899.50 | 743 | 927 | 779 | 240 |
| 2/12/2010 | 55,420 | 1899.50 | 778 | 901 | 735 | 240 |
| 2/13/2010 | 55,420 | 1899.50 | 772 | 910 | 732 | 240 |
| 2/14/2010 | 55,382 | 1899.40 | 738 | 892 | 687 | 240 |
| 2/15/2010 | 55,401 | 1899.50 | 742 | 883 | 651 | 240 |
| 2/16/2010 | 55,476 | 1899.50 | 837 | 928 | 692 | 240 |
| 2/17/2010 | 55,457 | 1899.50 | 833 | 960 | 775 | 186 |
| 2/18/2010 | 55,420 | 1899.50 | 813 | 950 | 755 | 189 |
| 2/19/2010 | 55,326 | 1899.40 | 719 | 900 | 644 | 235 |
| 2/20/2010 | 55,251 | 1899.40 | 647 | 842 | 574 | 235 |
| 2/21/2010 | 55,139 | 1899.30 | 556 | 777 | 507 | 236 |
| 2/22/2010 | 55,026 | 1899.30 | 445 | 700 | 432 | 236 |
| 2/23/2010 | 55,382 | 1899.40 | 628 | 691 | 417 | 236 |
| 2/24/2010 | 55,758 | 1899.60 | 1,243 | 1,130 | 1,080 | 140 |
| 2/25/2010 | 55,513 | 1899.50 | 871 | 1,070 | 1,020 | 125 |
| 2/26/2010 | 57,472 | 1900.60 | 2,216 | 1,440 | 1,230 | 240 |
| 2/27/2010 | 56,858 | 1900.20 | 2,316 | 2,610 | 2,940 | 239 |
| 2/28/2010 | 56,211 | 1899.90 | 1,453 | 1,800 | 1,900 | 239 |
| 3/1/2010 | 55,852 | 1899.70 | 1,114 | 1,350 | 1,280 | 239 |
| 3/2/2010 | 56,079 | 1899.80 | 1,321 | 1,290 | 1,200 | 239 |
| 3/3/2010 | 56,439 | 1900.00 | 1,765 | 1,650 | 1,770 | 240 |
| 3/4/2010 | 56,117 | 1899.80 | 1,375 | 1,580 | 1,700 | 240 |
| 3/5/2010 | 55,852 | 1899.70 | 1,138 | 1,320 | 1,310 | 240 |
| 3/6/2010 | 55,664 | 1899.60 | 963 | 1,140 | 1,040 | 240 |
| 3/7/2010 | 55,495 | 1899.50 | 832 | 1,010 | 979 | 114 |
| 3/8/2010 | 55,382 | 1899.40 | 760 | 931 | 919 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/9/2010 | 55,307 | 1899.40 | 664 | 858 | 825 | 46 |
| 3/10/2010 | 55,195 | 1899.30 | 585 | 804 | 757 | 46 |
| 3/11/2010 | 55,101 | 1899.30 | 505 | 738 | 681 | 46 |
| 3/12/2010 | 56,591 | 1900.10 | 1,539 | 1,010 | 1,140 | 46 |
| 3/13/2010 | 56,249 | 1899.90 | 1,604 | 1,780 | 2,250 | 46 |
| 3/14/2010 | 55,852 | 1899.70 | 1,115 | 1,360 | 1,660 | 46 |
| 3/15/2010 | 55,626 | 1899.60 | 925 | 1,110 | 1,260 | 46 |
| 3/16/2010 | 55,495 | 1899.50 | 840 | 1,000 | 1,050 | 46 |
| 3/17/2010 | 55,438 | 1899.50 | 796 | 948 | 1,150 | 46 |
| 3/18/2010 | 55,382 | 1899.40 | 758 | 918 | 1,090 | 45 |
| 3/19/2010 | 55,307 | 1899.40 | 690 | 870 | 991 | 45 |
| 3/20/2010 | 55,251 | 1899.40 | 649 | 831 | 915 | 45 |
| 3/21/2010 | 55,195 | 1899.30 | 591 | 789 | 858 | 45 |
| 3/22/2010 | 55,176 | 1899.30 | 567 | 764 | 805 | 59 |
| 3/23/2010 | 55,101 | 1899.30 | 513 | 731 | 743 | 65 |
| 3/24/2010 | 55,139 | 1899.30 | 536 | 699 | 697 | 65 |
| 3/25/2010 | 55,120 | 1899.30 | 640 | 735 | 765 | 65 |
| 3/26/2010 | 55,026 | 1899.30 | 545 | 693 | 724 | 65 |
| 3/27/2010 | 54,933 | 1899.20 | 472 | 619 | 626 | 65 |
| 3/28/2010 | 54,914 | 1899.20 | 449 | 561 | 559 | 65 |
| 3/29/2010 | 55,288 | 1899.40 | 730 | 662 | 709 | 65 |
| 3/30/2010 | 56,610 | 1900.10 | 2,034 | 1,510 | 1,950 | 65 |
| 3/31/2010 | 56,230 | 1899.90 | 1,451 | 1,720 | 2,240 | 65 |
| 4/1/2010 | 56,401 | 1900.00 | 1,173 | 1,170 | 1,610 | 65 |
| 4/2/2010 | 58,733 | 1901.20 | 1,683 | 657 | 1,200 | 52 |
| 4/3/2010 | 61,535 | 1902.70 | 1,626 | 275 | 879 | 44 |
| 4/4/2010 | 64,274 | 1904.00 | 1,497 | 232 | 750 | 44 |
| 4/5/2010 | 66,662 | 1905.20 | 1,390 | 236 | 729 | 45 |
| 4/6/2010 | 68,542 | 1906.10 | 1,128 | 248 | 652 | 45 |
| 4/7/2010 | 70,189 | 1906.90 | 1,028 | 281 | 585 | 55 |
| 4/8/2010 | 69,645 | 1906.60 | 1,144 | 1,530 | 1,700 | 60 |
| 4/9/2010 | 69,124 | 1906.40 | 1,079 | 1,440 | 1,780 | 55 |
| 4/10/2010 | 68,284 | 1906.00 | 1,000 | 1,530 | 1,840 | 48 |
| 4/11/2010 | 67,920 | 1905.80 | 1,246 | 1,560 | 2,010 | 44 |
| 4/12/2010 | 69,493 | 1906.50 | 2,857 | 2,200 | 3,070 | 45 |
| 4/13/2010 | 67,984 | 1905.80 | 1,872 | 2,790 | 3,670 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/14/2010 | 67,257 | 1905.50 | 1,551 | 2,080 | 2,700 | 44 |
| 4/15/2010 | 66,981 | 1905.30 | 1,300 | 1,570 | 2,030 | 72 |
| 4/16/2010 | 67,321 | 1905.50 | 1,198 | 1,170 | 1,510 | 88 |
| 4/17/2010 | 68,069 | 1905.90 | 1,155 | 907 | 1,040 | 89 |
| 4/18/2010 | 68,649 | 1906.10 | 1,130 | 968 | 1,070 | 89 |
| 4/19/2010 | 68,951 | 1906.30 | 1,088 | 1,080 | 1,160 | 89 |
| 4/20/2010 | 69,406 | 1906.50 | 1,336 | 1,250 | 1,400 | 89 |
| 4/21/2010 | 69,319 | 1906.50 | 1,079 | 1,270 | 1,450 | 89 |
| 4/22/2010 | 69,298 | 1906.50 | 931 | 1,090 | 1,200 | 89 |
| 4/23/2010 | 69,298 | 1906.50 | 864 | 1,020 | 1,030 | 89 |
| 4/24/2010 | 69,124 | 1906.40 | 835 | 1,070 | 1,060 | 89 |
| 4/25/2010 | 68,995 | 1906.30 | 857 | 1,140 | 1,130 | 89 |
| 4/26/2010 | 68,305 | 1906.00 | 594 | 1,170 | 1,160 | 89 |
| 4/27/2010 | 69,471 | 1906.50 | 2,852 | 2,510 | 2,680 | 89 |
| 4/28/2010 | 68,177 | 1905.90 | 2,574 | 3,360 | 4,210 | 89 |
| 4/29/2010 | 67,642 | 1905.70 | 1,832 | 2,240 | 2,750 | 89 |
| 4/30/2010 | 67,343 | 1905.50 | 1,433 | 1,720 | 2,120 | 88 |
| 5/1/2010 | 67,706 | 1905.70 | 1,238 | 1,190 | 1,390 | 88 |
| 5/2/2010 | 67,856 | 1905.80 | 1,126 | 1,200 | 1,310 | 89 |
| 5/3/2010 | 68,005 | 1905.80 | 1,052 | 1,110 | 1,210 | 89 |
| 5/4/2010 | 68,327 | 1906.00 | 1,041 | 1,020 | 1,030 | 90 |
| 5/5/2010 | 68,327 | 1906.00 | 898 | 1,030 | 1,020 | 89 |
| 5/6/2010 | 68,262 | 1906.00 | 800 | 991 | 973 | 89 |
| 5/7/2010 | 68,198 | 1905.90 | 730 | 908 | 855 | 90 |
| 5/8/2010 | 68,305 | 1906.00 | 688 | 772 | 731 | 89 |
| 5/9/2010 | 68,585 | 1906.10 | 663 | 664 | 579 | 90 |
| 5/10/2010 | 69,038 | 1906.30 | 754 | 627 | 585 | 90 |
| 5/11/2010 | 69,298 | 1906.50 | 616 | 586 | 541 | 90 |
| 5/12/2010 | 69,536 | 1906.60 | 567 | 543 | 480 | 93 |
| 5/13/2010 | 69,819 | 1906.70 | 533 | 496 | 443 | 96 |
| 5/14/2010 | 69,971 | 1906.80 | 512 | 451 | 378 | 116 |
| 5/15/2010 | 70,539 | 1907.00 | 485 | 387 | 309 | 129 |
| 5/16/2010 | 71,021 | 1907.30 | 504 | 356 | 270 | 130 |
| 5/17/2010 | 71,594 | 1907.50 | 566 | 388 | 284 | 130 |
| 5/18/2010 | 72,059 | 1907.80 | 632 | 549 | 381 | 130 |
| 5/19/2010 | 72,014 | 1907.70 | 551 | 743 | 567 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/20/2010 | 71,926 | 1907.70 | 531 | 742 | 569 | 129 |
| 5/21/2010 | 71,704 | 1907.60 | 484 | 763 | 574 | 129 |
| 5/22/2010 | 71,329 | 1907.40 | 445 | 786 | 621 | 129 |
| 5/23/2010 | 71,131 | 1907.30 | 395 | 607 | 497 | 129 |
| 5/24/2010 | 71,153 | 1907.30 | 327 | 438 | 340 | 129 |
| 5/25/2010 | 71,373 | 1907.40 | 403 | 391 | 300 | 129 |
| 5/26/2010 | 71,594 | 1907.50 | 491 | 486 | 367 | 130 |
| 5/27/2010 | 71,881 | 1907.70 | 621 | 591 | 487 | 130 |
| 5/28/2010 | 71,881 | 1907.70 | 651 | 758 | 679 | 129 |
| 5/29/2010 | 71,771 | 1907.60 | 537 | 709 | 616 | 129 |
| 5/30/2010 | 71,749 | 1907.60 | 474 | 596 | 502 | 129 |
| 5/31/2010 | 71,771 | 1907.60 | 477 | 567 | 459 | 130 |
| 6/1/2010 | 71,771 | 1907.60 | 459 | 564 | 451 | 130 |
| 6/2/2010 | 71,837 | 1907.70 | 483 | 564 | 447 | 129 |
| 6/3/2010 | 71,837 | 1907.70 | 539 | 667 | 487 | 130 |
| 6/4/2010 | 71,970 | 1907.70 | 768 | 809 | 703 | 129 |
| 6/5/2010 | 72,059 | 1907.80 | 678 | 716 | 658 | 129 |
| 6/6/2010 | 72,369 | 1907.90 | 524 | 473 | 416 | 130 |
| 6/7/2010 | 72,657 | 1908.00 | 436 | 382 | 324 | 129 |
| 6/8/2010 | 72,902 | 1908.20 | 369 | 361 | 293 | 129 |
| 6/9/2010 | 73,058 | 1908.20 | 343 | 360 | 288 | 130 |
| 6/10/2010 | 73,170 | 1908.30 | 304 | 358 | 284 | 129 |
| 6/11/2010 | 73,170 | 1908.30 | 263 | 355 | 278 | 129 |
| 6/12/2010 | 73,170 | 1908.30 | 230 | 351 | 270 | 129 |
| 6/13/2010 | 73,125 | 1908.30 | 215 | 349 | 264 | 130 |
| 6/14/2010 | 73,080 | 1908.20 | 209 | 349 | 257 | 130 |
| 6/15/2010 | 72,991 | 1908.20 | 185 | 337 | 251 | 130 |
| 6/16/2010 | 72,924 | 1908.20 | 157 | 291 | 219 | 129 |
| 6/17/2010 | 72,924 | 1908.20 | 151 | 221 | 153 | 129 |
| 6/18/2010 | 72,969 | 1908.20 | 162 | 187 | 102 | 130 |
| 6/19/2010 | 72,991 | 1908.20 | 154 | 179 | 105 | 129 |
| 6/20/2010 | 73,014 | 1908.20 | 149 | 179 | 102 | 130 |
| 6/21/2010 | 73,014 | 1908.20 | 141 | 171 | 96 | 129 |
| 6/22/2010 | 73,014 | 1908.20 | 140 | 161 | 82 | 130 |
| 6/23/2010 | 73,014 | 1908.20 | 130 | 158 | 76 | 129 |
| 6/24/2010 | 73,014 | 1908.20 | 123 | 175 | 74 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/25/2010 | 72,991 | 1908.20 | 114 | 166 | 58 | 130 |
| 6/26/2010 | 72,947 | 1908.20 | 107 | 159 | 51 | 130 |
| 6/27/2010 | 72,902 | 1908.20 | 95 | 159 | 50 | 129 |
| 6/28/2010 | 72,835 | 1908.10 | 78 | 157 | 45 | 130 |
| 6/29/2010 | 72,724 | 1908.10 | 78 | 153 | 38 | 130 |
| 6/30/2010 | 72,635 | 1908.00 | 75 | 155 | 37 | 130 |
| 7/1/2010 | 72,524 | 1908.00 | 72 | 155 | 37 | 130 |
| 7/2/2010 | 72,435 | 1907.90 | 68 | 151 | 34 | 130 |
| 7/3/2010 | 72,302 | 1907.90 | 61 | 148 | 31 | 129 |
| 7/4/2010 | 72,192 | 1907.80 | 57 | 147 | 31 | 129 |
| 7/5/2010 | 72,037 | 1907.70 | 48 | 149 | 29 | 129 |
| 7/6/2010 | 71,881 | 1907.70 | 47 | 154 | 34 | 129 |
| 7/7/2010 | 71,726 | 1907.60 | 43 | 155 | 34 | 129 |
| 7/8/2010 | 71,550 | 1907.50 | 39 | 151 | 30 | 130 |
| 7/9/2010 | 71,395 | 1907.40 | 37 | 148 | 27 | 129 |
| 7/10/2010 | 71,197 | 1907.40 | 34 | 155 | 27 | 130 |
| 7/11/2010 | 71,021 | 1907.30 | 37 | 161 | 25 | 130 |
| 7/12/2010 | 70,845 | 1907.20 | 33 | 156 | 25 | 130 |
| 7/13/2010 | 70,626 | 1907.10 | 27 | 153 | 26 | 129 |
| 7/14/2010 | 70,429 | 1907.00 | 29 | 151 | 25 | 130 |
| 7/15/2010 | 70,233 | 1906.90 | 34 | 146 | 24 | 130 |
| 7/16/2010 | 69,993 | 1906.80 | 36 | 151 | 25 | 130 |
| 7/17/2010 | 69,776 | 1906.70 | 27 | 151 | 27 | 130 |
| 7/18/2010 | 69,536 | 1906.60 | 23 | 150 | 26 | 130 |
| 7/19/2010 | 69,298 | 1906.50 | 22 | 151 | 26 | 129 |
| 7/20/2010 | 69,038 | 1906.30 | 20 | 151 | 26 | 129 |
| 7/21/2010 | 68,779 | 1906.20 | 15 | 150 | 25 | 130 |
| 7/22/2010 | 68,542 | 1906.10 | 19 | 152 | 25 | 129 |
| 7/23/2010 | 68,284 | 1906.00 | 18 | 152 | 24 | 130 |
| 7/24/2010 | 68,027 | 1905.80 | 13 | 151 | 22 | 130 |
| 7/25/2010 | 67,749 | 1905.70 | 11 | 148 | 22 | 130 |
| 7/26/2010 | 67,492 | 1905.60 | 6 | 147 | 22 | 130 |
| 7/27/2010 | 67,215 | 1905.50 | 5 | 147 | 22 | 129 |
| 7/28/2010 | 66,959 | 1905.30 | 4 | 147 | 21 | 129 |
| 7/29/2010 | 66,684 | 1905.20 | 4 | 149 | 20 | 130 |
| 7/30/2010 | 66,387 | 1905.10 | 4 | 152 | 22 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/31/2010 | 66,113 | 1904.90 | 6 | 153 | 22 | 130 |
| 8/1/2010 | 65,840 | 1904.80 | 3 | 152 | 22 | 130 |
| 8/2/2010 | 65,567 | 1904.70 | 3 | 152 | 22 | 129 |
| 8/3/2010 | 65,274 | 1904.50 | 4 | 154 | 21 | 129 |
| 8/4/2010 | 64,960 | 1904.40 | 7 | 175 | 22 | 130 |
| 8/5/2010 | 64,585 | 1904.20 | 16 | 209 | 69 | 135 |
| 8/6/2010 | 64,315 | 1904.00 | 7 | 160 | 33 | 130 |
| 8/7/2010 | 64,005 | 1903.90 | 5 | 158 | 20 | 129 |
| 8/8/2010 | 63,737 | 1903.80 | 4 | 156 | 19 | 129 |
| 8/9/2010 | 63,449 | 1903.60 | 4 | 154 | 19 | 129 |
| 8/10/2010 | 63,161 | 1903.50 | -1 | 151 | 19 | 129 |
| 8/11/2010 | 62,854 | 1903.30 | -7 | 149 | 19 | 129 |
| 8/12/2010 | 62,569 | 1903.20 | -4 | 149 | 19 | 129 |
| 8/13/2010 | 62,284 | 1903.00 | -2 | 148 | 19 | 130 |
| 8/14/2010 | 62,000 | 1902.90 | -3 | 149 | 18 | 130 |
| 8/15/2010 | 61,676 | 1902.70 | -2 | 176 | 21 | 130 |
| 8/16/2010 | 61,293 | 1902.50 | 15 | 213 | 96 | 131 |
| 8/17/2010 | 60,992 | 1902.40 | -4 | 149 | 24 | 130 |
| 8/18/2010 | 60,671 | 1902.20 | -9 | 168 | 17 | 130 |
| 8/19/2010 | 60,292 | 1902.00 | 10 | 205 | 80 | 134 |
| 8/20/2010 | 59,994 | 1901.90 | -5 | 153 | 21 | 130 |
| 8/21/2010 | 59,697 | 1901.70 | -11 | 153 | 19 | 131 |
| 8/22/2010 | 59,400 | 1901.60 | -4 | 156 | 19 | 131 |
| 8/23/2010 | 59,105 | 1901.40 | 4 | 158 | 19 | 131 |
| 8/24/2010 | 58,811 | 1901.30 | -2 | 152 | 18 | 130 |
| 8/25/2010 | 58,518 | 1901.10 | -6 | 149 | 17 | 130 |
| 8/26/2010 | 58,226 | 1901.00 | -6 | 151 | 17 | 130 |
| 8/27/2010 | 57,916 | 1900.80 | -10 | 151 | 16 | 130 |
| 8/28/2010 | 57,587 | 1900.60 | -15 | 151 | 16 | 130 |
| 8/29/2010 | 57,299 | 1900.50 | -7 | 151 | 16 | 130 |
| 8/30/2010 | 57,011 | 1900.30 | -1 | 151 | 15 | 130 |
| 8/31/2010 | 56,725 | 1900.20 | 1 | 151 | 15 | 130 |
| 9/1/2010 | 56,458 | 1900.00 | 2 | 151 | 15 | 130 |
| 9/2/2010 | 56,155 | 1899.90 | -4 | 151 | 16 | 130 |
| 9/3/2010 | 55,833 | 1899.70 | 3 | 151 | 23 | 131 |
| 9/4/2010 | 55,476 | 1899.50 | -2 | 151 | 35 | 144 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/5/2010 | 55,139 | 1899.30 | 5 | 173 | 27 | 152 |
| 9/6/2010 | 54,765 | 1899.10 | -16 | 193 | 27 | 152 |
| 9/7/2010 | 54,413 | 1898.90 | -16 | 193 | 26 | 152 |
| 9/8/2010 | 54,024 | 1898.70 | -12 | 217 | 23 | 163 |
| 9/9/2010 | 53,601 | 1898.50 | -2 | 239 | 20 | 188 |
| 9/10/2010 | 53,197 | 1898.30 | 3 | 243 | 21 | 193 |
| 9/11/2010 | 52,795 | 1898.00 | 5 | 245 | 17 | 196 |
| 9/12/2010 | 52,378 | 1897.80 | 4 | 250 | 19 | 198 |
| 9/13/2010 | 51,963 | 1897.60 | 0 | 245 | 18 | 199 |
| 9/14/2010 | 51,530 | 1897.30 | -7 | 244 | 20 | 197 |
| 9/15/2010 | 51,100 | 1897.10 | -2 | 246 | 25 | 196 |
| 9/16/2010 | 50,797 | 1896.90 | -1 | 181 | 91 | 117 |
| 9/17/2010 | 50,566 | 1896.80 | -3 | 120 | 48 | 79 |
| 9/18/2010 | 50,353 | 1896.70 | -3 | 117 | 49 | 74 |
| 9/19/2010 | 50,177 | 1896.60 | 16 | 118 | 32 | 90 |
| 9/20/2010 | 49,964 | 1896.40 | 2 | 119 | 29 | 90 |
| 9/21/2010 | 49,753 | 1896.30 | 9 | 118 | 27 | 90 |
| 9/22/2010 | 49,542 | 1896.20 | 6 | 117 | 23 | 90 |
| 9/23/2010 | 49,331 | 1896.10 | 7 | 116 | 23 | 90 |
| 9/24/2010 | 49,121 | 1896.00 | 3 | 115 | 23 | 90 |
| 9/25/2010 | 48,912 | 1895.80 | 6 | 115 | 22 | 90 |
| 9/26/2010 | 48,704 | 1895.70 | 5 | 116 | 22 | 90 |
| 9/27/2010 | 48,513 | 1895.60 | 8 | 116 | 22 | 90 |
| 9/28/2010 | 48,288 | 1895.50 | 6 | 116 | 22 | 91 |
| 9/29/2010 | 48,063 | 1895.40 | -8 | 117 | 21 | 91 |
| 9/30/2010 | 47,856 | 1895.20 | 67 | 119 | 22 | 91 |
| 10/1/2010 | 47,633 | 1895.10 | 2 | 120 | 25 | 91 |
| 10/2/2010 | 47,427 | 1895.00 | 10 | 120 | 24 | 91 |
| 10/3/2010 | 47,205 | 1894.90 | 3 | 121 | 25 | 91 |
| 10/4/2010 | 47,001 | 1894.70 | 13 | 122 | 26 | 91 |
| 10/5/2010 | 46,780 | 1894.60 | 5 | 122 | 27 | 91 |
| 10/6/2010 | 46,577 | 1894.50 | 14 | 122 | 28 | 91 |
| 10/7/2010 | 46,357 | 1894.40 | 5 | 122 | 28 | 91 |
| 10/8/2010 | 46,154 | 1894.20 | 14 | 122 | 28 | 91 |
| 10/9/2010 | 45,952 | 1894.10 | 14 | 122 | 27 | 91 |
| 10/10/2010 | 45,735 | 1894.00 | 6 | 122 | 27 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/11/2010 | 45,517 | 1893.90 | 7 | 123 | 28 | 91 |
| 10/12/2010 | 45,301 | 1893.70 | 9 | 124 | 29 | 91 |
| 10/13/2010 | 45,085 | 1893.60 | 12 | 127 | 30 | 91 |
| 10/14/2010 | 44,886 | 1893.50 | 21 | 128 | 32 | 91 |
| 10/15/2010 | 44,637 | 1893.30 | -3 | 129 | 32 | 91 |
| 10/16/2010 | 44,406 | 1893.20 | 5 | 128 | 32 | 91 |
| 10/17/2010 | 44,192 | 1893.10 | 14 | 128 | 32 | 91 |
| 10/18/2010 | 43,995 | 1892.90 | 12 | 117 | 34 | 83 |
| 10/19/2010 | 43,832 | 1892.80 | 18 | 105 | 37 | 61 |
| 10/20/2010 | 43,653 | 1892.70 | 9 | 105 | 36 | 60 |
| 10/21/2010 | 43,490 | 1892.60 | 18 | 105 | 38 | 60 |
| 10/22/2010 | 43,344 | 1892.60 | 24 | 103 | 48 | 52 |
| 10/23/2010 | 43,523 | 1892.70 | 185 | 100 | 64 | 45 |
| 10/24/2010 | 46,984 | 1894.70 | 1,848 | 108 | 483 | 45 |
| 10/25/2010 | 47,616 | 1895.10 | 440 | 128 | 189 | 45 |
| 10/26/2010 | 47,547 | 1895.10 | 127 | 170 | 139 | 44 |
| 10/27/2010 | 47,393 | 1895.00 | 114 | 202 | 162 | 44 |
| 10/28/2010 | 48,288 | 1895.50 | 642 | 201 | 275 | 45 |
| 10/29/2010 | 50,300 | 1896.60 | 1,206 | 202 | 397 | 46 |
| 10/30/2010 | 50,815 | 1896.90 | 443 | 193 | 249 | 46 |
| 10/31/2010 | 50,815 | 1896.90 | 208 | 219 | 199 | 46 |
| 11/1/2010 | 50,601 | 1896.80 | 144 | 265 | 235 | 49 |
| 11/2/2010 | 50,353 | 1896.70 | 121 | 259 | 189 | 87 |
| 11/3/2010 | 50,000 | 1896.50 | 96 | 289 | 145 | 131 |
| 11/4/2010 | 49,612 | 1896.20 | 107 | 318 | 206 | 120 |
| 11/5/2010 | 49,384 | 1896.10 | 66 | 190 | 144 | 83 |
| 11/6/2010 | 48,999 | 1895.90 | 60 | 267 | 155 | 98 |
| 11/7/2010 | 49,349 | 1896.10 | 503 | 344 | 299 | 168 |
| 11/8/2010 | 49,139 | 1896.00 | 200 | 322 | 231 | 171 |
| 11/9/2010 | 48,860 | 1895.80 | 163 | 320 | 203 | 171 |
| 11/10/2010 | 48,548 | 1895.60 | 147 | 320 | 211 | 165 |
| 11/11/2010 | 48,167 | 1895.40 | 115 | 323 | 204 | 162 |
| 11/12/2010 | 47,753 | 1895.20 | 94 | 318 | 197 | 162 |
| 11/13/2010 | 47,325 | 1894.90 | 80 | 311 | 193 | 162 |
| 11/14/2010 | 46,865 | 1894.70 | 62 | 309 | 192 | 161 |
| 11/15/2010 | 46,407 | 1894.40 | 59 | 305 | 191 | 161 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/16/2010 | 45,919 | 1894.10 | 45 | 306 | 181 | 170 |
| 11/17/2010 | 45,434 | 1893.80 | 49 | 309 | 174 | 174 |
| 11/18/2010 | 44,952 | 1893.50 | 52 | 310 | 166 | 180 |
| 11/19/2010 | 44,455 | 1893.20 | 43 | 309 | 155 | 192 |
| 11/20/2010 | 44,127 | 1893.00 | 131 | 312 | 161 | 196 |
| 11/21/2010 | 43,799 | 1892.80 | 131 | 312 | 171 | 197 |
| 11/22/2010 | 43,555 | 1892.70 | 174 | 313 | 200 | 196 |
| 11/23/2010 | 44,077 | 1893.00 | 563 | 316 | 483 | 196 |
| 11/24/2010 | 43,995 | 1892.90 | 257 | 314 | 275 | 197 |
| 11/25/2010 | 43,718 | 1892.80 | 158 | 313 | 215 | 192 |
| 11/26/2010 | 43,376 | 1892.60 | 123 | 311 | 197 | 187 |
| 11/27/2010 | 43,604 | 1892.70 | 416 | 317 | 295 | 193 |
| 11/28/2010 | 43,653 | 1892.70 | 324 | 315 | 262 | 200 |
| 11/29/2010 | 43,458 | 1892.60 | 200 | 314 | 209 | 201 |
| 11/30/2010 | 43,165 | 1892.40 | 149 | 312 | 187 | 201 |
| 12/1/2010 | 42,873 | 1892.30 | 148 | 311 | 176 | 201 |
| 12/2/2010 | 42,922 | 1892.30 | 325 | 316 | 196 | 200 |
| 12/3/2010 | 44,571 | 1893.30 | 1,137 | 322 | 299 | 200 |
| 12/4/2010 | 45,251 | 1893.70 | 651 | 324 | 245 | 200 |
| 12/5/2010 | 46,020 | 1894.20 | 699 | 328 | 244 | 200 |
| 12/6/2010 | 50,371 | 1896.70 | 2,516 | 339 | 620 | 199 |
| 12/7/2010 | 51,945 | 1897.60 | 1,114 | 337 | 393 | 195 |
| 12/8/2010 | 53,234 | 1898.30 | 976 | 343 | 359 | 197 |
| 12/9/2010 | 54,505 | 1899.00 | 986 | 363 | 426 | 124 |
| 12/10/2010 | 55,045 | 1899.30 | 1,006 | 772 | 553 | 165 |
| 12/11/2010 | 55,008 | 1899.20 | 915 | 983 | 688 | 210 |
| 12/12/2010 | 54,859 | 1899.20 | 770 | 889 | 600 | 210 |
| 12/13/2010 | 54,691 | 1899.10 | 618 | 740 | 463 | 210 |
| 12/14/2010 | 55,739 | 1899.60 | 1,668 | 1,200 | 1,070 | 210 |
| 12/15/2010 | 55,457 | 1899.50 | 1,254 | 1,470 | 1,440 | 210 |
| 12/16/2010 | 55,139 | 1899.30 | 960 | 1,180 | 1,020 | 210 |
| 12/17/2010 | 55,064 | 1899.30 | 869 | 954 | 721 | 210 |
| 12/18/2010 | 56,136 | 1899.90 | 2,042 | 1,580 | 1,450 | 210 |
| 12/19/2010 | 58,323 | 1901.00 | 4,152 | 3,210 | 3,800 | 195 |
| 12/20/2010 | 57,529 | 1900.60 | 3,200 | 3,790 | 4,560 | 174 |
| 12/21/2010 | 56,820 | 1900.20 | 2,397 | 2,900 | 3,250 | 174 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/22/2010 | 57,011 | 1900.30 | 2,861 | 2,910 | 3,240 | 172 |
| 12/23/2010 | 56,306 | 1899.90 | 1,982 | 2,460 | 2,560 | 198 |
| 12/24/2010 | 55,796 | 1899.70 | 1,481 | 1,830 | 1,800 | 199 |
| 12/25/2010 | 56,003 | 1899.80 | 1,586 | 1,560 | 1,500 | 199 |
| 12/26/2010 | 56,439 | 1900.00 | 2,072 | 1,950 | 2,030 | 200 |
| 12/27/2010 | 56,173 | 1899.90 | 1,908 | 2,150 | 2,300 | 200 |
| 12/28/2010 | 58,968 | 1901.30 | 3,566 | 2,270 | 2,630 | 199 |
| 12/29/2010 | 58,968 | 1901.30 | 6,669 | 7,020 | 9,010 | 11 |
| 12/30/2010 | 57,260 | 1900.40 | 2,664 | 3,710 | 5,020 | 67 |
| 12/31/2010 | 56,477 | 1900.00 | 1,847 | 2,360 | 2,990 | 82 |
| 1/1/2011 | 56,022 | 1899.80 | 1,500 | 1,820 | 2,160 | 83 |
| 1/2/2011 | 55,777 | 1899.70 | 1,301 | 1,500 | 1,640 | 155 |
| 1/3/2011 | 55,532 | 1899.50 | 1,093 | 1,280 | 1,310 | 200 |
| 1/4/2011 | 55,307 | 1899.40 | 893 | 1,060 | 1,040 | 200 |
| 1/5/2011 | 55,082 | 1899.30 | 757 | 916 | 876 | 190 |
| 1/6/2011 | 55,008 | 1899.20 | 724 | 801 | 708 | 200 |
| 1/7/2011 | 54,896 | 1899.20 | 624 | 716 | 615 | 200 |
| 1/8/2011 | 54,821 | 1899.10 | 594 | 665 | 547 | 200 |
| 1/9/2011 | 54,747 | 1899.10 | 547 | 615 | 502 | 206 |
| 1/10/2011 | 54,672 | 1899.10 | 492 | 557 | 459 | 210 |
| 1/11/2011 | 54,710 | 1899.10 | 526 | 534 | 425 | 209 |
| 1/12/2011 | 54,747 | 1899.10 | 578 | 589 | 419 | 216 |
| 1/13/2011 | 55,607 | 1899.60 | 1,220 | 827 | 504 | 221 |
| 1/14/2011 | 55,664 | 1899.60 | 1,292 | 1,330 | 979 | 220 |
| 1/15/2011 | 55,607 | 1899.60 | 1,159 | 1,250 | 954 | 220 |
| 1/16/2011 | 55,796 | 1899.70 | 1,311 | 1,280 | 969 | 220 |
| 1/17/2011 | 55,871 | 1899.70 | 1,416 | 1,450 | 1,014 | 219 |
| 1/18/2011 | 55,645 | 1899.60 | 1,169 | 1,350 | 979 | 219 |
| 1/19/2011 | 55,438 | 1899.50 | 1,026 | 1,190 | 864 | 220 |
| 1/20/2011 | 55,232 | 1899.40 | 884 | 1,040 | 744 | 220 |
| 1/21/2011 | 55,101 | 1899.30 | 797 | 908 | 674 | 220 |
| 1/22/2011 | 55,008 | 1899.20 | 733 | 821 | 394 | 220 |
| 1/23/2011 | 54,896 | 1899.20 | 649 | 743 | 319 | 220 |
| 1/24/2011 | 54,803 | 1899.10 | 587 | 667 | 354 | 214 |
| 1/25/2011 | 54,710 | 1899.10 | 523 | 600 | 381 | 223 |
| 1/26/2011 | 54,635 | 1899.00 | 463 | 527 | 343 | 223 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/27/2011 | 54,561 | 1899.00 | 416 | 477 | 311 | 223 |
| 1/28/2011 | 54,487 | 1899.00 | 374 | 433 | 313 | 188 |
| 1/29/2011 | 54,468 | 1899.00 | 375 | 405 | 278 | 201 |
| 1/30/2011 | 54,524 | 1899.00 | 437 | 430 | 284 | 224 |
| 1/31/2011 | 54,431 | 1898.90 | 351 | 419 | 270 | 224 |
| 2/1/2011 | 54,302 | 1898.90 | 297 | 381 | 241 | 224 |
| 2/2/2011 | 54,191 | 1898.80 | 279 | 353 | 219 | 224 |
| 2/3/2011 | 54,043 | 1898.70 | 257 | 349 | 209 | 224 |
| 2/4/2011 | 53,877 | 1898.60 | 250 | 351 | 205 | 224 |
| 2/5/2011 | 53,693 | 1898.50 | 240 | 350 | 203 | 223 |
| 2/6/2011 | 53,509 | 1898.40 | 239 | 349 | 201 | 223 |
| 2/7/2011 | 53,307 | 1898.30 | 231 | 350 | 200 | 223 |
| 2/8/2011 | 53,105 | 1898.20 | 231 | 350 | 199 | 223 |
| 2/9/2011 | 52,850 | 1898.10 | 204 | 350 | 197 | 223 |
| 2/10/2011 | 52,595 | 1897.90 | 204 | 350 | 196 | 223 |
| 2/11/2011 | 52,324 | 1897.80 | 195 | 350 | 194 | 222 |
| 2/12/2011 | 52,071 | 1897.60 | 189 | 333 | 193 | 221 |
| 2/13/2011 | 52,053 | 1897.60 | 183 | 202 | 210 | 49 |
| 2/14/2011 | 52,378 | 1897.80 | 357 | 203 | 255 | 1 |
| 2/15/2011 | 57,819 | 1900.70 | 3,304 | 590 | 1,150 | 1 |
| 2/16/2011 | 58,032 | 1900.90 | 4,192 | 4,300 | 5,580 | 1 |
| 2/17/2011 | 56,992 | 1900.30 | 1,936 | 2,590 | 3,430 | 1 |
| 2/18/2011 | 56,401 | 1900.00 | 1,479 | 1,870 | 2,290 | 1 |
| 2/19/2011 | 56,079 | 1899.80 | 1,234 | 1,470 | 1,690 | 57 |
| 2/20/2011 | 55,758 | 1899.60 | 1,007 | 1,230 | 1,190 | 224 |
| 2/21/2011 | 55,495 | 1899.50 | 884 | 1,070 | 973 | 232 |
| 2/22/2011 | 55,307 | 1899.40 | 796 | 937 | 799 | 237 |
| 2/23/2011 | 55,213 | 1899.40 | 758 | 848 | 677 | 234 |
| 2/24/2011 | 55,232 | 1899.40 | 774 | 805 | 623 | 232 |
| 2/25/2011 | 55,232 | 1899.40 | 796 | 838 | 673 | 233 |
| 2/26/2011 | 55,139 | 1899.30 | 709 | 796 | 615 | 233 |
| 2/27/2011 | 55,045 | 1899.30 | 639 | 722 | 533 | 232 |
| 2/28/2011 | 55,026 | 1899.30 | 640 | 684 | 493 | 232 |
| 3/1/2011 | 55,045 | 1899.30 | 655 | 680 | 495 | 232 |
| 3/2/2011 | 56,534 | 1900.10 | 1,891 | 1,200 | 1,100 | 231 |
| 3/3/2011 | 56,496 | 1900.00 | 1,843 | 1,960 | 2,110 | 231 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/4/2011 | 56,136 | 1899.90 | 1,376 | 1,640 | 1,750 | 231 |
| 3/5/2011 | 55,984 | 1899.80 | 1,244 | 1,390 | 1,400 | 231 |
| 3/6/2011 | 56,686 | 1900.10 | 1,940 | 1,670 | 1,780 | 230 |
| 3/7/2011 | 56,686 | 1900.10 | 1,966 | 2,070 | 2,260 | 230 |
| 3/8/2011 | 56,401 | 1900.00 | 1,585 | 1,820 | 2,020 | 150 |
| 3/9/2011 | 56,230 | 1899.90 | 1,424 | 1,590 | 1,780 | 46 |
| 3/10/2011 | 56,344 | 1900.00 | 1,492 | 1,510 | 1,690 | 46 |
| 3/11/2011 | 56,211 | 1899.90 | 1,386 | 1,530 | 1,740 | 46 |
| 3/12/2011 | 56,003 | 1899.80 | 1,197 | 1,370 | 1,520 | 45 |
| 3/13/2011 | 56,629 | 1900.10 | 1,589 | 1,340 | 1,470 | 45 |
| 3/14/2011 | 56,858 | 1900.20 | 2,177 | 2,170 | 2,470 | 45 |
| 3/15/2011 | 59,007 | 1901.40 | 3,449 | 2,490 | 2,960 | 45 |
| 3/16/2011 | 58,948 | 1901.30 | 4,891 | 5,180 | 6,730 | 45 |
| 3/17/2011 | 57,819 | 1900.70 | 2,927 | 3,680 | 4,740 | 45 |
| 3/18/2011 | 58,752 | 1901.20 | 4,033 | 3,750 | 4,980 | 45 |
| 3/19/2011 | 58,245 | 1901.00 | 3,326 | 3,770 | 5,030 | 45 |
| 3/20/2011 | 59,046 | 1901.40 | 4,575 | 4,390 | 5,880 | 45 |
| 3/21/2011 | 57,780 | 1900.70 | 2,791 | 3,610 | 4,840 | 45 |
| 3/22/2011 | 57,145 | 1900.40 | 2,178 | 2,630 | 3,410 | 45 |
| 3/23/2011 | 57,606 | 1900.60 | 2,684 | 2,580 | 3,400 | 45 |
| 3/24/2011 | 58,421 | 1901.10 | 3,650 | 3,410 | 4,800 | 45 |
| 3/25/2011 | 58,460 | 1901.10 | 3,582 | 3,750 | 5,460 | 46 |
| 3/26/2011 | 58,694 | 1901.20 | 4,117 | 4,210 | 6,270 | 45 |
| 3/27/2011 | 58,071 | 1900.90 | 3,172 | 3,670 | 5,300 | 45 |
| 3/28/2011 | 57,645 | 1900.70 | 2,787 | 3,160 | 4,190 | 45 |
| 3/29/2011 | 57,299 | 1900.50 | 2,390 | 2,700 | 3,480 | 45 |
| 3/30/2011 | 57,780 | 1900.70 | 2,789 | 2,680 | 3,310 | 45 |
| 3/31/2011 | 58,246 | 1901.00 | 3,436 | 3,370 | 4,120 | 45 |
| 4/1/2011 | 58,323 | 1901.00 | 3,564 | 3,710 | 4,500 | 46 |
| 4/2/2011 | 59,341 | 1901.50 | 3,610 | 3,260 | 4,270 | 45 |
| 4/3/2011 | 61,576 | 1902.70 | 2,846 | 1,810 | 2,360 | 45 |
| 4/4/2011 | 64,212 | 1904.00 | 2,336 | 1,060 | 1,230 | 51 |
| 4/5/2011 | 65,483 | 1904.60 | 2,313 | 1,760 | 1,820 | 49 |
| 4/6/2011 | 65,106 | 1904.40 | 2,023 | 2,330 | 2,590 | 46 |
| 4/7/2011 | 64,440 | 1904.10 | 1,745 | 2,190 | 2,400 | 46 |
| 4/8/2011 | 64,026 | 1903.90 | 1,492 | 1,790 | 1,950 | 55 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/9/2011 | 64,026 | 1903.90 | 1,301 | 1,370 | 1,510 | 61 |
| 4/10/2011 | 64,523 | 1904.10 | 1,153 | 950 | 941 | 61 |
| 4/11/2011 | 64,793 | 1904.30 | 1,096 | 1,010 | 982 | 61 |
| 4/12/2011 | 65,588 | 1904.70 | 1,003 | 634 | 761 | 67 |
| 4/13/2011 | 66,917 | 1905.30 | 1,015 | 292 | 390 | 70 |
| 4/14/2011 | 67,984 | 1905.80 | 879 | 295 | 375 | 70 |
| 4/15/2011 | 69,059 | 1906.30 | 875 | 297 | 360 | 84 |
| 4/16/2011 | 70,386 | 1907.00 | 1,013 | 300 | 356 | 90 |
| 4/17/2011 | 71,021 | 1907.30 | 1,280 | 1,010 | 814 | 89 |
| 4/18/2011 | 71,043 | 1907.30 | 1,425 | 1,510 | 1,350 | 89 |
| 4/19/2011 | 70,364 | 1907.00 | 1,349 | 1,790 | 1,750 | 89 |
| 4/20/2011 | 69,841 | 1906.70 | 1,187 | 1,540 | 1,530 | 89 |
| 4/21/2011 | 69,993 | 1906.80 | 1,312 | 1,300 | 1,230 | 89 |
| 4/22/2011 | 69,885 | 1906.70 | 1,123 | 1,240 | 1,310 | 89 |
| 4/23/2011 | 70,015 | 1906.80 | 1,006 | 967 | 982 | 90 |
| 4/24/2011 | 70,758 | 1907.10 | 932 | 576 | 509 | 89 |
| 4/25/2011 | 70,977 | 1907.30 | 976 | 908 | 741 | 89 |
| 4/26/2011 | 71,109 | 1907.30 | 925 | 899 | 890 | 90 |
| 4/27/2011 | 71,700 | 1907.60 | 827 | 527 | 466 | 90 |
| 4/28/2011 | 72,147 | 1907.80 | 865 | 535 | 466 | 90 |
| 4/29/2011 | 72,457 | 1907.90 | 672 | 519 | 457 | 90 |
| 4/30/2011 | 72,724 | 1908.10 | 624 | 503 | 443 | 90 |
| 5/1/2011 | 72,835 | 1908.10 | 600 | 573 | 467 | 90 |
| 5/2/2011 | 72,880 | 1908.10 | 603 | 617 | 501 | 90 |
| 5/3/2011 | 73,058 | 1908.20 | 670 | 616 | 502 | 90 |
| 5/4/2011 | 73,125 | 1908.30 | 608 | 606 | 492 | 90 |
| 5/5/2011 | 73,259 | 1908.30 | 639 | 604 | 488 | 89 |
| 5/6/2011 | 73,371 | 1908.40 | 678 | 651 | 500 | 89 |
| 5/7/2011 | 73,170 | 1908.30 | 698 | 845 | 650 | 89 |
| 5/8/2011 | 72,902 | 1908.20 | 654 | 837 | 717 | 89 |
| 5/9/2011 | 72,924 | 1908.20 | 565 | 576 | 505 | 89 |
| 5/10/2011 | 73,036 | 1908.20 | 492 | 434 | 373 | 89 |
| 5/11/2011 | 73,125 | 1908.30 | 482 | 435 | 373 | 89 |
| 5/12/2011 | 73,058 | 1908.20 | 497 | 554 | 410 | 90 |
| 5/13/2011 | 72,902 | 1908.20 | 510 | 631 | 479 | 98 |
| 5/14/2011 | 72,769 | 1908.10 | 481 | 584 | 430 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/15/2011 | 72,724 | 1908.10 | 551 | 598 | 430 | 129 |
| 5/16/2011 | 72,657 | 1908.00 | 498 | 565 | 417 | 130 |
| 5/17/2011 | 72,746 | 1908.10 | 512 | 491 | 376 | 130 |
| 5/18/2011 | 72,813 | 1908.10 | 528 | 510 | 391 | 130 |
| 5/19/2011 | 72,791 | 1908.10 | 462 | 485 | 375 | 130 |
| 5/20/2011 | 72,835 | 1908.10 | 440 | 415 | 331 | 130 |
| 5/21/2011 | 72,969 | 1908.20 | 423 | 341 | 262 | 130 |
| 5/22/2011 | 73,103 | 1908.20 | 417 | 333 | 251 | 130 |
| 5/23/2011 | 73,192 | 1908.30 | 396 | 328 | 245 | 130 |
| 5/24/2011 | 73,237 | 1908.30 | 383 | 342 | 246 | 130 |
| 5/25/2011 | 73,505 | 1908.40 | 538 | 403 | 273 | 130 |
| 5/26/2011 | 73,483 | 1908.40 | 484 | 513 | 380 | 131 |
| 5/27/2011 | 73,505 | 1908.40 | 402 | 384 | 282 | 131 |
| 5/28/2011 | 73,528 | 1908.40 | 396 | 384 | 277 | 131 |
| 5/29/2011 | 73,483 | 1908.40 | 364 | 377 | 272 | 131 |
| 5/30/2011 | 73,416 | 1908.40 | 347 | 366 | 256 | 131 |
| 5/31/2011 | 73,573 | 1908.50 | 446 | 369 | 282 | 131 |
| 6/1/2011 | 73,148 | 1908.30 | 538 | 810 | 543 | 131 |
| 6/2/2011 | 72,325 | 1907.90 | 591 | 1,070 | 975 | 131 |
| 6/3/2011 | 72,192 | 1907.80 | 446 | 537 | 412 | 133 |
| 6/4/2011 | 72,924 | 1908.20 | 807 | 506 | 384 | 131 |
| 6/5/2011 | 73,036 | 1908.20 | 1,723 | 1,870 | 1,440 | 130 |
| 6/6/2011 | 71,771 | 1907.60 | 1,345 | 1,730 | 1,800 | 129 |
| 6/7/2011 | 71,837 | 1907.70 | 935 | 949 | 827 | 129 |
| 6/8/2011 | 71,881 | 1907.70 | 797 | 819 | 626 | 129 |
| 6/9/2011 | 72,037 | 1907.70 | 689 | 638 | 522 | 129 |
| 6/10/2011 | 72,258 | 1907.90 | 598 | 486 | 388 | 130 |
| 6/11/2011 | 72,480 | 1908.00 | 528 | 414 | 335 | 130 |
| 6/12/2011 | 72,613 | 1908.00 | 480 | 409 | 327 | 130 |
| 6/13/2011 | 72,746 | 1908.10 | 466 | 401 | 319 | 129 |
| 6/14/2011 | 72,791 | 1908.10 | 427 | 399 | 311 | 129 |
| 6/15/2011 | 72,880 | 1908.10 | 366 | 333 | 276 | 129 |
| 6/16/2011 | 73,014 | 1908.20 | 327 | 286 | 218 | 129 |
| 6/17/2011 | 72,991 | 1908.20 | 292 | 304 | 225 | 129 |
| 6/18/2011 | 72,947 | 1908.20 | 274 | 307 | 236 | 130 |
| 6/19/2011 | 72,924 | 1908.20 | 257 | 281 | 213 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/20/2011 | 72,924 | 1908.20 | 239 | 271 | 193 | 129 |
| 6/21/2011 | 72,924 | 1908.20 | 227 | 253 | 179 | 129 |
| 6/22/2011 | 72,947 | 1908.20 | 200 | 221 | 143 | 129 |
| 6/23/2011 | 72,991 | 1908.20 | 187 | 193 | 108 | 129 |
| 6/24/2011 | 73,014 | 1908.20 | 184 | 180 | 89 | 129 |
| 6/25/2011 | 72,991 | 1908.20 | 170 | 180 | 87 | 129 |
| 6/26/2011 | 72,969 | 1908.20 | 160 | 180 | 85 | 130 |
| 6/27/2011 | 72,924 | 1908.20 | 149 | 179 | 82 | 130 |
| 6/28/2011 | 73,036 | 1908.20 | 240 | 197 | 98 | 130 |
| 6/29/2011 | 73,036 | 1908.20 | 215 | 228 | 140 | 131 |
| 6/30/2011 | 72,969 | 1908.20 | 165 | 209 | 119 | 130 |
| 7/1/2011 | 72,924 | 1908.20 | 150 | 198 | 95 | 130 |
| 7/2/2011 | 72,835 | 1908.10 | 129 | 186 | 81 | 129 |
| 7/3/2011 | 72,724 | 1908.10 | 117 | 187 | 78 | 129 |
| 7/4/2011 | 72,613 | 1908.00 | 177 | 190 | 77 | 129 |
| 7/5/2011 | 72,480 | 1908.00 | 105 | 188 | 75 | 130 |
| 7/6/2011 | 72,325 | 1907.90 | 101 | 187 | 73 | 130 |
| 7/7/2011 | 72,170 | 1907.80 | 79 | 186 | 72 | 130 |
| 7/8/2011 | 71,992 | 1907.70 | 85 | 185 | 71 | 130 |
| 7/9/2011 | 71,815 | 1907.60 | 71 | 181 | 70 | 130 |
| 7/10/2011 | 71,616 | 1907.60 | 64 | 181 | 69 | 130 |
| 7/11/2011 | 71,395 | 1907.40 | 64 | 183 | 68 | 130 |
| 7/12/2011 | 71,197 | 1907.40 | 64 | 183 | 68 | 130 |
| 7/13/2011 | 70,977 | 1907.30 | 53 | 171 | 63 | 130 |
| 7/14/2011 | 70,779 | 1907.20 | 52 | 157 | 53 | 130 |
| 7/15/2011 | 70,604 | 1907.10 | 49 | 154 | 48 | 130 |
| 7/16/2011 | 70,429 | 1907.00 | 48 | 153 | 47 | 130 |
| 7/17/2011 | 70,233 | 1906.90 | 47 | 152 | 47 | 130 |
| 7/18/2011 | 70,059 | 1906.80 | 47 | 152 | 47 | 130 |
| 7/19/2011 | 69,885 | 1906.70 | 48 | 152 | 46 | 130 |
| 7/20/2011 | 69,667 | 1906.60 | 36 | 152 | 45 | 130 |
| 7/21/2011 | 69,471 | 1906.50 | 41 | 152 | 44 | 130 |
| 7/22/2011 | 69,276 | 1906.40 | 37 | 152 | 43 | 130 |
| 7/23/2011 | 69,059 | 1906.30 | 34 | 152 | 42 | 130 |
| 7/24/2011 | 68,843 | 1906.20 | 26 | 151 | 42 | 130 |
| 7/25/2011 | 68,606 | 1906.10 | 24 | 151 | 41 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/26/2011 | 68,370 | 1906.00 | 19 | 151 | 40 | 130 |
| 7/27/2011 | 68,155 | 1905.90 | 24 | 151 | 40 | 130 |
| 7/28/2011 | 67,877 | 1905.80 | 16 | 172 | 39 | 130 |
| 7/29/2011 | 67,449 | 1905.60 | 25 | 252 | 136 | 132 |
| 7/30/2011 | 67,194 | 1905.40 | 12 | 160 | 67 | 131 |
| 7/31/2011 | 66,938 | 1905.30 | 13 | 155 | 41 | 131 |
| 8/1/2011 | 66,684 | 1905.20 | 9 | 152 | 37 | 130 |
| 8/2/2011 | 66,409 | 1905.10 | 4 | 152 | 36 | 130 |
| 8/3/2011 | 66,134 | 1904.90 | 1 | 152 | 36 | 130 |
| 8/4/2011 | 65,882 | 1904.80 | 5 | 153 | 35 | 130 |
| 8/5/2011 | 65,609 | 1904.70 | 7 | 153 | 35 | 130 |
| 8/6/2011 | 65,357 | 1904.60 | 6 | 153 | 35 | 130 |
| 8/7/2011 | 65,085 | 1904.40 | 5 | 155 | 34 | 130 |
| 8/8/2011 | 64,814 | 1904.30 | 4 | 156 | 34 | 130 |
| 8/9/2011 | 64,544 | 1904.20 | 11 | 162 | 33 | 130 |
| 8/10/2011 | 64,253 | 1904.00 | 6 | 162 | 36 | 131 |
| 8/11/2011 | 63,964 | 1903.90 | -4 | 153 | 35 | 131 |
| 8/12/2011 | 63,675 | 1903.70 | -10 | 152 | 34 | 131 |
| 8/13/2011 | 63,387 | 1903.60 | -7 | 152 | 34 | 130 |
| 8/14/2011 | 63,100 | 1903.40 | -11 | 152 | 34 | 129 |
| 8/15/2011 | 62,813 | 1903.30 | -7 | 152 | 34 | 130 |
| 8/16/2011 | 62,528 | 1903.20 | -9 | 151 | 33 | 130 |
| 8/17/2011 | 62,223 | 1903.00 | -11 | 151 | 32 | 130 |
| 8/18/2011 | 61,939 | 1902.90 | -8 | 151 | 32 | 130 |
| 8/19/2011 | 61,656 | 1902.70 | -10 | 151 | 32 | 130 |
| 8/20/2011 | 61,374 | 1902.60 | -8 | 151 | 32 | 129 |
| 8/21/2011 | 61,072 | 1902.40 | -9 | 151 | 31 | 129 |
| 8/22/2011 | 60,791 | 1902.30 | -5 | 151 | 31 | 129 |
| 8/23/2011 | 60,451 | 1902.10 | -1 | 171 | 32 | 138 |
| 8/24/2011 | 60,212 | 1902.00 | 2 | 182 | 31 | 155 |
| 8/25/2011 | 59,875 | 1901.80 | -3 | 181 | 28 | 158 |
| 8/26/2011 | 59,499 | 1901.60 | -3 | 191 | 29 | 158 |
| 8/27/2011 | 59,105 | 1901.40 | -1 | 209 | 39 | 172 |
| 8/28/2011 | 58,713 | 1901.20 | -2 | 209 | 28 | 181 |
| 8/29/2011 | 58,323 | 1901.00 | -1 | 209 | 29 | 180 |
| 8/30/2011 | 57,935 | 1900.80 | -10 | 209 | 28 | 180 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/31/2011 | 57,529 | 1900.60 | -11 | 210 | 29 | 180 |
| 9/1/2011 | 57,126 | 1900.40 | -4 | 210 | 31 | 180 |
| 9/2/2011 | 56,725 | 1900.20 | -9 | 218 | 33 | 182 |
| 9/3/2011 | 56,287 | 1899.90 | -9 | 225 | 33 | 193 |
| 9/4/2011 | 55,852 | 1899.70 | -10 | 227 | 34 | 195 |
| 9/5/2011 | 55,420 | 1899.50 | -11 | 227 | 32 | 196 |
| 9/6/2011 | 54,989 | 1899.20 | -9 | 226 | 31 | 196 |
| 9/7/2011 | 54,561 | 1899.00 | -8 | 226 | 30 | 196 |
| 9/8/2011 | 54,135 | 1898.80 | -12 | 225 | 29 | 196 |
| 9/9/2011 | 53,693 | 1898.50 | -8 | 226 | 29 | 196 |
| 9/10/2011 | 53,252 | 1898.30 | -17 | 230 | 34 | 197 |
| 9/11/2011 | 52,814 | 1898.00 | -9 | 229 | 32 | 198 |
| 9/12/2011 | 52,378 | 1897.80 | -8 | 229 | 31 | 198 |
| 9/13/2011 | 51,927 | 1897.60 | -14 | 228 | 30 | 198 |
| 9/14/2011 | 51,512 | 1897.30 | -6 | 228 | 29 | 199 |
| 9/15/2011 | 51,064 | 1897.10 | -9 | 228 | 29 | 199 |
| 9/16/2011 | 50,637 | 1896.80 | -8 | 228 | 29 | 199 |
| 9/17/2011 | 50,212 | 1896.60 | -6 | 228 | 28 | 199 |
| 9/18/2011 | 49,788 | 1896.30 | -3 | 227 | 28 | 199 |
| 9/19/2011 | 49,349 | 1896.10 | -5 | 227 | 27 | 199 |
| 9/20/2011 | 48,947 | 1895.90 | -6 | 228 | 26 | 199 |
| 9/21/2011 | 48,513 | 1895.60 | -5 | 229 | 29 | 199 |
| 9/22/2011 | 48,080 | 1895.40 | -6 | 231 | 29 | 199 |
| 9/23/2011 | 47,633 | 1895.10 | -6 | 232 | 32 | 198 |
| 9/24/2011 | 47,205 | 1894.90 | -7 | 231 | 30 | 199 |
| 9/25/2011 | 46,763 | 1894.60 | -6 | 231 | 30 | 199 |
| 9/26/2011 | 46,323 | 1894.30 | -6 | 231 | 30 | 200 |
| 9/27/2011 | 45,919 | 1894.10 | -4 | 231 | 29 | 200 |
| 9/28/2011 | 45,401 | 1893.80 | -46 | 230 | 29 | 200 |
| 9/29/2011 | 44,969 | 1893.50 | -2 | 233 | 30 | 200 |
| 9/30/2011 | 44,521 | 1893.30 | -3 | 235 | 34 | 200 |
| 10/1/2011 | 44,077 | 1893.00 | 28 | 235 | 34 | 200 |
| 10/2/2011 | 43,636 | 1892.70 | -42 | 234 | 34 | 200 |
| 10/3/2011 | 43,441 | 1892.60 | 120 | 236 | 45 | 200 |
| 10/4/2011 | 43,197 | 1892.50 | 92 | 236 | 44 | 200 |
| 10/5/2011 | 43,100 | 1892.40 | 184 | 237 | 69 | 201 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/6/2011 | 42,776 | 1892.20 | 68 | 243 | 51 | 201 |
| 10/7/2011 | 42,342 | 1891.90 | 36 | 265 | 71 | 196 |
| 10/8/2011 | 41,880 | 1891.60 | 27 | 274 | 82 | 202 |
| 10/9/2011 | 41,387 | 1891.30 | 20 | 279 | 83 | 203 |
| 10/10/2011 | 41,134 | 1891.20 | 148 | 283 | 119 | 204 |
| 10/11/2011 | 40,866 | 1891.00 | 144 | 286 | 122 | 204 |
| 10/12/2011 | 40,412 | 1890.70 | 59 | 308 | 113 | 204 |
| 10/13/2011 | 39,837 | 1890.40 | 38 | 349 | 131 | 228 |
| 10/14/2011 | 39,250 | 1890.00 | 31 | 347 | 122 | 241 |
| 10/15/2011 | 38,716 | 1889.60 | 30 | 311 | 102 | 231 |
| 10/16/2011 | 38,185 | 1889.30 | 25 | 306 | 92 | 231 |
| 10/17/2011 | 37,643 | 1888.90 | 24 | 306 | 91 | 231 |
| 10/18/2011 | 37,107 | 1888.60 | 23 | 304 | 88 | 231 |
| 10/19/2011 | 36,603 | 1888.20 | 22 | 297 | 80 | 232 |
| 10/20/2011 | 36,104 | 1887.90 | 24 | 282 | 74 | 223 |
| 10/21/2011 | 35,668 | 1887.60 | 22 | 260 | 70 | 206 |
| 10/22/2011 | 35,263 | 1887.30 | 23 | 239 | 53 | 197 |
| 10/23/2011 | 34,890 | 1887.00 | 21 | 229 | 52 | 185 |
| 10/24/2011 | 34,519 | 1886.80 | 22 | 226 | 52 | 180 |
| 10/25/2011 | 34,151 | 1886.50 | 19 | 221 | 54 | 174 |
| 10/26/2011 | 33,897 | 1886.30 | 19 | 156 | 107 | 84 |
| 10/27/2011 | 33,729 | 1886.20 | 14 | 109 | 82 | 45 |
| 10/28/2011 | 33,575 | 1886.10 | 17 | 102 | 71 | 45 |
| 10/29/2011 | 33,435 | 1886.00 | 19 | 99 | 58 | 51 |
| 10/30/2011 | 33,296 | 1885.90 | 18 | 99 | 51 | 58 |
| 10/31/2011 | 33,144 | 1885.80 | 19 | 99 | 49 | 60 |
| 11/1/2011 | 32,992 | 1885.70 | 5 | 103 | 49 | 60 |
| 11/2/2011 | 32,785 | 1885.50 | 10 | 117 | 59 | 69 |
| 11/3/2011 | 32,579 | 1885.40 | 14 | 126 | 59 | 83 |
| 11/4/2011 | 32,373 | 1885.20 | 8 | 125 | 51 | 90 |
| 11/5/2011 | 32,237 | 1885.10 | 60 | 139 | 61 | 93 |
| 11/6/2011 | 32,088 | 1885.00 | 53 | 145 | 66 | 100 |
| 11/7/2011 | 31,763 | 1884.80 | -15 | 161 | 59 | 112 |
| 11/8/2011 | 31,508 | 1884.60 | 18 | 167 | 54 | 127 |
| 11/9/2011 | 31,227 | 1884.40 | 20 | 174 | 51 | 132 |
| 11/10/2011 | 30,935 | 1884.20 | 21 | 187 | 58 | 134 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/11/2011 | 30,657 | 1884.00 | 32 | 191 | 60 | 138 |
| 11/12/2011 | 30,356 | 1883.70 | 25 | 190 | 59 | 139 |
| 11/13/2011 | 30,069 | 1883.50 | 27 | 189 | 58 | 139 |
| 11/14/2011 | 29,771 | 1883.30 | 23 | 189 | 57 | 138 |
| 11/15/2011 | 29,475 | 1883.00 | 23 | 189 | 56 | 139 |
| 11/16/2011 | 29,194 | 1882.80 | 27 | 183 | 53 | 139 |
| 11/17/2011 | 28,928 | 1882.60 | 17 | 170 | 54 | 129 |
| 11/18/2011 | 28,701 | 1882.40 | 26 | 151 | 65 | 105 |
| 11/19/2011 | 28,601 | 1882.30 | 55 | 108 | 80 | 57 |
| 11/20/2011 | 28,526 | 1882.30 | 56 | 100 | 76 | 46 |
| 11/21/2011 | 28,414 | 1882.20 | 36 | 100 | 70 | 46 |
| 11/22/2011 | 28,314 | 1882.10 | 44 | 100 | 74 | 46 |
| 11/23/2011 | 28,289 | 1882.10 | 82 | 100 | 82 | 46 |
| 11/24/2011 | 28,701 | 1882.40 | 303 | 100 | 151 | 46 |
| 11/25/2011 | 28,777 | 1882.50 | 151 | 119 | 109 | 46 |
| 11/26/2011 | 28,676 | 1882.40 | 97 | 155 | 136 | 46 |
| 11/27/2011 | 28,501 | 1882.30 | 68 | 164 | 148 | 46 |
| 11/28/2011 | 28,314 | 1882.10 | 61 | 164 | 144 | 46 |
| 11/29/2011 | 28,091 | 1881.90 | 58 | 179 | 143 | 53 |
| 11/30/2011 | 27,832 | 1881.70 | 47 | 186 | 139 | 67 |
| 12/1/2011 | 27,624 | 1881.50 | 72 | 186 | 128 | 74 |
| 12/2/2011 | 27,345 | 1881.30 | 21 | 170 | 141 | 55 |
| 12/3/2011 | 27,128 | 1881.10 | 44 | 162 | 139 | 45 |
| 12/4/2011 | 26,900 | 1880.90 | 33 | 162 | 137 | 45 |
| 12/5/2011 | 26,496 | 1880.60 | -56 | 167 | 136 | 45 |
| 12/6/2011 | 26,261 | 1880.40 | 43 | 170 | 136 | 45 |
| 12/7/2011 | 26,040 | 1880.20 | 44 | 164 | 132 | 46 |
| 12/8/2011 | 25,831 | 1880.00 | 47 | 160 | 127 | 45 |
| 12/9/2011 | 25,612 | 1879.80 | 40 | 159 | 126 | 46 |
| 12/10/2011 | 25,395 | 1879.70 | 42 | 160 | 125 | 46 |
| 12/11/2011 | 25,180 | 1879.50 | 33 | 160 | 125 | 46 |
| 12/12/2011 | 24,966 | 1879.30 | 43 | 158 | 125 | 46 |
| 12/13/2011 | 24,743 | 1879.10 | 29 | 161 | 125 | 46 |
| 12/14/2011 | 24,510 | 1878.90 | 32 | 165 | 131 | 46 |
| 12/15/2011 | 24,312 | 1878.70 | 54 | 162 | 132 | 46 |
| 12/16/2011 | 24,093 | 1878.50 | 42 | 160 | 127 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/17/2011 | 23,878 | 1878.30 | 43 | 159 | 127 | 46 |
| 12/18/2011 | 23,664 | 1878.10 | 30 | 159 | 126 | 46 |
| 12/19/2011 | 23,440 | 1877.90 | 27 | 158 | 126 | 46 |
| 12/20/2011 | 23,219 | 1877.70 | 28 | 159 | 126 | 46 |
| 12/21/2011 | 23,000 | 1877.50 | 30 | 160 | 125 | 46 |
| 12/22/2011 | 22,773 | 1877.30 | 24 | 159 | 124 | 46 |
| 12/23/2011 | 22,538 | 1877.00 | 25 | 159 | 124 | 46 |
| 12/24/2011 | 22,314 | 1876.80 | 27 | 159 | 123 | 46 |
| 12/25/2011 | 22,093 | 1876.60 | 27 | 158 | 123 | 46 |
| 12/26/2011 | 21,875 | 1876.40 | 26 | 158 | 123 | 46 |
| 12/27/2011 | 21,649 | 1876.20 | 27 | 162 | 125 | 46 |
| 12/28/2011 | 21,414 | 1875.90 | 29 | 164 | 130 | 46 |
| 12/29/2011 | 21,192 | 1875.70 | 30 | 164 | 131 | 46 |
| 12/30/2011 | 20,990 | 1875.50 | 42 | 164 | 136 | 46 |
| 12/31/2011 | 20,791 | 1875.30 | 46 | 163 | 134 | 46 |
| 1/1/2012 | 20,575 | 1875.10 | 38 | 163 | 132 | 46 |
| 1/2/2012 | 20,369 | 1874.90 | 34 | 162 | 131 | 46 |
| 1/3/2012 | 20,146 | 1874.60 | 32 | 162 | 131 | 46 |
| 1/4/2012 | 19,918 | 1874.40 | 26 | 162 | 129 | 46 |
| 1/5/2012 | 19,691 | 1874.10 | 30 | 162 | 128 | 46 |
| 1/6/2012 | 19,466 | 1873.90 | 28 | 161 | 127 | 46 |
| 1/7/2012 | 19,243 | 1873.70 | 29 | 163 | 127 | 46 |
| 1/8/2012 | 19,006 | 1873.40 | 30 | 163 | 126 | 46 |
| 1/9/2012 | 18,788 | 1873.10 | 29 | 163 | 126 | 46 |
| 1/10/2012 | 18,564 | 1872.90 | 28 | 159 | 123 | 46 |
| 1/11/2012 | 18,342 | 1872.60 | 26 | 156 | 118 | 46 |
| 1/12/2012 | 18,123 | 1872.40 | 28 | 156 | 118 | 46 |
| 1/13/2012 | 17,905 | 1872.10 | 25 | 156 | 117 | 45 |
| 1/14/2012 | 17,905 | 1872.10 | 26 | 156 | 117 | 45 |
| 1/15/2012 | 17,461 | 1871.60 | 25 | 155 | 116 | 45 |
| 1/16/2012 | 17,234 | 1871.30 | 23 | 159 | 116 | 45 |
| 1/17/2012 | 16,985 | 1871.00 | 21 | 165 | 126 | 45 |
| 1/18/2012 | 16,770 | 1870.80 | 36 | 165 | 126 | 45 |
| 1/19/2012 | 16,739 | 1870.70 | 134 | 167 | 160 | 45 |
| 1/20/2012 | 19,181 | 1873.60 | 1,242 | 180 | 543 | 45 |
| 1/21/2012 | 23,409 | 1877.90 | 2,591 | 182 | 886 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/22/2012 | 24,754 | 1879.10 | 868 | 182 | 372 | 43 |
| 1/23/2012 | 28,852 | 1882.50 | 2,212 | 195 | 908 | 44 |
| 1/24/2012 | 29,849 | 1883.30 | 858 | 186 | 362 | 45 |
| 1/25/2012 | 30,750 | 1884.00 | 635 | 186 | 277 | 45 |
| 1/26/2012 | 32,088 | 1885.00 | 875 | 187 | 263 | 45 |
| 1/27/2012 | 32,881 | 1885.60 | 611 | 198 | 237 | 45 |
| 1/28/2012 | 33,227 | 1885.90 | 377 | 204 | 220 | 45 |
| 1/29/2012 | 33,407 | 1886.00 | 284 | 204 | 206 | 45 |
| 1/30/2012 | 33,505 | 1886.10 | 231 | 204 | 198 | 45 |
| 1/31/2012 | 33,519 | 1886.10 | 199 | 204 | 191 | 45 |
| 2/1/2012 | 33,547 | 1886.10 | 193 | 205 | 192 | 45 |
| 2/2/2012 | 33,505 | 1886.10 | 166 | 204 | 186 | 45 |
| 2/3/2012 | 33,435 | 1886.00 | 146 | 204 | 182 | 45 |
| 2/4/2012 | 33,338 | 1885.90 | 132 | 204 | 179 | 45 |
| 2/5/2012 | 33,213 | 1885.90 | 125 | 204 | 177 | 45 |
| 2/6/2012 | 33,088 | 1885.80 | 124 | 204 | 175 | 45 |
| 2/7/2012 | 33,282 | 1885.90 | 282 | 205 | 192 | 45 |
| 2/8/2012 | 33,533 | 1886.10 | 334 | 204 | 202 | 45 |
| 2/9/2012 | 33,589 | 1886.10 | 224 | 204 | 188 | 45 |
| 2/10/2012 | 33,631 | 1886.20 | 208 | 202 | 185 | 45 |
| 2/11/2012 | 33,687 | 1886.20 | 219 | 202 | 188 | 45 |
| 2/12/2012 | 33,729 | 1886.20 | 211 | 202 | 184 | 45 |
| 2/13/2012 | 33,827 | 1886.30 | 248 | 203 | 219 | 45 |
| 2/14/2012 | 33,925 | 1886.40 | 237 | 202 | 215 | 45 |
| 2/15/2012 | 33,925 | 1886.40 | 197 | 202 | 202 | 45 |
| 2/16/2012 | 33,897 | 1886.30 | 175 | 204 | 193 | 45 |
| 2/17/2012 | 33,841 | 1886.30 | 163 | 204 | 188 | 45 |
| 2/18/2012 | 33,771 | 1886.30 | 158 | 204 | 186 | 44 |
| 2/19/2012 | 33,701 | 1886.20 | 149 | 205 | 183 | 45 |
| 2/20/2012 | 33,603 | 1886.10 | 140 | 204 | 181 | 44 |
| 2/21/2012 | 33,491 | 1886.10 | 133 | 204 | 179 | 44 |
| 2/22/2012 | 33,366 | 1886.00 | 127 | 203 | 177 | 44 |
| 2/23/2012 | 33,241 | 1885.90 | 119 | 197 | 172 | 44 |
| 2/24/2012 | 33,116 | 1885.80 | 116 | 192 | 167 | 44 |
| 2/25/2012 | 32,992 | 1885.70 | 102 | 182 | 158 | 44 |
| 2/26/2012 | 32,868 | 1885.60 | 104 | 175 | 150 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/27/2012 | 32,744 | 1885.50 | 91 | 167 | 147 | 45 |
| 2/28/2012 | 32,716 | 1885.50 | 131 | 162 | 146 | 46 |
| 2/29/2012 | 33,061 | 1885.70 | 313 | 150 | 219 | 46 |
| 3/1/2012 | 33,407 | 1886.00 | 327 | 150 | 262 | 46 |
| 3/2/2012 | 33,631 | 1886.20 | 268 | 149 | 223 | 46 |
| 3/3/2012 | 33,785 | 1886.30 | 224 | 149 | 194 | 45 |
| 3/4/2012 | 34,010 | 1886.40 | 263 | 159 | 193 | 45 |
| 3/5/2012 | 34,222 | 1886.60 | 268 | 170 | 194 | 45 |
| 3/6/2012 | 34,335 | 1886.70 | 225 | 183 | 195 | 45 |
| 3/7/2012 | 34,377 | 1886.70 | 190 | 189 | 191 | 45 |
| 3/8/2012 | 34,363 | 1886.70 | 167 | 189 | 184 | 45 |
| 3/9/2012 | 34,335 | 1886.70 | 157 | 189 | 180 | 45 |
| 3/10/2012 | 34,307 | 1886.60 | 155 | 189 | 177 | 45 |
| 3/11/2012 | 34,278 | 1886.60 | 155 | 189 | 179 | 45 |
| 3/12/2012 | 34,250 | 1886.60 | 150 | 189 | 176 | 45 |
| 3/13/2012 | 38,534 | 1889.50 | 2,327 | 203 | 671 | 45 |
| 3/14/2012 | 44,028 | 1893.00 | 3,142 | 206 | 912 | 45 |
| 3/15/2012 | 47,273 | 1894.90 | 1,909 | 206 | 563 | 45 |
| 3/16/2012 | 54,672 | 1899.10 | 3,941 | 217 | 1,166 | 45 |
| 3/17/2012 | 57,107 | 1900.40 | 3,701 | 2,406 | 2,987 | 45 |
| 3/18/2012 | 56,325 | 1900.00 | 1,883 | 2,341 | 2,826 | 45 |
| 3/19/2012 | 55,890 | 1899.70 | 1,215 | 1,429 | 1,682 | 45 |
| 3/20/2012 | 55,720 | 1899.60 | 934 | 974 | 1,088 | 45 |
| 3/21/2012 | 55,909 | 1899.70 | 907 | 770 | 933 | 45 |
| 3/22/2012 | 56,915 | 1900.30 | 942 | 414 | 574 | 45 |
| 3/23/2012 | 57,858 | 1900.80 | 696 | 207 | 324 | 45 |
| 3/24/2012 | 58,792 | 1901.20 | 689 | 207 | 324 | 45 |
| 3/25/2012 | 59,875 | 1901.80 | 760 | 209 | 350 | 45 |
| 3/26/2012 | 60,791 | 1902.30 | 684 | 209 | 341 | 45 |
| 3/27/2012 | 62,528 | 1903.20 | 1,136 | 1,242 | 1,151 | 55 |
| 3/28/2012 | 63,161 | 1903.50 | 6,403 | 6,431 | 7,819 | 60 |
| 3/29/2012 | 60,731 | 1902.20 | 2,459 | 3,811 | 4,640 | 51 |
| 3/30/2012 | 60,571 | 1902.20 | 2,890 | 3,052 | 3,803 | 45 |
| 3/31/2012 | 61,959 | 1902.90 | 4,085 | 3,563 | 4,386 | 45 |
| 4/1/2012 | 61,899 | 1902.80 | 3,524 | 3,692 | 4,677 | 45 |
| 4/2/2012 | 62,406 | 1903.10 | 2,322 | 2,005 | 2,689 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/3/2012 | 63,881 | 1903.80 | 1,777 | 938 | 1,301 | 44 |
| 4/4/2012 | 64,710 | 1904.20 | 1,459 | 988 | 1,066 | 44 |
| 4/5/2012 | 64,898 | 1904.30 | 1,226 | 1,065 | 1,224 | 52 |
| 4/6/2012 | 64,835 | 1904.30 | 1,032 | 999 | 1,052 | 63 |
| 4/7/2012 | 64,752 | 1904.30 | 902 | 892 | 959 | 66 |
| 4/8/2012 | 64,898 | 1904.30 | 780 | 669 | 738 | 55 |
| 4/9/2012 | 65,106 | 1904.40 | 662 | 544 | 573 | 45 |
| 4/10/2012 | 65,546 | 1904.60 | 714 | 577 | 632 | 45 |
| 4/11/2012 | 66,366 | 1905.00 | 1,037 | 518 | 609 | 45 |
| 4/12/2012 | 67,130 | 1905.40 | 1,764 | 1,396 | 1,508 | 45 |
| 4/13/2012 | 67,321 | 1905.50 | 1,931 | 1,797 | 2,110 | 45 |
| 4/14/2012 | 67,321 | 1905.50 | 1,507 | 1,455 | 1,779 | 57 |
| 4/15/2012 | 67,620 | 1905.70 | 1,304 | 1,087 | 1,204 | 90 |
| 4/16/2012 | 67,941 | 1905.80 | 1,209 | 977 | 1,078 | 90 |
| 4/17/2012 | 68,219 | 1905.90 | 1,120 | 918 | 965 | 90 |
| 4/18/2012 | 68,606 | 1906.10 | 1,069 | 802 | 859 | 90 |
| 4/19/2012 | 69,059 | 1906.30 | 1,016 | 737 | 751 | 90 |
| 4/20/2012 | 69,623 | 1906.60 | 1,108 | 748 | 741 | 89 |
| 4/21/2012 | 70,189 | 1906.90 | 1,110 | 763 | 739 | 91 |
| 4/22/2012 | 70,342 | 1907.00 | 1,099 | 957 | 859 | 91 |
| 4/23/2012 | 70,211 | 1906.90 | 1,040 | 1,059 | 987 | 91 |
| 4/24/2012 | 69,863 | 1906.70 | 922 | 1,050 | 969 | 91 |
| 4/25/2012 | 69,667 | 1906.60 | 833 | 881 | 854 | 91 |
| 4/26/2012 | 69,754 | 1906.70 | 829 | 730 | 680 | 91 |
| 4/27/2012 | 69,580 | 1906.60 | 690 | 728 | 659 | 91 |
| 4/28/2012 | 69,471 | 1906.50 | 589 | 618 | 585 | 90 |
| 4/29/2012 | 69,493 | 1906.50 | 527 | 521 | 458 | 90 |
| 4/30/2012 | 69,558 | 1906.60 | 491 | 470 | 437 | 90 |
| 5/1/2012 | 69,732 | 1906.70 | 429 | 375 | 346 | 91 |
| 5/2/2012 | 69,798 | 1906.70 | 376 | 374 | 340 | 91 |
| 5/3/2012 | 69,928 | 1906.80 | 407 | 367 | 340 | 91 |
| 5/4/2012 | 70,037 | 1906.80 | 384 | 340 | 316 | 91 |
| 5/5/2012 | 70,059 | 1906.80 | 330 | 321 | 285 | 91 |
| 5/6/2012 | 70,059 | 1906.80 | 300 | 321 | 279 | 91 |
| 5/7/2012 | 70,037 | 1906.80 | 294 | 322 | 275 | 91 |
| 5/8/2012 | 69,972 | 1906.80 | 275 | 322 | 273 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/9/2012 | 69,907 | 1906.70 | 270 | 322 | 271 | 90 |
| 5/10/2012 | 69,819 | 1906.70 | 262 | 322 | 269 | 90 |
| 5/11/2012 | 69,689 | 1906.60 | 240 | 322 | 264 | 90 |
| 5/12/2012 | 69,536 | 1906.60 | 228 | 321 | 262 | 90 |
| 5/13/2012 | 69,384 | 1906.50 | 221 | 321 | 260 | 90 |
| 5/14/2012 | 69,146 | 1906.40 | 210 | 349 | 260 | 108 |
| 5/15/2012 | 68,887 | 1906.30 | 202 | 362 | 258 | 130 |
| 5/16/2012 | 68,585 | 1906.10 | 192 | 363 | 256 | 90 |
| 5/17/2012 | 68,262 | 1906.00 | 113 | 352 | 250 | 131 |
| 5/18/2012 | 67,449 | 1905.60 | 163 | 644 | 394 | 131 |
| 5/19/2012 | 66,324 | 1905.00 | 164 | 808 | 687 | 129 |
| 5/20/2012 | 65,525 | 1904.60 | 160 | 610 | 479 | 129 |
| 5/21/2012 | 64,918 | 1904.30 | 144 | 492 | 378 | 129 |
| 5/22/2012 | 64,441 | 1904.10 | 126 | 405 | 303 | 129 |
| 5/23/2012 | 63,964 | 1903.90 | 88 | 365 | 261 | 130 |
| 5/24/2012 | 63,552 | 1903.70 | 91 | 318 | 225 | 130 |
| 5/25/2012 | 63,285 | 1903.50 | 106 | 270 | 192 | 130 |
| 5/26/2012 | 63,100 | 1903.40 | 132 | 237 | 152 | 130 |
| 5/27/2012 | 62,854 | 1903.30 | 106 | 237 | 150 | 130 |
| 5/28/2012 | 62,650 | 1903.20 | 95 | 225 | 140 | 130 |
| 5/29/2012 | 62,426 | 1903.10 | 93 | 216 | 122 | 127 |
| 5/30/2012 | 62,182 | 1903.00 | 84 | 232 | 134 | 130 |
| 5/31/2012 | 61,919 | 1902.80 | 82 | 232 | 135 | 130 |
| 6/1/2012 | 61,636 | 1902.70 | 70 | 228 | 129 | 130 |
| 6/2/2012 | 61,354 | 1902.60 | 63 | 228 | 127 | 130 |
| 6/3/2012 | 61,052 | 1902.40 | 79 | 228 | 126 | 130 |
| 6/4/2012 | 60,791 | 1902.30 | 58 | 226 | 127 | 130 |
| 6/5/2012 | 60,531 | 1902.10 | 59 | 220 | 120 | 130 |
| 6/6/2012 | 60,272 | 1902.00 | 69 | 217 | 114 | 130 |
| 6/7/2012 | 59,994 | 1901.90 | 60 | 217 | 112 | 130 |
| 6/8/2012 | 59,697 | 1901.70 | 57 | 217 | 109 | 130 |
| 6/9/2012 | 59,302 | 1901.50 | 54 | 266 | 157 | 132 |
| 6/10/2012 | 58,948 | 1901.30 | 46 | 247 | 125 | 136 |
| 6/11/2012 | 58,655 | 1901.20 | 46 | 215 | 112 | 130 |
| 6/12/2012 | 58,343 | 1901.00 | 33 | 211 | 99 | 130 |
| 6/13/2012 | 58,051 | 1900.90 | 38 | 204 | 94 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/14/2012 | 57,742 | 1900.70 | 30 | 194 | 83 | 130 |
| 6/15/2012 | 57,472 | 1900.60 | 40 | 185 | 72 | 130 |
| 6/16/2012 | 57,222 | 1900.40 | 47 | 182 | 68 | 130 |
| 6/17/2012 | 56,954 | 1900.30 | 27 | 178 | 65 | 129 |
| 6/18/2012 | 56,686 | 1900.10 | 20 | 171 | 59 | 129 |
| 6/19/2012 | 56,420 | 1900.00 | 15 | 166 | 53 | 129 |
| 6/20/2012 | 56,136 | 1899.90 | 15 | 163 | 49 | 129 |
| 6/21/2012 | 55,871 | 1899.70 | 22 | 163 | 50 | 129 |
| 6/22/2012 | 55,626 | 1899.60 | 31 | 163 | 52 | 130 |
| 6/23/2012 | 55,382 | 1899.40 | 29 | 160 | 51 | 130 |
| 6/24/2012 | 55,120 | 1899.30 | 16 | 157 | 45 | 130 |
| 6/25/2012 | 54,877 | 1899.20 | 14 | 154 | 39 | 130 |
| 6/26/2012 | 54,635 | 1899.00 | 17 | 149 | 34 | 130 |
| 6/27/2012 | 54,394 | 1898.90 | 15 | 148 | 28 | 130 |
| 6/28/2012 | 54,154 | 1898.80 | 12 | 148 | 27 | 129 |
| 6/29/2012 | 53,914 | 1898.60 | 19 | 147 | 27 | 129 |
| 6/30/2012 | 53,693 | 1898.50 | 28 | 147 | 27 | 129 |
| 7/1/2012 | 53,435 | 1898.40 | 12 | 147 | 27 | 129 |
| 7/2/2012 | 53,179 | 1898.20 | 2 | 147 | 26 | 129 |
| 7/3/2012 | 52,923 | 1898.10 | 7 | 147 | 26 | 130 |
| 7/4/2012 | 52,668 | 1898.00 | 3 | 147 | 25 | 130 |
| 7/5/2012 | 52,396 | 1897.80 | -2 | 147 | 25 | 130 |
| 7/6/2012 | 52,143 | 1897.70 | -2 | 146 | 25 | 130 |
| 7/7/2012 | 51,872 | 1897.50 | 0 | 146 | 25 | 130 |
| 7/8/2012 | 51,602 | 1897.40 | -5 | 146 | 24 | 130 |
| 7/9/2012 | 51,315 | 1897.20 | -6 | 146 | 24 | 129 |
| 7/10/2012 | 51,046 | 1897.10 | -3 | 146 | 25 | 129 |
| 7/11/2012 | 50,779 | 1896.90 | 4 | 146 | 25 | 128 |
| 7/12/2012 | 50,460 | 1896.70 | -5 | 146 | 24 | 129 |
| 7/13/2012 | 50,230 | 1896.60 | 0 | 146 | 23 | 130 |
| 7/14/2012 | 49,947 | 1896.40 | -17 | 146 | 23 | 130 |
| 7/15/2012 | 49,682 | 1896.30 | -8 | 145 | 23 | 130 |
| 7/16/2012 | 49,384 | 1896.10 | -15 | 145 | 23 | 130 |
| 7/17/2012 | 49,086 | 1895.90 | -12 | 145 | 23 | 130 |
| 7/18/2012 | 48,825 | 1895.80 | -6 | 145 | 23 | 130 |
| 7/19/2012 | 48,530 | 1895.60 | -10 | 145 | 23 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/20/2012 | 48,271 | 1895.50 | -10 | 145 | 22 | 130 |
| 7/21/2012 | 47,977 | 1895.30 | -8 | 145 | 22 | 130 |
| 7/22/2012 | 47,701 | 1895.10 | -11 | 146 | 22 | 129 |
| 7/23/2012 | 47,461 | 1895.00 | 11 | 147 | 23 | 129 |
| 7/24/2012 | 47,154 | 1894.80 | -11 | 147 | 24 | 129 |
| 7/25/2012 | 46,882 | 1894.70 | -12 | 147 | 23 | 129 |
| 7/26/2012 | 46,577 | 1894.50 | -14 | 148 | 22 | 129 |
| 7/27/2012 | 46,272 | 1894.30 | -13 | 148 | 22 | 130 |
| 7/28/2012 | 45,986 | 1894.10 | -5 | 147 | 22 | 130 |
| 7/29/2012 | 45,684 | 1894.00 | -19 | 147 | 22 | 130 |
| 7/30/2012 | 45,384 | 1893.80 | -13 | 149 | 21 | 131 |
| 7/31/2012 | 45,085 | 1893.60 | 10 | 153 | 26 | 129 |
| 8/1/2012 | 44,786 | 1893.40 | -40 | 153 | 26 | 129 |
| 8/2/2012 | 44,472 | 1893.20 | -16 | 153 | 25 | 130 |
| 8/3/2012 | 44,176 | 1893.10 | -11 | 153 | 24 | 131 |
| 8/4/2012 | 43,865 | 1892.90 | -15 | 152 | 24 | 131 |
| 8/5/2012 | 43,555 | 1892.70 | -14 | 152 | 24 | 130 |
| 8/6/2012 | 43,246 | 1892.50 | -16 | 152 | 24 | 131 |
| 8/7/2012 | 42,938 | 1892.30 | -17 | 151 | 23 | 131 |
| 8/8/2012 | 42,631 | 1892.10 | -26 | 149 | 23 | 131 |
| 8/9/2012 | 42,326 | 1891.90 | -22 | 148 | 23 | 131 |
| 8/10/2012 | 42,023 | 1891.70 | -22 | 148 | 23 | 131 |
| 8/11/2012 | 41,705 | 1891.50 | -24 | 148 | 23 | 130 |
| 8/12/2012 | 41,403 | 1891.30 | -22 | 148 | 23 | 130 |
| 8/13/2012 | 41,086 | 1891.10 | -22 | 148 | 23 | 130 |
| 8/14/2012 | 40,787 | 1891.00 | -21 | 147 | 22 | 131 |
| 8/15/2012 | 40,475 | 1890.80 | -23 | 147 | 21 | 132 |
| 8/16/2012 | 40,163 | 1890.60 | -23 | 147 | 21 | 132 |
| 8/17/2012 | 39,852 | 1890.40 | -26 | 147 | 21 | 132 |
| 8/18/2012 | 39,543 | 1890.20 | -27 | 146 | 21 | 132 |
| 8/19/2012 | 39,235 | 1890.00 | -24 | 146 | 21 | 131 |
| 8/20/2012 | 38,929 | 1889.80 | -24 | 146 | 21 | 131 |
| 8/21/2012 | 38,625 | 1889.60 | -23 | 146 | 21 | 130 |
| 8/22/2012 | 38,276 | 1889.30 | -26 | 161 | 28 | 125 |
| 8/23/2012 | 37,883 | 1889.10 | -24 | 195 | 70 | 134 |
| 8/24/2012 | 37,584 | 1888.90 | -19 | 145 | 33 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/25/2012 | 37,286 | 1888.70 | -20 | 145 | 19 | 130 |
| 8/26/2012 | 36,973 | 1888.50 | -25 | 145 | 19 | 131 |
| 8/27/2012 | 36,677 | 1888.30 | -19 | 146 | 19 | 131 |
| 8/28/2012 | 36,383 | 1888.10 | -17 | 146 | 19 | 130 |
| 8/29/2012 | 36,090 | 1887.90 | -20 | 146 | 19 | 130 |
| 8/30/2012 | 35,784 | 1887.70 | -22 | 145 | 19 | 129 |
| 8/31/2012 | 35,480 | 1887.50 | -18 | 146 | 19 | 130 |
| 9/1/2012 | 35,191 | 1887.30 | -12 | 147 | 19 | 130 |
| 9/2/2012 | 34,904 | 1887.10 | -14 | 147 | 19 | 130 |
| 9/3/2012 | 34,604 | 1886.80 | -13 | 147 | 19 | 130 |
| 9/4/2012 | 34,321 | 1886.60 | -12 | 147 | 19 | 130 |
| 9/5/2012 | 34,038 | 1886.40 | -9 | 147 | 19 | 130 |
| 9/6/2012 | 33,743 | 1886.20 | -14 | 147 | 19 | 130 |
| 9/7/2012 | 33,449 | 1886.00 | -10 | 147 | 19 | 130 |
| 9/8/2012 | 33,171 | 1885.80 | -11 | 147 | 19 | 128 |
| 9/9/2012 | 32,868 | 1885.60 | -13 | 147 | 19 | 129 |
| 9/10/2012 | 32,592 | 1885.40 | -10 | 148 | 19 | 130 |
| 9/11/2012 | 32,305 | 1885.20 | -7 | 147 | 19 | 130 |
| 9/12/2012 | 32,020 | 1885.00 | -8 | 148 | 19 | 130 |
| 9/13/2012 | 31,736 | 1884.80 | -1 | 148 | 19 | 130 |
| 9/14/2012 | 31,455 | 1884.60 | -11 | 148 | 19 | 130 |
| 9/15/2012 | 31,174 | 1884.30 | -6 | 148 | 19 | 130 |
| 9/16/2012 | 30,935 | 1884.20 | -8 | 129 | 30 | 105 |
| 9/17/2012 | 30,710 | 1884.00 | -13 | 116 | 21 | 90 |
| 9/18/2012 | 30,487 | 1883.80 | -11 | 116 | 21 | 90 |
| 9/19/2012 | 30,264 | 1883.60 | -12 | 115 | 21 | 90 |
| 9/20/2012 | 30,056 | 1883.50 | -11 | 114 | 20 | 90 |
| 9/21/2012 | 29,849 | 1883.30 | -9 | 112 | 20 | 90 |
| 9/22/2012 | 29,629 | 1883.20 | -9 | 112 | 19 | 90 |
| 9/23/2012 | 29,411 | 1883.00 | -11 | 111 | 19 | 90 |
| 9/24/2012 | 29,220 | 1882.80 | -8 | 111 | 19 | 90 |
| 9/25/2012 | 29,004 | 1882.70 | -8 | 111 | 19 | 90 |
| 9/26/2012 | 28,802 | 1882.50 | -7 | 111 | 19 | 90 |
| 9/27/2012 | 28,601 | 1882.30 | 5 | 111 | 19 | 90 |
| 9/28/2012 | 28,389 | 1882.20 | -1 | 111 | 19 | 90 |
| 9/29/2012 | 28,177 | 1882.00 | -1 | 111 | 19 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/30/2012 | 27,980 | 1881.80 | -7 | 111 | 19 | 90 |
| 10/1/2012 | 27,783 | 1881.70 | -6 | 111 | 19 | 90 |
| 10/2/2012 | 27,563 | 1881.50 | 7 | 118 | 20 | 90 |
| 10/3/2012 | 27,333 | 1881.30 | -26 | 122 | 26 | 90 |
| 10/4/2012 | 27,104 | 1881.10 | -10 | 122 | 27 | 90 |
| 10/5/2012 | 26,876 | 1880.90 | -9 | 122 | 27 | 90 |
| 10/6/2012 | 26,650 | 1880.70 | -7 | 121 | 26 | 90 |
| 10/7/2012 | 26,437 | 1880.60 | -9 | 120 | 26 | 90 |
| 10/8/2012 | 26,203 | 1880.40 | -7 | 123 | 28 | 91 |
| 10/9/2012 | 25,970 | 1880.20 | -7 | 125 | 33 | 91 |
| 10/10/2012 | 25,750 | 1880.00 | -5 | 125 | 33 | 91 |
| 10/11/2012 | 25,521 | 1879.80 | -7 | 125 | 33 | 90 |
| 10/12/2012 | 25,293 | 1879.60 | -6 | 125 | 33 | 91 |
| 10/13/2012 | 25,078 | 1879.40 | -2 | 125 | 33 | 91 |
| 10/14/2012 | 24,854 | 1879.20 | -2 | 125 | 32 | 90 |
| 10/15/2012 | 24,632 | 1879.00 | -3 | 125 | 32 | 91 |
| 10/16/2012 | 24,433 | 1878.80 | -2 | 116 | 42 | 78 |
| 10/17/2012 | 24,246 | 1878.60 | 0 | 108 | 32 | 72 |
| 10/18/2012 | 24,061 | 1878.50 | -1 | 108 | 32 | 72 |
| 10/19/2012 | 23,889 | 1878.30 | 0 | 104 | 38 | 66 |
| 10/20/2012 | 23,707 | 1878.10 | 0 | 100 | 36 | 61 |
| 10/21/2012 | 23,547 | 1878.00 | 4 | 100 | 37 | 60 |
| 10/22/2012 | 23,462 | 1877.90 | 51 | 96 | 52 | 51 |
| 10/23/2012 | 23,366 | 1877.80 | 25 | 93 | 45 | 46 |
| 10/24/2012 | 23,292 | 1877.80 | 49 | 93 | 52 | 46 |
| 10/25/2012 | 23,177 | 1877.60 | 19 | 93 | 44 | 46 |
| 10/26/2012 | 23,041 | 1877.50 | 17 | 93 | 42 | 46 |
| 10/27/2012 | 22,927 | 1877.40 | 20 | 92 | 42 | 46 |
| 10/28/2012 | 22,804 | 1877.30 | 18 | 92 | 42 | 46 |
| 10/29/2012 | 22,671 | 1877.20 | 18 | 92 | 42 | 46 |
| 10/30/2012 | 22,548 | 1877.00 | 26 | 92 | 42 | 46 |
| 10/31/2012 | 22,477 | 1877.00 | 47 | 93 | 48 | 46 |
| 11/1/2012 | 22,365 | 1876.90 | 28 | 93 | 50 | 46 |
| 11/2/2012 | 22,254 | 1876.80 | 26 | 93 | 45 | 46 |
| 11/3/2012 | 22,143 | 1876.70 | 23 | 92 | 45 | 46 |
| 11/4/2012 | 22,023 | 1876.50 | 19 | 92 | 44 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/5/2012 | 21,895 | 1876.40 | 20 | 93 | 45 | 46 |
| 11/6/2012 | 21,688 | 1876.20 | 10 | 134 | 87 | 53 |
| 11/7/2012 | 21,561 | 1876.10 | 17 | 96 | 51 | 46 |
| 11/8/2012 | 21,453 | 1876.00 | 32 | 94 | 49 | 46 |
| 11/9/2012 | 21,327 | 1875.80 | 19 | 93 | 45 | 46 |
| 11/10/2012 | 21,201 | 1875.70 | 15 | 92 | 42 | 46 |
| 11/11/2012 | 21,076 | 1875.60 | 17 | 92 | 41 | 46 |
| 11/12/2012 | 20,952 | 1875.50 | 18 | 92 | 41 | 46 |
| 11/13/2012 | 20,857 | 1875.40 | 31 | 92 | 42 | 46 |
| 11/14/2012 | 20,744 | 1875.30 | 23 | 92 | 41 | 46 |
| 11/15/2012 | 20,622 | 1875.10 | 23 | 92 | 40 | 46 |
| 11/16/2012 | 20,546 | 1875.00 | 41 | 92 | 42 | 46 |
| 11/17/2012 | 20,612 | 1875.10 | 119 | 93 | 60 | 46 |
| 11/18/2012 | 20,820 | 1875.30 | 191 | 92 | 71 | 46 |
| 11/19/2012 | 20,810 | 1875.30 | 91 | 102 | 65 | 47 |
| 11/20/2012 | 22,722 | 1877.20 | 1,008 | 116 | 334 | 47 |
| 11/21/2012 | 24,202 | 1878.60 | 1,015 | 142 | 296 | 47 |
| 11/22/2012 | 24,356 | 1878.70 | 268 | 191 | 201 | 46 |
| 11/23/2012 | 24,246 | 1878.60 | 143 | 209 | 209 | 46 |
| 11/24/2012 | 24,072 | 1878.50 | 100 | 208 | 201 | 47 |
| 11/25/2012 | 23,846 | 1878.30 | 79 | 208 | 197 | 47 |
| 11/26/2012 | 23,568 | 1878.00 | 67 | 229 | 195 | 47 |
| 11/27/2012 | 23,177 | 1877.60 | 68 | 288 | 278 | 50 |
| 11/28/2012 | 23,942 | 1878.40 | 556 | 215 | 264 | 46 |
| 11/29/2012 | 28,513 | 1882.30 | 2,407 | 226 | 642 | 46 |
| 11/30/2012 | 43,767 | 1892.80 | 8,490 | 255 | 1,954 | 46 |
| 12/1/2012 | 49,911 | 1896.40 | 3,531 | 254 | 859 | 46 |
| 12/2/2012 | 61,717 | 1902.70 | 15,574 | 10,101 | 12,102 | 7 |
| 12/3/2012 | 57,626 | 1900.60 | 2,833 | 4,770 | 6,850 | 73 |
| 12/4/2012 | 58,071 | 1900.90 | 2,580 | 2,480 | 3,168 | 150 |
| 12/5/2012 | 58,518 | 1901.10 | 4,429 | 4,425 | 6,043 | 95 |
| 12/6/2012 | 56,973 | 1900.30 | 1,964 | 2,803 | 3,858 | 151 |
| 12/7/2012 | 56,117 | 1899.80 | 1,232 | 1,690 | 2,127 | 151 |
| 12/8/2012 | 55,589 | 1899.60 | 873 | 1,154 | 1,352 | 151 |
| 12/9/2012 | 55,213 | 1899.40 | 673 | 878 | 921 | 170 |
| 12/10/2012 | 54,933 | 1899.20 | 542 | 698 | 632 | 199 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/11/2012 | 54,728 | 1899.10 | 470 | 583 | 470 | 199 |
| 12/12/2012 | 54,561 | 1899.00 | 412 | 508 | 386 | 199 |
| 12/13/2012 | 54,413 | 1898.90 | 342 | 433 | 311 | 200 |
| 12/14/2012 | 54,265 | 1898.80 | 295 | 391 | 262 | 201 |
| 12/15/2012 | 54,228 | 1898.80 | 326 | 373 | 241 | 202 |
| 12/16/2012 | 54,302 | 1898.90 | 399 | 382 | 281 | 202 |
| 12/17/2012 | 56,174 | 1899.90 | 1,866 | 1,143 | 1,389 | 201 |
| 12/18/2012 | 55,626 | 1899.60 | 978 | 1,265 | 1,509 | 200 |
| 12/19/2012 | 55,213 | 1899.40 | 690 | 912 | 976 | 200 |
| 12/20/2012 | 55,195 | 1899.30 | 715 | 749 | 769 | 200 |
| 12/21/2012 | 60,431 | 1902.10 | 7,097 | 4,888 | 6,593 | 82 |
| 12/22/2012 | 59,282 | 1901.50 | 5,139 | 5,925 | 8,404 | 1 |
| 12/23/2012 | 60,411 | 1902.10 | 5,776 | 5,482 | 7,965 | 16 |
| 12/24/2012 | 58,032 | 1900.90 | 3,216 | 4,542 | 6,687 | 24 |
| 12/25/2012 | 57,241 | 1900.40 | 2,124 | 2,664 | 3,797 | 46 |
| 12/26/2012 | 56,992 | 1900.30 | 2,040 | 2,277 | 3,172 | 109 |
| 12/27/2012 | 56,420 | 1900.00 | 1,572 | 1,940 | 2,524 | 153 |
| 12/28/2012 | 56,003 | 1899.80 | 1,204 | 1,489 | 1,825 | 153 |
| 12/29/2012 | 55,702 | 1899.60 | 1,004 | 1,217 | 1,396 | 154 |
| 12/30/2012 | 55,457 | 1899.50 | 851 | 1,027 | 1,107 | 155 |
| 12/31/2012 | 55,251 | 1899.40 | 736 | 882 | 902 | 155 |
| 1/1/2013 | 55,101 | 1899.30 | 647 | 766 | 740 | 155 |
| 1/2/2013 | 54,970 | 1899.20 | 589 | 682 | 623 | 155 |
| 1/3/2013 | 54,877 | 1899.20 | 541 | 624 | 539 | 155 |
| 1/4/2013 | 54,803 | 1899.10 | 508 | 578 | 481 | 155 |
| 1/5/2013 | 54,896 | 1899.20 | 578 | 565 | 473 | 155 |
| 1/6/2013 | 55,101 | 1899.30 | 734 | 676 | 641 | 155 |
| 1/7/2013 | 55,082 | 1899.30 | 679 | 713 | 666 | 155 |
| 1/8/2013 | 55,120 | 1899.30 | 704 | 717 | 654 | 155 |
| 1/9/2013 | 55,157 | 1899.30 | 763 | 749 | 670 | 155 |
| 1/10/2013 | 55,101 | 1899.30 | 730 | 758 | 678 | 155 |
| 1/11/2013 | 55,008 | 1899.20 | 645 | 705 | 615 | 155 |
| 1/12/2013 | 54,914 | 1899.20 | 588 | 651 | 543 | 155 |
| 1/13/2013 | 54,784 | 1899.10 | 531 | 596 | 479 | 156 |
| 1/14/2013 | 54,710 | 1899.10 | 486 | 546 | 428 | 156 |
| 1/15/2013 | 54,635 | 1899.00 | 444 | 497 | 383 | 156 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/16/2013 | 54,580 | 1899.00 | 428 | 465 | 351 | 155 |
| 1/17/2013 | 54,580 | 1899.00 | 425 | 451 | 336 | 155 |
| 1/18/2013 | 54,561 | 1899.00 | 428 | 448 | 330 | 155 |
| 1/19/2013 | 54,561 | 1899.00 | 419 | 444 | 324 | 155 |
| 1/20/2013 | 54,524 | 1899.00 | 402 | 432 | 313 | 155 |
| 1/21/2013 | 54,450 | 1898.90 | 392 | 438 | 310 | 155 |
| 1/22/2013 | 54,394 | 1898.90 | 387 | 429 | 305 | 155 |
| 1/23/2013 | 54,821 | 1899.10 | 559 | 418 | 297 | 155 |
| 1/24/2013 | 55,589 | 1899.60 | 1,243 | 901 | 681 | 155 |
| 1/25/2013 | 55,476 | 1899.50 | 899 | 1,006 | 859 | 170 |
| 1/26/2013 | 55,326 | 1899.40 | 800 | 922 | 727 | 202 |
| 1/27/2013 | 55,157 | 1899.30 | 727 | 815 | 616 | 202 |
| 1/28/2013 | 55,008 | 1899.20 | 613 | 704 | 500 | 201 |
| 1/29/2013 | 54,859 | 1899.20 | 540 | 621 | 418 | 201 |
| 1/30/2013 | 54,747 | 1899.10 | 492 | 560 | 361 | 201 |
| 1/31/2013 | 54,672 | 1899.10 | 453 | 508 | 320 | 201 |
| 2/1/2013 | 54,598 | 1899.00 | 420 | 470 | 288 | 201 |
| 2/2/2013 | 54,524 | 1899.00 | 385 | 437 | 261 | 201 |
| 2/3/2013 | 54,450 | 1898.90 | 367 | 412 | 242 | 201 |
| 2/4/2013 | 54,394 | 1898.90 | 346 | 393 | 228 | 201 |
| 2/5/2013 | 54,339 | 1898.90 | 335 | 377 | 214 | 201 |
| 2/6/2013 | 54,265 | 1898.80 | 312 | 363 | 203 | 201 |
| 2/7/2013 | 54,283 | 1898.80 | 345 | 358 | 203 | 201 |
| 2/8/2013 | 54,209 | 1898.80 | 295 | 351 | 197 | 201 |
| 2/9/2013 | 54,172 | 1898.80 | 278 | 309 | 187 | 176 |
| 2/10/2013 | 54,135 | 1898.80 | 261 | 290 | 182 | 151 |
| 2/11/2013 | 54,117 | 1898.80 | 245 | 271 | 174 | 143 |
| 2/12/2013 | 54,135 | 1898.80 | 234 | 239 | 171 | 115 |
| 2/13/2013 | 54,154 | 1898.80 | 225 | 227 | 166 | 100 |
| 2/14/2013 | 54,154 | 1898.80 | 212 | 228 | 166 | 99 |
| 2/15/2013 | 54,172 | 1898.80 | 221 | 227 | 164 | 99 |
| 2/16/2013 | 54,172 | 1898.80 | 212 | 227 | 163 | 99 |
| 2/17/2013 | 54,172 | 1898.80 | 211 | 227 | 162 | 99 |
| 2/18/2013 | 54,154 | 1898.80 | 203 | 227 | 162 | 99 |
| 2/19/2013 | 54,209 | 1898.80 | 237 | 211 | 195 | 22 |
| 2/20/2013 | 54,265 | 1898.80 | 199 | 195 | 199 | 22 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/21/2013 | 54,265 | 1898.80 | 177 | 187 | 195 | 22 |
| 2/22/2013 | 54,339 | 1898.90 | 212 | 184 | 188 | 22 |
| 2/23/2013 | 54,320 | 1898.90 | 170 | 188 | 192 | 22 |
| 2/24/2013 | 54,339 | 1898.90 | 177 | 183 | 188 | 22 |
| 2/25/2013 | 54,339 | 1898.90 | 172 | 181 | 185 | 22 |
| 2/26/2013 | 54,320 | 1898.90 | 160 | 179 | 183 | 22 |
| 2/27/2013 | 54,320 | 1898.90 | 168 | 177 | 180 | 22 |
| 2/28/2013 | 54,302 | 1898.90 | 152 | 175 | 177 | 22 |
| 3/1/2013 | 54,302 | 1898.90 | 162 | 174 | 177 | 22 |
| 3/2/2013 | 54,320 | 1898.90 | 169 | 176 | 177 | 22 |
| 3/3/2013 | 54,339 | 1898.90 | 171 | 177 | 177 | 22 |
| 3/4/2013 | 54,357 | 1898.90 | 176 | 180 | 180 | 22 |
| 3/5/2013 | 54,431 | 1898.90 | 224 | 210 | 197 | 22 |
| 3/6/2013 | 54,989 | 1899.20 | 647 | 365 | 384 | 22 |
| 3/7/2013 | 54,877 | 1899.20 | 418 | 460 | 486 | 22 |
| 3/8/2013 | 54,710 | 1899.10 | 300 | 379 | 396 | 22 |
| 3/9/2013 | 54,617 | 1899.00 | 266 | 321 | 322 | 22 |
| 3/10/2013 | 54,561 | 1899.00 | 255 | 288 | 280 | 46 |
| 3/11/2013 | 54,542 | 1899.00 | 248 | 271 | 260 | 46 |
| 3/12/2013 | 54,542 | 1899.00 | 280 | 272 | 255 | 45 |
| 3/13/2013 | 54,580 | 1899.00 | 284 | 279 | 255 | 46 |
| 3/14/2013 | 54,561 | 1899.00 | 272 | 280 | 255 | 46 |
| 3/15/2013 | 54,524 | 1899.00 | 259 | 275 | 251 | 46 |
| 3/16/2013 | 54,505 | 1899.00 | 253 | 269 | 245 | 46 |
| 3/17/2013 | 54,487 | 1899.00 | 240 | 256 | 235 | 46 |
| 3/18/2013 | 54,450 | 1898.90 | 225 | 246 | 227 | 46 |
| 3/19/2013 | 54,487 | 1899.00 | 235 | 225 | 214 | 46 |
| 3/20/2013 | 55,064 | 1899.30 | 518 | 227 | 232 | 59 |
| 3/21/2013 | 55,420 | 1899.50 | 427 | 223 | 215 | 66 |
| 3/22/2013 | 55,626 | 1899.60 | 325 | 219 | 201 | 66 |
| 3/23/2013 | 55,777 | 1899.70 | 283 | 213 | 199 | 60 |
| 3/24/2013 | 55,871 | 1899.70 | 258 | 209 | 195 | 56 |
| 3/25/2013 | 55,947 | 1899.70 | 247 | 210 | 193 | 55 |
| 3/26/2013 | 55,984 | 1899.80 | 226 | 209 | 191 | 55 |
| 3/27/2013 | 56,060 | 1899.80 | 227 | 199 | 188 | 52 |
| 3/28/2013 | 56,136 | 1899.90 | 217 | 188 | 179 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/29/2013 | 56,192 | 1899.90 | 207 | 187 | 177 | 45 |
| 3/30/2013 | 56,287 | 1899.90 | 226 | 188 | 177 | 45 |
| 3/31/2013 | 56,534 | 1900.10 | 296 | 180 | 175 | 45 |
| 4/1/2013 | 56,725 | 1900.20 | 296 | 202 | 177 | 45 |
| 4/2/2013 | 56,744 | 1900.20 | 242 | 227 | 209 | 45 |
| 4/3/2013 | 56,801 | 1900.20 | 239 | 223 | 203 | 45 |
| 4/4/2013 | 57,587 | 1900.60 | 626 | 225 | 237 | 45 |
| 4/5/2013 | 58,382 | 1901.00 | 643 | 226 | 246 | 45 |
| 4/6/2013 | 58,870 | 1901.30 | 485 | 227 | 235 | 45 |
| 4/7/2013 | 59,223 | 1901.50 | 420 | 228 | 230 | 45 |
| 4/8/2013 | 59,499 | 1901.60 | 396 | 247 | 246 | 50 |
| 4/9/2013 | 59,677 | 1901.70 | 335 | 257 | 238 | 60 |
| 4/10/2013 | 59,756 | 1901.70 | 307 | 258 | 236 | 60 |
| 4/11/2013 | 59,835 | 1901.80 | 282 | 251 | 228 | 60 |
| 4/12/2013 | 59,855 | 1901.80 | 266 | 253 | 226 | 60 |
| 4/13/2013 | 59,835 | 1901.80 | 242 | 252 | 224 | 60 |
| 4/14/2013 | 59,776 | 1901.80 | 225 | 265 | 224 | 60 |
| 4/15/2013 | 59,637 | 1901.70 | 208 | 294 | 236 | 81 |
| 4/16/2013 | 59,479 | 1901.60 | 211 | 289 | 224 | 90 |
| 4/17/2013 | 59,341 | 1901.50 | 209 | 286 | 220 | 89 |
| 4/18/2013 | 59,145 | 1901.40 | 175 | 288 | 217 | 90 |
| 4/19/2013 | 58,909 | 1901.30 | 157 | 290 | 219 | 91 |
| 4/20/2013 | 58,713 | 1901.20 | 165 | 278 | 210 | 91 |
| 4/21/2013 | 58,518 | 1901.10 | 137 | 264 | 198 | 91 |
| 4/22/2013 | 58,362 | 1901.00 | 133 | 253 | 187 | 90 |
| 4/23/2013 | 58,168 | 1900.90 | 116 | 243 | 174 | 90 |
| 4/24/2013 | 58,013 | 1900.80 | 117 | 236 | 167 | 90 |
| 4/25/2013 | 57,838 | 1900.80 | 109 | 237 | 159 | 90 |
| 4/26/2013 | 57,645 | 1900.70 | 108 | 243 | 166 | 90 |
| 4/27/2013 | 57,472 | 1900.60 | 105 | 237 | 158 | 90 |
| 4/28/2013 | 57,279 | 1900.50 | 108 | 230 | 148 | 90 |
| 4/29/2013 | 57,107 | 1900.40 | 104 | 227 | 142 | 89 |
| 4/30/2013 | 56,915 | 1900.30 | 112 | 220 | 135 | 89 |
| 5/1/2013 | 56,725 | 1900.20 | 71 | 215 | 124 | 89 |
| 5/2/2013 | 56,515 | 1900.10 | 83 | 215 | 121 | 90 |
| 5/3/2013 | 56,344 | 1900.00 | 118 | 215 | 120 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/4/2013 | 56,098 | 1899.80 | 82 | 217 | 120 | 89 |
| 5/5/2013 | 55,890 | 1899.70 | 75 | 218 | 121 | 90 |
| 5/6/2013 | 55,777 | 1899.70 | 126 | 218 | 126 | 91 |
| 5/7/2013 | 55,626 | 1899.60 | 96 | 212 | 120 | 91 |
| 5/8/2013 | 55,438 | 1899.50 | 89 | 213 | 118 | 89 |
| 5/9/2013 | 55,139 | 1899.30 | 87 | 270 | 160 | 95 |
| 5/10/2013 | 54,933 | 1899.20 | 75 | 224 | 130 | 91 |
| 5/11/2013 | 54,710 | 1899.10 | 71 | 218 | 122 | 90 |
| 5/12/2013 | 54,450 | 1898.90 | 57 | 216 | 119 | 90 |
| 5/13/2013 | 54,209 | 1898.80 | 54 | 217 | 122 | 90 |
| 5/14/2013 | 53,895 | 1898.60 | 75 | 246 | 124 | 105 |
| 5/15/2013 | 53,693 | 1898.50 | 131 | 264 | 122 | 131 |
| 5/16/2013 | 53,325 | 1898.30 | 44 | 260 | 124 | 131 |
| 5/17/2013 | 52,978 | 1898.10 | 39 | 255 | 117 | 132 |
| 5/18/2013 | 52,650 | 1898.00 | 36 | 240 | 109 | 131 |
| 5/19/2013 | 52,342 | 1897.80 | 35 | 227 | 97 | 129 |
| 5/20/2013 | 52,035 | 1897.60 | 29 | 223 | 94 | 129 |
| 5/21/2013 | 51,674 | 1897.40 | 10 | 222 | 93 | 129 |
| 5/22/2013 | 51,351 | 1897.20 | 7 | 216 | 89 | 129 |
| 5/23/2013 | 51,046 | 1897.10 | 22 | 214 | 85 | 129 |
| 5/24/2013 | 50,761 | 1896.90 | 22 | 207 | 82 | 129 |
| 5/25/2013 | 50,477 | 1896.70 | 19 | 198 | 75 | 129 |
| 5/26/2013 | 50,212 | 1896.60 | 18 | 193 | 69 | 129 |
| 5/27/2013 | 49,982 | 1896.50 | 29 | 187 | 68 | 129 |
| 5/28/2013 | 49,753 | 1896.30 | 24 | 183 | 66 | 129 |
| 5/29/2013 | 49,489 | 1896.20 | 15 | 183 | 65 | 128 |
| 5/30/2013 | 49,226 | 1896.00 | 14 | 184 | 64 | 129 |
| 5/31/2013 | 48,947 | 1895.90 | 6 | 186 | 64 | 129 |
| 6/1/2013 | 48,704 | 1895.70 | 33 | 172 | 76 | 107 |
| 6/2/2013 | 48,496 | 1895.60 | 39 | 155 | 68 | 96 |
| 6/3/2013 | 48,288 | 1895.50 | 30 | 147 | 55 | 96 |
| 6/4/2013 | 48,080 | 1895.40 | 25 | 145 | 46 | 96 |
| 6/5/2013 | 47,891 | 1895.30 | 29 | 143 | 47 | 95 |
| 6/6/2013 | 47,684 | 1895.10 | 23 | 142 | 44 | 95 |
| 6/7/2013 | 47,478 | 1895.00 | 25 | 142 | 44 | 94 |
| 6/8/2013 | 47,256 | 1894.90 | 20 | 142 | 44 | 94 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/9/2013 | 47,052 | 1894.80 | 17 | 142 | 45 | 94 |
| 6/10/2013 | 46,814 | 1894.60 | 18 | 146 | 46 | 94 |
| 6/11/2013 | 46,594 | 1894.50 | 20 | 143 | 48 | 94 |
| 6/12/2013 | 46,357 | 1894.40 | 7 | 139 | 44 | 94 |
| 6/13/2013 | 46,137 | 1894.20 | 12 | 137 | 37 | 94 |
| 6/14/2013 | 45,919 | 1894.10 | 16 | 142 | 35 | 94 |
| 6/15/2013 | 45,701 | 1894.00 | 22 | 149 | 38 | 95 |
| 6/16/2013 | 45,480 | 1893.80 | 20 | 148 | 33 | 95 |
| 6/17/2013 | 45,270 | 1893.70 | 25 | 150 | 33 | 95 |
| 6/18/2013 | 45,052 | 1893.60 | 13 | 132 | 31 | 95 |
| 6/19/2013 | 44,819 | 1893.40 | -5 | 119 | 29 | 95 |
| 6/20/2013 | 44,604 | 1893.30 | 3 | 119 | 29 | 95 |
| 6/21/2013 | 44,406 | 1893.20 | 4 | 116 | 28 | 95 |
| 6/22/2013 | 44,209 | 1893.10 | 5 | 114 | 25 | 95 |
| 6/23/2013 | 44,012 | 1893.00 | 9 | 116 | 25 | 95 |
| 6/24/2013 | 43,832 | 1892.80 | 27 | 120 | 31 | 95 |
| 6/25/2013 | 43,946 | 1892.90 | 171 | 119 | 49 | 95 |
| 6/26/2013 | 43,881 | 1892.90 | 75 | 118 | 39 | 95 |
| 6/27/2013 | 43,750 | 1892.80 | 46 | 118 | 31 | 94 |
| 6/28/2013 | 43,571 | 1892.70 | 26 | 119 | 29 | 94 |
| 6/29/2013 | 43,393 | 1892.60 | 20 | 119 | 27 | 95 |
| 6/30/2013 | 43,181 | 1892.50 | 9 | 120 | 27 | 94 |
| 7/1/2013 | 43,003 | 1892.30 | 14 | 121 | 27 | 94 |
| 7/2/2013 | 42,793 | 1892.20 | 8 | 121 | 26 | 94 |
| 7/3/2013 | 42,567 | 1892.10 | 0 | 122 | 26 | 94 |
| 7/4/2013 | 42,358 | 1891.90 | 1 | 118 | 25 | 95 |
| 7/5/2013 | 42,166 | 1891.80 | 0 | 118 | 23 | 95 |
| 7/6/2013 | 41,943 | 1891.70 | -3 | 118 | 23 | 95 |
| 7/7/2013 | 41,721 | 1891.50 | 0 | 118 | 22 | 95 |
| 7/8/2013 | 41,514 | 1891.40 | 0 | 118 | 22 | 95 |
| 7/9/2013 | 41,292 | 1891.30 | -10 | 118 | 22 | 95 |
| 7/10/2013 | 41,071 | 1891.10 | -5 | 119 | 22 | 95 |
| 7/11/2013 | 40,835 | 1891.00 | -7 | 118 | 22 | 95 |
| 7/12/2013 | 40,599 | 1890.80 | -10 | 120 | 22 | 95 |
| 7/13/2013 | 40,366 | 1890.70 | -7 | 123 | 25 | 95 |
| 7/14/2013 | 40,132 | 1890.50 | -9 | 120 | 24 | 95 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/15/2013 | 39,899 | 1890.40 | -12 | 119 | 24 | 95 |
| 7/16/2013 | 39,666 | 1890.20 | -12 | 120 | 24 | 95 |
| 7/17/2013 | 39,435 | 1890.10 | -10 | 121 | 24 | 95 |
| 7/18/2013 | 39,199 | 1889.90 | -10 | 122 | 24 | 95 |
| 7/19/2013 | 38,970 | 1889.80 | -6 | 123 | 23 | 95 |
| 7/20/2013 | 38,736 | 1889.60 | -6 | 123 | 23 | 95 |
| 7/21/2013 | 38,503 | 1889.50 | -8 | 123 | 23 | 95 |
| 7/22/2013 | 38,270 | 1889.30 | -9 | 123 | 23 | 94 |
| 7/23/2013 | 38,034 | 1889.20 | -9 | 124 | 23 | 94 |
| 7/24/2013 | 37,793 | 1889.00 | -10 | 125 | 23 | 94 |
| 7/25/2013 | 37,548 | 1888.90 | -13 | 123 | 23 | 94 |
| 7/26/2013 | 37,305 | 1888.70 | -13 | 126 | 22 | 94 |
| 7/27/2013 | 37,061 | 1888.50 | -13 | 124 | 22 | 94 |
| 7/28/2013 | 36,817 | 1888.40 | -13 | 126 | 21 | 95 |
| 7/29/2013 | 36,573 | 1888.20 | -13 | 126 | 21 | 96 |
| 7/30/2013 | 36,333 | 1888.00 | -13 | 121 | 21 | 96 |
| 7/31/2013 | 36,094 | 1887.90 | -17 | 119 | 21 | 96 |
| 8/1/2013 | 35,861 | 1887.70 | -15 | 118 | 21 | 96 |
| 8/2/2013 | 35,625 | 1887.60 | -16 | 118 | 21 | 96 |
| 8/3/2013 | 35,393 | 1887.40 | -12 | 117 | 21 | 96 |
| 8/4/2013 | 35,162 | 1887.20 | -14 | 117 | 21 | 96 |
| 8/5/2013 | 34,932 | 1887.10 | -12 | 117 | 21 | 96 |
| 8/6/2013 | 34,704 | 1886.90 | -10 | 118 | 20 | 96 |
| 8/7/2013 | 34,462 | 1886.70 | -12 | 120 | 20 | 96 |
| 8/8/2013 | 34,222 | 1886.60 | -9 | 120 | 21 | 96 |
| 8/9/2013 | 33,996 | 1886.40 | -10 | 119 | 21 | 96 |
| 8/10/2013 | 33,757 | 1886.20 | -12 | 118 | 20 | 95 |
| 8/11/2013 | 33,533 | 1886.10 | -8 | 116 | 20 | 95 |
| 8/12/2013 | 33,296 | 1885.90 | -8 | 118 | 20 | 95 |
| 8/13/2013 | 33,074 | 1885.80 | -8 | 117 | 20 | 95 |
| 8/14/2013 | 32,840 | 1885.60 | -8 | 117 | 20 | 95 |
| 8/15/2013 | 32,606 | 1885.40 | -8 | 114 | 20 | 95 |
| 8/16/2013 | 32,373 | 1885.20 | -6 | 115 | 20 | 95 |
| 8/17/2013 | 32,155 | 1885.10 | -7 | 116 | 20 | 95 |
| 8/18/2013 | 31,925 | 1884.90 | -2 | 118 | 20 | 95 |
| 8/19/2013 | 31,710 | 1884.70 | -1 | 110 | 20 | 95 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/20/2013 | 31,484 | 1884.60 | -1 | 115 | 20 | 95 |
| 8/21/2013 | 31,254 | 1884.40 | -2 | 116 | 19 | 95 |
| 8/22/2013 | 31,028 | 1884.20 | -4 | 116 | 19 | 95 |
| 8/23/2013 | 30,789 | 1884.10 | -6 | 115 | 19 | 95 |
| 8/24/2013 | 30,558 | 1883.90 | -3 | 115 | 19 | 95 |
| 8/25/2013 | 30,330 | 1883.70 | -3 | 113 | 19 | 95 |
| 8/26/2013 | 30,098 | 1883.50 | -3 | 116 | 19 | 95 |
| 8/27/2013 | 29,875 | 1883.30 | -8 | 114 | 19 | 95 |
| 8/28/2013 | 29,655 | 1883.20 | -11 | 114 | 19 | 95 |
| 8/29/2013 | 29,424 | 1883.00 | -9 | 114 | 19 | 95 |
| 8/30/2013 | 29,201 | 1882.80 | -11 | 114 | 19 | 95 |
| 8/31/2013 | 28,966 | 1882.60 | -12 | 117 | 19 | 95 |
| 9/1/2013 | 28,739 | 1882.50 | -10 | 118 | 20 | 95 |
| 9/2/2013 | 28,501 | 1882.30 | -12 | 118 | 20 | 95 |
| 9/3/2013 | 28,265 | 1882.10 | -11 | 118 | 20 | 95 |
| 9/4/2013 | 28,029 | 1881.90 | -13 | 118 | 20 | 95 |
| 9/5/2013 | 27,801 | 1881.70 | -12 | 117 | 20 | 95 |
| 9/6/2013 | 27,575 | 1881.50 | -10 | 117 | 20 | 95 |
| 9/7/2013 | 27,345 | 1881.30 | -9 | 117 | 20 | 95 |
| 9/8/2013 | 27,116 | 1881.10 | -7 | 118 | 20 | 95 |
| 9/9/2013 | 26,888 | 1880.90 | -9 | 118 | 20 | 95 |
| 9/10/2013 | 26,662 | 1880.70 | -9 | 118 | 20 | 95 |
| 9/11/2013 | 26,432 | 1880.60 | -8 | 118 | 20 | 95 |
| 9/12/2013 | 26,215 | 1880.40 | -6 | 118 | 20 | 95 |
| 9/13/2013 | 25,993 | 1880.20 | -6 | 118 | 20 | 95 |
| 9/14/2013 | 25,762 | 1880.00 | -8 | 118 | 20 | 95 |
| 9/15/2013 | 25,532 | 1879.80 | -7 | 118 | 20 | 95 |
| 9/16/2013 | 25,315 | 1879.60 | -7 | 118 | 20 | 92 |
| 9/17/2013 | 25,078 | 1879.40 | -14 | 113 | 21 | 90 |
| 9/18/2013 | 24,866 | 1879.20 | -5 | 108 | 20 | 90 |
| 9/19/2013 | 24,654 | 1879.00 | -5 | 110 | 19 | 90 |
| 9/20/2013 | 24,460 | 1878.80 | 2 | 111 | 20 | 90 |
| 9/21/2013 | 24,284 | 1878.70 | 17 | 111 | 24 | 91 |
| 9/22/2013 | 24,072 | 1878.50 | -2 | 112 | 20 | 91 |
| 9/23/2013 | 23,867 | 1878.30 | 1 | 111 | 20 | 91 |
| 9/24/2013 | 23,681 | 1878.10 | 58 | 111 | 20 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/25/2013 | 23,494 | 1877.90 | -60 | 111 | 20 | 91 |
| 9/26/2013 | 23,239 | 1877.70 | -4 | 112 | 20 | 91 |
| 9/27/2013 | 23,036 | 1877.50 | 2 | 112 | 20 | 91 |
| 9/28/2013 | 22,835 | 1877.30 | 5 | 113 | 20 | 91 |
| 9/29/2013 | 22,680 | 1877.20 | 26 | 114 | 21 | 91 |
| 9/30/2013 | 22,507 | 1877.00 | 27 | 115 | 29 | 91 |
| 10/1/2013 | 22,334 | 1876.80 | 19 | 115 | 20 | 91 |
| 10/2/2013 | 22,128 | 1876.60 | 14 | 114 | 20 | 91 |
| 10/3/2013 | 21,939 | 1876.50 | 6 | 113 | 20 | 91 |
| 10/4/2013 | 21,717 | 1876.20 | -9 | 116 | 20 | 91 |
| 10/5/2013 | 21,521 | 1876.00 | 7 | 119 | 23 | 91 |
| 10/6/2013 | 21,317 | 1875.80 | 4 | 119 | 24 | 91 |
| 10/7/2013 | 21,105 | 1875.60 | 4 | 123 | 27 | 91 |
| 10/8/2013 | 20,891 | 1875.40 | 5 | 123 | 30 | 90 |
| 10/9/2013 | 20,683 | 1875.20 | -1 | 119 | 28 | 90 |
| 10/10/2013 | 20,490 | 1875.00 | 2 | 117 | 24 | 89 |
| 10/11/2013 | 20,285 | 1874.80 | 2 | 117 | 24 | 90 |
| 10/12/2013 | 20,082 | 1874.60 | 1 | 116 | 24 | 90 |
| 10/13/2013 | 19,890 | 1874.40 | 3 | 117 | 24 | 90 |
| 10/14/2013 | 19,691 | 1874.10 | 1 | 117 | 25 | 90 |
| 10/15/2013 | 19,484 | 1873.90 | 0 | 116 | 25 | 90 |
| 10/16/2013 | 19,309 | 1873.70 | 6 | 102 | 46 | 67 |
| 10/17/2013 | 19,163 | 1873.60 | 11 | 85 | 45 | 51 |
| 10/18/2013 | 19,027 | 1873.40 | 11 | 80 | 33 | 51 |
| 10/19/2013 | 18,888 | 1873.30 | 11 | 83 | 30 | 51 |
| 10/20/2013 | 18,736 | 1873.10 | 9 | 86 | 40 | 51 |
| 10/21/2013 | 18,500 | 1872.80 | -64 | 86 | 40 | 51 |
| 10/22/2013 | 18,309 | 1872.60 | 12 | 85 | 39 | 52 |
| 10/23/2013 | 18,173 | 1872.40 | 12 | 83 | 34 | 52 |
| 10/24/2013 | 18,031 | 1872.30 | 22 | 82 | 34 | 52 |
| 10/25/2013 | 17,889 | 1872.10 | 1 | 82 | 33 | 52 |
| 10/26/2013 | 17,752 | 1871.90 | 22 | 82 | 33 | 52 |
| 10/27/2013 | 17,607 | 1871.80 | 4 | 82 | 34 | 52 |
| 10/28/2013 | 17,469 | 1871.60 | 20 | 82 | 34 | 52 |
| 10/29/2013 | 17,331 | 1871.40 | 3 | 82 | 35 | 52 |
| 10/30/2013 | 17,189 | 1871.30 | 11 | 81 | 35 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/31/2013 | 17,049 | 1871.10 | 12 | 81 | 34 | 52 |
| 11/1/2013 | 16,933 | 1871.00 | 18 | 75 | 37 | 46 |
| 11/2/2013 | 16,794 | 1870.80 | 17 | 74 | 31 | 46 |
| 11/3/2013 | 16,668 | 1870.60 | 8 | 74 | 31 | 46 |
| 11/4/2013 | 16,546 | 1870.50 | 8 | 74 | 31 | 46 |
| 11/5/2013 | 16,429 | 1870.30 | 17 | 74 | 32 | 46 |
| 11/6/2013 | 16,313 | 1870.20 | 18 | 73 | 32 | 46 |
| 11/7/2013 | 16,178 | 1870.00 | 16 | 73 | 32 | 46 |
| 11/8/2013 | 16,067 | 1869.90 | 16 | 73 | 32 | 46 |
| 11/9/2013 | 15,941 | 1869.70 | 17 | 73 | 31 | 46 |
| 11/10/2013 | 15,831 | 1869.50 | 18 | 73 | 31 | 46 |
| 11/11/2013 | 15,706 | 1869.40 | 16 | 73 | 31 | 46 |
| 11/12/2013 | 15,586 | 1869.20 | 17 | 73 | 31 | 46 |
| 11/13/2013 | 15,467 | 1869.10 | 18 | 74 | 31 | 46 |
| 11/14/2013 | 15,281 | 1868.80 | 10 | 108 | 58 | 47 |
| 11/15/2013 | 15,156 | 1868.60 | 19 | 82 | 49 | 46 |
| 11/16/2013 | 15,031 | 1868.50 | 19 | 77 | 29 | 46 |
| 11/17/2013 | 14,907 | 1868.30 | 20 | 77 | 29 | 46 |
| 11/18/2013 | 14,794 | 1868.20 | 24 | 77 | 29 | 46 |
| 11/19/2013 | 14,806 | 1868.20 | 89 | 79 | 46 | 46 |
| 11/20/2013 | 14,798 | 1868.20 | 82 | 80 | 52 | 46 |
| 11/21/2013 | 14,728 | 1868.10 | 47 | 80 | 39 | 46 |
| 11/22/2013 | 14,610 | 1867.90 | 28 | 79 | 35 | 45 |
| 11/23/2013 | 14,495 | 1867.70 | 28 | 84 | 37 | 44 |
| 11/24/2013 | 14,381 | 1867.60 | 30 | 85 | 47 | 45 |
| 11/25/2013 | 14,264 | 1867.40 | 27 | 82 | 39 | 45 |
| 11/26/2013 | 14,143 | 1867.20 | 24 | 84 | 38 | 45 |
| 11/27/2013 | 14,024 | 1867.10 | 23 | 86 | 42 | 45 |
| 11/28/2013 | 13,898 | 1866.90 | 12 | 85 | 40 | 45 |
| 11/29/2013 | 13,770 | 1866.70 | 12 | 85 | 38 | 46 |
| 11/30/2013 | 13,607 | 1866.50 | 3 | 103 | 39 | 46 |
| 12/1/2013 | 13,330 | 1866.00 | -1 | 179 | 121 | 46 |
| 12/2/2013 | 13,039 | 1865.60 | 0 | 182 | 130 | 46 |
| 12/3/2013 | 12,747 | 1865.20 | -2 | 183 | 132 | 46 |
| 12/4/2013 | 12,458 | 1864.70 | 1 | 182 | 130 | 46 |
| 12/5/2013 | 12,177 | 1864.30 | 1 | 181 | 129 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/6/2013 | 11,936 | 1863.90 | 19 | 180 | 132 | 46 |
| 12/7/2013 | 11,660 | 1863.40 | 10 | 181 | 131 | 46 |
| 12/8/2013 | 11,383 | 1863.00 | 73 | 181 | 129 | 46 |
| 12/9/2013 | 11,117 | 1862.60 | 8 | 179 | 128 | 46 |
| 12/10/2013 | 10,846 | 1862.10 | 13 | 178 | 127 | 46 |
| 12/11/2013 | 10,586 | 1861.70 | 9 | 177 | 126 | 45 |
| 12/12/2013 | 10,417 | 1861.40 | 17 | 129 | 117 | 38 |
| 12/13/2013 | 10,361 | 1861.30 | 29 | 54 | 47 | 15 |
| 12/14/2013 | 10,307 | 1861.20 | 27 | 53 | 39 | 15 |
| 12/15/2013 | 10,255 | 1861.10 | 29 | 53 | 39 | 15 |
| 12/16/2013 | 10,203 | 1861.00 | 28 | 52 | 39 | 15 |
| 12/17/2013 | 10,126 | 1860.80 | 14 | 52 | 38 | 15 |
| 12/18/2013 | 10,094 | 1860.80 | 36 | 52 | 38 | 15 |
| 12/19/2013 | 10,024 | 1860.70 | 49 | 52 | 38 | 15 |
| 12/20/2013 | 9,957 | 1860.50 | 22 | 52 | 38 | 15 |
| 12/21/2013 | 9,913 | 1860.50 | 22 | 53 | 37 | 16 |
| 12/22/2013 | 9,860 | 1860.40 | 21 | 53 | 37 | 16 |
| 12/23/2013 | 9,804 | 1860.30 | 25 | 59 | 37 | 16 |
| 12/24/2013 | 9,749 | 1860.20 | 28 | 62 | 37 | 16 |
| 12/25/2013 | 9,693 | 1860.10 | 30 | 65 | 37 | 16 |
| 12/26/2013 | 9,638 | 1860.00 | 32 | 64 | 37 | 16 |
| 12/27/2013 | 9,583 | 1859.90 | 31 | 63 | 37 | 16 |
| 12/28/2013 | 9,523 | 1859.80 | 32 | 60 | 37 | 16 |
| 12/29/2013 | 9,471 | 1859.70 | 31 | 59 | 35 | 16 |
| 12/30/2013 | 9,409 | 1859.50 | 31 | 59 | 40 | 16 |
| 12/31/2013 | 9,347 | 1859.40 | 32 | 55 | 38 | 16 |
| 1/1/2014 | 9,288 | 1859.30 | 32 | 52 | 38 | 16 |
| 1/2/2014 | 9,227 | 1859.20 | 31 | 51 | 38 | 15 |
| 1/3/2014 | 9,171 | 1859.10 | 24 | 50 | 37 | 15 |
| 1/4/2014 | 9,113 | 1859.00 | 20 | 49 | 36 | 15 |
| 1/5/2014 | 9,060 | 1858.90 | 14 | 49 | 36 | 15 |
| 1/6/2014 | 9,005 | 1858.80 | 16 | 49 | 36 | 14 |
| 1/7/2014 | 8,950 | 1858.70 | 17 | 48 | 36 | 13 |
| 1/8/2014 | 8,898 | 1858.60 | 16 | 47 | 35 | 13 |
| 1/9/2014 | 8,846 | 1858.50 | 17 | 47 | 35 | 13 |
| 1/10/2014 | 8,806 | 1858.40 | 17 | 45 | 36 | 12 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/11/2014 | 8,773 | 1858.30 | 23 | 43 | 37 | 10 |
| 1/12/2014 | 8,739 | 1858.20 | 16 | 42 | 34 | 10 |
| 1/13/2014 | 8,706 | 1858.20 | 19 | 42 | 33 | 10 |
| 1/14/2014 | 8,664 | 1858.10 | 18 | 42 | 33 | 10 |
| 1/15/2014 | 8,638 | 1858.00 | 21 | 42 | 33 | 10 |
| 1/16/2014 | 8,585 | 1857.90 | 18 | 42 | 33 | 10 |
| 1/17/2014 | 8,542 | 1857.90 | 18 | 42 | 33 | 10 |
| 1/18/2014 | 8,502 | 1857.80 | 18 | 42 | 33 | 10 |
| 1/19/2014 | 8,457 | 1857.70 | 17 | 42 | 33 | 10 |
| 1/20/2014 | 8,413 | 1857.60 | 16 | 42 | 33 | 10 |
| 1/21/2014 | 8,373 | 1857.50 | 16 | 41 | 32 | 10 |
| 1/22/2014 | 8,329 | 1857.40 | 18 | 41 | 32 | 10 |
| 1/23/2014 | 8,288 | 1857.30 | 14 | 41 | 33 | 10 |
| 1/24/2014 | 8,240 | 1857.20 | 16 | 40 | 31 | 10 |
| 1/25/2014 | 8,196 | 1857.10 | 16 | 42 | 32 | 10 |
| 1/26/2014 | 8,148 | 1857.00 | 16 | 42 | 33 | 10 |
| 1/27/2014 | 8,105 | 1857.00 | 17 | 42 | 33 | 10 |
| 1/28/2014 | 8,061 | 1856.90 | 17 | 42 | 33 | 10 |
| 1/29/2014 | 8,044 | 1856.80 | 31 | 43 | 37 | 10 |
| 1/30/2014 | 8,015 | 1856.80 | 24 | 42 | 35 | 10 |
| 1/31/2014 | 7,975 | 1856.70 | 26 | 45 | 34 | 10 |
| 2/1/2014 | 7,944 | 1856.60 | 21 | 40 | 37 | 10 |
| 2/2/2014 | 7,908 | 1856.50 | 19 | 40 | 33 | 10 |
| 2/3/2014 | 7,870 | 1856.50 | 18 | 40 | 32 | 10 |
| 2/4/2014 | 7,833 | 1856.40 | 17 | 39 | 32 | 10 |
| 2/5/2014 | 7,809 | 1856.30 | 21 | 37 | 32 | 9 |
| 2/6/2014 | 7,816 | 1856.30 | 39 | 37 | 35 | 9 |
| 2/7/2014 | 7,927 | 1856.60 | 94 | 38 | 62 | 9 |
| 2/8/2014 | 8,588 | 1857.90 | 881 | 46 | 483 | 9 |
| 2/9/2014 | 14,105 | 1867.20 | 2,500 | 49 | 570 | 9 |
| 2/10/2014 | 16,374 | 1870.30 | 1,265 | 73 | 376 | 9 |
| 2/11/2014 | 16,857 | 1870.90 | 404 | 87 | 176 | 9 |
| 2/12/2014 | 17,129 | 1871.20 | 191 | 47 | 98 | 9 |
| 2/13/2014 | 17,339 | 1871.50 | 154 | 45 | 71 | 9 |
| 2/14/2014 | 17,513 | 1871.70 | 135 | 45 | 64 | 9 |
| 2/15/2014 | 17,674 | 1871.90 | 129 | 45 | 62 | 9 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/16/2014 | 18,013 | 1872.30 | 216 | 42 | 117 | 9 |
| 2/17/2014 | 18,232 | 1872.50 | 161 | 42 | 80 | 9 |
| 2/18/2014 | 18,388 | 1872.70 | 122 | 42 | 65 | 9 |
| 2/19/2014 | 18,508 | 1872.80 | 105 | 42 | 60 | 9 |
| 2/20/2014 | 18,607 | 1872.90 | 93 | 43 | 55 | 9 |
| 2/21/2014 | 18,684 | 1873.00 | 82 | 43 | 51 | 9 |
| 2/22/2014 | 18,754 | 1873.10 | 77 | 43 | 49 | 9 |
| 2/23/2014 | 18,801 | 1873.20 | 68 | 43 | 48 | 9 |
| 2/24/2014 | 18,844 | 1873.20 | 64 | 44 | 46 | 9 |
| 2/25/2014 | 18,875 | 1873.20 | 60 | 45 | 45 | 9 |
| 2/26/2014 | 18,910 | 1873.30 | 81 | 45 | 46 | 9 |
| 2/27/2014 | 21,929 | 1876.40 | 1,578 | 48 | 229 | 9 |
| 2/28/2014 | 24,165 | 1878.60 | 1,222 | 50 | 259 | 9 |
| 3/1/2014 | 25,964 | 1880.20 | 1,032 | 49 | 221 | 9 |
| 3/2/2014 | 26,916 | 1881.00 | 559 | 48 | 144 | 9 |
| 3/3/2014 | 28,128 | 1882.00 | 1,374 | 52 | 390 | 9 |
| 3/4/2014 | 35,912 | 1887.80 | 3,462 | 54 | 599 | 9 |
| 3/5/2014 | 37,300 | 1888.70 | 933 | 51 | 288 | 9 |
| 3/6/2014 | 42,530 | 1892.10 | 2,668 | 55 | 486 | 9 |
| 3/7/2014 | 44,363 | 1893.20 | 1,062 | 54 | 291 | 9 |
| 3/8/2014 | 45,550 | 1893.90 | 690 | 55 | 213 | 9 |
| 3/9/2014 | 46,003 | 1894.20 | 813 | 55 | 180 | 9 |
| 3/10/2014 | 50,749 | 1896.90 | 2,027 | 57 | 400 | 9 |
| 3/11/2014 | 52,595 | 1897.90 | 1,055 | 49 | 269 | 9 |
| 3/12/2014 | 53,811 | 1898.60 | 691 | 45 | 206 | 9 |
| 3/13/2014 | 54,662 | 1899.10 | 533 | 59 | 174 | 11 |
| 3/14/2014 | 55,307 | 1899.40 | 434 | 73 | 149 | 13 |
| 3/15/2014 | 55,739 | 1899.60 | 397 | 105 | 167 | 15 |
| 3/16/2014 | 56,061 | 1899.80 | 343 | 108 | 158 | 15 |
| 3/17/2014 | 56,307 | 1899.90 | 308 | 110 | 147 | 14 |
| 3/18/2014 | 56,496 | 1900.00 | 285 | 127 | 151 | 14 |
| 3/19/2014 | 56,580 | 1900.10 | 273 | 147 | 177 | 14 |
| 3/20/2014 | 56,858 | 1900.20 | 337 | 158 | 174 | 14 |
| 3/21/2014 | 56,820 | 1900.20 | 159 | 185 | 189 | 22 |
| 3/22/2014 | 56,774 | 1900.20 | 152 | 187 | 184 | 24 |
| 3/23/2014 | 56,706 | 1900.20 | 140 | 183 | 181 | 20 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/24/2014 | 56,639 | 1900.10 | 131 | 182 | 177 | 20 |
| 3/25/2014 | 56,667 | 1900.10 | 192 | 183 | 183 | 17 |
| 3/26/2014 | 56,810 | 1900.20 | 241 | 181 | 204 | 15 |
| 3/27/2014 | 56,975 | 1900.30 | 262 | 180 | 219 | 15 |
| 3/28/2014 | 57,472 | 1900.60 | 411 | 180 | 252 | 15 |
| 3/29/2014 | 67,445 | 1905.60 | 5,119 | 198 | 950 | 15 |
| 3/30/2014 | 69,808 | 1906.70 | 1,794 | 525 | 774 | 15 |
| 3/31/2014 | 70,102 | 1906.80 | 1,496 | 1,404 | 1,548 | 15 |
| 4/1/2014 | 70,037 | 1906.80 | 1,475 | 1,511 | 1,939 | 36 |
| 4/2/2014 | 70,234 | 1906.90 | 1,339 | 1,281 | 1,616 | 45 |
| 4/3/2014 | 70,322 | 1906.90 | 1,091 | 1,052 | 1,207 | 46 |
| 4/4/2014 | 70,384 | 1907.00 | 928 | 890 | 1,007 | 45 |
| 4/5/2014 | 70,485 | 1907.00 | 795 | 725 | 816 | 45 |
| 4/6/2014 | 70,714 | 1907.10 | 673 | 556 | 652 | 45 |
| 4/7/2014 | 71,032 | 1907.30 | 575 | 443 | 506 | 45 |
| 4/8/2014 | 71,285 | 1907.40 | 490 | 406 | 465 | 45 |
| 4/9/2014 | 71,505 | 1907.50 | 418 | 358 | 410 | 44 |
| 4/10/2014 | 71,671 | 1907.60 | 359 | 321 | 353 | 44 |
| 4/11/2014 | 71,815 | 1907.60 | 320 | 291 | 317 | 44 |
| 4/12/2014 | 71,837 | 1907.70 | 265 | 280 | 293 | 44 |
| 4/13/2014 | 71,904 | 1907.70 | 281 | 279 | 282 | 44 |
| 4/14/2014 | 71,859 | 1907.70 | 230 | 279 | 271 | 44 |
| 4/15/2014 | 71,771 | 1907.60 | 225 | 300 | 262 | 62 |
| 4/16/2014 | 71,594 | 1907.50 | 200 | 306 | 245 | 80 |
| 4/17/2014 | 71,505 | 1907.50 | 208 | 308 | 243 | 80 |
| 4/18/2014 | 71,258 | 1907.40 | 141 | 304 | 238 | 80 |
| 4/19/2014 | 71,011 | 1907.30 | 141 | 303 | 234 | 80 |
| 4/20/2014 | 70,764 | 1907.20 | 135 | 297 | 227 | 80 |
| 4/21/2014 | 70,518 | 1907.00 | 124 | 288 | 218 | 80 |
| 4/22/2014 | 70,298 | 1906.90 | 110 | 269 | 202 | 80 |
| 4/23/2014 | 70,081 | 1906.80 | 105 | 257 | 191 | 80 |
| 4/24/2014 | 69,885 | 1906.70 | 116 | 255 | 187 | 80 |
| 4/25/2014 | 69,689 | 1906.60 | 112 | 255 | 186 | 80 |
| 4/26/2014 | 69,471 | 1906.50 | 107 | 257 | 184 | 80 |
| 4/27/2014 | 69,254 | 1906.40 | 109 | 258 | 183 | 80 |
| 4/28/2014 | 69,015 | 1906.30 | 89 | 256 | 180 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/29/2014 | 68,757 | 1906.20 | 86 | 255 | 177 | 80 |
| 4/30/2014 | 68,499 | 1906.10 | 69 | 242 | 172 | 80 |
| 5/1/2014 | 68,252 | 1906.00 | 60 | 233 | 165 | 80 |
| 5/2/2014 | 67,995 | 1905.80 | 50 | 226 | 161 | 80 |
| 5/3/2014 | 67,728 | 1905.70 | 41 | 222 | 157 | 80 |
| 5/4/2014 | 67,461 | 1905.60 | 36 | 222 | 155 | 81 |
| 5/5/2014 | 67,215 | 1905.50 | 35 | 211 | 154 | 81 |
| 5/6/2014 | 66,954 | 1905.30 | 230 | 188 | 135 | 81 |
| 5/7/2014 | 66,796 | 1905.30 | 105 | 187 | 132 | 80 |
| 5/8/2014 | 66,595 | 1905.20 | 29 | 186 | 131 | 81 |
| 5/9/2014 | 66,368 | 1905.00 | 47 | 186 | 130 | 81 |
| 5/10/2014 | 66,131 | 1904.90 | 58 | 185 | 127 | 81 |
| 5/11/2014 | 65,890 | 1904.80 | 64 | 185 | 125 | 81 |
| 5/12/2014 | 65,675 | 1904.70 | 64 | 186 | 123 | 81 |
| 5/13/2014 | 65,428 | 1904.60 | 56 | 187 | 121 | 80 |
| 5/14/2014 | 65,179 | 1904.50 | 59 | 188 | 119 | 80 |
| 5/15/2014 | 64,939 | 1904.30 | 57 | 186 | 117 | 80 |
| 5/16/2014 | 64,702 | 1904.20 | 43 | 174 | 106 | 80 |
| 5/17/2014 | 64,458 | 1904.10 | 45 | 168 | 90 | 80 |
| 5/18/2014 | 64,235 | 1904.00 | 46 | 167 | 86 | 80 |
| 5/19/2014 | 64,053 | 1903.90 | 53 | 160 | 79 | 81 |
| 5/20/2014 | 63,885 | 1903.80 | 55 | 149 | 68 | 80 |
| 5/21/2014 | 63,730 | 1903.80 | 44 | 141 | 58 | 78 |
| 5/22/2014 | 63,563 | 1903.70 | 33 | 135 | 53 | 79 |
| 5/23/2014 | 63,394 | 1903.60 | 25 | 134 | 49 | 79 |
| 5/24/2014 | 63,224 | 1903.50 | 22 | 133 | 49 | 79 |
| 5/25/2014 | 63,032 | 1903.40 | 13 | 134 | 49 | 79 |
| 5/26/2014 | 62,844 | 1903.30 | 15 | 133 | 50 | 79 |
| 5/27/2014 | 62,661 | 1903.20 | 8 | 127 | 48 | 79 |
| 5/28/2014 | 62,450 | 1903.10 | -2 | 124 | 42 | 79 |
| 5/29/2014 | 62,267 | 1903.00 | 15 | 124 | 42 | 79 |
| 5/30/2014 | 62,073 | 1902.90 | 13 | 123 | 43 | 80 |
| 5/31/2014 | 61,922 | 1902.80 | 14 | 117 | 39 | 80 |
| 6/1/2014 | 61,758 | 1902.80 | 11 | 115 | 34 | 80 |
| 6/2/2014 | 61,572 | 1902.70 | 4 | 115 | 34 | 80 |
| 6/3/2014 | 61,377 | 1902.60 | 9 | 114 | 34 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/4/2014 | 61,203 | 1902.50 | -2 | 113 | 33 | 80 |
| 6/5/2014 | 61,021 | 1902.40 | 5 | 112 | 32 | 80 |
| 6/6/2014 | 60,819 | 1902.30 | 1 | 112 | 30 | 80 |
| 6/7/2014 | 60,644 | 1902.20 | 6 | 112 | 30 | 80 |
| 6/8/2014 | 60,439 | 1902.10 | 3 | 109 | 29 | 79 |
| 6/9/2014 | 60,261 | 1902.00 | 1 | 107 | 25 | 80 |
| 6/10/2014 | 60,073 | 1901.90 | 1 | 106 | 24 | 80 |
| 6/11/2014 | 59,883 | 1901.80 | -4 | 103 | 22 | 80 |
| 6/12/2014 | 59,670 | 1901.70 | -17 | 100 | 18 | 80 |
| 6/13/2014 | 59,480 | 1901.60 | -5 | 96 | 16 | 80 |
| 6/14/2014 | 59,320 | 1901.50 | -6 | 94 | 13 | 80 |
| 6/15/2014 | 59,129 | 1901.40 | -12 | 92 | 11 | 80 |
| 6/16/2014 | 58,912 | 1901.30 | -19 | 91 | 10 | 79 |
| 6/17/2014 | 58,749 | 1901.20 | -8 | 91 | 10 | 79 |
| 6/18/2014 | 58,567 | 1901.10 | -7 | 90 | 10 | 79 |
| 6/19/2014 | 58,375 | 1901.00 | -13 | 93 | 10 | 79 |
| 6/20/2014 | 58,194 | 1900.90 | -12 | 89 | 14 | 79 |
| 6/21/2014 | 58,019 | 1900.80 | -12 | 89 | 10 | 79 |
| 6/22/2014 | 57,832 | 1900.70 | -12 | 89 | 10 | 79 |
| 6/23/2014 | 57,635 | 1900.60 | -14 | 89 | 9 | 79 |
| 6/24/2014 | 57,437 | 1900.50 | -22 | 88 | 9 | 79 |
| 6/25/2014 | 57,279 | 1900.50 | -4 | 90 | 9 | 79 |
| 6/26/2014 | 57,097 | 1900.40 | -10 | 91 | 10 | 79 |
| 6/27/2014 | 56,918 | 1900.30 | -4 | 91 | 10 | 79 |
| 6/28/2014 | 56,734 | 1900.20 | -16 | 90 | 9 | 79 |
| 6/29/2014 | 56,544 | 1900.10 | -12 | 91 | 9 | 79 |
| 6/30/2014 | 56,356 | 1900.00 | -13 | 92 | 10 | 79 |
| 7/1/2014 | 56,155 | 1899.90 | -18 | 92 | 10 | 80 |
| 7/2/2014 | 55,953 | 1899.80 | -14 | 92 | 9 | 80 |
| 7/3/2014 | 55,761 | 1899.70 | -18 | 92 | 9 | 80 |
| 7/4/2014 | 55,535 | 1899.50 | -25 | 92 | 9 | 80 |
| 7/5/2014 | 55,330 | 1899.40 | -23 | 93 | 9 | 80 |
| 7/6/2014 | 55,145 | 1899.30 | -18 | 92 | 9 | 80 |
| 7/7/2014 | 54,939 | 1899.20 | -16 | 91 | 9 | 80 |
| 7/8/2014 | 54,779 | 1899.10 | -10 | 91 | 9 | 80 |
| 7/9/2014 | 54,570 | 1899.00 | -16 | 92 | 9 | 80 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/10/2014 | 54,351 | 1898.90 | -19 | 95 | 9 | 80 |
| 7/11/2014 | 54,138 | 1898.80 | -23 | 96 | 11 | 80 |
| 7/12/2014 | 53,950 | 1898.70 | -18 | 95 | 10 | 80 |
| 7/13/2014 | 53,716 | 1898.50 | -23 | 95 | 10 | 80 |
| 7/14/2014 | 53,512 | 1898.40 | -20 | 94 | 10 | 80 |
| 7/15/2014 | 53,316 | 1898.30 | -20 | 94 | 10 | 80 |
| 7/16/2014 | 53,124 | 1898.20 | -8 | 93 | 10 | 80 |
| 7/17/2014 | 52,917 | 1898.10 | -20 | 92 | 10 | 80 |
| 7/18/2014 | 52,722 | 1898.00 | -19 | 90 | 10 | 80 |
| 7/19/2014 | 52,518 | 1897.90 | -18 | 90 | 10 | 80 |
| 7/20/2014 | 52,305 | 1897.80 | -28 | 89 | 9 | 80 |
| 7/21/2014 | 52,110 | 1897.70 | -19 | 90 | 9 | 80 |
| 7/22/2014 | 51,912 | 1897.50 | -18 | 90 | 10 | 80 |
| 7/23/2014 | 51,706 | 1897.40 | -27 | 89 | 9 | 80 |
| 7/24/2014 | 51,494 | 1897.30 | -19 | 88 | 9 | 80 |
| 7/25/2014 | 51,278 | 1897.20 | -29 | 89 | 9 | 80 |
| 7/26/2014 | 51,100 | 1897.10 | -14 | 89 | 10 | 80 |
| 7/27/2014 | 50,868 | 1897.00 | -24 | 90 | 10 | 80 |
| 7/28/2014 | 50,643 | 1896.80 | -38 | 89 | 10 | 80 |
| 7/29/2014 | 50,437 | 1896.70 | -27 | 89 | 9 | 80 |
| 7/30/2014 | 50,250 | 1896.60 | -24 | 89 | 10 | 80 |
| 7/31/2014 | 50,124 | 1896.50 | 36 | 89 | 9 | 80 |
| 8/1/2014 | 49,917 | 1896.40 | -27 | 89 | 9 | 80 |
| 8/2/2014 | 49,709 | 1896.30 | -24 | 89 | 9 | 80 |
| 8/3/2014 | 49,530 | 1896.20 | -22 | 89 | 10 | 80 |
| 8/4/2014 | 49,332 | 1896.10 | -15 | 90 | 10 | 80 |
| 8/5/2014 | 49,159 | 1896.00 | -4 | 90 | 10 | 80 |
| 8/6/2014 | 48,982 | 1895.90 | -6 | 90 | 10 | 81 |
| 8/7/2014 | 48,843 | 1895.80 | -24 | 91 | 10 | 80 |
| 8/8/2014 | 48,571 | 1895.70 | -10 | 91 | 9 | 80 |
| 8/9/2014 | 48,363 | 1895.50 | -41 | 90 | 9 | 80 |
| 8/10/2014 | 48,149 | 1895.40 | -24 | 90 | 9 | 80 |
| 8/11/2014 | 47,943 | 1895.30 | -31 | 89 | 9 | 80 |
| 8/12/2014 | 47,751 | 1895.20 | -26 | 90 | 9 | 80 |
| 8/13/2014 | 47,542 | 1895.10 | -23 | 89 | 10 | 80 |
| 8/14/2014 | 47,343 | 1894.90 | -18 | 90 | 9 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/15/2014 | 47,118 | 1894.80 | -25 | 103 | 14 | 80 |
| 8/16/2014 | 46,880 | 1894.70 | -23 | 108 | 28 | 80 |
| 8/17/2014 | 46,646 | 1894.50 | -25 | 107 | 29 | 80 |
| 8/18/2014 | 46,417 | 1894.40 | -20 | 107 | 27 | 80 |
| 8/19/2014 | 46,138 | 1894.20 | -46 | 108 | 27 | 80 |
| 8/20/2014 | 45,905 | 1894.10 | -19 | 109 | 29 | 80 |
| 8/21/2014 | 45,681 | 1894.00 | -16 | 109 | 29 | 80 |
| 8/22/2014 | 45,446 | 1893.80 | -23 | 109 | 29 | 80 |
| 8/23/2014 | 45,223 | 1893.70 | -17 | 107 | 28 | 80 |
| 8/24/2014 | 44,984 | 1893.50 | -25 | 106 | 26 | 80 |
| 8/25/2014 | 44,763 | 1893.40 | -22 | 106 | 27 | 80 |
| 8/26/2014 | 44,533 | 1893.30 | -19 | 105 | 26 | 80 |
| 8/27/2014 | 44,300 | 1893.10 | -22 | 105 | 26 | 80 |
| 8/28/2014 | 44,074 | 1893.00 | -21 | 105 | 26 | 80 |
| 8/29/2014 | 43,849 | 1892.90 | -20 | 105 | 26 | 80 |
| 8/30/2014 | 43,611 | 1892.70 | -27 | 104 | 26 | 80 |
| 8/31/2014 | 43,381 | 1892.60 | -21 | 105 | 28 | 80 |
| 9/1/2014 | 43,160 | 1892.40 | -19 | 105 | 28 | 80 |
| 9/2/2014 | 42,925 | 1892.30 | -21 | 105 | 27 | 80 |
| 9/3/2014 | 42,700 | 1892.20 | -19 | 105 | 27 | 80 |
| 9/4/2014 | 42,458 | 1892.00 | -26 | 105 | 27 | 80 |
| 9/5/2014 | 42,232 | 1891.90 | -21 | 104 | 27 | 80 |
| 9/6/2014 | 42,006 | 1891.70 | -23 | 105 | 27 | 80 |
| 9/7/2014 | 41,768 | 1891.60 | -21 | 105 | 27 | 80 |
| 9/8/2014 | 41,540 | 1891.40 | -22 | 105 | 26 | 80 |
| 9/9/2014 | 41,316 | 1891.30 | -19 | 104 | 26 | 80 |
| 9/10/2014 | 41,085 | 1891.10 | -18 | 105 | 27 | 80 |
| 9/11/2014 | 40,848 | 1891.00 | -27 | 107 | 28 | 80 |
| 9/12/2014 | 40,619 | 1890.90 | -18 | 109 | 28 | 80 |
| 9/13/2014 | 40,401 | 1890.70 | -13 | 107 | 28 | 80 |
| 9/14/2014 | 40,179 | 1890.60 | -17 | 106 | 28 | 80 |
| 9/15/2014 | 39,947 | 1890.40 | -21 | 108 | 25 | 82 |
| 9/16/2014 | 39,696 | 1890.30 | -32 | 110 | 21 | 89 |
| 9/17/2014 | 39,450 | 1890.10 | -22 | 111 | 21 | 89 |
| 9/18/2014 | 39,239 | 1890.00 | -6 | 112 | 23 | 89 |
| 9/19/2014 | 39,027 | 1889.80 | -9 | 111 | 22 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/20/2014 | 38,800 | 1889.70 | -14 | 113 | 21 | 89 |
| 9/21/2014 | 38,562 | 1889.50 | -19 | 115 | 24 | 89 |
| 9/22/2014 | 38,329 | 1889.40 | -16 | 115 | 24 | 90 |
| 9/23/2014 | 38,104 | 1889.20 | -11 | 115 | 22 | 90 |
| 9/24/2014 | 37,889 | 1889.10 | -1 | 115 | 24 | 91 |
| 9/25/2014 | 37,711 | 1889.00 | 7 | 115 | 25 | 91 |
| 9/26/2014 | 37,488 | 1888.80 | -10 | 115 | 26 | 91 |
| 9/27/2014 | 37,266 | 1888.70 | -7 | 114 | 23 | 91 |
| 9/28/2014 | 37,041 | 1888.50 | -14 | 113 | 23 | 90 |
| 9/29/2014 | 36,814 | 1888.40 | -13 | 113 | 21 | 91 |
| 9/30/2014 | 36,582 | 1888.20 | -14 | 113 | 21 | 90 |
| 10/1/2014 | 36,354 | 1888.10 | -17 | 113 | 21 | 91 |
| 10/2/2014 | 36,118 | 1887.90 | -18 | 115 | 23 | 91 |
| 10/3/2014 | 35,889 | 1887.70 | -15 | 115 | 24 | 91 |
| 10/4/2014 | 35,660 | 1887.60 | -12 | 114 | 24 | 91 |
| 10/5/2014 | 35,431 | 1887.40 | -13 | 114 | 24 | 91 |
| 10/6/2014 | 35,198 | 1887.30 | -14 | 114 | 23 | 90 |
| 10/7/2014 | 34,964 | 1887.10 | -17 | 115 | 23 | 91 |
| 10/8/2014 | 34,744 | 1886.90 | -11 | 115 | 23 | 91 |
| 10/9/2014 | 34,518 | 1886.80 | -12 | 115 | 23 | 91 |
| 10/10/2014 | 34,286 | 1886.60 | -11 | 114 | 23 | 90 |
| 10/11/2014 | 34,064 | 1886.50 | -10 | 114 | 24 | 90 |
| 10/12/2014 | 33,837 | 1886.30 | -12 | 114 | 24 | 90 |
| 10/13/2014 | 33,603 | 1886.10 | -14 | 115 | 25 | 90 |
| 10/14/2014 | 33,384 | 1886.00 | -7 | 116 | 27 | 90 |
| 10/15/2014 | 33,238 | 1885.90 | 25 | 111 | 33 | 90 |
| 10/16/2014 | 33,106 | 1885.80 | 2 | 72 | 31 | 51 |
| 10/17/2014 | 32,984 | 1885.70 | 4 | 72 | 26 | 45 |
| 10/18/2014 | 32,867 | 1885.60 | 12 | 74 | 26 | 45 |
| 10/19/2014 | 32,731 | 1885.50 | 2 | 76 | 31 | 45 |
| 10/20/2014 | 32,606 | 1885.40 | 8 | 76 | 34 | 45 |
| 10/21/2014 | 32,469 | 1885.30 | 3 | 77 | 31 | 45 |
| 10/22/2014 | 32,334 | 1885.20 | 6 | 77 | 31 | 45 |
| 10/23/2014 | 32,170 | 1885.10 | -14 | 77 | 34 | 45 |
| 10/24/2014 | 32,042 | 1885.00 | 4 | 76 | 33 | 45 |
| 10/25/2014 | 32,037 | 1885.00 | 70 | 77 | 44 | 45 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/26/2014 | 31,967 | 1884.90 | 35 | 76 | 38 | 45 |
| 10/27/2014 | 31,869 | 1884.90 | 18 | 76 | 33 | 45 |
| 10/28/2014 | 31,745 | 1884.80 | 9 | 76 | 32 | 45 |
| 10/29/2014 | 31,629 | 1884.70 | 10 | 76 | 31 | 45 |
| 10/30/2014 | 31,510 | 1884.60 | 7 | 76 | 31 | 45 |
| 10/31/2014 | 31,439 | 1884.50 | 33 | 77 | 37 | 45 |
| 11/1/2014 | 31,308 | 1884.40 | 3 | 77 | 34 | 45 |
| 11/2/2014 | 31,183 | 1884.40 | 5 | 77 | 33 | 45 |
| 11/3/2014 | 31,048 | 1884.20 | 4 | 77 | 34 | 45 |
| 11/4/2014 | 30,925 | 1884.20 | 6 | 77 | 33 | 45 |
| 11/5/2014 | 30,806 | 1884.10 | 10 | 77 | 33 | 45 |
| 11/6/2014 | 30,672 | 1884.00 | 3 | 77 | 33 | 45 |
| 11/7/2014 | 30,548 | 1883.90 | 7 | 77 | 33 | 45 |
| 11/8/2014 | 30,426 | 1883.80 | 4 | 77 | 33 | 45 |
| 11/9/2014 | 30,297 | 1883.70 | 8 | 78 | 33 | 45 |
| 11/10/2014 | 30,172 | 1883.60 | 4 | 78 | 33 | 45 |
| 11/11/2014 | 30,043 | 1883.50 | 5 | 78 | 33 | 45 |
| 11/12/2014 | 29,925 | 1883.40 | 16 | 78 | 33 | 45 |
| 11/13/2014 | 29,875 | 1883.30 | 41 | 79 | 39 | 45 |
| 11/14/2014 | 29,758 | 1883.30 | 13 | 77 | 34 | 45 |
| 11/15/2014 | 29,642 | 1883.20 | 8 | 75 | 39 | 25 |
| 11/16/2014 | 29,539 | 1883.10 | 10 | 69 | 46 | 0 |
| 11/17/2014 | 29,439 | 1883.00 | 8 | 69 | 44 | 0 |
| 11/18/2014 | 29,340 | 1882.90 | 13 | 69 | 42 | 0 |
| 11/19/2014 | 29,335 | 1882.90 | 62 | 69 | 48 | 0 |
| 11/20/2014 | 29,373 | 1883.00 | 84 | 69 | 57 | 0 |
| 11/21/2014 | 29,417 | 1883.00 | 90 | 68 | 59 | 0 |
| 11/22/2014 | 30,276 | 1883.70 | 502 | 68 | 211 | 0 |
| 11/23/2014 | 30,500 | 1883.80 | 174 | 68 | 89 | 0 |
| 11/24/2014 | 30,396 | 1883.80 | 25 | 90 | 60 | 0 |
| 11/25/2014 | 30,162 | 1883.60 | 19 | 162 | 160 | 0 |
| 11/26/2014 | 29,900 | 1883.40 | 34 | 190 | 171 | 0 |
| 11/27/2014 | 29,627 | 1883.20 | 31 | 190 | 169 | 0 |
| 11/28/2014 | 29,452 | 1883.00 | 77 | 191 | 172 | 0 |
| 11/29/2014 | 29,373 | 1883.00 | 132 | 191 | 194 | 0 |
| 11/30/2014 | 29,574 | 1883.10 | 275 | 192 | 246 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/1/2014 | 29,646 | 1883.20 | 216 | 192 | 205 | 0 |
| 12/2/2014 | 29,778 | 1883.30 | 247 | 194 | 200 | 0 |
| 12/3/2014 | 33,972 | 1886.40 | 2,335 | 206 | 581 | 0 |
| 12/4/2014 | 35,600 | 1887.50 | 1,112 | 205 | 432 | 0 |
| 12/5/2014 | 36,866 | 1888.40 | 808 | 208 | 398 | 0 |
| 12/6/2014 | 39,428 | 1890.10 | 1,593 | 208 | 601 | 0 |
| 12/7/2014 | 40,098 | 1890.50 | 561 | 208 | 307 | 0 |
| 12/8/2014 | 41,063 | 1891.10 | 682 | 208 | 336 | 0 |
| 12/9/2014 | 41,927 | 1891.70 | 671 | 208 | 349 | 0 |
| 12/10/2014 | 43,243 | 1892.50 | 837 | 211 | 363 | 0 |
| 12/11/2014 | 61,154 | 1902.50 | 12,782 | 3,541 | 5,772 | 0 |
| 12/12/2014 | 57,962 | 1900.80 | 4,102 | 5,322 | 7,946 | 0 |
| 12/13/2014 | 56,477 | 1900.00 | 1,669 | 2,284 | 3,270 | 0 |
| 12/14/2014 | 55,772 | 1899.70 | 1,085 | 1,310 | 1,694 | 0 |
| 12/15/2014 | 55,584 | 1899.60 | 977 | 975 | 1,153 | 0 |
| 12/16/2014 | 56,065 | 1899.80 | 1,271 | 971 | 1,186 | 0 |
| 12/17/2014 | 56,420 | 1900.00 | 1,913 | 1,577 | 2,279 | 0 |
| 12/18/2014 | 56,177 | 1899.90 | 1,550 | 1,537 | 2,191 | 0 |
| 12/19/2014 | 57,396 | 1900.50 | 2,817 | 2,091 | 2,963 | 0 |
| 12/20/2014 | 56,999 | 1900.30 | 2,211 | 2,273 | 3,294 | 0 |
| 12/21/2014 | 56,995 | 1900.30 | 2,355 | 2,207 | 3,084 | 0 |
| 12/22/2014 | 56,498 | 1900.00 | 1,859 | 1,946 | 2,661 | 0 |
| 12/23/2014 | 55,994 | 1899.80 | 1,321 | 1,444 | 1,896 | 0 |
| 12/24/2014 | 55,718 | 1899.60 | 1,093 | 1,126 | 1,385 | 0 |
| 12/25/2014 | 55,463 | 1899.50 | 903 | 932 | 1,080 | 0 |
| 12/26/2014 | 55,251 | 1899.40 | 718 | 740 | 852 | 0 |
| 12/27/2014 | 55,064 | 1899.30 | 571 | 604 | 699 | 0 |
| 12/28/2014 | 54,948 | 1899.20 | 476 | 504 | 590 | 0 |
| 12/29/2014 | 54,837 | 1899.20 | 388 | 429 | 512 | 0 |
| 12/30/2014 | 54,750 | 1899.10 | 335 | 378 | 454 | 0 |
| 12/31/2014 | 54,680 | 1899.10 | 286 | 324 | 394 | 0 |
| 1/1/2015 | 54,613 | 1899.00 | 251 | 296 | 358 | 0 |
| 1/2/2015 | 54,545 | 1899.00 | 222 | 271 | 327 | 0 |
| 1/3/2015 | 54,488 | 1899.00 | 203 | 251 | 300 | 0 |
| 1/4/2015 | 54,418 | 1898.90 | 177 | 233 | 275 | 0 |
| 1/5/2015 | 54,364 | 1898.90 | 166 | 219 | 254 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/6/2015 | 54,309 | 1898.90 | 154 | 207 | 236 | 0 |
| 1/7/2015 | 54,261 | 1898.80 | 146 | 198 | 221 | 0 |
| 1/8/2015 | 54,186 | 1898.80 | 125 | 193 | 210 | 0 |
| 1/9/2015 | 54,116 | 1898.80 | 126 | 190 | 203 | 0 |
| 1/10/2015 | 54,025 | 1898.70 | 116 | 189 | 198 | 0 |
| 1/11/2015 | 53,934 | 1898.70 | 111 | 189 | 196 | 0 |
| 1/12/2015 | 53,831 | 1898.60 | 106 | 188 | 194 | 0 |
| 1/13/2015 | 53,718 | 1898.50 | 101 | 188 | 193 | 0 |
| 1/14/2015 | 53,578 | 1898.50 | 90 | 188 | 192 | 0 |
| 1/15/2015 | 53,429 | 1898.40 | 85 | 188 | 191 | 0 |
| 1/16/2015 | 53,409 | 1898.40 | 149 | 188 | 202 | 0 |
| 1/17/2015 | 53,317 | 1898.30 | 114 | 187 | 198 | 0 |
| 1/18/2015 | 53,279 | 1898.30 | 137 | 187 | 210 | 0 |
| 1/19/2015 | 53,258 | 1898.30 | 150 | 188 | 207 | 0 |
| 1/20/2015 | 53,178 | 1898.20 | 116 | 189 | 198 | 0 |
| 1/21/2015 | 53,063 | 1898.20 | 100 | 189 | 195 | 0 |
| 1/22/2015 | 52,935 | 1898.10 | 93 | 189 | 192 | 0 |
| 1/23/2015 | 52,783 | 1898.00 | 79 | 188 | 191 | 0 |
| 1/24/2015 | 52,641 | 1897.90 | 87 | 188 | 190 | 0 |
| 1/25/2015 | 52,480 | 1897.90 | 77 | 187 | 190 | 0 |
| 1/26/2015 | 52,306 | 1897.80 | 67 | 186 | 187 | 0 |
| 1/27/2015 | 52,130 | 1897.70 | 62 | 184 | 185 | 0 |
| 1/28/2015 | 51,963 | 1897.60 | 64 | 181 | 181 | 0 |
| 1/29/2015 | 51,790 | 1897.50 | 61 | 179 | 178 | 0 |
| 1/30/2015 | 51,621 | 1897.40 | 52 | 176 | 175 | 0 |
| 1/31/2015 | 51,441 | 1897.30 | 56 | 176 | 173 | 0 |
| 2/1/2015 | 51,255 | 1897.20 | 48 | 176 | 173 | 0 |
| 2/2/2015 | 51,071 | 1897.10 | 47 | 175 | 173 | 0 |
| 2/3/2015 | 50,896 | 1897.00 | 50 | 173 | 171 | 0 |
| 2/4/2015 | 50,716 | 1896.90 | 49 | 173 | 170 | 0 |
| 2/5/2015 | 50,538 | 1896.80 | 56 | 174 | 170 | 0 |
| 2/6/2015 | 56,926 | 1900.30 | 3,299 | 287 | 615 | 0 |
| 2/7/2015 | 58,117 | 1900.90 | 5,301 | 4,273 | 5,709 | 0 |
| 2/8/2015 | 57,473 | 1900.60 | 2,346 | 2,545 | 3,422 | 0 |
| 2/9/2015 | 58,578 | 1901.10 | 4,216 | 3,675 | 5,131 | 0 |
| 2/10/2015 | 57,259 | 1900.40 | 2,157 | 2,934 | 4,194 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/11/2015 | 56,453 | 1900.00 | 1,367 | 1,857 | 2,466 | 0 |
| 2/12/2015 | 55,948 | 1899.80 | 970 | 1,288 | 1,624 | 0 |
| 2/13/2015 | 55,625 | 1899.60 | 776 | 991 | 1,139 | 0 |
| 2/14/2015 | 55,399 | 1899.50 | 638 | 792 | 886 | 0 |
| 2/15/2015 | 55,215 | 1899.40 | 532 | 651 | 730 | 0 |
| 2/16/2015 | 55,081 | 1899.30 | 464 | 561 | 621 | 0 |
| 2/17/2015 | 54,970 | 1899.20 | 403 | 483 | 539 | 0 |
| 2/18/2015 | 54,869 | 1899.20 | 345 | 413 | 476 | 0 |
| 2/19/2015 | 54,786 | 1899.10 | 310 | 367 | 427 | 0 |
| 2/20/2015 | 54,724 | 1899.10 | 283 | 331 | 387 | 0 |
| 2/21/2015 | 54,631 | 1899.00 | 254 | 314 | 354 | 14 |
| 2/22/2015 | 54,531 | 1899.00 | 226 | 290 | 320 | 46 |
| 2/23/2015 | 54,410 | 1898.90 | 209 | 284 | 288 | 57 |
| 2/24/2015 | 54,306 | 1898.90 | 197 | 269 | 255 | 71 |
| 2/25/2015 | 54,220 | 1898.80 | 184 | 252 | 235 | 73 |
| 2/26/2015 | 54,120 | 1898.80 | 179 | 244 | 222 | 70 |
| 2/27/2015 | 54,039 | 1898.70 | 178 | 242 | 218 | 70 |
| 2/28/2015 | 53,937 | 1898.70 | 163 | 242 | 215 | 70 |
| 3/1/2015 | 53,785 | 1898.60 | 159 | 240 | 212 | 69 |
| 3/2/2015 | 53,693 | 1898.50 | 145 | 220 | 215 | 46 |
| 3/3/2015 | 53,582 | 1898.50 | 141 | 213 | 203 | 46 |
| 3/4/2015 | 53,463 | 1898.40 | 129 | 207 | 194 | 45 |
| 3/5/2015 | 53,343 | 1898.30 | 118 | 206 | 191 | 45 |
| 3/6/2015 | 53,206 | 1898.30 | 118 | 205 | 190 | 45 |
| 3/7/2015 | 53,069 | 1898.20 | 111 | 205 | 190 | 45 |
| 3/8/2015 | 52,932 | 1898.10 | 109 | 201 | 187 | 45 |
| 3/9/2015 | 52,777 | 1898.00 | 100 | 199 | 183 | 45 |
| 3/10/2015 | 52,650 | 1898.00 | 102 | 194 | 183 | 45 |
| 3/11/2015 | 52,468 | 1897.90 | 107 | 195 | 182 | 45 |
| 3/12/2015 | 52,324 | 1897.80 | 104 | 193 | 180 | 45 |
| 3/13/2015 | 52,152 | 1897.70 | 99 | 188 | 176 | 45 |
| 3/14/2015 | 52,017 | 1897.60 | 98 | 179 | 169 | 45 |
| 3/15/2015 | 51,863 | 1897.50 | 82 | 172 | 161 | 45 |
| 3/16/2015 | 51,728 | 1897.40 | 90 | 172 | 160 | 45 |
| 3/17/2015 | 51,566 | 1897.30 | 82 | 172 | 160 | 45 |
| 3/18/2015 | 51,404 | 1897.30 | 77 | 174 | 155 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/19/2015 | 51,216 | 1897.20 | 71 | 176 | 153 | 55 |
| 3/20/2015 | 51,037 | 1897.10 | 71 | 172 | 151 | 56 |
| 3/21/2015 | 50,868 | 1897.00 | 70 | 168 | 147 | 55 |
| 3/22/2015 | 50,779 | 1896.90 | 100 | 164 | 150 | 49 |
| 3/23/2015 | 50,672 | 1896.80 | 99 | 161 | 155 | 44 |
| 3/24/2015 | 50,601 | 1896.80 | 109 | 161 | 153 | 44 |
| 3/25/2015 | 50,513 | 1896.80 | 105 | 161 | 152 | 45 |
| 3/26/2015 | 50,371 | 1896.70 | 81 | 162 | 149 | 45 |
| 3/27/2015 | 50,212 | 1896.60 | 65 | 162 | 148 | 45 |
| 3/28/2015 | 50,053 | 1896.50 | 67 | 161 | 147 | 45 |
| 3/29/2015 | 49,894 | 1896.40 | 59 | 161 | 147 | 45 |
| 3/30/2015 | 49,700 | 1896.30 | 55 | 162 | 145 | 46 |
| 3/31/2015 | 49,506 | 1896.20 | 39 | 162 | 145 | 46 |
| 4/1/2015 | 49,296 | 1896.10 | 51 | 166 | 142 | 52 |
| 4/2/2015 | 49,069 | 1895.90 | 40 | 172 | 143 | 60 |
| 4/3/2015 | 48,843 | 1895.80 | 47 | 176 | 133 | 76 |
| 4/4/2015 | 48,600 | 1895.70 | 45 | 176 | 128 | 81 |
| 4/5/2015 | 48,444 | 1895.60 | 73 | 171 | 136 | 72 |
| 4/6/2015 | 48,288 | 1895.50 | 80 | 166 | 133 | 68 |
| 4/7/2015 | 48,409 | 1895.60 | 205 | 168 | 159 | 68 |
| 4/8/2015 | 48,340 | 1895.50 | 126 | 167 | 150 | 68 |
| 4/9/2015 | 48,219 | 1895.40 | 94 | 167 | 147 | 63 |
| 4/10/2015 | 48,063 | 1895.40 | 80 | 167 | 150 | 54 |
| 4/11/2015 | 47,925 | 1895.30 | 79 | 167 | 150 | 50 |
| 4/12/2015 | 47,770 | 1895.20 | 80 | 167 | 149 | 50 |
| 4/13/2015 | 47,598 | 1895.10 | 63 | 167 | 147 | 50 |
| 4/14/2015 | 47,359 | 1894.90 | 52 | 183 | 151 | 49 |
| 4/15/2015 | 47,103 | 1894.80 | 62 | 206 | 158 | 77 |
| 4/16/2015 | 46,814 | 1894.60 | 54 | 204 | 145 | 90 |
| 4/17/2015 | 46,560 | 1894.50 | 53 | 200 | 141 | 90 |
| 4/18/2015 | 46,306 | 1894.30 | 52 | 196 | 137 | 90 |
| 4/19/2015 | 46,053 | 1894.20 | 54 | 196 | 135 | 90 |
| 4/20/2015 | 45,785 | 1894.00 | 52 | 194 | 134 | 90 |
| 4/21/2015 | 45,384 | 1893.80 | 43 | 257 | 164 | 90 |
| 4/22/2015 | 45,002 | 1893.60 | 43 | 243 | 194 | 90 |
| 4/23/2015 | 44,753 | 1893.40 | 38 | 193 | 136 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/24/2015 | 44,488 | 1893.20 | 46 | 193 | 135 | 90 |
| 4/25/2015 | 44,241 | 1893.10 | 67 | 193 | 139 | 90 |
| 4/26/2015 | 43,946 | 1892.90 | 52 | 193 | 135 | 90 |
| 4/27/2015 | 43,701 | 1892.80 | 48 | 193 | 134 | 90 |
| 4/28/2015 | 43,409 | 1892.60 | 37 | 194 | 133 | 90 |
| 4/29/2015 | 43,116 | 1892.40 | 35 | 194 | 133 | 90 |
| 4/30/2015 | 42,825 | 1892.20 | 30 | 194 | 132 | 90 |
| 5/1/2015 | 42,503 | 1892.00 | 26 | 194 | 131 | 90 |
| 5/2/2015 | 42,198 | 1891.80 | 26 | 193 | 130 | 90 |
| 5/3/2015 | 41,880 | 1891.60 | 23 | 193 | 129 | 90 |
| 5/4/2015 | 41,561 | 1891.40 | 20 | 193 | 128 | 90 |
| 5/5/2015 | 41,102 | 1891.20 | 16 | 255 | 157 | 90 |
| 5/6/2015 | 40,662 | 1890.90 | 14 | 244 | 189 | 90 |
| 5/7/2015 | 40,365 | 1890.70 | 10 | 198 | 136 | 90 |
| 5/8/2015 | 40,007 | 1890.50 | 18 | 197 | 135 | 90 |
| 5/9/2015 | 39,682 | 1890.30 | 19 | 198 | 135 | 90 |
| 5/10/2015 | 39,343 | 1890.00 | 20 | 198 | 134 | 90 |
| 5/11/2015 | 39,006 | 1889.80 | 6 | 191 | 134 | 86 |
| 5/12/2015 | 38,655 | 1889.60 | 3 | 186 | 134 | 79 |
| 5/13/2015 | 38,351 | 1889.40 | 19 | 186 | 133 | 79 |
| 5/14/2015 | 37,883 | 1889.10 | 29 | 287 | 167 | 96 |
| 5/15/2015 | 37,449 | 1888.80 | 24 | 272 | 191 | 118 |
| 5/16/2015 | 37,122 | 1888.60 | 24 | 201 | 118 | 112 |
| 5/17/2015 | 36,884 | 1888.40 | 71 | 190 | 113 | 105 |
| 5/18/2015 | 36,633 | 1888.20 | 30 | 173 | 110 | 95 |
| 5/19/2015 | 36,412 | 1888.10 | 24 | 148 | 92 | 77 |
| 5/20/2015 | 36,236 | 1888.00 | 34 | 137 | 78 | 77 |
| 5/21/2015 | 36,090 | 1887.90 | 48 | 131 | 67 | 77 |
| 5/22/2015 | 35,959 | 1887.80 | 49 | 130 | 64 | 77 |
| 5/23/2015 | 35,784 | 1887.70 | 27 | 130 | 63 | 76 |
| 5/24/2015 | 35,596 | 1887.50 | 20 | 130 | 62 | 77 |
| 5/25/2015 | 35,393 | 1887.40 | 14 | 130 | 61 | 76 |
| 5/26/2015 | 35,205 | 1887.30 | 17 | 126 | 59 | 76 |
| 5/27/2015 | 35,004 | 1887.10 | 17 | 124 | 51 | 78 |
| 5/28/2015 | 34,832 | 1887.00 | 21 | 116 | 53 | 71 |
| 5/29/2015 | 34,675 | 1886.90 | 17 | 109 | 45 | 67 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/30/2015 | 34,533 | 1886.80 | 19 | 102 | 43 | 61 |
| 5/31/2015 | 34,377 | 1886.70 | 17 | 100 | 41 | 59 |
| 6/1/2015 | 34,250 | 1886.60 | 20 | 86 | 38 | 56 |
| 6/2/2015 | 34,123 | 1886.50 | 17 | 91 | 38 | 52 |
| 6/3/2015 | 33,967 | 1886.40 | 8 | 100 | 40 | 56 |
| 6/4/2015 | 33,827 | 1886.30 | 14 | 99 | 37 | 67 |
| 6/5/2015 | 33,672 | 1886.20 | 13 | 96 | 31 | 66 |
| 6/6/2015 | 33,519 | 1886.10 | 9 | 96 | 31 | 66 |
| 6/7/2015 | 33,393 | 1886.00 | 13 | 87 | 31 | 63 |
| 6/8/2015 | 33,268 | 1885.90 | 11 | 80 | 26 | 56 |
| 6/9/2015 | 33,102 | 1885.80 | 3 | 98 | 29 | 60 |
| 6/10/2015 | 32,923 | 1885.60 | 6 | 104 | 27 | 77 |
| 6/11/2015 | 32,757 | 1885.50 | 8 | 102 | 24 | 77 |
| 6/12/2015 | 32,565 | 1885.40 | 0 | 99 | 22 | 77 |
| 6/13/2015 | 32,387 | 1885.20 | -2 | 101 | 20 | 77 |
| 6/14/2015 | 32,210 | 1885.10 | -5 | 98 | 20 | 77 |
| 6/15/2015 | 32,033 | 1885.00 | -4 | 97 | 18 | 77 |
| 6/16/2015 | 31,844 | 1884.80 | -6 | 96 | 18 | 77 |
| 6/17/2015 | 31,656 | 1884.70 | -9 | 99 | 17 | 77 |
| 6/18/2015 | 31,468 | 1884.60 | -13 | 99 | 21 | 78 |
| 6/19/2015 | 31,267 | 1884.40 | -8 | 95 | 17 | 77 |
| 6/20/2015 | 31,094 | 1884.30 | -9 | 95 | 17 | 77 |
| 6/21/2015 | 30,895 | 1884.10 | -14 | 95 | 17 | 77 |
| 6/22/2015 | 30,710 | 1884.00 | -11 | 95 | 17 | 77 |
| 6/23/2015 | 30,526 | 1883.90 | -9 | 95 | 16 | 76 |
| 6/24/2015 | 30,343 | 1883.70 | -5 | 96 | 16 | 76 |
| 6/25/2015 | 30,173 | 1883.60 | -5 | 97 | 15 | 76 |
| 6/26/2015 | 29,978 | 1883.40 | -8 | 100 | 18 | 76 |
| 6/27/2015 | 29,797 | 1883.30 | -9 | 91 | 19 | 71 |
| 6/28/2015 | 29,616 | 1883.10 | -12 | 87 | 18 | 65 |
| 6/29/2015 | 29,462 | 1883.00 | -9 | 84 | 17 | 65 |
| 6/30/2015 | 29,283 | 1882.90 | -9 | 84 | 15 | 65 |
| 7/1/2015 | 29,118 | 1882.80 | -12 | 85 | 17 | 65 |
| 7/2/2015 | 28,941 | 1882.60 | -9 | 85 | 16 | 65 |
| 7/3/2015 | 28,777 | 1882.50 | -3 | 94 | 19 | 67 |
| 7/4/2015 | 28,576 | 1882.30 | -12 | 96 | 19 | 73 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/5/2015 | 28,389 | 1882.20 | -14 | 94 | 16 | 73 |
| 7/6/2015 | 28,190 | 1882.00 | -14 | 94 | 17 | 73 |
| 7/7/2015 | 28,004 | 1881.90 | -14 | 94 | 17 | 73 |
| 7/8/2015 | 27,857 | 1881.70 | 3 | 93 | 18 | 73 |
| 7/9/2015 | 27,758 | 1881.70 | 37 | 92 | 19 | 73 |
| 7/10/2015 | 27,636 | 1881.60 | 18 | 92 | 18 | 73 |
| 7/11/2015 | 27,478 | 1881.40 | 7 | 92 | 17 | 73 |
| 7/12/2015 | 27,309 | 1881.30 | -3 | 92 | 16 | 73 |
| 7/13/2015 | 27,176 | 1881.20 | 0 | 77 | 18 | 65 |
| 7/14/2015 | 27,032 | 1881.10 | -5 | 73 | 15 | 53 |
| 7/15/2015 | 26,888 | 1880.90 | -5 | 76 | 19 | 54 |
| 7/16/2015 | 26,733 | 1880.80 | -7 | 77 | 17 | 57 |
| 7/17/2015 | 26,567 | 1880.70 | -10 | 79 | 15 | 58 |
| 7/18/2015 | 26,414 | 1880.50 | -12 | 80 | 17 | 62 |
| 7/19/2015 | 26,250 | 1880.40 | -7 | 79 | 16 | 62 |
| 7/20/2015 | 26,121 | 1880.30 | 0 | 76 | 16 | 59 |
| 7/21/2015 | 25,982 | 1880.20 | -7 | 73 | 19 | 52 |
| 7/22/2015 | 25,797 | 1880.00 | -18 | 84 | 20 | 55 |
| 7/23/2015 | 25,612 | 1879.80 | -15 | 85 | 19 | 68 |
| 7/24/2015 | 25,441 | 1879.70 | -12 | 84 | 15 | 68 |
| 7/25/2015 | 25,270 | 1879.50 | -15 | 84 | 15 | 67 |
| 7/26/2015 | 25,090 | 1879.40 | -13 | 84 | 16 | 67 |
| 7/27/2015 | 24,921 | 1879.20 | -12 | 90 | 16 | 68 |
| 7/28/2015 | 24,732 | 1879.10 | -10 | 91 | 17 | 73 |
| 7/29/2015 | 24,554 | 1878.90 | -13 | 90 | 14 | 73 |
| 7/30/2015 | 24,378 | 1878.80 | -9 | 90 | 14 | 73 |
| 7/31/2015 | 24,202 | 1878.60 | -7 | 92 | 14 | 73 |
| 8/1/2015 | 24,029 | 1878.40 | -6 | 92 | 14 | 73 |
| 8/2/2015 | 23,856 | 1878.30 | -9 | 92 | 14 | 73 |
| 8/3/2015 | 23,674 | 1878.10 | -7 | 92 | 14 | 73 |
| 8/4/2015 | 23,493 | 1877.90 | -5 | 92 | 14 | 73 |
| 8/5/2015 | 23,335 | 1877.80 | -6 | 89 | 15 | 71 |
| 8/6/2015 | 23,155 | 1877.60 | -7 | 87 | 16 | 67 |
| 8/7/2015 | 22,989 | 1877.50 | -11 | 87 | 15 | 66 |
| 8/8/2015 | 22,824 | 1877.30 | -9 | 84 | 17 | 64 |
| 8/9/2015 | 22,660 | 1877.20 | -10 | 80 | 21 | 58 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/10/2015 | 22,517 | 1877.00 | -8 | 76 | 18 | 56 |
| 8/11/2015 | 22,385 | 1876.90 | -8 | 72 | 18 | 52 |
| 8/12/2015 | 22,244 | 1876.80 | -11 | 71 | 15 | 52 |
| 8/13/2015 | 22,093 | 1876.60 | -11 | 79 | 17 | 54 |
| 8/14/2015 | 21,934 | 1876.40 | -5 | 83 | 17 | 60 |
| 8/15/2015 | 21,786 | 1876.30 | -2 | 80 | 17 | 60 |
| 8/16/2015 | 21,639 | 1876.20 | -6 | 79 | 13 | 60 |
| 8/17/2015 | 21,492 | 1876.00 | -7 | 79 | 12 | 60 |
| 8/18/2015 | 21,337 | 1875.90 | -6 | 80 | 12 | 60 |
| 8/19/2015 | 21,182 | 1875.70 | -5 | 82 | 16 | 60 |
| 8/20/2015 | 21,029 | 1875.50 | -4 | 82 | 17 | 60 |
| 8/21/2015 | 20,886 | 1875.40 | -2 | 81 | 15 | 60 |
| 8/22/2015 | 20,725 | 1875.20 | -2 | 85 | 16 | 61 |
| 8/23/2015 | 20,556 | 1875.10 | -5 | 87 | 17 | 64 |
| 8/24/2015 | 20,397 | 1874.90 | -4 | 86 | 16 | 64 |
| 8/25/2015 | 20,248 | 1874.70 | -4 | 82 | 17 | 62 |
| 8/26/2015 | 20,091 | 1874.60 | -5 | 78 | 18 | 56 |
| 8/27/2015 | 19,954 | 1874.40 | -3 | 79 | 17 | 56 |
| 8/28/2015 | 19,808 | 1874.30 | 1 | 80 | 17 | 56 |
| 8/29/2015 | 19,645 | 1874.10 | -3 | 85 | 18 | 58 |
| 8/30/2015 | 19,483 | 1873.90 | -1 | 88 | 15 | 65 |
| 8/31/2015 | 19,307 | 1873.70 | 1 | 87 | 16 | 65 |
| 9/1/2015 | 19,148 | 1873.50 | -3 | 81 | 16 | 61 |
| 9/2/2015 | 18,998 | 1873.40 | -4 | 77 | 15 | 55 |
| 9/3/2015 | 18,858 | 1873.20 | -4 | 76 | 15 | 55 |
| 9/4/2015 | 18,710 | 1873.10 | -8 | 75 | 15 | 55 |
| 9/5/2015 | 18,571 | 1872.90 | -1 | 75 | 18 | 52 |
| 9/6/2015 | 18,425 | 1872.70 | -5 | 73 | 18 | 50 |
| 9/7/2015 | 18,297 | 1872.60 | -2 | 70 | 16 | 50 |
| 9/8/2015 | 18,170 | 1872.40 | -3 | 70 | 14 | 50 |
| 9/9/2015 | 18,035 | 1872.30 | 0 | 78 | 15 | 52 |
| 9/10/2015 | 17,884 | 1872.10 | 0 | 82 | 14 | 59 |
| 9/11/2015 | 17,742 | 1871.90 | 0 | 81 | 14 | 59 |
| 9/12/2015 | 17,601 | 1871.80 | 0 | 81 | 14 | 59 |
| 9/13/2015 | 17,462 | 1871.60 | 7 | 81 | 15 | 59 |
| 9/14/2015 | 17,331 | 1871.40 | 5 | 80 | 17 | 57 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/15/2015 | 17,185 | 1871.30 | -23 | 76 | 20 | 53 |
| 9/16/2015 | 17,097 | 1871.20 | 24 | 73 | 24 | 53 |
| 9/17/2015 | 16,969 | 1871.00 | 1 | 71 | 22 | 52 |
| 9/18/2015 | 16,849 | 1870.90 | 1 | 67 | 23 | 47 |
| 9/19/2015 | 16,731 | 1870.70 | -2 | 64 | 22 | 43 |
| 9/20/2015 | 16,613 | 1870.60 | 0 | 61 | 18 | 43 |
| 9/21/2015 | 16,503 | 1870.40 | 0 | 60 | 15 | 43 |
| 9/22/2015 | 16,387 | 1870.30 | -2 | 59 | 14 | 42 |
| 9/23/2015 | 16,278 | 1870.10 | 1 | 58 | 14 | 42 |
| 9/24/2015 | 16,170 | 1870.00 | 1 | 58 | 14 | 43 |
| 9/25/2015 | 16,055 | 1869.80 | 0 | 57 | 14 | 42 |
| 9/26/2015 | 15,941 | 1869.70 | 0 | 56 | 14 | 43 |
| 9/27/2015 | 15,835 | 1869.60 | 1 | 56 | 14 | 43 |
| 9/28/2015 | 15,729 | 1869.40 | 0 | 56 | 13 | 43 |
| 9/29/2015 | 15,601 | 1869.20 | -3 | 56 | 13 | 43 |
| 9/30/2015 | 15,497 | 1869.10 | 4 | 56 | 13 | 43 |
| 10/1/2015 | 15,400 | 1869.00 | 4 | 57 | 15 | 43 |
| 10/2/2015 | 15,296 | 1868.80 | 4 | 55 | 15 | 41 |
| 10/3/2015 | 15,185 | 1868.70 | 0 | 54 | 17 | 38 |
| 10/4/2015 | 15,090 | 1868.60 | 2 | 53 | 15 | 38 |
| 10/5/2015 | 14,972 | 1868.40 | 0 | 59 | 16 | 39 |
| 10/6/2015 | 14,849 | 1868.20 | 1 | 63 | 19 | 45 |
| 10/7/2015 | 14,747 | 1868.10 | 2 | 62 | 15 | 45 |
| 10/8/2015 | 14,625 | 1867.90 | 1 | 62 | 15 | 45 |
| 10/9/2015 | 14,517 | 1867.80 | 1 | 62 | 15 | 45 |
| 10/10/2015 | 14,403 | 1867.60 | 1 | 62 | 15 | 45 |
| 10/11/2015 | 14,281 | 1867.40 | 3 | 62 | 15 | 45 |
| 10/12/2015 | 14,182 | 1867.30 | 3 | 62 | 15 | 45 |
| 10/13/2015 | 14,063 | 1867.10 | -3 | 62 | 14 | 45 |
| 10/14/2015 | 13,950 | 1867.00 | 10 | 62 | 14 | 45 |
| 10/15/2015 | 13,846 | 1866.80 | 4 | 62 | 14 | 45 |
| 10/16/2015 | 13,749 | 1866.70 | 4 | 56 | 21 | 38 |
| 10/17/2015 | 13,666 | 1866.50 | 8 | 45 | 16 | 33 |
| 10/18/2015 | 13,597 | 1866.40 | 20 | 50 | 12 | 32 |
| 10/19/2015 | 13,507 | 1866.30 | 8 | 49 | 18 | 33 |
| 10/20/2015 | 13,425 | 1866.20 | 9 | 49 | 12 | 33 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/21/2015 | 13,330 | 1866.00 | 4 | 51 | 15 | 33 |
| 10/22/2015 | 13,228 | 1865.90 | -1 | 52 | 17 | 33 |
| 10/23/2015 | 13,134 | 1865.70 | 0 | 52 | 16 | 33 |
| 10/24/2015 | 13,053 | 1865.60 | 11 | 51 | 16 | 33 |
| 10/25/2015 | 12,959 | 1865.50 | 3 | 49 | 14 | 33 |
| 10/26/2015 | 12,879 | 1865.40 | 8 | 49 | 13 | 33 |
| 10/27/2015 | 12,800 | 1865.20 | 8 | 49 | 15 | 33 |
| 10/28/2015 | 12,720 | 1865.10 | 9 | 50 | 15 | 33 |
| 10/29/2015 | 12,654 | 1865.00 | 9 | 39 | 30 | 19 |
| 10/30/2015 | 12,615 | 1865.00 | 14 | 27 | 20 | 11 |
| 10/31/2015 | 12,576 | 1864.90 | 15 | 28 | 14 | 11 |
| 11/1/2015 | 12,563 | 1864.90 | 23 | 28 | 16 | 11 |
| 11/2/2015 | 12,523 | 1864.80 | 1 | 28 | 17 | 11 |
| 11/3/2015 | 12,484 | 1864.80 | 35 | 29 | 16 | 11 |
| 11/4/2015 | 12,452 | 1864.70 | 15 | 29 | 17 | 11 |
| 11/5/2015 | 12,419 | 1864.70 | -9 | 27 | 16 | 11 |
| 11/6/2015 | 12,380 | 1864.60 | -1 | 27 | 13 | 11 |
| 11/7/2015 | 12,348 | 1864.50 | 31 | 28 | 14 | 11 |
| 11/8/2015 | 12,322 | 1864.50 | 12 | 28 | 18 | 11 |
| 11/9/2015 | 12,329 | 1864.50 | 28 | 29 | 22 | 11 |
| 11/10/2015 | 12,290 | 1864.40 | 13 | 28 | 18 | 11 |
| 11/11/2015 | 12,284 | 1864.40 | 14 | 28 | 17 | 11 |
| 11/12/2015 | 12,252 | 1864.40 | 9 | 28 | 16 | 11 |
| 11/13/2015 | 12,219 | 1864.30 | 9 | 28 | 16 | 11 |
| 11/14/2015 | 12,187 | 1864.30 | 10 | 28 | 15 | 11 |
| 11/15/2015 | 12,207 | 1864.30 | 36 | 31 | 28 | 4 |
| 11/16/2015 | 12,168 | 1864.30 | 4 | 28 | 21 | 0 |
| 11/17/2015 | 12,149 | 1864.20 | 16 | 28 | 20 | 0 |
| 11/18/2015 | 12,124 | 1864.20 | 13 | 28 | 19 | 0 |
| 11/19/2015 | 12,104 | 1864.20 | 16 | 28 | 18 | 0 |
| 11/20/2015 | 12,073 | 1864.10 | 14 | 29 | 18 | 0 |
| 11/21/2015 | 12,047 | 1864.10 | 12 | 29 | 19 | 0 |
| 11/22/2015 | 12,022 | 1864.00 | 12 | 29 | 19 | 0 |
| 11/23/2015 | 11,977 | 1864.00 | 14 | 39 | 20 | 0 |
| 11/24/2015 | 11,933 | 1863.90 | 22 | 48 | 38 | 0 |
| 11/25/2015 | 11,877 | 1863.80 | 3 | 50 | 38 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/26/2015 | 11,820 | 1863.70 | 35 | 51 | 39 | 0 |
| 11/27/2015 | 11,752 | 1863.60 | 21 | 51 | 39 | 0 |
| 11/28/2015 | 11,620 | 1863.40 | 16 | 80 | 51 | 0 |
| 11/29/2015 | 11,484 | 1863.20 | 32 | 88 | 86 | 0 |
| 11/30/2015 | 11,349 | 1862.90 | 28 | 83 | 66 | 0 |
| 12/1/2015 | 11,209 | 1862.70 | 45 | 90 | 77 | 0 |
| 12/2/2015 | 11,102 | 1862.50 | 20 | 78 | 74 | 0 |
| 12/3/2015 | 11,105 | 1862.50 | 48 | 53 | 58 | 0 |
| 12/4/2015 | 11,166 | 1862.60 | 87 | 53 | 44 | 0 |
| 12/5/2015 | 11,159 | 1862.60 | 56 | 66 | 58 | 0 |
| 12/6/2015 | 11,318 | 1862.90 | 151 | 67 | 90 | 0 |
| 12/7/2015 | 11,490 | 1863.20 | 147 | 66 | 77 | 0 |
| 12/8/2015 | 11,502 | 1863.20 | 73 | 66 | 63 | 0 |
| 12/9/2015 | 11,533 | 1863.20 | 82 | 67 | 67 | 0 |
| 12/10/2015 | 13,981 | 1867.00 | 1,304 | 71 | 358 | 0 |
| 12/11/2015 | 15,046 | 1868.50 | 680 | 72 | 234 | 0 |
| 12/12/2015 | 15,360 | 1868.90 | 240 | 71 | 124 | 0 |
| 12/13/2015 | 17,942 | 1872.20 | 1,386 | 75 | 378 | 0 |
| 12/14/2015 | 18,754 | 1873.10 | 535 | 74 | 182 | 0 |
| 12/15/2015 | 19,084 | 1873.50 | 263 | 75 | 117 | 0 |
| 12/16/2015 | 19,216 | 1873.60 | 160 | 84 | 102 | 0 |
| 12/17/2015 | 19,305 | 1873.70 | 128 | 87 | 102 | 0 |
| 12/18/2015 | 21,316 | 1875.80 | 1,035 | 81 | 320 | 0 |
| 12/19/2015 | 23,145 | 1877.60 | 1,128 | 74 | 310 | 0 |
| 12/20/2015 | 25,000 | 1879.30 | 1,003 | 74 | 187 | 0 |
| 12/21/2015 | 32,730 | 1885.50 | 3,981 | 93 | 1,488 | 0 |
| 12/22/2015 | 41,000 | 1891.10 | 4,300 | 149 | 1,964 | 0 |
| 12/23/2015 | 44,862 | 1893.50 | 2,105 | 175 | 620 | 0 |
| 12/24/2015 | 46,289 | 1894.30 | 947 | 176 | 448 | 0 |
| 12/25/2015 | 47,069 | 1894.80 | 605 | 181 | 356 | 0 |
| 12/26/2015 | 47,564 | 1895.10 | 429 | 180 | 291 | 0 |
| 12/27/2015 | 47,908 | 1895.30 | 359 | 180 | 259 | 0 |
| 12/28/2015 | 48,201 | 1895.40 | 314 | 180 | 245 | 0 |
| 12/29/2015 | 48,340 | 1895.50 | 264 | 180 | 228 | 0 |
| 12/30/2015 | 48,461 | 1895.60 | 233 | 180 | 216 | 0 |
| 12/31/2015 | 48,530 | 1895.60 | 199 | 180 | 207 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/1/2016 | 48,513 | 1895.60 | 172 | 179 | 199 | 0 |
| 1/2/2016 | 48,496 | 1895.60 | 163 | 178 | 194 | 0 |
| 1/3/2016 | 48,548 | 1895.60 | 197 | 179 | 203 | 0 |
| 1/4/2016 | 48,999 | 1895.90 | 391 | 181 | 234 | 0 |
| 1/5/2016 | 53,241 | 1898.30 | 2,356 | 189 | 564 | 0 |
| 1/6/2016 | 55,758 | 1899.60 | 1,938 | 629 | 782 | 0 |
| 1/7/2016 | 55,758 | 1899.60 | 1,429 | 1,472 | 1,628 | 0 |
| 1/8/2016 | 55,420 | 1899.50 | 971 | 1,196 | 1,249 | 0 |
| 1/9/2016 | 55,364 | 1899.40 | 939 | 1,024 | 997 | 0 |
| 1/10/2016 | 55,251 | 1899.40 | 878 | 975 | 936 | 0 |
| 1/11/2016 | 55,101 | 1899.30 | 715 | 831 | 796 | 0 |
| 1/12/2016 | 55,131 | 1899.30 | 721 | 744 | 704 | 0 |
| 1/13/2016 | 57,780 | 1900.70 | 3,655 | 2,502 | 2,691 | 0 |
| 1/14/2016 | 57,646 | 1900.70 | 2,921 | 3,162 | 4,060 | 0 |
| 1/15/2016 | 57,107 | 1900.40 | 2,828 | 3,197 | 4,240 | 0 |
| 1/16/2016 | 57,819 | 1900.70 | 3,530 | 3,371 | 4,232 | 0 |
| 1/17/2016 | 63,408 | 1903.60 | 8,393 | 6,405 | 7,432 | 0 |
| 1/18/2016 | 59,302 | 1901.50 | 7,373 | 9,401 | 11,809 | 0 |
| 1/19/2016 | 59,519 | 1901.60 | 5,210 | 5,407 | 6,926 | 0 |
| 1/20/2016 | 57,846 | 1900.80 | 3,370 | 4,378 | 5,649 | 0 |
| 1/21/2016 | 56,954 | 1900.30 | 2,249 | 2,835 | 3,409 | 0 |
| 1/22/2016 | 57,318 | 1900.50 | 2,728 | 2,684 | 2,965 | 0 |
| 1/23/2016 | 58,071 | 1900.90 | 3,479 | 3,297 | 4,075 | 0 |
| 1/24/2016 | 57,318 | 1900.50 | 2,683 | 3,202 | 4,018 | 0 |
| 1/25/2016 | 56,744 | 1900.20 | 2,078 | 2,471 | 2,895 | 0 |
| 1/26/2016 | 56,325 | 1900.00 | 1,644 | 1,951 | 2,155 | 0 |
| 1/27/2016 | 56,003 | 1899.80 | 1,340 | 1,578 | 1,671 | 0 |
| 1/28/2016 | 55,909 | 1899.70 | 1,261 | 1,376 | 1,369 | 0 |
| 1/29/2016 | 58,129 | 1900.90 | 3,157 | 2,231 | 2,627 | 0 |
| 1/30/2016 | 57,337 | 1900.50 | 2,842 | 3,344 | 4,265 | 0 |
| 1/31/2016 | 56,667 | 1900.10 | 1,965 | 2,420 | 2,865 | 0 |
| 2/1/2016 | 56,192 | 1899.90 | 1,496 | 1,815 | 2,013 | 0 |
| 2/2/2016 | 55,890 | 1899.70 | 1,250 | 1,469 | 1,548 | 0 |
| 2/3/2016 | 55,720 | 1899.60 | 1,095 | 1,250 | 1,213 | 0 |
| 2/4/2016 | 55,532 | 1899.50 | 971 | 1,116 | 1,042 | 0 |
| 2/5/2016 | 55,382 | 1899.40 | 838 | 971 | 889 | 0 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/6/2016 | 55,232 | 1899.40 | 735 | 844 | 772 | 0 |
| 2/7/2016 | 55,101 | 1899.30 | 652 | 740 | 681 | 0 |
| 2/8/2016 | 55,045 | 1899.30 | 595 | 667 | 615 | 0 |
| 2/9/2016 | 54,970 | 1899.20 | 540 | 604 | 568 | 0 |
| 2/10/2016 | 54,914 | 1899.20 | 485 | 545 | 525 | 0 |
| 2/11/2016 | 54,859 | 1899.20 | 444 | 496 | 487 | 0 |
| 2/12/2016 | 54,821 | 1899.10 | 415 | 459 | 457 | 0 |
| 2/13/2016 | 54,765 | 1899.10 | 377 | 427 | 431 | 0 |
| 2/14/2016 | 54,728 | 1899.10 | 356 | 398 | 406 | 0 |
| 2/15/2016 | 54,710 | 1899.10 | 354 | 378 | 386 | 0 |
| 2/16/2016 | 54,672 | 1899.10 | 331 | 364 | 372 | 0 |
| 2/17/2016 | 54,765 | 1899.10 | 394 | 359 | 368 | 0 |
| 2/18/2016 | 55,344 | 1899.40 | 839 | 604 | 563 | 0 |
| 2/19/2016 | 56,117 | 1899.80 | 1,519 | 1,193 | 1,080 | 37 |
| 2/20/2016 | 55,645 | 1899.60 | 1,197 | 1,489 | 1,327 | 147 |
| 2/21/2016 | 55,288 | 1899.40 | 891 | 1,147 | 924 | 148 |
| 2/22/2016 | 55,045 | 1899.30 | 754 | 909 | 685 | 148 |
| 2/23/2016 | 54,914 | 1899.20 | 666 | 767 | 567 | 148 |
| 2/24/2016 | 54,822 | 1899.10 | 604 | 685 | 498 | 148 |
| 2/25/2016 | 54,765 | 1899.10 | 560 | 623 | 451 | 147 |
| 2/26/2016 | 54,784 | 1899.10 | 567 | 601 | 425 | 147 |
| 2/27/2016 | 54,728 | 1899.10 | 552 | 595 | 416 | 147 |
| 2/28/2016 | 54,654 | 1899.10 | 490 | 554 | 391 | 148 |
| 2/29/2016 | 54,580 | 1899.00 | 444 | 507 | 357 | 147 |
| 3/1/2016 | 54,524 | 1899.00 | 409 | 463 | 325 | 148 |
| 3/2/2016 | 54,524 | 1899.00 | 410 | 440 | 305 | 147 |
| 3/3/2016 | 54,598 | 1899.00 | 496 | 475 | 340 | 147 |
| 3/4/2016 | 54,561 | 1899.00 | 455 | 483 | 338 | 147 |
| 3/5/2016 | 59,756 | 1901.70 | 3,584 | 1,532 | 1,265 | 146 |
| 3/6/2016 | 59,716 | 1901.70 | 6,754 | 6,776 | 8,457 | 11 |
| 3/7/2016 | 58,323 | 1901.00 | 4,189 | 5,043 | 6,657 | 78 |
| 3/8/2016 | 57,241 | 1900.40 | 2,564 | 3,268 | 3,928 | 147 |
| 3/9/2016 | 57,126 | 1900.40 | 2,447 | 2,646 | 2,858 | 147 |
| 3/10/2016 | 60,073 | 1901.90 | 5,795 | 4,615 | 5,536 | 61 |
| 3/11/2016 | 59,460 | 1901.60 | 5,360 | 5,932 | 7,613 | 1 |
| 3/12/2016 | 60,591 | 1902.20 | 5,849 | 5,627 | 7,282 | 36 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/13/2016 | 62,936 | 1903.40 | 9,568 | 9,076 | 11,528 | 1 |
| 3/14/2016 | 60,155 | 1902.00 | 7,822 | 9,386 | 11,879 | 0 |
| 3/15/2016 | 58,421 | 1901.10 | 3,809 | 4,897 | 6,528 | 26 |
| 3/16/2016 | 57,452 | 1900.50 | 2,665 | 3,291 | 4,069 | 45 |
| 3/17/2016 | 56,877 | 1900.20 | 2,120 | 2,532 | 2,897 | 45 |
| 3/18/2016 | 56,458 | 1900.00 | 1,747 | 2,053 | 2,247 | 45 |
| 3/19/2016 | 56,173 | 1899.90 | 1,485 | 1,717 | 1,812 | 44 |
| 3/20/2016 | 56,514 | 1900.10 | 1,684 | 1,625 | 1,641 | 45 |
| 3/21/2016 | 57,761 | 1900.70 | 2,726 | 2,249 | 2,484 | 45 |
| 3/22/2016 | 57,222 | 1900.40 | 2,600 | 2,964 | 3,669 | 45 |
| 3/23/2016 | 56,667 | 1900.10 | 1,912 | 2,297 | 2,635 | 44 |
| 3/24/2016 | 56,325 | 1900.00 | 1,613 | 1,871 | 2,034 | 45 |
| 3/25/2016 | 56,060 | 1899.80 | 1,408 | 1,615 | 1,696 | 45 |
| 3/26/2016 | 55,909 | 1899.70 | 1,255 | 1,421 | 1,411 | 45 |
| 3/27/2016 | 55,720 | 1899.60 | 1,139 | 1,282 | 1,207 | 45 |
| 3/28/2016 | 55,551 | 1899.50 | 1,010 | 1,146 | 1,048 | 45 |
| 3/29/2016 | 55,438 | 1899.50 | 916 | 1,032 | 927 | 45 |
| 3/30/2016 | 55,344 | 1899.40 | 827 | 927 | 828 | 45 |
| 3/31/2016 | 55,213 | 1899.40 | 750 | 946 | 817 | 45 |
| 4/1/2016 | 55,495 | 1899.50 | 699 | 632 | 648 | 45 |
| 4/2/2016 | 56,060 | 1899.80 | 539 | 286 | 386 | 45 |
| 4/3/2016 | 56,553 | 1900.10 | 502 | 276 | 362 | 45 |
| 4/4/2016 | 56,992 | 1900.30 | 475 | 267 | 343 | 45 |
| 4/5/2016 | 57,414 | 1900.50 | 461 | 261 | 326 | 45 |
| 4/6/2016 | 57,742 | 1900.70 | 435 | 262 | 318 | 45 |
| 4/7/2016 | 58,071 | 1900.90 | 420 | 263 | 312 | 45 |
| 4/8/2016 | 58,362 | 1901.00 | 405 | 264 | 307 | 45 |
| 4/9/2016 | 58,655 | 1901.20 | 404 | 264 | 304 | 45 |
| 4/10/2016 | 58,929 | 1901.30 | 392 | 264 | 300 | 45 |
| 4/11/2016 | 59,145 | 1901.40 | 370 | 265 | 296 | 45 |
| 4/12/2016 | 59,282 | 1901.50 | 341 | 266 | 292 | 45 |
| 4/13/2016 | 59,499 | 1901.60 | 341 | 266 | 290 | 45 |
| 4/14/2016 | 59,637 | 1901.70 | 361 | 278 | 306 | 45 |
| 4/15/2016 | 59,690 | 1901.70 | 313 | 305 | 287 | 75 |
| 4/16/2016 | 59,697 | 1901.70 | 293 | 303 | 260 | 90 |
| 4/17/2016 | 59,677 | 1901.70 | 270 | 297 | 247 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/18/2016 | 59,617 | 1901.70 | 258 | 299 | 243 | 90 |
| 4/19/2016 | 59,558 | 1901.60 | 250 | 305 | 248 | 90 |
| 4/20/2016 | 59,440 | 1901.60 | 236 | 305 | 246 | 89 |
| 4/21/2016 | 59,381 | 1901.60 | 235 | 305 | 245 | 89 |
| 4/22/2016 | 59,657 | 1901.70 | 449 | 308 | 318 | 90 |
| 4/23/2016 | 59,795 | 1901.80 | 370 | 306 | 306 | 90 |
| 4/24/2016 | 59,756 | 1901.70 | 282 | 306 | 266 | 89 |
| 4/25/2016 | 59,697 | 1901.70 | 243 | 304 | 254 | 90 |
| 4/26/2016 | 59,598 | 1901.70 | 241 | 304 | 248 | 90 |
| 4/27/2016 | 59,519 | 1901.60 | 258 | 305 | 250 | 90 |
| 4/28/2016 | 59,460 | 1901.60 | 253 | 305 | 247 | 90 |
| 4/29/2016 | 59,302 | 1901.50 | 198 | 305 | 241 | 89 |
| 4/30/2016 | 59,105 | 1901.40 | 192 | 305 | 236 | 89 |
| 5/1/2016 | 58,935 | 1901.30 | 201 | 305 | 233 | 89 |
| 5/2/2016 | 58,733 | 1901.20 | 180 | 300 | 227 | 89 |
| 5/3/2016 | 58,499 | 1901.10 | 185 | 337 | 252 | 89 |
| 5/4/2016 | 57,916 | 1900.80 | 249 | 579 | 459 | 89 |
| 5/5/2016 | 57,606 | 1900.60 | 325 | 530 | 474 | 89 |
| 5/6/2016 | 57,299 | 1900.50 | 263 | 436 | 385 | 89 |
| 5/7/2016 | 57,011 | 1900.30 | 217 | 391 | 328 | 89 |
| 5/8/2016 | 56,705 | 1900.20 | 205 | 378 | 306 | 89 |
| 5/9/2016 | 56,420 | 1900.00 | 188 | 361 | 288 | 89 |
| 5/10/2016 | 56,117 | 1899.80 | 171 | 352 | 268 | 89 |
| 5/11/2016 | 55,780 | 1899.70 | 155 | 345 | 276 | 89 |
| 5/12/2016 | 55,462 | 1899.50 | 141 | 330 | 262 | 89 |
| 5/13/2016 | 55,127 | 1899.30 | 131 | 315 | 245 | 89 |
| 5/14/2016 | 54,830 | 1899.10 | 121 | 303 | 235 | 89 |
| 5/15/2016 | 54,553 | 1899.00 | 119 | 287 | 193 | 117 |
| 5/16/2016 | 54,278 | 1898.80 | 117 | 274 | 171 | 130 |
| 5/17/2016 | 54,002 | 1898.70 | 96 | 253 | 158 | 129 |
| 5/18/2016 | 53,747 | 1898.60 | 90 | 245 | 147 | 129 |
| 5/19/2016 | 53,473 | 1898.40 | 72 | 245 | 146 | 130 |
| 5/20/2016 | 53,145 | 1898.20 | 87 | 245 | 146 | 129 |
| 5/21/2016 | 52,928 | 1898.10 | 109 | 249 | 154 | 127 |
| 5/22/2016 | 52,657 | 1898.00 | 96 | 245 | 148 | 129 |
| 5/23/2016 | 52,405 | 1897.80 | 88 | 238 | 142 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/24/2016 | 52,280 | 1897.70 | 127 | 231 | 137 | 130 |
| 5/25/2016 | 52,066 | 1897.60 | 118 | 225 | 130 | 130 |
| 5/26/2016 | 51,869 | 1897.50 | 93 | 223 | 125 | 130 |
| 5/27/2016 | 51,619 | 1897.40 | 80 | 222 | 120 | 130 |
| 5/28/2016 | 51,369 | 1897.20 | 72 | 221 | 118 | 130 |
| 5/29/2016 | 51,103 | 1897.10 | 64 | 222 | 116 | 130 |
| 5/30/2016 | 50,819 | 1896.90 | 66 | 224 | 119 | 130 |
| 5/31/2016 | 50,555 | 1896.80 | 57 | 221 | 116 | 130 |
| 6/1/2016 | 50,256 | 1896.60 | 50 | 218 | 112 | 129 |
| 6/2/2016 | 49,957 | 1896.40 | 45 | 217 | 110 | 129 |
| 6/3/2016 | 49,677 | 1896.30 | 42 | 217 | 109 | 130 |
| 6/4/2016 | 49,381 | 1896.10 | 45 | 213 | 106 | 130 |
| 6/5/2016 | 49,085 | 1895.90 | 41 | 210 | 101 | 129 |
| 6/6/2016 | 48,809 | 1895.80 | 36 | 210 | 99 | 129 |
| 6/7/2016 | 48,498 | 1895.60 | 30 | 209 | 98 | 129 |
| 6/8/2016 | 48,189 | 1895.40 | 24 | 200 | 90 | 130 |
| 6/9/2016 | 47,914 | 1895.30 | 20 | 190 | 81 | 130 |
| 6/10/2016 | 47,590 | 1895.10 | 11 | 184 | 74 | 129 |
| 6/11/2016 | 47,318 | 1894.90 | 17 | 179 | 69 | 130 |
| 6/12/2016 | 47,047 | 1894.80 | 10 | 173 | 63 | 130 |
| 6/13/2016 | 46,743 | 1894.60 | 10 | 171 | 60 | 130 |
| 6/14/2016 | 46,474 | 1894.40 | -3 | 172 | 60 | 130 |
| 6/15/2016 | 46,189 | 1894.30 | 15 | 179 | 60 | 130 |
| 6/16/2016 | 45,938 | 1894.10 | 29 | 179 | 62 | 130 |
| 6/17/2016 | 45,772 | 1894.00 | 67 | 180 | 63 | 130 |
| 6/18/2016 | 45,557 | 1893.90 | 59 | 180 | 65 | 130 |
| 6/19/2016 | 45,309 | 1893.70 | 35 | 176 | 61 | 130 |
| 6/20/2016 | 45,062 | 1893.60 | 24 | 174 | 56 | 130 |
| 6/21/2016 | 44,781 | 1893.40 | 18 | 179 | 55 | 130 |
| 6/22/2016 | 44,469 | 1893.20 | 14 | 208 | 81 | 131 |
| 6/23/2016 | 44,142 | 1893.00 | 8 | 192 | 69 | 132 |
| 6/24/2016 | 43,849 | 1892.90 | 13 | 189 | 64 | 133 |
| 6/25/2016 | 43,541 | 1892.70 | 11 | 182 | 63 | 129 |
| 6/26/2016 | 43,251 | 1892.50 | 5 | 166 | 45 | 131 |
| 6/27/2016 | 42,993 | 1892.30 | 2 | 160 | 39 | 131 |
| 6/28/2016 | 42,736 | 1892.20 | -2 | 155 | 34 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/29/2016 | 42,464 | 1892.00 | 0 | 154 | 31 | 131 |
| 6/30/2016 | 42,226 | 1891.90 | 3 | 146 | 31 | 125 |
| 7/1/2016 | 41,972 | 1891.70 | -2 | 142 | 33 | 115 |
| 7/2/2016 | 41,736 | 1891.60 | -4 | 141 | 33 | 115 |
| 7/3/2016 | 41,483 | 1891.40 | -8 | 141 | 33 | 115 |
| 7/4/2016 | 41,215 | 1891.20 | -6 | 141 | 32 | 115 |
| 7/5/2016 | 40,949 | 1891.10 | -11 | 141 | 32 | 115 |
| 7/6/2016 | 40,699 | 1890.90 | -8 | 141 | 31 | 115 |
| 7/7/2016 | 40,435 | 1890.70 | -15 | 140 | 31 | 115 |
| 7/8/2016 | 40,172 | 1890.60 | -12 | 139 | 31 | 115 |
| 7/9/2016 | 39,894 | 1890.40 | -15 | 137 | 28 | 115 |
| 7/10/2016 | 39,648 | 1890.20 | -14 | 136 | 26 | 115 |
| 7/11/2016 | 39,403 | 1890.10 | -9 | 135 | 26 | 115 |
| 7/12/2016 | 39,158 | 1889.90 | 52 | 136 | 27 | 115 |
| 7/13/2016 | 38,914 | 1889.80 | -4 | 135 | 25 | 115 |
| 7/14/2016 | 38,655 | 1889.60 | -4 | 137 | 25 | 115 |
| 7/15/2016 | 38,412 | 1889.40 | -10 | 128 | 28 | 110 |
| 7/16/2016 | 38,230 | 1889.30 | 0 | 104 | 41 | 65 |
| 7/17/2016 | 38,049 | 1889.20 | -4 | 100 | 34 | 65 |
| 7/18/2016 | 37,853 | 1889.10 | -2 | 99 | 31 | 65 |
| 7/19/2016 | 37,688 | 1889.00 | -3 | 99 | 30 | 65 |
| 7/20/2016 | 37,539 | 1888.90 | 5 | 96 | 26 | 65 |
| 7/21/2016 | 37,315 | 1888.70 | -7 | 107 | 26 | 70 |
| 7/22/2016 | 37,122 | 1888.60 | -4 | 109 | 33 | 79 |
| 7/23/2016 | 36,929 | 1888.40 | -2 | 104 | 24 | 78 |
| 7/24/2016 | 36,736 | 1888.30 | -1 | 104 | 19 | 78 |
| 7/25/2016 | 36,559 | 1888.20 | -2 | 103 | 22 | 78 |
| 7/26/2016 | 36,353 | 1888.10 | -1 | 103 | 19 | 78 |
| 7/27/2016 | 36,163 | 1887.90 | -5 | 102 | 19 | 77 |
| 7/28/2016 | 35,973 | 1887.80 | 1 | 104 | 18 | 79 |
| 7/29/2016 | 35,770 | 1887.70 | -6 | 105 | 23 | 79 |
| 7/30/2016 | 35,552 | 1887.50 | -7 | 104 | 20 | 79 |
| 7/31/2016 | 35,349 | 1887.40 | -9 | 103 | 18 | 79 |
| 8/1/2016 | 35,148 | 1887.20 | -9 | 102 | 19 | 78 |
| 8/2/2016 | 34,947 | 1887.10 | -10 | 103 | 20 | 78 |
| 8/3/2016 | 34,747 | 1886.90 | -9 | 102 | 20 | 78 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/4/2016 | 34,547 | 1886.80 | -12 | 102 | 23 | 74 |
| 8/5/2016 | 34,335 | 1886.70 | -12 | 100 | 24 | 72 |
| 8/6/2016 | 34,151 | 1886.50 | -9 | 101 | 24 | 72 |
| 8/7/2016 | 33,925 | 1886.40 | -13 | 100 | 24 | 72 |
| 8/8/2016 | 33,743 | 1886.20 | -9 | 99 | 22 | 72 |
| 8/9/2016 | 33,561 | 1886.10 | -5 | 97 | 25 | 67 |
| 8/10/2016 | 33,366 | 1886.00 | -6 | 99 | 23 | 68 |
| 8/11/2016 | 33,171 | 1885.80 | -5 | 100 | 19 | 74 |
| 8/12/2016 | 32,992 | 1885.70 | -5 | 100 | 18 | 74 |
| 8/13/2016 | 32,799 | 1885.50 | -7 | 102 | 20 | 74 |
| 8/14/2016 | 32,606 | 1885.40 | -7 | 99 | 21 | 72 |
| 8/15/2016 | 32,435 | 1885.30 | -3 | 97 | 23 | 67 |
| 8/16/2016 | 32,246 | 1885.10 | -2 | 97 | 22 | 67 |
| 8/17/2016 | 32,074 | 1885.00 | -2 | 96 | 21 | 67 |
| 8/18/2016 | 31,898 | 1884.90 | 0 | 98 | 19 | 67 |
| 8/19/2016 | 31,709 | 1884.70 | -4 | 99 | 24 | 67 |
| 8/20/2016 | 31,522 | 1884.60 | -1 | 99 | 23 | 67 |
| 8/21/2016 | 31,347 | 1884.50 | -5 | 99 | 23 | 68 |
| 8/22/2016 | 31,159 | 1884.30 | -3 | 99 | 22 | 68 |
| 8/23/2016 | 30,944 | 1884.20 | -11 | 106 | 26 | 70 |
| 8/24/2016 | 30,717 | 1884.00 | -12 | 109 | 29 | 78 |
| 8/25/2016 | 30,517 | 1883.80 | -6 | 106 | 27 | 78 |
| 8/26/2016 | 30,318 | 1883.70 | -6 | 106 | 24 | 77 |
| 8/27/2016 | 30,107 | 1883.50 | -6 | 106 | 24 | 77 |
| 8/28/2016 | 29,910 | 1883.40 | -3 | 101 | 24 | 74 |
| 8/29/2016 | 29,739 | 1883.20 | -3 | 98 | 22 | 67 |
| 8/30/2016 | 29,556 | 1883.10 | 1 | 97 | 22 | 68 |
| 8/31/2016 | 29,387 | 1883.00 | 0 | 94 | 21 | 66 |
| 9/1/2016 | 29,219 | 1882.80 | 0 | 93 | 20 | 63 |
| 9/2/2016 | 29,039 | 1882.70 | -1 | 93 | 20 | 63 |
| 9/3/2016 | 28,884 | 1882.60 | -1 | 93 | 19 | 63 |
| 9/4/2016 | 28,706 | 1882.40 | -1 | 93 | 20 | 63 |
| 9/5/2016 | 28,540 | 1882.30 | 1 | 92 | 20 | 63 |
| 9/6/2016 | 28,376 | 1882.20 | 4 | 92 | 20 | 63 |
| 9/7/2016 | 28,212 | 1882.00 | 5 | 93 | 19 | 63 |
| 9/8/2016 | 28,061 | 1881.90 | 3 | 90 | 21 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/9/2016 | 27,910 | 1881.80 | 2 | 88 | 21 | 58 |
| 9/10/2016 | 27,748 | 1881.70 | 4 | 88 | 21 | 58 |
| 9/11/2016 | 27,587 | 1881.50 | -1 | 88 | 21 | 58 |
| 9/12/2016 | 27,415 | 1881.40 | -1 | 88 | 21 | 58 |
| 9/13/2016 | 27,267 | 1881.30 | 0 | 87 | 22 | 58 |
| 9/14/2016 | 27,108 | 1881.10 | 1 | 87 | 22 | 58 |
| 9/15/2016 | 26,962 | 1881.00 | 2 | 86 | 22 | 58 |
| 9/16/2016 | 26,805 | 1880.90 | 1 | 84 | 22 | 56 |
| 9/17/2016 | 26,648 | 1880.70 | 1 | 82 | 25 | 53 |
| 9/18/2016 | 26,515 | 1880.60 | 2 | 81 | 22 | 53 |
| 9/19/2016 | 26,360 | 1880.50 | 3 | 85 | 22 | 54 |
| 9/20/2016 | 26,194 | 1880.30 | -1 | 89 | 22 | 60 |
| 9/21/2016 | 26,005 | 1880.20 | -7 | 94 | 23 | 62 |
| 9/22/2016 | 25,818 | 1880.00 | -10 | 98 | 24 | 71 |
| 9/23/2016 | 25,643 | 1879.90 | 0 | 98 | 24 | 71 |
| 9/24/2016 | 25,468 | 1879.70 | 5 | 97 | 23 | 71 |
| 9/25/2016 | 25,307 | 1879.60 | 1 | 97 | 20 | 71 |
| 9/26/2016 | 25,135 | 1879.40 | 5 | 94 | 20 | 69 |
| 9/27/2016 | 24,976 | 1879.30 | 4 | 92 | 20 | 64 |
| 9/28/2016 | 24,817 | 1879.10 | 4 | 91 | 21 | 63 |
| 9/29/2016 | 24,659 | 1879.00 | -1 | 89 | 19 | 63 |
| 9/30/2016 | 24,502 | 1878.90 | -1 | 85 | 21 | 60 |
| 10/1/2016 | 24,350 | 1878.70 | 1 | 75 | 24 | 54 |
| 10/2/2016 | 24,220 | 1878.60 | 14 | 74 | 25 | 54 |
| 10/3/2016 | 24,113 | 1878.50 | 25 | 74 | 25 | 54 |
| 10/4/2016 | 23,997 | 1878.40 | 10 | 68 | 24 | 51 |
| 10/5/2016 | 23,881 | 1878.30 | 7 | 64 | 20 | 44 |
| 10/6/2016 | 23,775 | 1878.20 | 11 | 64 | 20 | 44 |
| 10/7/2016 | 23,671 | 1878.10 | 12 | 63 | 27 | 37 |
| 10/8/2016 | 23,566 | 1878.00 | 9 | 59 | 30 | 33 |
| 10/9/2016 | 23,483 | 1877.90 | 13 | 57 | 24 | 33 |
| 10/10/2016 | 23,379 | 1877.80 | 7 | 56 | 21 | 33 |
| 10/11/2016 | 23,286 | 1877.70 | 6 | 55 | 20 | 33 |
| 10/12/2016 | 23,187 | 1877.70 | 9 | 55 | 19 | 33 |
| 10/13/2016 | 23,124 | 1877.60 | 12 | 54 | 20 | 33 |
| 10/14/2016 | 23,198 | 1877.70 | 94 | 55 | 42 | 33 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/15/2016 | 23,261 | 1877.70 | 84 | 55 | 33 | 33 |
| 10/16/2016 | 23,377 | 1877.80 | 114 | 55 | 39 | 33 |
| 10/17/2016 | 23,579 | 1878.00 | 160 | 55 | 56 | 33 |
| 10/18/2016 | 23,632 | 1878.10 | 79 | 54 | 36 | 33 |
| 10/19/2016 | 23,621 | 1878.10 | 48 | 54 | 30 | 33 |
| 10/20/2016 | 23,579 | 1878.00 | 32 | 54 | 29 | 33 |
| 10/21/2016 | 23,536 | 1878.00 | 30 | 53 | 25 | 33 |
| 10/22/2016 | 23,493 | 1877.90 | 25 | 52 | 23 | 33 |
| 10/23/2016 | 23,430 | 1877.90 | 21 | 51 | 20 | 33 |
| 10/24/2016 | 23,462 | 1877.90 | 70 | 60 | 31 | 34 |
| 10/25/2016 | 23,728 | 1878.20 | 221 | 88 | 89 | 34 |
| 10/26/2016 | 23,856 | 1878.30 | 189 | 123 | 103 | 33 |
| 10/27/2016 | 24,213 | 1878.60 | 317 | 140 | 159 | 33 |
| 10/28/2016 | 24,532 | 1878.90 | 326 | 146 | 164 | 33 |
| 10/29/2016 | 25,022 | 1879.30 | 398 | 149 | 179 | 33 |
| 10/30/2016 | 26,472 | 1880.60 | 870 | 152 | 303 | 33 |
| 10/31/2016 | 27,893 | 1881.80 | 956 | 170 | 307 | 41 |
| 11/1/2016 | 29,719 | 1883.20 | 1,158 | 182 | 372 | 44 |
| 11/2/2016 | 30,238 | 1883.60 | 479 | 182 | 214 | 44 |
| 11/3/2016 | 30,421 | 1883.80 | 287 | 182 | 178 | 44 |
| 11/4/2016 | 30,460 | 1883.80 | 203 | 183 | 166 | 44 |
| 11/5/2016 | 30,434 | 1883.80 | 169 | 183 | 159 | 44 |
| 11/6/2016 | 30,382 | 1883.70 | 161 | 184 | 162 | 44 |
| 11/7/2016 | 30,277 | 1883.70 | 135 | 190 | 157 | 44 |
| 11/8/2016 | 30,134 | 1883.50 | 124 | 202 | 168 | 44 |
| 11/9/2016 | 29,952 | 1883.40 | 114 | 203 | 167 | 44 |
| 11/10/2016 | 29,758 | 1883.30 | 92 | 202 | 164 | 44 |
| 11/11/2016 | 29,578 | 1883.10 | 80 | 200 | 163 | 44 |
| 11/12/2016 | 29,398 | 1883.00 | 85 | 196 | 160 | 44 |
| 11/13/2016 | 29,220 | 1882.80 | 71 | 187 | 153 | 44 |
| 11/14/2016 | 29,042 | 1882.70 | 70 | 178 | 146 | 44 |
| 11/15/2016 | 28,865 | 1882.60 | 70 | 172 | 141 | 44 |
| 11/16/2016 | 28,701 | 1882.40 | 61 | 167 | 136 | 44 |
| 11/17/2016 | 28,538 | 1882.30 | 53 | 162 | 131 | 45 |
| 11/18/2016 | 28,376 | 1882.20 | 56 | 160 | 126 | 44 |
| 11/19/2016 | 29,849 | 1883.30 | 870 | 163 | 315 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/20/2016 | 31,307 | 1884.40 | 935 | 174 | 314 | 44 |
| 11/21/2016 | 31,979 | 1884.90 | 552 | 193 | 245 | 45 |
| 11/22/2016 | 32,278 | 1885.20 | 356 | 203 | 209 | 45 |
| 11/23/2016 | 32,689 | 1885.50 | 410 | 205 | 270 | 45 |
| 11/24/2016 | 32,936 | 1885.60 | 318 | 204 | 230 | 45 |
| 11/25/2016 | 33,061 | 1885.70 | 266 | 203 | 207 | 45 |
| 11/26/2016 | 33,925 | 1886.40 | 613 | 208 | 327 | 45 |
| 11/27/2016 | 35,364 | 1887.40 | 971 | 209 | 546 | 45 |
| 11/28/2016 | 36,515 | 1888.20 | 799 | 209 | 370 | 45 |
| 11/29/2016 | 37,167 | 1888.60 | 552 | 209 | 295 | 45 |
| 11/30/2016 | 37,554 | 1888.90 | 408 | 209 | 252 | 45 |
| 12/1/2016 | 37,793 | 1889.00 | 327 | 209 | 226 | 45 |
| 12/2/2016 | 37,928 | 1889.10 | 270 | 209 | 208 | 45 |
| 12/3/2016 | 38,004 | 1889.20 | 231 | 209 | 196 | 45 |
| 12/4/2016 | 38,049 | 1889.20 | 222 | 209 | 190 | 45 |
| 12/5/2016 | 38,079 | 1889.20 | 210 | 210 | 187 | 45 |
| 12/6/2016 | 38,064 | 1889.20 | 189 | 209 | 184 | 45 |
| 12/7/2016 | 38,155 | 1889.30 | 241 | 209 | 184 | 45 |
| 12/8/2016 | 38,823 | 1889.70 | 531 | 211 | 247 | 45 |
| 12/9/2016 | 40,334 | 1890.70 | 1,004 | 212 | 298 | 45 |
| 12/10/2016 | 55,176 | 1899.30 | 7,952 | 271 | 2,274 | 44 |
| 12/11/2016 | 56,515 | 1900.10 | 2,816 | 1,705 | 2,107 | 44 |
| 12/12/2016 | 55,928 | 1899.70 | 1,573 | 1,559 | 1,980 | 67 |
| 12/13/2016 | 55,495 | 1899.50 | 1,118 | 1,113 | 1,231 | 118 |
| 12/14/2016 | 55,965 | 1899.80 | 1,612 | 1,171 | 1,127 | 189 |
| 12/15/2016 | 62,650 | 1903.20 | 11,412 | 8,592 | 9,784 | 55 |
| 12/16/2016 | 58,460 | 1901.10 | 5,454 | 7,537 | 9,481 | 4 |
| 12/17/2016 | 56,801 | 1900.20 | 2,343 | 2,919 | 3,760 | 133 |
| 12/18/2016 | 56,003 | 1899.80 | 1,606 | 1,703 | 1,992 | 200 |
| 12/19/2016 | 55,532 | 1899.50 | 1,239 | 1,230 | 1,291 | 200 |
| 12/20/2016 | 55,213 | 1899.40 | 1,024 | 1,000 | 934 | 200 |
| 12/21/2016 | 54,970 | 1899.20 | 872 | 846 | 748 | 199 |
| 12/22/2016 | 54,803 | 1899.10 | 741 | 718 | 613 | 200 |
| 12/23/2016 | 55,645 | 1899.60 | 1,453 | 901 | 845 | 200 |
| 12/24/2016 | 55,326 | 1899.40 | 1,193 | 1,113 | 1,107 | 200 |
| 12/25/2016 | 55,026 | 1899.30 | 890 | 895 | 825 | 200 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/26/2016 | 54,803 | 1899.10 | 720 | 725 | 645 | 200 |
| 12/27/2016 | 54,635 | 1899.00 | 615 | 611 | 516 | 200 |
| 12/28/2016 | 54,524 | 1899.00 | 548 | 540 | 433 | 200 |
| 12/29/2016 | 54,431 | 1898.90 | 491 | 485 | 369 | 200 |
| 12/30/2016 | 54,339 | 1898.90 | 448 | 445 | 320 | 200 |
| 12/31/2016 | 54,246 | 1898.80 | 419 | 418 | 280 | 200 |
| 1/1/2017 | 54,172 | 1898.80 | 391 | 397 | 248 | 200 |
| 1/2/2017 | 54,154 | 1898.80 | 397 | 384 | 233 | 201 |
| 1/3/2017 | 55,045 | 1899.30 | 845 | 418 | 306 | 202 |
| 1/4/2017 | 56,915 | 1900.30 | 3,401 | 2,136 | 2,013 | 206 |
| 1/5/2017 | 56,060 | 1899.80 | 1,386 | 1,827 | 1,746 | 207 |
| 1/6/2017 | 55,551 | 1899.50 | 873 | 1,250 | 1,165 | 201 |
| 1/7/2017 | 56,820 | 1900.20 | 1,877 | 1,413 | 1,300 | 201 |
| 1/8/2017 | 66,599 | 1905.20 | 15,618 | 12,062 | 10,499 | 89 |
| 1/9/2017 | 61,656 | 1902.70 | 11,521 | 14,155 | 15,188 | 1 |
| 1/10/2017 | 68,262 | 1906.00 | 19,834 | 18,208 | 18,006 | 1 |
| 1/11/2017 | 61,374 | 1902.60 | 13,329 | 16,735 | 18,766 | 1 |
| 1/12/2017 | 58,831 | 1901.30 | 4,676 | 6,211 | 8,774 | 1 |
| 1/13/2017 | 57,549 | 1900.60 | 2,701 | 3,577 | 5,308 | 25 |
| 1/14/2017 | 56,858 | 1900.20 | 1,950 | 2,509 | 3,700 | 48 |
| 1/15/2017 | 56,344 | 1900.00 | 1,526 | 1,930 | 2,676 | 43 |
| 1/16/2017 | 56,022 | 1899.80 | 1,224 | 1,541 | 2,112 | 43 |
| 1/17/2017 | 55,758 | 1899.60 | 1,063 | 1,315 | 1,745 | 43 |
| 1/18/2017 | 58,577 | 1901.10 | 2,712 | 1,683 | 2,247 | 43 |
| 1/19/2017 | 58,752 | 1901.20 | 4,618 | 4,655 | 6,393 | 43 |
| 1/20/2017 | 59,460 | 1901.60 | 5,092 | 5,069 | 6,706 | 30 |
| 1/21/2017 | 59,007 | 1901.40 | 4,393 | 4,895 | 6,949 | 20 |
| 1/22/2017 | 58,674 | 1901.20 | 4,359 | 4,787 | 6,679 | 43 |
| 1/23/2017 | 57,544 | 1900.60 | 2,657 | 3,457 | 4,890 | 45 |
| 1/24/2017 | 56,819 | 1900.20 | 1,883 | 2,447 | 3,467 | 79 |
| 1/25/2017 | 56,365 | 1900.00 | 1,457 | 1,858 | 2,497 | 105 |
| 1/26/2017 | 56,085 | 1899.80 | 1,195 | 1,497 | 1,992 | 106 |
| 1/27/2017 | 55,871 | 1899.70 | 1,125 | 1,304 | 1,710 | 105 |
| 1/28/2017 | 55,758 | 1899.60 | 1,028 | 1,200 | 1,526 | 106 |
| 1/29/2017 | 55,701 | 1899.60 | 1,023 | 1,155 | 1,413 | 105 |
| 1/30/2017 | 55,701 | 1899.60 | 1,023 | 1,135 | 1,370 | 105 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/31/2017 | 55,739 | 1899.60 | 1,052 | 1,146 | 1,369 | 105 |
| 2/1/2017 | 55,683 | 1899.60 | 1,007 | 1,136 | 1,358 | 105 |
| 2/2/2017 | 56,022 | 1899.80 | 1,223 | 1,182 | 1,406 | 105 |
| 2/3/2017 | 58,577 | 1901.10 | 3,555 | 2,543 | 3,066 | 105 |
| 2/4/2017 | 58,988 | 1901.40 | 4,323 | 4,366 | 5,446 | 105 |
| 2/5/2017 | 58,596 | 1901.10 | 3,538 | 4,022 | 5,193 | 105 |
| 2/6/2017 | 59,697 | 1901.70 | 5,133 | 4,883 | 6,250 | 70 |
| 2/7/2017 | 64,336 | 1904.10 | 19,373 | 18,165 | 18,233 | 3 |
| 2/8/2017 | 62,854 | 1903.30 | 9,996 | 11,291 | 12,874 | 1 |
| 2/9/2017 | 65,693 | 1904.70 | 14,495 | 13,993 | 14,630 | 1 |
| 2/10/2017 | 61,253 | 1902.50 | 9,840 | 12,304 | 14,165 | 1 |
| 2/11/2017 | 59,105 | 1901.40 | 4,650 | 6,034 | 7,986 | 13 |
| 2/12/2017 | 57,974 | 1900.80 | 2,977 | 3,833 | 5,097 | 44 |
| 2/13/2017 | 57,299 | 1900.50 | 2,263 | 2,876 | 3,663 | 65 |
| 2/14/2017 | 56,839 | 1900.20 | 1,845 | 2,325 | 2,655 | 144 |
| 2/15/2017 | 56,534 | 1900.10 | 1,598 | 1,959 | 2,127 | 172 |
| 2/16/2017 | 58,090 | 1900.90 | 3,439 | 2,944 | 3,504 | 172 |
| 2/17/2017 | 58,479 | 1901.10 | 3,205 | 3,256 | 3,922 | 172 |
| 2/18/2017 | 58,323 | 1901.00 | 3,773 | 4,060 | 4,705 | 172 |
| 2/19/2017 | 58,499 | 1901.10 | 3,411 | 3,578 | 4,326 | 172 |
| 2/20/2017 | 64,585 | 1904.20 | 8,713 | 6,552 | 7,226 | 35 |
| 2/21/2017 | 62,223 | 1903.00 | 13,153 | 14,616 | 15,442 | 1 |
| 2/22/2017 | 59,558 | 1901.60 | 5,445 | 7,067 | 8,491 | 0 |
| 2/23/2017 | 58,265 | 1901.00 | 3,356 | 4,237 | 5,149 | 43 |
| 2/24/2017 | 57,510 | 1900.60 | 2,516 | 3,097 | 3,726 | 44 |
| 2/25/2017 | 56,992 | 1900.30 | 2,062 | 2,492 | 2,838 | 66 |
| 2/26/2017 | 56,648 | 1900.10 | 1,764 | 2,093 | 2,265 | 85 |
| 2/27/2017 | 56,344 | 1900.00 | 1,508 | 1,804 | 1,940 | 85 |
| 2/28/2017 | 56,079 | 1899.80 | 1,287 | 1,541 | 1,699 | 85 |
| 3/1/2017 | 55,852 | 1899.70 | 1,139 | 1,323 | 1,472 | 85 |
| 3/2/2017 | 55,701 | 1899.60 | 1,056 | 1,175 | 1,303 | 84 |
| 3/3/2017 | 55,589 | 1899.60 | 994 | 1,072 | 1,185 | 83 |
| 3/4/2017 | 55,796 | 1899.70 | 1,131 | 1,048 | 1,169 | 85 |
| 3/5/2017 | 55,965 | 1899.80 | 1,348 | 1,330 | 1,530 | 86 |
| 3/6/2017 | 55,947 | 1899.70 | 1,192 | 1,276 | 1,474 | 86 |
| 3/7/2017 | 55,890 | 1899.70 | 1,200 | 1,281 | 1,510 | 72 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/8/2017 | 55,984 | 1899.80 | 1,247 | 1,278 | 1,537 | 44 |
| 3/9/2017 | 56,060 | 1899.80 | 1,351 | 1,392 | 1,649 | 45 |
| 3/10/2017 | 56,003 | 1899.80 | 1,294 | 1,410 | 1,645 | 45 |
| 3/11/2017 | 55,890 | 1899.70 | 1,204 | 1,337 | 1,542 | 45 |
| 3/12/2017 | 55,796 | 1899.70 | 1,132 | 1,253 | 1,426 | 44 |
| 3/13/2017 | 55,664 | 1899.60 | 1,061 | 1,154 | 1,302 | 44 |
| 3/14/2017 | 55,570 | 1899.50 | 1,014 | 1,079 | 1,205 | 44 |
| 3/15/2017 | 55,589 | 1899.60 | 987 | 980 | 1,109 | 44 |
| 3/16/2017 | 55,645 | 1899.60 | 964 | 898 | 1,005 | 43 |
| 3/17/2017 | 55,551 | 1899.50 | 891 | 933 | 1,037 | 44 |
| 3/18/2017 | 55,513 | 1899.50 | 893 | 884 | 981 | 44 |
| 3/19/2017 | 55,476 | 1899.50 | 882 | 871 | 965 | 44 |
| 3/20/2017 | 55,570 | 1899.50 | 909 | 855 | 933 | 44 |
| 3/21/2017 | 56,363 | 1900.00 | 1,619 | 1,281 | 1,267 | 45 |
| 3/22/2017 | 57,896 | 1900.80 | 1,876 | 1,140 | 1,463 | 44 |
| 3/23/2017 | 59,954 | 1901.80 | 1,325 | 228 | 445 | 44 |
| 3/24/2017 | 64,544 | 1904.20 | 2,577 | 237 | 667 | 44 |
| 3/25/2017 | 67,941 | 1905.80 | 2,089 | 246 | 642 | 44 |
| 3/26/2017 | 69,081 | 1906.40 | 1,641 | 1,082 | 1,045 | 44 |
| 3/27/2017 | 69,124 | 1906.40 | 1,550 | 1,624 | 1,760 | 68 |
| 3/28/2017 | 69,168 | 1906.40 | 1,264 | 1,297 | 1,334 | 80 |
| 3/29/2017 | 68,951 | 1906.30 | 1,124 | 1,299 | 1,322 | 80 |
| 3/30/2017 | 69,146 | 1906.40 | 1,200 | 1,125 | 1,191 | 59 |
| 3/31/2017 | 69,319 | 1906.50 | 1,050 | 936 | 1,011 | 45 |
| 4/1/2017 | 69,580 | 1906.60 | 960 | 787 | 902 | 45 |
| 4/2/2017 | 69,754 | 1906.70 | 906 | 756 | 822 | 45 |
| 4/3/2017 | 69,602 | 1906.60 | 830 | 914 | 928 | 44 |
| 4/4/2017 | 68,800 | 1906.20 | 718 | 1,160 | 1,097 | 44 |
| 4/5/2017 | 68,112 | 1905.90 | 702 | 1,100 | 1,135 | 44 |
| 4/6/2017 | 69,103 | 1906.40 | 1,401 | 987 | 1,048 | 44 |
| 4/7/2017 | 68,348 | 1906.00 | 2,936 | 3,530 | 3,684 | 44 |
| 4/8/2017 | 68,520 | 1906.10 | 2,127 | 2,145 | 2,562 | 44 |
| 4/9/2017 | 68,995 | 1906.30 | 1,553 | 1,371 | 1,526 | 43 |
| 4/10/2017 | 69,103 | 1906.40 | 1,312 | 1,327 | 1,441 | 43 |
| 4/11/2017 | 69,298 | 1906.50 | 1,341 | 1,309 | 1,404 | 43 |
| 4/12/2017 | 69,493 | 1906.50 | 1,498 | 1,522 | 1,558 | 43 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/13/2017 | 69,776 | 1906.70 | 2,273 | 2,292 | 2,384 | 44 |
| 4/14/2017 | 68,951 | 1906.30 | 1,646 | 2,242 | 2,407 | 44 |
| 4/15/2017 | 68,370 | 1906.00 | 1,390 | 1,832 | 1,871 | 88 |
| 4/16/2017 | 68,649 | 1906.10 | 1,368 | 1,279 | 1,447 | 88 |
| 4/17/2017 | 68,995 | 1906.30 | 1,293 | 1,136 | 1,209 | 88 |
| 4/18/2017 | 69,298 | 1906.50 | 1,282 | 1,154 | 1,245 | 88 |
| 4/19/2017 | 69,493 | 1906.50 | 1,225 | 1,160 | 1,219 | 89 |
| 4/20/2017 | 70,080 | 1906.80 | 1,444 | 1,178 | 1,277 | 91 |
| 4/21/2017 | 70,233 | 1906.90 | 1,189 | 1,132 | 1,210 | 90 |
| 4/22/2017 | 70,582 | 1907.10 | 1,122 | 895 | 1,010 | 89 |
| 4/23/2017 | 71,021 | 1907.30 | 1,026 | 764 | 857 | 90 |
| 4/24/2017 | 71,175 | 1907.30 | 966 | 875 | 914 | 91 |
| 4/25/2017 | 71,131 | 1907.30 | 898 | 864 | 905 | 95 |
| 4/26/2017 | 71,219 | 1907.40 | 878 | 806 | 833 | 126 |
| 4/27/2017 | 71,550 | 1907.50 | 855 | 640 | 753 | 91 |
| 4/28/2017 | 72,081 | 1907.80 | 744 | 444 | 550 | 90 |
| 4/29/2017 | 72,502 | 1908.00 | 676 | 431 | 500 | 90 |
| 4/30/2017 | 72,769 | 1908.10 | 650 | 483 | 542 | 90 |
| 5/1/2017 | 72,880 | 1908.10 | 650 | 568 | 582 | 90 |
| 5/2/2017 | 72,880 | 1908.10 | 692 | 639 | 668 | 90 |
| 5/3/2017 | 72,880 | 1908.10 | 687 | 625 | 646 | 90 |
| 5/4/2017 | 72,902 | 1908.20 | 662 | 612 | 640 | 90 |
| 5/5/2017 | 72,880 | 1908.10 | 579 | 573 | 597 | 90 |
| 5/6/2017 | 72,969 | 1908.20 | 510 | 488 | 539 | 90 |
| 5/7/2017 | 73,170 | 1908.30 | 465 | 364 | 389 | 90 |
| 5/8/2017 | 73,326 | 1908.30 | 429 | 378 | 380 | 90 |
| 5/9/2017 | 73,326 | 1908.30 | 407 | 420 | 421 | 89 |
| 5/10/2017 | 73,326 | 1908.30 | 385 | 405 | 419 | 89 |
| 5/11/2017 | 73,393 | 1908.40 | 371 | 359 | 364 | 89 |
| 5/12/2017 | 73,461 | 1908.40 | 348 | 337 | 324 | 90 |
| 5/13/2017 | 73,461 | 1908.40 | 314 | 335 | 323 | 90 |
| 5/14/2017 | 73,461 | 1908.40 | 325 | 346 | 327 | 91 |
| 5/15/2017 | 73,393 | 1908.40 | 303 | 364 | 306 | 130 |
| 5/16/2017 | 73,304 | 1908.30 | 292 | 361 | 303 | 130 |
| 5/17/2017 | 73,214 | 1908.30 | 295 | 354 | 291 | 130 |
| 5/18/2017 | 73,103 | 1908.20 | 284 | 355 | 291 | 130 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/19/2017 | 72,924 | 1908.20 | 270 | 356 | 287 | 130 |
| 5/20/2017 | 72,769 | 1908.10 | 255 | 357 | 285 | 130 |
| 5/21/2017 | 72,591 | 1908.00 | 251 | 356 | 282 | 130 |
| 5/22/2017 | 72,147 | 1907.80 | 227 | 474 | 337 | 129 |
| 5/23/2017 | 71,461 | 1907.50 | 212 | 605 | 543 | 129 |
| 5/24/2017 | 70,823 | 1907.20 | 204 | 566 | 493 | 129 |
| 5/25/2017 | 70,233 | 1906.90 | 195 | 527 | 459 | 129 |
| 5/26/2017 | 69,754 | 1906.70 | 190 | 469 | 396 | 130 |
| 5/27/2017 | 69,341 | 1906.50 | 188 | 426 | 342 | 130 |
| 5/28/2017 | 68,930 | 1906.30 | 182 | 404 | 309 | 130 |
| 5/29/2017 | 68,520 | 1906.10 | 170 | 395 | 291 | 131 |
| 5/30/2017 | 68,112 | 1905.90 | 159 | 401 | 293 | 131 |
| 5/31/2017 | 67,685 | 1905.70 | 154 | 387 | 283 | 131 |
| 6/1/2017 | 67,300 | 1905.50 | 151 | 367 | 252 | 131 |
| 6/2/2017 | 66,981 | 1905.30 | 149 | 322 | 213 | 131 |
| 6/3/2017 | 66,726 | 1905.20 | 140 | 282 | 168 | 130 |
| 6/4/2017 | 66,472 | 1905.10 | 125 | 269 | 158 | 129 |
| 6/5/2017 | 66,240 | 1905.00 | 125 | 253 | 147 | 129 |
| 6/6/2017 | 66,008 | 1904.90 | 112 | 239 | 135 | 129 |
| 6/7/2017 | 65,777 | 1904.80 | 114 | 230 | 126 | 130 |
| 6/8/2017 | 65,714 | 1904.70 | 182 | 225 | 128 | 130 |
| 6/9/2017 | 65,525 | 1904.60 | 122 | 214 | 113 | 130 |
| 6/10/2017 | 65,357 | 1904.60 | 107 | 206 | 101 | 131 |
| 6/11/2017 | 65,169 | 1904.50 | 113 | 204 | 99 | 131 |
| 6/12/2017 | 65,002 | 1904.40 | 119 | 204 | 97 | 130 |
| 6/13/2017 | 64,856 | 1904.30 | 115 | 197 | 93 | 130 |
| 6/14/2017 | 64,689 | 1904.20 | 104 | 191 | 81 | 130 |
| 6/15/2017 | 64,523 | 1904.10 | 95 | 189 | 79 | 130 |
| 6/16/2017 | 64,336 | 1904.10 | 89 | 185 | 73 | 130 |
| 6/17/2017 | 64,150 | 1904.00 | 83 | 180 | 68 | 129 |
| 6/18/2017 | 63,964 | 1903.90 | 79 | 178 | 64 | 129 |
| 6/19/2017 | 63,778 | 1903.80 | 73 | 178 | 64 | 130 |
| 6/20/2017 | 63,573 | 1903.70 | 67 | 178 | 62 | 130 |
| 6/21/2017 | 63,367 | 1903.60 | 57 | 174 | 61 | 130 |
| 6/22/2017 | 63,038 | 1903.40 | 58 | 231 | 92 | 142 |
| 6/23/2017 | 62,813 | 1903.30 | 40 | 172 | 62 | 131 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/24/2017 | 62,568 | 1903.20 | 39 | 171 | 55 | 131 |
| 6/25/2017 | 62,324 | 1903.10 | 35 | 171 | 54 | 131 |
| 6/26/2017 | 62,040 | 1902.90 | 22 | 169 | 55 | 130 |
| 6/27/2017 | 61,798 | 1902.80 | 25 | 167 | 56 | 131 |
| 6/28/2017 | 61,535 | 1902.70 | 21 | 167 | 55 | 131 |
| 6/29/2017 | 61,253 | 1902.50 | 17 | 166 | 54 | 131 |
| 6/30/2017 | 60,992 | 1902.40 | 19 | 166 | 53 | 131 |
| 7/1/2017 | 60,731 | 1902.20 | 18 | 166 | 53 | 130 |
| 7/2/2017 | 60,451 | 1902.10 | 17 | 166 | 52 | 130 |
| 7/3/2017 | 60,172 | 1902.00 | 15 | 166 | 51 | 130 |
| 7/4/2017 | 59,895 | 1901.80 | 8 | 166 | 52 | 130 |
| 7/5/2017 | 59,598 | 1901.70 | 6 | 165 | 50 | 130 |
| 7/6/2017 | 59,302 | 1901.50 | 7 | 169 | 51 | 130 |
| 7/7/2017 | 59,007 | 1901.40 | 6 | 172 | 54 | 130 |
| 7/8/2017 | 58,674 | 1901.20 | 2 | 171 | 54 | 130 |
| 7/9/2017 | 58,382 | 1901.00 | -2 | 171 | 52 | 130 |
| 7/10/2017 | 58,071 | 1900.90 | 2 | 171 | 52 | 130 |
| 7/11/2017 | 57,761 | 1900.70 | 2 | 171 | 52 | 130 |
| 7/12/2017 | 57,452 | 1900.50 | 3 | 171 | 52 | 130 |
| 7/13/2017 | 57,145 | 1900.40 | 0 | 171 | 48 | 130 |
| 7/14/2017 | 56,820 | 1900.20 | -6 | 166 | 47 | 130 |
| 7/15/2017 | 56,496 | 1900.00 | -7 | 163 | 44 | 130 |
| 7/16/2017 | 56,192 | 1899.90 | -11 | 163 | 44 | 130 |
| 7/17/2017 | 55,890 | 1899.70 | -12 | 162 | 44 | 130 |
| 7/18/2017 | 55,551 | 1899.50 | -18 | 162 | 44 | 130 |
| 7/19/2017 | 55,232 | 1899.40 | -14 | 162 | 43 | 130 |
| 7/20/2017 | 54,933 | 1899.20 | -12 | 162 | 43 | 130 |
| 7/21/2017 | 54,617 | 1899.00 | -10 | 162 | 42 | 130 |
| 7/22/2017 | 54,302 | 1898.90 | -8 | 161 | 42 | 130 |
| 7/23/2017 | 53,987 | 1898.70 | -10 | 161 | 41 | 130 |
| 7/24/2017 | 53,674 | 1898.50 | -14 | 163 | 42 | 130 |
| 7/25/2017 | 53,362 | 1898.30 | -18 | 164 | 44 | 130 |
| 7/26/2017 | 53,014 | 1898.20 | -13 | 164 | 44 | 130 |
| 7/27/2017 | 52,704 | 1898.00 | -13 | 163 | 43 | 130 |
| 7/28/2017 | 52,360 | 1897.80 | -20 | 163 | 43 | 130 |
| 7/29/2017 | 52,053 | 1897.60 | -17 | 163 | 43 | 130 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/30/2017 | 51,728 | 1897.40 | -15 | 164 | 43 | 130 |
| 7/31/2017 | 51,404 | 1897.30 | -14 | 164 | 42 | 130 |
| 8/1/2017 | 51,082 | 1897.10 | -13 | 164 | 42 | 130 |
| 8/2/2017 | 50,743 | 1896.90 | -18 | 164 | 42 | 130 |
| 8/3/2017 | 50,406 | 1896.70 | -22 | 164 | 41 | 129 |
| 8/4/2017 | 50,070 | 1896.50 | -15 | 166 | 43 | 130 |
| 8/5/2017 | 49,735 | 1896.30 | -19 | 165 | 42 | 130 |
| 8/6/2017 | 49,401 | 1896.10 | -19 | 165 | 42 | 130 |
| 8/7/2017 | 49,069 | 1895.90 | -17 | 166 | 42 | 130 |
| 8/8/2017 | 48,738 | 1895.70 | -17 | 166 | 41 | 130 |
| 8/9/2017 | 48,427 | 1895.60 | -8 | 166 | 40 | 130 |
| 8/10/2017 | 48,115 | 1895.40 | -7 | 166 | 40 | 130 |
| 8/11/2017 | 47,787 | 1895.20 | -6 | 166 | 39 | 131 |
| 8/12/2017 | 47,461 | 1895.00 | -15 | 166 | 39 | 132 |
| 8/13/2017 | 47,120 | 1894.80 | -17 | 166 | 40 | 131 |
| 8/14/2017 | 46,797 | 1894.60 | -12 | 167 | 40 | 131 |
| 8/15/2017 | 46,475 | 1894.40 | -10 | 167 | 40 | 130 |
| 8/16/2017 | 46,154 | 1894.20 | -15 | 166 | 40 | 130 |
| 8/17/2017 | 45,818 | 1894.00 | -13 | 170 | 44 | 130 |
| 8/18/2017 | 45,484 | 1893.80 | -12 | 171 | 43 | 130 |
| 8/19/2017 | 45,151 | 1893.60 | -13 | 169 | 41 | 130 |
| 8/20/2017 | 44,803 | 1893.40 | -16 | 168 | 40 | 130 |
| 8/21/2017 | 44,472 | 1893.20 | -12 | 169 | 40 | 130 |
| 8/22/2017 | 44,159 | 1893.00 | -10 | 169 | 40 | 130 |
| 8/23/2017 | 43,816 | 1892.80 | -12 | 168 | 40 | 130 |
| 8/24/2017 | 43,474 | 1892.60 | -16 | 168 | 39 | 130 |
| 8/25/2017 | 43,132 | 1892.40 | -13 | 167 | 39 | 130 |
| 8/26/2017 | 42,793 | 1892.20 | -17 | 169 | 39 | 130 |
| 8/27/2017 | 42,471 | 1892.00 | -14 | 169 | 40 | 130 |
| 8/28/2017 | 42,135 | 1891.80 | -13 | 168 | 39 | 129 |
| 8/29/2017 | 41,800 | 1891.60 | -21 | 168 | 38 | 129 |
| 8/30/2017 | 41,450 | 1891.40 | -21 | 167 | 38 | 130 |
| 8/31/2017 | 41,118 | 1891.20 | -13 | 167 | 38 | 130 |
| 9/1/2017 | 40,787 | 1891.00 | -19 | 169 | 38 | 129 |
| 9/2/2017 | 40,428 | 1890.70 | -12 | 172 | 41 | 129 |
| 9/3/2017 | 40,099 | 1890.50 | -12 | 171 | 40 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/4/2017 | 39,753 | 1890.30 | -17 | 172 | 40 | 130 |
| 9/5/2017 | 39,409 | 1890.10 | -12 | 172 | 40 | 130 |
| 9/6/2017 | 39,068 | 1889.90 | -12 | 173 | 40 | 131 |
| 9/7/2017 | 38,728 | 1889.60 | -12 | 173 | 40 | 131 |
| 9/8/2017 | 38,450 | 1889.50 | -6 | 172 | 39 | 131 |
| 9/9/2017 | 38,053 | 1889.20 | -11 | 171 | 39 | 131 |
| 9/10/2017 | 37,718 | 1889.00 | -12 | 170 | 38 | 131 |
| 9/11/2017 | 37,385 | 1888.70 | -9 | 169 | 39 | 131 |
| 9/12/2017 | 37,039 | 1888.50 | -18 | 169 | 40 | 131 |
| 9/13/2017 | 36,709 | 1888.30 | -11 | 168 | 40 | 131 |
| 9/14/2017 | 36,352 | 1888.10 | -20 | 168 | 40 | 131 |
| 9/15/2017 | 36,012 | 1887.80 | -14 | 167 | 39 | 131 |
| 9/16/2017 | 35,717 | 1887.60 | -18 | 147 | 59 | 90 |
| 9/17/2017 | 35,452 | 1887.40 | -19 | 144 | 54 | 91 |
| 9/18/2017 | 35,131 | 1887.20 | -25 | 143 | 52 | 91 |
| 9/19/2017 | 34,840 | 1887.00 | -24 | 140 | 49 | 91 |
| 9/20/2017 | 34,580 | 1886.80 | -12 | 138 | 48 | 91 |
| 9/21/2017 | 34,307 | 1886.60 | -20 | 135 | 45 | 92 |
| 9/22/2017 | 34,048 | 1886.50 | -10 | 133 | 43 | 92 |
| 9/23/2017 | 33,805 | 1886.30 | -15 | 129 | 40 | 91 |
| 9/24/2017 | 33,564 | 1886.10 | -13 | 131 | 40 | 91 |
| 9/25/2017 | 33,309 | 1885.90 | -15 | 132 | 42 | 91 |
| 9/26/2017 | 33,056 | 1885.70 | -13 | 132 | 41 | 91 |
| 9/27/2017 | 32,804 | 1885.60 | -13 | 132 | 41 | 91 |
| 9/28/2017 | 32,553 | 1885.40 | -15 | 132 | 41 | 91 |
| 9/29/2017 | 32,290 | 1885.20 | -17 | 132 | 41 | 91 |
| 9/30/2017 | 33,483 | 1886.00 | -14 | 132 | 41 | 91 |
| 10/1/2017 | 33,216 | 1885.90 | -19 | 118 | 41 | 91 |
| 10/2/2017 | 32,951 | 1885.70 | -15 | 118 | 41 | 92 |
| 10/3/2017 | 32,687 | 1885.50 | -24 | 117 | 41 | 91 |
| 10/4/2017 | 32,424 | 1885.30 | -16 | 117 | 42 | 90 |
| 10/5/2017 | 32,163 | 1885.10 | -18 | 117 | 42 | 90 |
| 10/6/2017 | 31,916 | 1884.90 | -15 | 116 | 42 | 90 |
| 10/7/2017 | 31,658 | 1884.70 | -15 | 116 | 41 | 90 |
| 10/8/2017 | 31,387 | 1884.50 | -23 | 117 | 41 | 90 |
| 10/9/2017 | 31,397 | 1884.50 | -20 | 119 | 47 | 78 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/10/2017 | 30,879 | 1884.10 | -19 | 119 | 43 | 50 |
| 10/11/2017 | 30,708 | 1884.00 | -16 | 122 | 39 | 92 |
| 10/12/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/13/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/14/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/15/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/16/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/17/2017 | 29,557 | 1883.10 | -11 | 94 | 42 | 91 |
| 10/18/2017 | 28,999 | 1882.70 | -20 | 84 | 56 | 47 |
| 10/19/2017 | 28,861 | 1882.50 | 4 | 84 | 57 | 45 |
| 10/20/2017 | 28,698 | 1882.40 | -2 | 79 | 58 | 45 |
| 10/21/2017 | 28,561 | 1882.30 | -4 | 70 | 49 | 45 |
| 10/22/2017 | 28,412 | 1882.20 | -9 | 66 | 45 | 45 |
| 10/23/2017 | 28,276 | 1882.10 | 7 | 78 | 43 | 45 |
| 10/24/2017 | 28,165 | 1882.00 | 14 | 83 | 41 | 44 |
| 10/25/2017 | 28,017 | 1881.90 | 14 | 84 | 41 | 44 |
| 10/26/2017 | 27,895 | 1881.80 | 13 | 84 | 41 | 41 |
| 10/27/2017 | 27,748 | 1881.60 | 9 | 85 | 41 | 45 |
| 10/28/2017 | 27,614 | 1881.50 | 19 | 89 | 40 | 45 |
| 10/29/2017 | 27,468 | 1881.40 | 17 | 90 | 41 | 45 |
| 10/30/2017 | 27,299 | 1881.30 | 16 | 90 | 41 | 45 |
| 10/31/2017 | 27,191 | 1881.20 | 15 | 90 | 41 | 45 |
| 11/1/2017 | 27,059 | 1881.10 | 17 | 90 | 41 | 45 |
| 11/2/2017 | 26,916 | 1881.00 | 16 | 90 | 41 | 45 |
| 11/3/2017 | 26,844 | 1880.90 | 50 | 91 | 45 | 45 |
| 11/4/2017 | 26,725 | 1880.80 | 34 | 91 | 45 | 45 |
| 11/5/2017 | 26,630 | 1880.70 | 32 | 91 | 43 | 45 |
| 11/6/2017 | 26,500 | 1880.60 | 27 | 91 | 43 | 45 |
| 11/7/2017 | 26,477 | 1880.60 | 76 | 92 | 42 | 45 |
| 11/8/2017 | 26,382 | 1880.50 | 58 | 92 | 47 | 45 |
| 11/9/2017 | 26,500 | 1880.60 | 151 | 91 | 59 | 45 |
| 11/10/2017 | 26,701 | 1880.80 | 212 | 91 | 61 | 45 |
| 11/11/2017 | 26,618 | 1880.70 | 110 | 155 | 71 | 71 |
| 11/12/2017 | 26,335 | 1880.50 | 61 | 216 | 67 | 149 |
| 11/13/2017 | 26,008 | 1880.20 | 87 | 250 | 95 | 150 |
| 11/14/2017 | 25,672 | 1879.90 | 139 | 313 | 225 | 95 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/15/2017 | 25,857 | 1880.10 | 368 | 319 | 331 | 71 |
| 11/16/2017 | 26,559 | 1880.70 | 711 | 302 | 437 | 57 |
| 11/17/2017 | 26,595 | 1880.70 | 235 | 213 | 200 | 65 |
| 11/18/2017 | 26,382 | 1880.50 | 132 | 245 | 172 | 99 |
| 11/19/2017 | 26,090 | 1880.30 | 98 | 259 | 166 | 99 |
| 11/20/2017 | 26,785 | 1880.80 | 580 | 256 | 309 | 99 |
| 11/21/2017 | 27,275 | 1881.30 | 553 | 257 | 266 | 99 |
| 11/22/2017 | 27,203 | 1881.20 | 221 | 256 | 187 | 99 |
| 11/23/2017 | 26,999 | 1881.00 | 152 | 256 | 176 | 99 |
| 11/24/2017 | 26,737 | 1880.80 | 118 | 256 | 171 | 99 |
| 11/25/2017 | 26,435 | 1880.60 | 101 | 254 | 167 | 99 |
| 11/26/2017 | 26,808 | 1880.90 | 406 | 257 | 264 | 99 |
| 11/27/2017 | 27,456 | 1881.40 | 635 | 259 | 358 | 100 |
| 11/28/2017 | 27,505 | 1881.40 | 287 | 258 | 219 | 100 |
| 11/29/2017 | 27,372 | 1881.30 | 195 | 258 | 188 | 100 |
| 11/30/2017 | 27,167 | 1881.20 | 153 | 258 | 179 | 100 |
| 12/1/2017 | 26,987 | 1881.00 | 128 | 258 | 174 | 100 |
| 12/2/2017 | 26,654 | 1880.70 | 114 | 259 | 172 | 100 |
| 12/3/2017 | 26,335 | 1880.50 | 101 | 260 | 171 | 100 |
| 12/4/2017 | 26,020 | 1880.20 | 87 | 259 | 169 | 100 |
| 12/5/2017 | 25,695 | 1879.90 | 87 | 259 | 167 | 100 |
| 12/6/2017 | 25,339 | 1879.60 | 64 | 252 | 164 | 100 |
| 12/7/2017 | 25,027 | 1879.30 | 66 | 231 | 154 | 100 |
| 12/8/2017 | 24,729 | 1879.10 | 58 | 219 | 142 | 101 |
| 12/9/2017 | 24,495 | 1878.90 | 58 | 186 | 152 | 59 |
| 12/10/2017 | 24,295 | 1878.70 | 55 | 160 | 139 | 44 |
| 12/11/2017 | 24,119 | 1878.50 | 52 | 155 | 134 | 44 |
| 12/12/2017 | 23,912 | 1878.30 | 51 | 151 | 130 | 44 |
| 12/13/2017 | 23,728 | 1878.20 | 49 | 149 | 125 | 44 |
| 12/14/2017 | 23,555 | 1878.00 | 48 | 149 | 125 | 44 |
| 12/15/2017 | 23,351 | 1877.80 | 50 | 145 | 124 | 44 |
| 12/16/2017 | 23,138 | 1877.60 | 44 | 145 | 124 | 44 |
| 12/17/2017 | 22,959 | 1877.40 | 45 | 144 | 123 | 44 |
| 12/18/2017 | 22,749 | 1877.20 | 45 | 144 | 122 | 44 |
| 12/19/2017 | 22,583 | 1877.10 | 50 | 144 | 123 | 44 |
| 12/20/2017 | 22,375 | 1876.90 | 45 | 145 | 124 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/21/2017 | 22,170 | 1876.70 | 42 | 144 | 122 | 44 |
| 12/22/2017 | 21,976 | 1876.50 | 41 | 143 | 121 | 44 |
| 12/23/2017 | 21,773 | 1876.30 | 42 | 143 | 120 | 44 |
| 12/24/2017 | 21,572 | 1876.10 | 40 | 143 | 120 | 44 |
| 12/25/2017 | 21,382 | 1875.90 | 41 | 143 | 119 | 44 |
| 12/26/2017 | 21,174 | 1875.70 | 38 | 143 | 119 | 44 |
| 12/27/2017 | 20,987 | 1875.50 | 38 | 142 | 119 | 44 |
| 12/28/2017 | 20,773 | 1875.30 | 37 | 142 | 118 | 44 |
| 12/29/2017 | 20,560 | 1875.10 | 35 | 146 | 122 | 44 |
| 12/30/2017 | 20,339 | 1874.80 | 37 | 144 | 123 | 44 |
| 12/31/2017 | 20,139 | 1874.60 | 35 | 143 | 119 | 44 |
| 1/1/2018 | 19,933 | 1874.40 | 38 | 143 | 119 | 44 |
| 1/2/2018 | 19,718 | 1874.20 | 35 | 143 | 119 | 44 |
| 1/3/2018 | 19,515 | 1874.00 | 38 | 143 | 118 | 44 |
| 1/4/2018 | 19,332 | 1873.80 | 46 | 142 | 120 | 44 |
| 1/5/2018 | 19,323 | 1873.70 | 130 | 143 | 127 | 44 |
| 1/6/2018 | 19,286 | 1873.70 | 139 | 142 | 128 | 44 |
| 1/7/2018 | 19,168 | 1873.60 | 80 | 142 | 124 | 44 |
| 1/8/2018 | 20,017 | 1874.50 | 526 | 145 | 231 | 44 |
| 1/9/2018 | 22,293 | 1876.80 | 1,399 | 148 | 399 | 44 |
| 1/10/2018 | 22,802 | 1877.30 | 452 | 171 | 206 | 44 |
| 1/11/2018 | 22,970 | 1877.50 | 285 | 179 | 188 | 44 |
| 1/12/2018 | 23,022 | 1877.50 | 208 | 179 | 178 | 44 |
| 1/13/2018 | 22,980 | 1877.50 | 163 | 179 | 172 | 44 |
| 1/14/2018 | 22,906 | 1877.40 | 140 | 178 | 168 | 44 |
| 1/15/2018 | 22,802 | 1877.30 | 127 | 178 | 165 | 44 |
| 1/16/2018 | 22,728 | 1877.20 | 139 | 178 | 166 | 44 |
| 1/17/2018 | 22,624 | 1877.10 | 122 | 177 | 163 | 44 |
| 1/18/2018 | 22,666 | 1877.20 | 192 | 177 | 209 | 44 |
| 1/19/2018 | 23,043 | 1877.50 | 385 | 179 | 313 | 44 |
| 1/20/2018 | 23,149 | 1877.60 | 242 | 178 | 221 | 44 |
| 1/21/2018 | 23,544 | 1878.00 | 366 | 179 | 235 | 44 |
| 1/22/2018 | 26,230 | 1880.40 | 1,605 | 186 | 704 | 44 |
| 1/23/2018 | 26,951 | 1881.00 | 582 | 186 | 350 | 44 |
| 1/24/2018 | 28,214 | 1882.00 | 809 | 187 | 459 | 44 |
| 1/25/2018 | 29,955 | 1883.40 | 1,131 | 189 | 651 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/26/2018 | 30,918 | 1884.10 | 731 | 188 | 517 | 45 |
| 1/27/2018 | 31,502 | 1884.60 | 499 | 188 | 400 | 45 |
| 1/28/2018 | 32,186 | 1885.10 | 540 | 188 | 372 | 45 |
| 1/29/2018 | 32,758 | 1885.50 | 499 | 188 | 335 | 46 |
| 1/30/2018 | 33,088 | 1885.80 | 433 | 248 | 342 | 45 |
| 1/31/2018 | 33,184 | 1885.80 | 385 | 343 | 464 | 51 |
| 2/1/2018 | 33,447 | 1886.00 | 327 | 191 | 277 | 46 |
| 2/2/2018 | 33,670 | 1886.20 | 311 | 189 | 232 | 46 |
| 2/3/2018 | 33,823 | 1886.30 | 279 | 187 | 217 | 46 |
| 2/4/2018 | 33,991 | 1886.40 | 269 | 185 | 203 | 46 |
| 2/5/2018 | 34,019 | 1886.40 | 199 | 185 | 196 | 46 |
| 2/6/2018 | 34,075 | 1886.50 | 214 | 186 | 192 | 46 |
| 2/7/2018 | 34,075 | 1886.50 | 187 | 186 | 189 | 45 |
| 2/8/2018 | 34,019 | 1886.40 | 167 | 186 | 188 | 45 |
| 2/9/2018 | 33,963 | 1886.40 | 146 | 186 | 186 | 45 |
| 2/10/2018 | 33,879 | 1886.30 | 134 | 185 | 185 | 45 |
| 2/11/2018 | 33,753 | 1886.20 | 125 | 186 | 183 | 45 |
| 2/12/2018 | 33,670 | 1886.20 | 113 | 185 | 182 | 45 |
| 2/13/2018 | 33,530 | 1886.10 | 106 | 185 | 180 | 45 |
| 2/14/2018 | 33,378 | 1886.00 | 101 | 183 | 178 | 45 |
| 2/15/2018 | 33,240 | 1885.90 | 93 | 174 | 171 | 45 |
| 2/16/2018 | 33,102 | 1885.80 | 90 | 166 | 163 | 45 |
| 2/17/2018 | 32,978 | 1885.70 | 87 | 160 | 157 | 45 |
| 2/18/2018 | 32,882 | 1885.60 | 92 | 150 | 149 | 45 |
| 2/19/2018 | 32,745 | 1885.50 | 75 | 148 | 146 | 45 |
| 2/20/2018 | 32,621 | 1885.40 | 64 | 148 | 145 | 45 |
| 2/21/2018 | 32,485 | 1885.30 | 83 | 147 | 145 | 45 |
| 2/22/2018 | 32,363 | 1885.20 | 78 | 147 | 145 | 45 |
| 2/23/2018 | 32,214 | 1885.10 | 62 | 147 | 144 | 45 |
| 2/24/2018 | 32,092 | 1885.00 | 71 | 145 | 143 | 45 |
| 2/25/2018 | 31,944 | 1884.90 | 67 | 143 | 141 | 45 |
| 2/26/2018 | 31,863 | 1884.90 | 98 | 143 | 146 | 45 |
| 2/27/2018 | 31,729 | 1884.80 | 64 | 143 | 142 | 45 |
| 2/28/2018 | 31,662 | 1884.70 | 102 | 143 | 143 | 45 |
| 3/1/2018 | 32,458 | 1885.30 | 529 | 146 | 368 | 45 |
| 3/2/2018 | 33,102 | 1885.80 | 519 | 147 | 372 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/3/2018 | 33,378 | 1886.00 | 324 | 177 | 293 | 45 |
| 3/4/2018 | 33,530 | 1886.10 | 273 | 196 | 309 | 45 |
| 3/5/2018 | 33,600 | 1886.10 | 231 | 195 | 280 | 45 |
| 3/6/2018 | 33,670 | 1886.20 | 227 | 195 | 267 | 45 |
| 3/7/2018 | 33,823 | 1886.30 | 267 | 195 | 271 | 45 |
| 3/8/2018 | 34,287 | 1886.60 | 430 | 194 | 329 | 45 |
| 3/9/2018 | 34,612 | 1886.90 | 362 | 194 | 294 | 45 |
| 3/10/2018 | 34,826 | 1887.00 | 308 | 194 | 271 | 45 |
| 3/11/2018 | 34,997 | 1887.10 | 281 | 194 | 253 | 45 |
| 3/12/2018 | 35,184 | 1887.30 | 288 | 194 | 240 | 45 |
| 3/13/2018 | 36,479 | 1888.10 | 831 | 196 | 323 | 45 |
| 3/14/2018 | 37,717 | 1889.00 | 866 | 196 | 383 | 45 |
| 3/15/2018 | 39,026 | 1889.80 | 856 | 200 | 410 | 45 |
| 3/16/2018 | 40,567 | 1890.80 | 1,027 | 203 | 582 | 45 |
| 3/17/2018 | 41,517 | 1891.40 | 708 | 203 | 495 | 45 |
| 3/18/2018 | 42,142 | 1891.80 | 534 | 203 | 423 | 45 |
| 3/19/2018 | 42,626 | 1892.10 | 447 | 203 | 378 | 45 |
| 3/20/2018 | 43,113 | 1892.40 | 441 | 203 | 356 | 45 |
| 3/21/2018 | 44,610 | 1893.30 | 945 | 204 | 413 | 45 |
| 3/22/2018 | 52,195 | 1897.70 | 4,191 | 213 | 729 | 45 |
| 3/23/2018 | 55,631 | 1899.60 | 2,037 | 215 | 594 | 45 |
| 3/24/2018 | 58,064 | 1900.90 | 1,517 | 223 | 589 | 45 |
| 3/25/2018 | 59,741 | 1901.70 | 1,122 | 226 | 529 | 45 |
| 3/26/2018 | 60,992 | 1902.40 | 884 | 227 | 488 | 45 |
| 3/27/2018 | 61,921 | 1902.80 | 725 | 228 | 444 | 45 |
| 3/28/2018 | 62,758 | 1903.30 | 651 | 229 | 411 | 44 |
| 3/29/2018 | 63,502 | 1903.60 | 610 | 226 | 377 | 44 |
| 3/30/2018 | 64,167 | 1904.00 | 566 | 220 | 345 | 44 |
| 3/31/2018 | 64,735 | 1904.20 | 510 | 212 | 311 | 44 |
| 4/1/2018 | 65,163 | 1904.50 | 465 | 258 | 340 | 45 |
| 4/2/2018 | 65,491 | 1904.60 | 434 | 260 | 339 | 45 |
| 4/3/2018 | 65,962 | 1904.80 | 497 | 260 | 327 | 45 |
| 4/4/2018 | 66,146 | 1904.90 | 342 | 259 | 316 | 45 |
| 4/5/2018 | 66,475 | 1905.10 | 413 | 260 | 316 | 45 |
| 4/6/2018 | 68,447 | 1906.00 | 4,387 | 3,469 | 3,869 | 33 |
| 4/7/2018 | 67,366 | 1905.50 | 5,863 | 6,806 | 8,307 | 9 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/8/2018 | 69,433 | 1906.50 | 2,386 | 1,336 | 2,026 | 45 |
| 4/9/2018 | 70,804 | 1907.20 | 1,414 | 769 | 1,016 | 45 |
| 4/10/2018 | 71,935 | 1907.70 | 1,029 | 508 | 756 | 45 |
| 4/11/2018 | 72,406 | 1907.90 | 865 | 634 | 739 | 45 |
| 4/12/2018 | 72,707 | 1908.10 | 777 | 702 | 932 | 44 |
| 4/13/2018 | 73,290 | 1908.30 | 610 | 341 | 494 | 45 |
| 4/14/2018 | 73,550 | 1908.50 | 530 | 458 | 542 | 56 |
| 4/15/2018 | 73,724 | 1908.50 | 557 | 548 | 616 | 81 |
| 4/16/2018 | 73,941 | 1908.60 | 588 | 534 | 619 | 81 |
| 4/17/2018 | 74,226 | 1908.80 | 493 | 419 | 508 | 80 |
| 4/18/2018 | 74,512 | 1908.90 | 450 | 337 | 393 | 80 |
| 4/19/2018 | 74,732 | 1909.00 | 413 | 333 | 374 | 80 |
| 4/20/2018 | 74,887 | 1909.10 | 397 | 312 | 339 | 80 |
| 4/21/2018 | 75,019 | 1909.10 | 368 | 306 | 319 | 80 |
| 4/22/2018 | 75,108 | 1909.20 | 343 | 306 | 308 | 80 |
| 4/23/2018 | 75,130 | 1909.20 | 312 | 306 | 298 | 80 |
| 4/24/2018 | 75,108 | 1909.20 | 292 | 306 | 290 | 80 |
| 4/25/2018 | 75,063 | 1909.10 | 274 | 305 | 282 | 80 |
| 4/26/2018 | 74,975 | 1909.10 | 249 | 305 | 276 | 80 |
| 4/27/2018 | 74,887 | 1909.10 | 246 | 304 | 272 | 80 |
| 4/28/2018 | 74,798 | 1909.00 | 249 | 304 | 271 | 80 |
| 4/29/2018 | 74,666 | 1909.00 | 234 | 303 | 273 | 80 |
| 4/30/2018 | 74,556 | 1908.90 | 230 | 304 | 261 | 80 |
| 5/1/2018 | 74,402 | 1908.80 | 221 | 303 | 256 | 80 |
| 5/2/2018 | 74,226 | 1908.80 | 199 | 304 | 251 | 80 |
| 5/3/2018 | 74,029 | 1908.70 | 179 | 293 | 239 | 80 |
| 5/4/2018 | 73,832 | 1908.60 | 166 | 283 | 226 | 80 |
| 5/5/2018 | 73,637 | 1908.50 | 162 | 280 | 221 | 80 |
| 5/6/2018 | 73,398 | 1908.40 | 138 | 275 | 216 | 80 |
| 5/7/2018 | 73,182 | 1908.30 | 142 | 271 | 210 | 80 |
| 5/8/2018 | 72,966 | 1908.20 | 130 | 260 | 202 | 80 |
| 5/9/2018 | 72,750 | 1908.10 | 113 | 246 | 188 | 80 |
| 5/10/2018 | 72,513 | 1908.00 | 121 | 241 | 184 | 80 |
| 5/11/2018 | 72,277 | 1907.90 | 97 | 240 | 183 | 80 |
| 5/12/2018 | 72,063 | 1907.80 | 102 | 230 | 178 | 80 |
| 5/13/2018 | 71,828 | 1907.60 | 89 | 213 | 167 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/14/2018 | 71,635 | 1907.60 | 89 | 203 | 157 | 80 |
| 5/15/2018 | 71,400 | 1907.40 | 84 | 225 | 164 | 80 |
| 5/16/2018 | 71,144 | 1907.30 | 101 | 250 | 191 | 79 |
| 5/17/2018 | 70,889 | 1907.20 | 101 | 241 | 184 | 79 |
| 5/18/2018 | 70,635 | 1907.10 | 94 | 235 | 180 | 80 |
| 5/19/2018 | 70,360 | 1907.00 | 76 | 231 | 174 | 81 |
| 5/20/2018 | 70,085 | 1906.80 | 68 | 230 | 176 | 81 |
| 5/21/2018 | 69,791 | 1906.70 | 74 | 227 | 173 | 81 |
| 5/22/2018 | 69,538 | 1906.60 | 74 | 222 | 171 | 81 |
| 5/23/2018 | 69,285 | 1906.40 | 70 | 210 | 163 | 81 |
| 5/24/2018 | 69,054 | 1906.30 | 64 | 202 | 156 | 81 |
| 5/25/2018 | 68,844 | 1906.20 | 79 | 198 | 156 | 81 |
| 5/26/2018 | 68,656 | 1906.10 | 89 | 194 | 154 | 80 |
| 5/27/2018 | 68,447 | 1906.00 | 77 | 193 | 152 | 80 |
| 5/28/2018 | 68,217 | 1905.90 | 58 | 187 | 149 | 80 |
| 5/29/2018 | 67,926 | 1905.80 | 49 | 208 | 155 | 80 |
| 5/30/2018 | 67,555 | 1905.60 | 35 | 224 | 172 | 80 |
| 5/31/2018 | 67,225 | 1905.50 | 23 | 225 | 173 | 81 |
| 6/1/2018 | 66,917 | 1905.30 | 33 | 213 | 166 | 81 |
| 6/2/2018 | 66,610 | 1905.20 | 38 | 209 | 157 | 80 |
| 6/3/2018 | 66,304 | 1905.00 | 27 | 205 | 156 | 80 |
| 6/4/2018 | 65,999 | 1904.90 | 14 | 197 | 146 | 80 |
| 6/5/2018 | 65,695 | 1904.70 | 20 | 186 | 140 | 80 |
| 6/6/2018 | 65,432 | 1904.60 | 17 | 180 | 131 | 80 |
| 6/7/2018 | 65,128 | 1904.40 | 7 | 179 | 129 | 80 |
| 6/8/2018 | 64,846 | 1904.30 | 10 | 172 | 123 | 80 |
| 6/9/2018 | 64,544 | 1904.20 | 1 | 168 | 115 | 80 |
| 6/10/2018 | 64,283 | 1904.00 | 6 | 163 | 110 | 81 |
| 6/11/2018 | 64,043 | 1903.90 | 15 | 161 | 104 | 80 |
| 6/12/2018 | 63,804 | 1903.80 | 14 | 159 | 101 | 80 |
| 6/13/2018 | 63,566 | 1903.70 | 4 | 149 | 90 | 80 |
| 6/14/2018 | 63,347 | 1903.60 | 3 | 141 | 77 | 80 |
| 6/15/2018 | 63,129 | 1903.50 | 0 | 137 | 71 | 81 |
| 6/16/2018 | 62,911 | 1903.30 | -4 | 128 | 62 | 81 |
| 6/17/2018 | 62,714 | 1903.20 | -3 | 120 | 51 | 81 |
| 6/18/2018 | 62,537 | 1903.20 | 1 | 118 | 48 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/19/2018 | 62,359 | 1903.10 | 29 | 121 | 49 | 80 |
| 6/20/2018 | 62,140 | 1903.00 | -5 | 121 | 48 | 80 |
| 6/21/2018 | 61,941 | 1902.90 | 5 | 112 | 45 | 80 |
| 6/22/2018 | 61,803 | 1902.80 | 8 | 100 | 33 | 80 |
| 6/23/2018 | 61,644 | 1902.70 | 6 | 96 | 28 | 80 |
| 6/24/2018 | 61,447 | 1902.60 | 0 | 94 | 23 | 80 |
| 6/25/2018 | 61,288 | 1902.50 | -3 | 94 | 20 | 81 |
| 6/26/2018 | 61,130 | 1902.40 | 0 | 93 | 20 | 81 |
| 6/27/2018 | 60,933 | 1902.30 | -4 | 95 | 19 | 81 |
| 6/28/2018 | 60,737 | 1902.20 | -11 | 96 | 22 | 81 |
| 6/29/2018 | 60,580 | 1902.20 | -6 | 94 | 21 | 81 |
| 6/30/2018 | 60,384 | 1902.10 | -6 | 93 | 18 | 80 |
| 7/1/2018 | 60,208 | 1902.00 | -6 | 93 | 17 | 80 |
| 7/2/2018 | 60,033 | 1901.90 | -6 | 94 | 17 | 80 |
| 7/3/2018 | 59,838 | 1901.80 | -13 | 94 | 16 | 80 |
| 7/4/2018 | 59,644 | 1901.70 | -13 | 95 | 16 | 79 |
| 7/5/2018 | 59,450 | 1901.60 | -12 | 92 | 16 | 79 |
| 7/6/2018 | 59,295 | 1901.50 | -7 | 87 | 13 | 79 |
| 7/7/2018 | 59,120 | 1901.40 | -8 | 90 | 12 | 79 |
| 7/8/2018 | 58,947 | 1901.30 | -14 | 93 | 16 | 79 |
| 7/9/2018 | 58,735 | 1901.20 | -19 | 92 | 15 | 79 |
| 7/10/2018 | 58,542 | 1901.10 | -13 | 92 | 15 | 79 |
| 7/11/2018 | 58,370 | 1901.00 | -11 | 91 | 15 | 79 |
| 7/12/2018 | 58,178 | 1900.90 | -15 | 90 | 14 | 79 |
| 7/13/2018 | 58,006 | 1900.80 | -10 | 88 | 12 | 80 |
| 7/14/2018 | 57,835 | 1900.70 | -10 | 87 | 11 | 80 |
| 7/15/2018 | 57,645 | 1900.60 | -18 | 90 | 11 | 80 |
| 7/16/2018 | 57,454 | 1900.50 | -14 | 94 | 12 | 80 |
| 7/17/2018 | 57,245 | 1900.40 | -21 | 93 | 13 | 80 |
| 7/18/2018 | 57,036 | 1900.30 | -20 | 93 | 13 | 80 |
| 7/19/2018 | 56,846 | 1900.20 | -16 | 92 | 12 | 80 |
| 7/20/2018 | 56,637 | 1900.10 | -22 | 94 | 13 | 80 |
| 7/21/2018 | 56,430 | 1900.00 | -19 | 93 | 13 | 80 |
| 7/22/2018 | 56,222 | 1899.90 | -16 | 94 | 13 | 80 |
| 7/23/2018 | 56,016 | 1899.80 | -17 | 93 | 12 | 80 |
| 7/24/2018 | 55,810 | 1899.70 | -22 | 92 | 11 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/25/2018 | 55,604 | 1899.60 | -24 | 95 | 11 | 80 |
| 7/26/2018 | 55,380 | 1899.40 | -21 | 94 | 15 | 80 |
| 7/27/2018 | 55,156 | 1899.30 | -26 | 93 | 12 | 80 |
| 7/28/2018 | 54,952 | 1899.20 | -17 | 93 | 13 | 80 |
| 7/29/2018 | 54,748 | 1899.10 | -24 | 93 | 12 | 80 |
| 7/30/2018 | 54,527 | 1899.00 | -29 | 93 | 13 | 80 |
| 7/31/2018 | 54,306 | 1898.90 | -21 | 93 | 12 | 80 |
| 8/1/2018 | 54,104 | 1898.80 | -23 | 93 | 12 | 80 |
| 8/2/2018 | 53,884 | 1898.60 | -22 | 93 | 12 | 80 |
| 8/3/2018 | 53,681 | 1898.50 | -25 | 92 | 11 | 81 |
| 8/4/2018 | 53,461 | 1898.40 | -28 | 92 | 10 | 81 |
| 8/5/2018 | 53,242 | 1898.30 | -22 | 91 | 10 | 81 |
| 8/6/2018 | 53,023 | 1898.20 | -26 | 94 | 11 | 81 |
| 8/7/2018 | 52,823 | 1898.00 | -17 | 95 | 15 | 81 |
| 8/8/2018 | 52,623 | 1897.90 | -7 | 94 | 13 | 81 |
| 8/9/2018 | 52,442 | 1897.80 | -8 | 94 | 13 | 81 |
| 8/10/2018 | 52,244 | 1897.70 | -15 | 94 | 12 | 81 |
| 8/11/2018 | 52,045 | 1897.60 | -16 | 94 | 12 | 81 |
| 8/12/2018 | 51,846 | 1897.50 | -11 | 94 | 12 | 81 |
| 8/13/2018 | 51,647 | 1897.40 | -12 | 94 | 12 | 81 |
| 8/14/2018 | 51,449 | 1897.30 | -11 | 94 | 12 | 81 |
| 8/15/2018 | 51,251 | 1897.20 | -15 | 94 | 13 | 81 |
| 8/16/2018 | 51,054 | 1897.10 | -10 | 94 | 12 | 81 |
| 8/17/2018 | 50,858 | 1897.00 | -13 | 93 | 11 | 81 |
| 8/18/2018 | 50,680 | 1896.90 | -9 | 92 | 11 | 81 |
| 8/19/2018 | 50,484 | 1896.70 | -12 | 92 | 11 | 81 |
| 8/20/2018 | 50,289 | 1896.60 | -9 | 91 | 11 | 81 |
| 8/21/2018 | 50,129 | 1896.50 | -7 | 91 | 11 | 81 |
| 8/22/2018 | 49,933 | 1896.40 | -5 | 90 | 11 | 81 |
| 8/23/2018 | 49,756 | 1896.30 | -12 | 88 | 9 | 80 |
| 8/24/2018 | 49,579 | 1896.20 | -6 | 89 | 10 | 80 |
| 8/25/2018 | 49,385 | 1896.10 | -7 | 91 | 10 | 80 |
| 8/26/2018 | 49,192 | 1896.00 | -7 | 94 | 13 | 81 |
| 8/27/2018 | 48,999 | 1895.90 | -9 | 93 | 13 | 81 |
| 8/28/2018 | 48,842 | 1895.80 | -5 | 91 | 10 | 81 |
| 8/29/2018 | 48,650 | 1895.70 | -8 | 91 | 11 | 81 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/30/2018 | 48,458 | 1895.60 | -11 | 91 | 11 | 80 |
| 8/31/2018 | 48,283 | 1895.50 | -6 | 91 | 10 | 80 |
| 9/1/2018 | 48,092 | 1895.40 | -5 | 91 | 10 | 80 |
| 9/2/2018 | 47,901 | 1895.30 | -8 | 91 | 10 | 80 |
| 9/3/2018 | 47,710 | 1895.20 | -12 | 91 | 10 | 80 |
| 9/4/2018 | 47,538 | 1895.10 | -6 | 90 | 10 | 80 |
| 9/5/2018 | 47,331 | 1894.90 | -13 | 90 | 10 | 80 |
| 9/6/2018 | 47,142 | 1894.80 | -12 | 89 | 10 | 80 |
| 9/7/2018 | 46,937 | 1894.70 | -12 | 92 | 10 | 80 |
| 9/8/2018 | 46,766 | 1894.60 | -7 | 94 | 14 | 80 |
| 9/9/2018 | 46,560 | 1894.50 | -11 | 93 | 12 | 80 |
| 9/10/2018 | 46,372 | 1894.40 | -15 | 92 | 12 | 80 |
| 9/11/2018 | 46,134 | 1894.20 | -14 | 92 | 12 | 80 |
| 9/12/2018 | 45,947 | 1894.10 | -45 | 92 | 12 | 81 |
| 9/13/2018 | 45,760 | 1894.00 | -17 | 91 | 12 | 81 |
| 9/14/2018 | 45,557 | 1893.90 | -14 | 91 | 12 | 80 |
| 9/15/2018 | 45,338 | 1893.80 | -17 | 96 | 12 | 81 |
| 9/16/2018 | 45,152 | 1893.60 | -12 | 103 | 13 | 90 |
| 9/17/2018 | 44,933 | 1893.50 | -12 | 103 | 13 | 90 |
| 9/18/2018 | 44,731 | 1893.40 | -9 | 103 | 12 | 90 |
| 9/19/2018 | 44,513 | 1893.30 | -10 | 106 | 12 | 90 |
| 9/20/2018 | 44,329 | 1893.20 | -5 | 106 | 12 | 90 |
| 9/21/2018 | 44,113 | 1893.00 | -10 | 106 | 13 | 90 |
| 9/22/2018 | 43,897 | 1892.90 | -11 | 106 | 12 | 90 |
| 9/23/2018 | 43,682 | 1892.80 | -6 | 106 | 11 | 90 |
| 9/24/2018 | 43,484 | 1892.60 | -9 | 106 | 11 | 90 |
| 9/25/2018 | 43,269 | 1892.50 | -6 | 106 | 11 | 90 |
| 9/26/2018 | 43,071 | 1892.40 | -8 | 106 | 11 | 90 |
| 9/27/2018 | 42,874 | 1892.30 | -5 | 106 | 11 | 90 |
| 9/28/2018 | 42,662 | 1892.10 | -8 | 106 | 11 | 90 |
| 9/29/2018 | 42,450 | 1892.00 | -6 | 107 | 11 | 90 |
| 9/30/2018 | 42,271 | 1891.90 | 4 | 107 | 13 | 90 |
| 10/1/2018 | 42,108 | 1891.80 | 15 | 102 | 12 | 90 |
| 10/2/2018 | 42,010 | 1891.70 | 56 | 104 | 18 | 90 |
| 10/3/2018 | 41,929 | 1891.70 | 62 | 106 | 22 | 90 |
| 10/4/2018 | 41,767 | 1891.60 | 25 | 106 | 20 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/5/2018 | 41,573 | 1891.50 | 11 | 115 | 20 | 90 |
| 10/6/2018 | 41,348 | 1891.30 | 5 | 121 | 31 | 90 |
| 10/7/2018 | 41,124 | 1891.20 | 3 | 119 | 29 | 90 |
| 10/8/2018 | 40,916 | 1891.00 | 8 | 120 | 29 | 91 |
| 10/9/2018 | 40,694 | 1890.90 | 6 | 122 | 30 | 90 |
| 10/10/2018 | 40,488 | 1890.80 | -18 | 122 | 31 | 90 |
| 10/11/2018 | 40,269 | 1890.60 | -4 | 116 | 29 | 91 |
| 10/12/2018 | 40,034 | 1890.50 | -3 | 115 | 30 | 90 |
| 10/13/2018 | 39,800 | 1890.30 | -4 | 117 | 33 | 90 |
| 10/14/2018 | 39,567 | 1890.20 | -3 | 116 | 32 | 90 |
| 10/15/2018 | 39,303 | 1890.00 | -24 | 117 | 33 | 90 |
| 10/16/2018 | 39,134 | 1889.90 | -2 | 94 | 38 | 75 |
| 10/17/2018 | 38,980 | 1889.80 | -8 | 71 | 38 | 45 |
| 10/18/2018 | 38,842 | 1889.70 | -6 | 71 | 36 | 45 |
| 10/19/2018 | 38,735 | 1889.60 | -5 | 71 | 37 | 45 |
| 10/20/2018 | 38,551 | 1889.50 | -4 | 72 | 37 | 45 |
| 10/21/2018 | 38,398 | 1889.40 | -4 | 72 | 37 | 45 |
| 10/22/2018 | 38,261 | 1889.30 | -6 | 73 | 37 | 45 |
| 10/23/2018 | 38,110 | 1889.20 | -3 | 73 | 37 | 46 |
| 10/24/2018 | 37,958 | 1889.10 | -2 | 75 | 33 | 47 |
| 10/25/2018 | 37,837 | 1889.00 | 3 | 76 | 34 | 47 |
| 10/26/2018 | 37,682 | 1888.90 | 3 | 77 | 34 | 47 |
| 10/27/2018 | 37,540 | 1888.90 | 5 | 78 | 32 | 47 |
| 10/28/2018 | 37,398 | 1888.80 | 1 | 80 | 35 | 47 |
| 10/29/2018 | 37,227 | 1888.60 | 2 | 80 | 34 | 47 |
| 10/30/2018 | 37,056 | 1888.50 | 8 | 91 | 34 | 48 |
| 10/31/2018 | 36,901 | 1888.40 | 15 | 110 | 49 | 54 |
| 11/1/2018 | 36,701 | 1888.30 | 18 | 112 | 38 | 65 |
| 11/2/2018 | 36,502 | 1888.20 | 14 | 111 | 35 | 68 |
| 11/3/2018 | 36,304 | 1888.00 | 10 | 112 | 35 | 68 |
| 11/4/2018 | 36,107 | 1887.90 | 9 | 112 | 34 | 68 |
| 11/5/2018 | 35,896 | 1887.70 | 11 | 117 | 35 | 68 |
| 11/6/2018 | 35,704 | 1887.60 | -24 | 124 | 42 | 78 |
| 11/7/2018 | 35,472 | 1887.50 | -2 | 113 | 42 | 73 |
| 11/8/2018 | 35,256 | 1887.30 | -5 | 113 | 43 | 76 |
| 11/9/2018 | 35,040 | 1887.20 | -1 | 114 | 37 | 83 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/10/2018 | 34,840 | 1887.00 | 0 | 113 | 34 | 85 |
| 11/11/2018 | 34,641 | 1886.90 | 1 | 112 | 34 | 85 |
| 11/12/2018 | 34,414 | 1886.70 | 2 | 112 | 34 | 85 |
| 11/13/2018 | 34,202 | 1886.60 | 2 | 112 | 35 | 84 |
| 11/14/2018 | 33,991 | 1886.40 | 5 | 112 | 35 | 84 |
| 11/15/2018 | 33,795 | 1886.30 | 5 | 112 | 35 | 84 |
| 11/16/2018 | 33,600 | 1886.10 | 4 | 112 | 34 | 84 |
| 11/17/2018 | 33,392 | 1886.00 | 4 | 112 | 33 | 84 |
| 11/18/2018 | 33,184 | 1885.80 | 4 | 113 | 34 | 84 |
| 11/19/2018 | 32,978 | 1885.70 | 4 | 114 | 39 | 81 |
| 11/20/2018 | 32,772 | 1885.50 | 4 | 108 | 38 | 77 |
| 11/21/2018 | 32,662 | 1885.40 | 38 | 104 | 40 | 73 |
| 11/22/2018 | 32,662 | 1885.40 | 100 | 103 | 45 | 73 |
| 11/23/2018 | 33,350 | 1885.90 | 439 | 102 | 80 | 73 |
| 11/24/2018 | 33,809 | 1886.30 | 386 | 115 | 100 | 71 |
| 11/25/2018 | 33,670 | 1886.20 | 87 | 154 | 88 | 70 |
| 11/26/2018 | 33,392 | 1886.00 | 44 | 190 | 135 | 71 |
| 11/27/2018 | 33,309 | 1885.90 | 142 | 201 | 175 | 71 |
| 11/28/2018 | 33,253 | 1885.90 | 181 | 203 | 181 | 70 |
| 11/29/2018 | 34,926 | 1887.10 | 1,082 | 216 | 319 | 71 |
| 11/30/2018 | 35,098 | 1887.20 | 333 | 219 | 208 | 71 |
| 12/1/2018 | 35,098 | 1887.20 | 222 | 218 | 200 | 71 |
| 12/2/2018 | 34,954 | 1887.10 | 141 | 217 | 187 | 71 |
| 12/3/2018 | 34,698 | 1886.90 | 88 | 216 | 176 | 71 |
| 12/4/2018 | 34,428 | 1886.70 | 69 | 216 | 171 | 71 |
| 12/5/2018 | 34,146 | 1886.50 | 62 | 215 | 169 | 71 |
| 12/6/2018 | 33,837 | 1886.30 | 52 | 215 | 167 | 71 |
| 12/7/2018 | 33,517 | 1886.10 | 45 | 215 | 165 | 71 |
| 12/8/2018 | 33,212 | 1885.80 | 48 | 208 | 161 | 71 |
| 12/9/2018 | 32,950 | 1885.70 | 44 | 189 | 148 | 71 |
| 12/10/2018 | 32,704 | 1885.50 | 20 | 176 | 134 | 70 |
| 12/11/2018 | 32,471 | 1885.30 | 42 | 174 | 130 | 71 |
| 12/12/2018 | 32,186 | 1885.10 | 39 | 174 | 129 | 71 |
| 12/13/2018 | 31,944 | 1884.90 | 45 | 173 | 128 | 71 |
| 12/14/2018 | 31,729 | 1884.80 | 60 | 175 | 130 | 71 |
| 12/15/2018 | 31,542 | 1884.60 | 75 | 174 | 132 | 71 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/16/2018 | 33,725 | 1886.20 | 1,171 | 176 | 263 | 71 |
| 12/17/2018 | 35,299 | 1887.30 | 1,149 | 178 | 280 | 71 |
| 12/18/2018 | 35,777 | 1887.70 | 444 | 198 | 198 | 71 |
| 12/19/2018 | 36,127 | 1887.90 | 420 | 216 | 264 | 70 |
| 12/20/2018 | 36,273 | 1888.00 | 290 | 220 | 202 | 71 |
| 12/21/2018 | 36,642 | 1888.20 | 416 | 230 | 240 | 70 |
| 12/22/2018 | 36,731 | 1888.30 | 270 | 228 | 197 | 71 |
| 12/23/2018 | 36,746 | 1888.30 | 232 | 229 | 186 | 71 |
| 12/24/2018 | 37,883 | 1889.10 | 760 | 231 | 338 | 71 |
| 12/25/2018 | 38,888 | 1889.70 | 801 | 232 | 353 | 71 |
| 12/26/2018 | 39,165 | 1889.90 | 386 | 232 | 237 | 71 |
| 12/27/2018 | 39,273 | 1890.00 | 274 | 232 | 201 | 71 |
| 12/28/2018 | 39,242 | 1890.00 | 209 | 230 | 188 | 71 |
| 12/29/2018 | 39,134 | 1889.90 | 173 | 228 | 180 | 71 |
| 12/30/2018 | 39,011 | 1889.80 | 148 | 225 | 173 | 71 |
| 12/31/2018 | 38,857 | 1889.70 | 133 | 223 | 168 | 71 |
| 1/1/2019 | 38,674 | 1889.60 | 111 | 222 | 166 | 71 |
| 1/2/2019 | 38,460 | 1889.50 | 105 | 221 | 164 | 71 |
| 1/3/2019 | 38,261 | 1889.30 | 100 | 221 | 162 | 71 |
| 1/4/2019 | 38,049 | 1889.20 | 94 | 216 | 159 | 71 |
| 1/5/2019 | 38,155 | 1889.30 | 244 | 209 | 171 | 71 |
| 1/6/2019 | 40,238 | 1890.60 | 1,231 | 212 | 464 | 70 |
| 1/7/2019 | 41,374 | 1891.30 | 879 | 214 | 429 | 70 |
| 1/8/2019 | 42,918 | 1892.30 | 963 | 216 | 347 | 70 |
| 1/9/2019 | 50,976 | 1897.00 | 4,382 | 228 | 728 | 70 |
| 1/10/2019 | 54,577 | 1899.00 | 2,235 | 227 | 565 | 70 |
| 1/11/2019 | 56,340 | 1900.00 | 1,180 | 229 | 387 | 70 |
| 1/12/2019 | 57,245 | 1900.40 | 1,002 | 543 | 597 | 70 |
| 1/13/2019 | 57,227 | 1900.40 | 716 | 753 | 866 | 70 |
| 1/14/2019 | 57,075 | 1900.30 | 550 | 662 | 781 | 70 |
| 1/15/2019 | 57,132 | 1900.40 | 606 | 605 | 688 | 70 |
| 1/16/2019 | 62,898 | 1903.30 | 4,183 | 1,911 | 1,690 | 69 |
| 1/17/2019 | 62,001 | 1902.90 | 8,364 | 8,797 | 10,266 | 0 |
| 1/18/2019 | 60,033 | 1901.90 | 3,355 | 4,533 | 5,878 | 34 |
| 1/19/2019 | 60,072 | 1901.90 | 3,324 | 3,489 | 4,295 | 46 |
| 1/20/2019 | 61,842 | 1902.80 | 5,161 | 4,592 | 5,780 | 35 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/21/2019 | 60,130 | 1901.90 | 3,933 | 4,916 | 6,441 | 39 |
| 1/22/2019 | 58,966 | 1901.30 | 2,161 | 2,868 | 3,582 | 59 |
| 1/23/2019 | 58,331 | 1901.00 | 1,519 | 1,935 | 2,248 | 62 |
| 1/24/2019 | 57,930 | 1900.80 | 1,171 | 1,434 | 1,772 | 48 |
| 1/25/2019 | 57,645 | 1900.60 | 916 | 1,120 | 1,476 | 58 |
| 1/26/2019 | 57,378 | 1900.50 | 740 | 911 | 1,238 | 81 |
| 1/27/2019 | 57,227 | 1900.40 | 631 | 747 | 1,056 | 81 |
| 1/28/2019 | 57,113 | 1900.40 | 566 | 653 | 926 | 81 |
| 1/29/2019 | 56,962 | 1900.30 | 526 | 637 | 824 | 127 |
| 1/30/2019 | 56,848 | 1900.20 | 488 | 583 | 720 | 155 |
| 1/31/2019 | 56,735 | 1900.20 | 450 | 526 | 593 | 132 |
| 2/1/2019 | 57,695 | 1900.70 | 474 | 491 | 521 | 83 |
| 2/2/2019 | 58,031 | 1900.90 | 1,664 | 1,065 | 1,022 | 83 |
| 2/3/2019 | 58,368 | 1901.00 | 1,415 | 1,473 | 1,489 | 82 |
| 2/4/2019 | 58,446 | 1901.10 | 1,958 | 1,881 | 1,841 | 83 |
| 2/5/2019 | 57,930 | 1900.80 | 1,269 | 1,606 | 1,665 | 86 |
| 2/6/2019 | 57,549 | 1900.60 | 951 | 1,186 | 1,305 | 86 |
| 2/7/2019 | 57,359 | 1900.50 | 803 | 953 | 1,070 | 84 |
| 2/8/2019 | 57,264 | 1900.40 | 714 | 803 | 905 | 82 |
| 2/9/2019 | 57,702 | 1900.70 | 970 | 836 | 1,017 | 83 |
| 2/10/2019 | 57,473 | 1900.60 | 938 | 1,057 | 1,246 | 83 |
| 2/11/2019 | 57,264 | 1900.40 | 700 | 852 | 1,025 | 82 |
| 2/12/2019 | 57,378 | 1900.50 | 764 | 748 | 902 | 84 |
| 2/13/2019 | 61,585 | 1902.70 | 4,639 | 2,846 | 3,537 | 84 |
| 2/14/2019 | 64,147 | 1904.00 | 12,640 | 11,958 | 13,387 | 1 |
| 2/15/2019 | 61,190 | 1902.50 | 5,433 | 7,146 | 9,185 | 1 |
| 2/16/2019 | 59,857 | 1901.80 | 3,056 | 3,891 | 5,137 | 43 |
| 2/17/2019 | 59,043 | 1901.40 | 2,211 | 2,744 | 3,406 | 46 |
| 2/18/2019 | 58,504 | 1901.10 | 1,691 | 2,068 | 2,317 | 47 |
| 2/19/2019 | 58,197 | 1900.90 | 1,394 | 1,617 | 1,822 | 46 |
| 2/20/2019 | 58,045 | 1900.90 | 1,243 | 1,396 | 1,572 | 46 |
| 2/21/2019 | 57,854 | 1900.80 | 1,091 | 1,246 | 1,397 | 46 |
| 2/22/2019 | 57,664 | 1900.70 | 929 | 1,073 | 1,237 | 46 |
| 2/23/2019 | 57,530 | 1900.60 | 823 | 940 | 1,075 | 66 |
| 2/24/2019 | 57,530 | 1900.60 | 828 | 873 | 980 | 79 |
| 2/25/2019 | 65,041 | 1904.40 | 5,884 | 2,846 | 3,632 | 63 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/26/2019 | 71,166 | 1907.30 | 20,419 | 18,769 | 19,130 | 1 |
| 2/27/2019 | 65,675 | 1904.70 | 23,398 | 26,638 | 27,437 | 1 |
| 2/28/2019 | 61,981 | 1902.90 | 6,512 | 8,666 | 10,897 | 1 |
| 3/1/2019 | 60,325 | 1902.00 | 3,614 | 4,623 | 6,553 | 29 |
| 3/2/2019 | 60,150 | 1901.90 | 3,329 | 3,635 | 5,041 | 45 |
| 3/3/2019 | 59,994 | 1901.90 | 3,042 | 3,321 | 4,362 | 45 |
| 3/4/2019 | 59,557 | 1901.60 | 2,715 | 3,088 | 3,890 | 45 |
| 3/5/2019 | 59,411 | 1901.60 | 2,428 | 2,674 | 3,171 | 45 |
| 3/6/2019 | 61,269 | 1902.50 | 4,713 | 4,067 | 4,620 | 45 |
| 3/7/2019 | 60,345 | 1902.00 | 3,629 | 4,290 | 5,046 | 45 |
| 3/8/2019 | 59,566 | 1901.70 | 2,670 | 3,253 | 3,866 | 46 |
| 3/9/2019 | 59,256 | 1901.50 | 2,306 | 2,651 | 3,037 | 46 |
| 3/10/2019 | 58,927 | 1901.30 | 2,106 | 2,429 | 2,725 | 46 |
| 3/11/2019 | 58,581 | 1901.10 | 1,721 | 2,053 | 2,219 | 46 |
| 3/12/2019 | 58,327 | 1901.00 | 1,494 | 1,767 | 1,909 | 46 |
| 3/13/2019 | 58,083 | 1900.90 | 1,267 | 1,518 | 1,708 | 45 |
| 3/14/2019 | 57,892 | 1900.80 | 1,120 | 1,332 | 1,524 | 45 |
| 3/15/2019 | 57,778 | 1900.70 | 1,023 | 1,207 | 1,361 | 44 |
| 3/16/2019 | 57,721 | 1900.70 | 982 | 1,132 | 1,242 | 44 |
| 3/17/2019 | 57,683 | 1900.70 | 972 | 1,104 | 1,188 | 45 |
| 3/18/2019 | 57,703 | 1900.70 | 991 | 1,108 | 1,228 | 47 |
| 3/19/2019 | 57,726 | 1900.70 | 999 | 1,118 | 1,264 | 46 |
| 3/20/2019 | 58,282 | 1901.00 | 1,458 | 1,289 | 1,478 | 45 |
| 3/21/2019 | 58,170 | 1900.90 | 1,461 | 1,579 | 1,636 | 46 |
| 3/22/2019 | 58,653 | 1901.20 | 1,643 | 1,546 | 1,616 | 46 |
| 3/23/2019 | 58,522 | 1901.10 | 1,877 | 2,020 | 1,942 | 45 |
| 3/24/2019 | 58,218 | 1901.00 | 1,428 | 1,686 | 1,722 | 45 |
| 3/25/2019 | 59,280 | 1901.50 | 2,268 | 1,887 | 1,892 | 45 |
| 3/26/2019 | 58,878 | 1901.30 | 2,109 | 2,417 | 2,374 | 44 |
| 3/27/2019 | 60,729 | 1902.20 | 4,509 | 3,576 | 3,824 | 44 |
| 3/28/2019 | 60,281 | 1902.00 | 3,322 | 3,774 | 4,419 | 44 |
| 3/29/2019 | 59,506 | 1901.60 | 2,669 | 3,199 | 3,698 | 44 |
| 3/30/2019 | 58,987 | 1901.40 | 2,121 | 2,526 | 2,797 | 44 |
| 3/31/2019 | 58,624 | 1901.20 | 1,789 | 2,085 | 2,252 | 44 |
| 4/1/2019 | 58,512 | 1901.10 | 1,652 | 1,822 | 1,959 | 44 |
| 4/2/2019 | 59,170 | 1901.40 | 1,745 | 1,499 | 1,733 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/3/2019 | 59,523 | 1901.60 | 1,568 | 1,460 | 1,590 | 44 |
| 4/4/2019 | 59,448 | 1901.60 | 1,372 | 1,511 | 1,605 | 44 |
| 4/5/2019 | 59,801 | 1901.80 | 1,609 | 1,533 | 1,640 | 44 |
| 4/6/2019 | 60,840 | 1902.30 | 1,625 | 1,128 | 1,425 | 44 |
| 4/7/2019 | 62,547 | 1903.20 | 1,659 | 808 | 1,045 | 44 |
| 4/8/2019 | 65,626 | 1904.70 | 2,387 | 922 | 1,297 | 44 |
| 4/9/2019 | 69,406 | 1906.50 | 2,854 | 841 | 1,258 | 44 |
| 4/10/2019 | 71,292 | 1907.40 | 1,970 | 1,014 | 1,317 | 45 |
| 4/11/2019 | 72,515 | 1908.00 | 1,623 | 1,030 | 1,289 | 45 |
| 4/12/2019 | 73,273 | 1908.30 | 1,387 | 1,048 | 1,266 | 45 |
| 4/13/2019 | 73,667 | 1908.50 | 1,251 | 1,099 | 1,279 | 45 |
| 4/14/2019 | 73,671 | 1908.50 | 1,160 | 1,259 | 1,356 | 45 |
| 4/15/2019 | 73,414 | 1908.40 | 1,128 | 1,339 | 1,416 | 73 |
| 4/16/2019 | 73,439 | 1908.40 | 1,084 | 1,135 | 1,288 | 90 |
| 4/17/2019 | 73,856 | 1908.60 | 967 | 727 | 877 | 90 |
| 4/18/2019 | 74,539 | 1908.90 | 871 | 533 | 597 | 90 |
| 4/19/2019 | 75,072 | 1909.10 | 898 | 666 | 731 | 90 |
| 4/20/2019 | 75,297 | 1909.20 | 880 | 791 | 816 | 90 |
| 4/21/2019 | 75,322 | 1909.30 | 800 | 839 | 826 | 90 |
| 4/22/2019 | 75,304 | 1909.20 | 738 | 795 | 855 | 90 |
| 4/23/2019 | 75,285 | 1909.20 | 720 | 784 | 798 | 90 |
| 4/24/2019 | 75,311 | 1909.30 | 753 | 799 | 801 | 90 |
| 4/25/2019 | 75,347 | 1909.30 | 731 | 748 | 791 | 90 |
| 4/26/2019 | 75,295 | 1909.20 | 669 | 736 | 718 | 90 |
| 4/27/2019 | 75,321 | 1909.30 | 627 | 660 | 698 | 90 |
| 4/28/2019 | 75,302 | 1909.20 | 601 | 625 | 627 | 90 |
| 4/29/2019 | 75,283 | 1909.20 | 576 | 612 | 618 | 90 |
| 4/30/2019 | 75,242 | 1909.20 | 526 | 572 | 586 | 91 |
| 5/1/2019 | 75,290 | 1909.20 | 461 | 466 | 461 | 91 |
| 5/2/2019 | 75,271 | 1909.20 | 418 | 454 | 413 | 91 |
| 5/3/2019 | 75,319 | 1909.30 | 402 | 420 | 383 | 91 |
| 5/4/2019 | 75,345 | 1909.30 | 387 | 402 | 342 | 91 |
| 5/5/2019 | 75,326 | 1909.30 | 371 | 427 | 362 | 91 |
| 5/6/2019 | 75,330 | 1909.30 | 364 | 398 | 334 | 91 |
| 5/7/2019 | 75,377 | 1909.30 | 366 | 376 | 294 | 91 |
| 5/8/2019 | 75,359 | 1909.30 | 362 | 403 | 308 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/9/2019 | 75,318 | 1909.30 | 348 | 406 | 324 | 91 |
| 5/10/2019 | 75,277 | 1909.20 | 319 | 382 | 288 | 90 |
| 5/11/2019 | 75,192 | 1909.20 | 297 | 379 | 276 | 90 |
| 5/12/2019 | 75,062 | 1909.10 | 279 | 381 | 275 | 90 |
| 5/13/2019 | 74,931 | 1909.10 | 318 | 379 | 272 | 90 |
| 5/14/2019 | 74,820 | 1909.00 | 263 | 380 | 251 | 110 |
| 5/15/2019 | 74,953 | 1909.10 | 512 | 494 | 272 | 130 |
| 5/16/2019 | 74,776 | 1909.00 | 1,485 | 1,715 | 1,507 | 130 |
| 5/17/2019 | 74,512 | 1908.90 | 1,195 | 1,407 | 1,448 | 129 |
| 5/18/2019 | 74,776 | 1909.00 | 1,103 | 1,047 | 1,048 | 130 |
| 5/19/2019 | 75,041 | 1909.10 | 1,447 | 1,381 | 1,361 | 130 |
| 5/20/2019 | 74,887 | 1909.10 | 1,022 | 1,168 | 1,157 | 130 |
| 5/21/2019 | 74,931 | 1909.10 | 1,154 | 1,217 | 1,273 | 131 |
| 5/22/2019 | 74,931 | 1909.10 | 1,147 | 1,214 | 1,362 | 130 |
| 5/23/2019 | 75,063 | 1909.10 | 910 | 877 | 960 | 130 |
| 5/24/2019 | 75,108 | 1909.20 | 796 | 823 | 863 | 130 |
| 5/25/2019 | 75,285 | 1909.20 | 736 | 674 | 737 | 130 |
| 5/26/2019 | 75,285 | 1909.20 | 651 | 675 | 677 | 130 |
| 5/27/2019 | 75,241 | 1909.20 | 572 | 636 | 653 | 130 |
| 5/28/2019 | 75,285 | 1909.20 | 517 | 530 | 542 | 131 |
| 5/29/2019 | 75,374 | 1909.30 | 530 | 531 | 482 | 135 |
| 5/30/2019 | 75,440 | 1909.30 | 637 | 657 | 642 | 131 |
| 5/31/2019 | 75,285 | 1909.20 | 603 | 695 | 693 | 131 |
| 6/1/2019 | 75,263 | 1909.20 | 480 | 525 | 501 | 131 |
| 6/2/2019 | 75,241 | 1909.20 | 484 | 540 | 454 | 131 |
| 6/3/2019 | 75,087 | 1909.20 | 422 | 541 | 521 | 131 |
| 6/4/2019 | 75,155 | 1909.20 | 373 | 376 | 312 | 130 |
| 6/5/2019 | 75,268 | 1909.20 | 329 | 316 | 191 | 130 |
| 6/6/2019 | 75,203 | 1909.20 | 301 | 355 | 231 | 130 |
| 6/7/2019 | 75,250 | 1909.20 | 270 | 306 | 195 | 130 |
| 6/8/2019 | 75,340 | 1909.30 | 259 | 260 | 160 | 130 |
| 6/9/2019 | 75,364 | 1909.30 | 246 | 259 | 151 | 130 |
| 6/10/2019 | 75,389 | 1909.30 | 232 | 270 | 160 | 130 |
| 6/11/2019 | 75,391 | 1909.30 | 221 | 262 | 152 | 129 |
| 6/12/2019 | 75,415 | 1909.30 | 210 | 242 | 139 | 129 |
| 6/13/2019 | 75,394 | 1909.30 | 190 | 219 | 130 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/14/2019 | 75,396 | 1909.30 | 186 | 223 | 121 | 129 |
| 6/15/2019 | 75,421 | 1909.30 | 180 | 204 | 110 | 129 |
| 6/16/2019 | 75,400 | 1909.30 | 178 | 226 | 111 | 130 |
| 6/17/2019 | 75,336 | 1909.30 | 168 | 241 | 134 | 129 |
| 6/18/2019 | 75,293 | 1909.20 | 155 | 207 | 109 | 129 |
| 6/19/2019 | 75,251 | 1909.20 | 140 | 188 | 91 | 129 |
| 6/20/2019 | 75,187 | 1909.20 | 126 | 180 | 80 | 129 |
| 6/21/2019 | 75,122 | 1909.20 | 153 | 185 | 82 | 130 |
| 6/22/2019 | 75,014 | 1909.10 | 134 | 189 | 84 | 130 |
| 6/23/2019 | 74,905 | 1909.10 | 110 | 187 | 84 | 129 |
| 6/24/2019 | 74,819 | 1909.00 | 98 | 178 | 78 | 129 |
| 6/25/2019 | 74,710 | 1909.00 | 98 | 173 | 70 | 129 |
| 6/26/2019 | 74,602 | 1908.90 | 106 | 160 | 64 | 129 |
| 6/27/2019 | 74,494 | 1908.90 | 108 | 160 | 59 | 130 |
| 6/28/2019 | 74,408 | 1908.80 | 108 | 165 | 64 | 129 |
| 6/29/2019 | 74,300 | 1908.80 | 108 | 165 | 63 | 130 |
| 6/30/2019 | 74,170 | 1908.70 | 102 | 165 | 62 | 129 |
| 7/1/2019 | 74,041 | 1908.70 | 94 | 170 | 63 | 130 |
| 7/2/2019 | 73,911 | 1908.60 | 94 | 169 | 64 | 130 |
| 7/3/2019 | 73,761 | 1908.50 | 94 | 165 | 61 | 130 |
| 7/4/2019 | 73,676 | 1908.50 | 91 | 165 | 60 | 130 |
| 7/5/2019 | 73,504 | 1908.40 | 85 | 165 | 59 | 130 |
| 7/6/2019 | 73,311 | 1908.30 | 79 | 175 | 62 | 130 |
| 7/7/2019 | 73,118 | 1908.30 | 74 | 179 | 67 | 130 |
| 7/8/2019 | 72,926 | 1908.20 | 74 | 179 | 66 | 130 |
| 7/9/2019 | 72,734 | 1908.10 | 75 | 177 | 66 | 130 |
| 7/10/2019 | 72,564 | 1908.00 | 74 | 177 | 65 | 130 |
| 7/11/2019 | 72,351 | 1907.90 | 74 | 179 | 65 | 129 |
| 7/12/2019 | 72,160 | 1907.80 | 67 | 180 | 66 | 130 |
| 7/13/2019 | 71,948 | 1907.70 | 64 | 180 | 65 | 130 |
| 7/14/2019 | 71,714 | 1907.60 | 57 | 180 | 63 | 130 |
| 7/15/2019 | 71,502 | 1907.50 | 56 | 178 | 64 | 130 |
| 7/16/2019 | 71,248 | 1907.40 | 58 | 181 | 63 | 130 |
| 7/17/2019 | 71,037 | 1907.30 | 57 | 183 | 65 | 130 |
| 7/18/2019 | 70,806 | 1907.20 | 66 | 183 | 62 | 130 |
| 7/19/2019 | 70,617 | 1907.10 | 88 | 183 | 62 | 130 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/20/2019 | 70,323 | 1906.90 | 35 | 183 | 62 | 130 |
| 7/21/2019 | 70,071 | 1906.80 | 56 | 183 | 62 | 130 |
| 7/22/2019 | 69,821 | 1906.70 | 57 | 183 | 61 | 130 |
| 7/23/2019 | 69,570 | 1906.60 | 37 | 182 | 61 | 130 |
| 7/24/2019 | 69,319 | 1906.50 | 39 | 181 | 60 | 130 |
| 7/25/2019 | 69,047 | 1906.30 | 42 | 181 | 59 | 130 |
| 7/26/2019 | 68,735 | 1906.20 | 30 | 183 | 60 | 130 |
| 7/27/2019 | 68,465 | 1906.10 | 37 | 186 | 62 | 130 |
| 7/28/2019 | 68,196 | 1905.90 | 34 | 185 | 62 | 130 |
| 7/29/2019 | 67,885 | 1905.80 | 81 | 185 | 61 | 131 |
| 7/30/2019 | 67,595 | 1905.60 | 27 | 184 | 61 | 131 |
| 7/31/2019 | 67,366 | 1905.50 | 28 | 184 | 60 | 130 |
| 8/1/2019 | 67,033 | 1905.40 | 22 | 184 | 59 | 131 |
| 8/2/2019 | 66,744 | 1905.20 | 25 | 178 | 56 | 131 |
| 8/3/2019 | 66,455 | 1905.10 | 23 | 175 | 51 | 130 |
| 8/4/2019 | 66,167 | 1904.90 | 23 | 174 | 52 | 130 |
| 8/5/2019 | 65,900 | 1904.80 | 27 | 174 | 51 | 130 |
| 8/6/2019 | 65,634 | 1904.70 | 22 | 174 | 51 | 130 |
| 8/7/2019 | 65,327 | 1904.50 | 16 | 169 | 48 | 130 |
| 8/8/2019 | 65,041 | 1904.40 | 12 | 167 | 46 | 131 |
| 8/9/2019 | 64,755 | 1904.30 | 22 | 167 | 47 | 131 |
| 8/10/2019 | 64,491 | 1904.10 | 20 | 166 | 46 | 132 |
| 8/11/2019 | 64,207 | 1904.00 | 23 | 166 | 46 | 131 |
| 8/12/2019 | 63,945 | 1903.90 | 21 | 163 | 45 | 130 |
| 8/13/2019 | 63,663 | 1903.70 | 25 | 163 | 44 | 130 |
| 8/14/2019 | 63,421 | 1903.60 | 18 | 162 | 41 | 131 |
| 8/15/2019 | 63,139 | 1903.50 | 18 | 162 | 42 | 130 |
| 8/16/2019 | 62,838 | 1903.30 | 18 | 163 | 41 | 130 |
| 8/17/2019 | 62,578 | 1903.20 | 15 | 160 | 40 | 130 |
| 8/18/2019 | 62,299 | 1903.00 | 12 | 157 | 36 | 130 |
| 8/19/2019 | 62,021 | 1902.90 | 1 | 157 | 36 | 130 |
| 8/20/2019 | 61,743 | 1902.80 | 13 | 157 | 34 | 130 |
| 8/21/2019 | 61,486 | 1902.60 | 11 | 156 | 34 | 130 |
| 8/22/2019 | 61,209 | 1902.50 | 18 | 154 | 33 | 130 |
| 8/23/2019 | 60,933 | 1902.30 | 12 | 153 | 29 | 131 |
| 8/24/2019 | 60,658 | 1902.20 | 12 | 156 | 30 | 131 |

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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/25/2019 | 60,384 | 1902.10 | 11 | 157 | 30 | 131 |
| 8/26/2019 | 60,111 | 1901.90 | 13 | 157 | 30 | 131 |
| 8/27/2019 | 59,835 | 1901.80 | 7 | 152 | 27 | 131 |
| 8/28/2019 | 59,538 | 1901.60 | -1 | 153 | 24 | 131 |
| 8/29/2019 | 59,241 | 1901.50 | -5 | 155 | 28 | 131 |
| 8/30/2019 | 58,964 | 1901.30 | 0 | 155 | 28 | 131 |
| 8/31/2019 | 58,688 | 1901.20 | -3 | 155 | 28 | 131 |
| 9/1/2019 | 58,375 | 1901.00 | -3 | 154 | 27 | 131 |
| 9/2/2019 | 58,101 | 1900.90 | -4 | 154 | 27 | 131 |
| 9/3/2019 | 57,809 | 1900.70 | -4 | 155 | 27 | 131 |
| 9/4/2019 | 57,537 | 1900.60 | 1 | 154 | 26 | 131 |
| 9/5/2019 | 57,227 | 1900.40 | -7 | 155 | 26 | 131 |
| 9/6/2019 | 56,957 | 1900.30 | -6 | 154 | 26 | 130 |
| 9/7/2019 | 56,649 | 1900.10 | -12 | 155 | 27 | 131 |
| 9/8/2019 | 56,361 | 1900.00 | -9 | 155 | 27 | 131 |
| 9/9/2019 | 56,056 | 1899.80 | -1 | 176 | 32 | 138 |
| 9/10/2019 | 55,714 | 1899.60 | 1 | 181 | 30 | 151 |
| 9/11/2019 | 55,411 | 1899.50 | 6 | 181 | 28 | 153 |
| 9/12/2019 | 55,109 | 1899.30 | 7 | 174 | 37 | 137 |
| 9/13/2019 | 54,826 | 1899.10 | 1 | 162 | 33 | 131 |
| 9/14/2019 | 54,526 | 1899.00 | -1 | 158 | 25 | 132 |
| 9/15/2019 | 54,246 | 1898.80 | -6 | 159 | 25 | 132 |
| 9/16/2019 | 53,984 | 1898.70 | 11 | 160 | 26 | 134 |
| 9/17/2019 | 53,722 | 1898.50 | 9 | 159 | 24 | 134 |
| 9/18/2019 | 53,461 | 1898.40 | 15 | 160 | 25 | 134 |
| 9/19/2019 | 53,219 | 1898.30 | 11 | 159 | 26 | 134 |
| 9/20/2019 | 52,941 | 1898.10 | 7 | 160 | 26 | 133 |
| 9/21/2019 | 52,682 | 1898.00 | 7 | 160 | 27 | 132 |
| 9/22/2019 | 52,406 | 1897.80 | -1 | 160 | 25 | 132 |
| 9/23/2019 | 52,132 | 1897.70 | 0 | 158 | 25 | 132 |
| 9/24/2019 | 51,838 | 1897.50 | 2 | 158 | 25 | 132 |
| 9/25/2019 | 51,582 | 1897.40 | 0 | 158 | 24 | 132 |
| 9/26/2019 | 51,329 | 1897.20 | -7 | 146 | 40 | 110 |
| 9/27/2019 | 51,059 | 1897.10 | -14 | 133 | 46 | 91 |
| 9/28/2019 | 50,825 | 1896.90 | -20 | 125 | 41 | 91 |
| 9/29/2019 | 50,628 | 1896.80 | 4 | 121 | 38 | 92 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/30/2019 | 50,395 | 1896.70 | -6 | 120 | 37 | 91 |
| 10/1/2019 | 50,198 | 1896.60 | 21 | 121 | 38 | 90 |
| 10/2/2019 | 49,983 | 1896.50 | 12 | 120 | 37 | 90 |
| 10/3/2019 | 49,769 | 1896.30 | 13 | 121 | 37 | 90 |
| 10/4/2019 | 49,556 | 1896.20 | 14 | 121 | 37 | 90 |
| 10/5/2019 | 49,360 | 1896.10 | 21 | 120 | 36 | 89 |
| 10/6/2019 | 49,183 | 1896.00 | 29 | 118 | 34 | 88 |
| 10/7/2019 | 49,006 | 1895.90 | 29 | 118 | 32 | 90 |
| 10/8/2019 | 48,778 | 1895.80 | 3 | 119 | 30 | 92 |
| 10/9/2019 | 48,567 | 1895.60 | 13 | 119 | 30 | 92 |
| 10/10/2019 | 48,356 | 1895.50 | 18 | 124 | 32 | 92 |
| 10/11/2019 | 48,145 | 1895.40 | 20 | 126 | 37 | 92 |
| 10/12/2019 | 47,918 | 1895.30 | 11 | 126 | 38 | 91 |
| 10/13/2019 | 47,709 | 1895.20 | 20 | 126 | 39 | 91 |
| 10/14/2019 | 47,500 | 1895.00 | 21 | 126 | 38 | 91 |
| 10/15/2019 | 47,310 | 1894.90 | 29 | 125 | 39 | 90 |
| 10/16/2019 | 47,103 | 1894.80 | 16 | 120 | 41 | 84 |
| 10/17/2019 | 46,882 | 1894.70 | 11 | 123 | 46 | 80 |
| 10/18/2019 | 46,729 | 1894.60 | 37 | 115 | 42 | 80 |
| 10/19/2019 | 46,541 | 1894.50 | 16 | 110 | 34 | 80 |
| 10/20/2019 | 46,371 | 1894.40 | 25 | 111 | 34 | 80 |
| 10/21/2019 | 46,219 | 1894.30 | 34 | 111 | 34 | 80 |
| 10/22/2019 | 46,100 | 1894.20 | 39 | 99 | 34 | 71 |
| 10/23/2019 | 45,948 | 1894.10 | 29 | 106 | 38 | 65 |
| 10/24/2019 | 45,746 | 1894.00 | 10 | 112 | 33 | 80 |
| 10/25/2019 | 45,578 | 1893.90 | 27 | 112 | 33 | 81 |
| 10/26/2019 | 45,399 | 1893.80 | 21 | 111 | 34 | 80 |
| 10/27/2019 | 45,194 | 1893.70 | 9 | 112 | 34 | 80 |
| 10/28/2019 | 45,010 | 1893.60 | 19 | 111 | 34 | 80 |
| 10/29/2019 | 44,843 | 1893.50 | 27 | 111 | 35 | 80 |
| 10/30/2019 | 44,659 | 1893.30 | 18 | 111 | 35 | 80 |
| 10/31/2019 | 44,493 | 1893.20 | 27 | 111 | 35 | 80 |
| 11/1/2019 | 44,311 | 1893.10 | 19 | 111 | 35 | 80 |
| 11/2/2019 | 44,146 | 1893.00 | 27 | 111 | 35 | 80 |
| 11/3/2019 | 43,981 | 1892.90 | 28 | 111 | 36 | 79 |
| 11/4/2019 | 43,833 | 1892.80 | 37 | 111 | 36 | 79 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/5/2019 | 43,653 | 1892.70 | 20 | 111 | 35 | 80 |
| 11/6/2019 | 43,506 | 1892.70 | 37 | 111 | 35 | 80 |
| 11/7/2019 | 43,326 | 1892.50 | 20 | 111 | 34 | 79 |
| 11/8/2019 | 43,162 | 1892.40 | 28 | 111 | 35 | 79 |
| 11/9/2019 | 43,015 | 1892.30 | 37 | 111 | 35 | 80 |
| 11/10/2019 | 42,836 | 1892.20 | 21 | 112 | 34 | 80 |
| 11/11/2019 | 42,658 | 1892.10 | 24 | 114 | 36 | 80 |
| 11/12/2019 | 42,367 | 1891.90 | 36 | 183 | 42 | 112 |
| 11/13/2019 | 42,014 | 1891.70 | 41 | 219 | 47 | 167 |
| 11/14/2019 | 41,645 | 1891.50 | 30 | 216 | 41 | 169 |
| 11/15/2019 | 41,405 | 1891.30 | 52 | 172 | 41 | 151 |
| 11/16/2019 | 41,215 | 1891.20 | 23 | 119 | 46 | 80 |
| 11/17/2019 | 41,040 | 1891.10 | 31 | 119 | 44 | 81 |
| 11/18/2019 | 40,866 | 1891.00 | 31 | 119 | 44 | 81 |
| 11/19/2019 | 40,551 | 1890.80 | -2 | 157 | 44 | 92 |
| 11/20/2019 | 40,238 | 1890.60 | 56 | 214 | 45 | 164 |
| 11/21/2019 | 39,893 | 1890.40 | 39 | 213 | 42 | 168 |
| 11/22/2019 | 39,551 | 1890.20 | 39 | 211 | 43 | 167 |
| 11/23/2019 | 39,211 | 1889.90 | 34 | 205 | 40 | 166 |
| 11/24/2019 | 38,857 | 1889.70 | 26 | 204 | 39 | 165 |
| 11/25/2019 | 38,505 | 1889.50 | 26 | 204 | 38 | 165 |
| 11/26/2019 | 38,398 | 1889.40 | 160 | 214 | 45 | 166 |
| 11/27/2019 | 38,216 | 1889.30 | 131 | 223 | 57 | 166 |
| 11/28/2019 | 37,913 | 1889.10 | 79 | 232 | 46 | 166 |
| 11/29/2019 | 37,551 | 1888.90 | 54 | 236 | 53 | 166 |
| 11/30/2019 | 37,252 | 1888.70 | 91 | 242 | 65 | 167 |
| 12/1/2019 | 37,401 | 1888.80 | 327 | 252 | 121 | 167 |
| 12/2/2019 | 37,446 | 1888.80 | 277 | 254 | 129 | 167 |
| 12/3/2019 | 37,327 | 1888.70 | 199 | 259 | 131 | 167 |
| 12/4/2019 | 37,013 | 1888.50 | 107 | 265 | 133 | 167 |
| 12/5/2019 | 36,672 | 1888.30 | 100 | 272 | 135 | 167 |
| 12/6/2019 | 36,672 | 1888.30 | 275 | 275 | 140 | 167 |
| 12/7/2019 | 41,677 | 1891.50 | 2,820 | 297 | 428 | 167 |
| 12/8/2019 | 43,981 | 1892.90 | 1,468 | 307 | 381 | 166 |
| 12/9/2019 | 44,361 | 1893.20 | 498 | 307 | 219 | 166 |
| 12/10/2019 | 44,377 | 1893.20 | 313 | 304 | 187 | 166 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/11/2019 | 44,444 | 1893.20 | 353 | 319 | 195 | 166 |
| 12/12/2019 | 44,859 | 1893.50 | 556 | 346 | 254 | 196 |
| 12/13/2019 | 45,294 | 1893.70 | 569 | 350 | 229 | 217 |
| 12/14/2019 | 45,948 | 1894.10 | 681 | 351 | 280 | 218 |
| 12/15/2019 | 46,033 | 1894.20 | 393 | 350 | 204 | 218 |
| 12/16/2019 | 45,914 | 1894.10 | 291 | 350 | 183 | 219 |
| 12/17/2019 | 45,679 | 1894.00 | 230 | 349 | 205 | 190 |
| 12/18/2019 | 45,511 | 1893.90 | 246 | 331 | 260 | 130 |
| 12/19/2019 | 45,461 | 1893.80 | 291 | 316 | 251 | 130 |
| 12/20/2019 | 45,578 | 1893.90 | 362 | 303 | 255 | 129 |
| 12/21/2019 | 45,511 | 1893.90 | 262 | 296 | 229 | 130 |
| 12/22/2019 | 45,662 | 1893.90 | 369 | 293 | 238 | 130 |
| 12/23/2019 | 45,713 | 1894.00 | 314 | 289 | 232 | 130 |
| 12/24/2019 | 45,629 | 1893.90 | 243 | 286 | 214 | 130 |
| 12/25/2019 | 45,545 | 1893.90 | 240 | 282 | 212 | 129 |
| 12/26/2019 | 45,377 | 1893.80 | 193 | 278 | 198 | 129 |
| 12/27/2019 | 45,160 | 1893.60 | 162 | 271 | 190 | 130 |
| 12/28/2019 | 44,943 | 1893.50 | 159 | 268 | 185 | 129 |
| 12/29/2019 | 44,826 | 1893.40 | 211 | 270 | 193 | 129 |
| 12/30/2019 | 44,709 | 1893.40 | 211 | 270 | 200 | 130 |
| 12/31/2019 | 44,510 | 1893.30 | 168 | 269 | 189 | 130 |
| 1/1/2020 | 44,295 | 1893.10 | 160 | 268 | 186 | 129 |
| 1/2/2020 | 44,097 | 1893.00 | 169 | 268 | 184 | 129 |
| 1/3/2020 | 43,850 | 1892.90 | 143 | 267 | 181 | 129 |
| 1/4/2020 | 43,620 | 1892.70 | 151 | 267 | 184 | 129 |
| 1/5/2020 | 43,358 | 1892.60 | 134 | 267 | 180 | 129 |
| 1/6/2020 | 43,113 | 1892.40 | 130 | 254 | 177 | 115 |
| 1/7/2020 | 42,950 | 1892.30 | 159 | 241 | 175 | 85 |
| 1/8/2020 | 42,836 | 1892.20 | 179 | 236 | 170 | 80 |
| 1/9/2020 | 42,804 | 1892.20 | 214 | 231 | 173 | 80 |
| 1/10/2020 | 42,690 | 1892.10 | 180 | 237 | 175 | 80 |
| 1/11/2020 | 42,545 | 1892.10 | 165 | 238 | 177 | 80 |
| 1/12/2020 | 42,432 | 1892.00 | 181 | 238 | 175 | 81 |
| 1/13/2020 | 42,367 | 1891.90 | 205 | 238 | 173 | 92 |
| 1/14/2020 | 42,367 | 1891.90 | 239 | 239 | 223 | 100 |
| 1/15/2020 | 42,319 | 1891.90 | 217 | 241 | 199 | 100 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/16/2020 | 43,097 | 1892.40 | 640 | 247 | 309 | 106 |
| 1/17/2020 | 43,522 | 1892.70 | 466 | 251 | 349 | 91 |
| 1/18/2020 | 43,719 | 1892.80 | 349 | 250 | 312 | 79 |
| 1/19/2020 | 43,899 | 1892.90 | 335 | 244 | 275 | 79 |
| 1/20/2020 | 44,179 | 1893.10 | 383 | 242 | 269 | 79 |
| 1/21/2020 | 45,377 | 1893.80 | 846 | 242 | 370 | 79 |
| 1/22/2020 | 46,422 | 1894.40 | 794 | 267 | 394 | 99 |
| 1/23/2020 | 46,899 | 1894.70 | 556 | 315 | 313 | 161 |
| 1/24/2020 | 47,377 | 1895.00 | 579 | 338 | 271 | 205 |
| 1/25/2020 | 48,551 | 1895.60 | 932 | 340 | 324 | 211 |
| 1/26/2020 | 52,898 | 1898.10 | 2,538 | 347 | 800 | 209 |
| 1/27/2020 | 54,853 | 1899.20 | 1,333 | 348 | 504 | 209 |
| 1/28/2020 | 56,190 | 1899.90 | 1,023 | 348 | 430 | 209 |
| 1/29/2020 | 56,792 | 1900.20 | 812 | 509 | 484 | 217 |
| 1/30/2020 | 56,829 | 1900.20 | 665 | 645 | 624 | 224 |
| 1/31/2020 | 56,735 | 1900.20 | 578 | 626 | 596 | 220 |
| 2/1/2020 | 56,603 | 1900.10 | 499 | 565 | 507 | 220 |
| 2/2/2020 | 56,472 | 1900.00 | 436 | 502 | 417 | 220 |
| 2/3/2020 | 56,322 | 1900.00 | 364 | 440 | 330 | 226 |
| 2/4/2020 | 56,247 | 1899.90 | 338 | 376 | 263 | 218 |
| 2/5/2020 | 56,190 | 1899.90 | 280 | 308 | 294 | 125 |
| 2/6/2020 | 56,491 | 1900.00 | 410 | 259 | 272 | 80 |
| 2/7/2020 | 56,303 | 1899.90 | 166 | 261 | 228 | 94 |
| 2/8/2020 | 56,303 | 1899.90 | 277 | 277 | 209 | 126 |
| 2/9/2020 | 56,303 | 1899.90 | 266 | 266 | 183 | 140 |
| 2/10/2020 | 56,184 | 1899.90 | 196 | 256 | 174 | 140 |
| 2/11/2020 | 56,066 | 1899.80 | 194 | 254 | 177 | 125 |
| 2/12/2020 | 55,947 | 1899.80 | 187 | 247 | 181 | 73 |
| 2/13/2020 | 55,891 | 1899.70 | 209 | 237 | 187 | 46 |
| 2/14/2020 | 55,873 | 1899.70 | 225 | 234 | 182 | 45 |
| 2/15/2020 | 55,817 | 1899.70 | 206 | 234 | 181 | 45 |
| 2/16/2020 | 55,780 | 1899.70 | 209 | 228 | 178 | 45 |
| 2/17/2020 | 55,705 | 1899.60 | 186 | 224 | 173 | 45 |
| 2/18/2020 | 55,668 | 1899.60 | 205 | 224 | 172 | 45 |
| 2/19/2020 | 55,575 | 1899.50 | 177 | 224 | 171 | 45 |
| 2/20/2020 | 55,482 | 1899.50 | 173 | 220 | 169 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/21/2020 | 55,426 | 1899.50 | 179 | 208 | 162 | 45 |
| 2/22/2020 | 55,352 | 1899.40 | 162 | 200 | 156 | 36 |
| 2/23/2020 | 55,296 | 1899.40 | 158 | 186 | 168 | 22 |
| 2/24/2020 | 55,296 | 1899.40 | 146 | 146 | 157 | 20 |
| 2/25/2020 | 55,278 | 1899.40 | 134 | 143 | 150 | 19 |
| 2/26/2020 | 55,296 | 1899.40 | 146 | 137 | 148 | 19 |
| 2/27/2020 | 55,333 | 1899.40 | 148 | 129 | 142 | 18 |
| 2/28/2020 | 55,352 | 1899.40 | 132 | 123 | 136 | 18 |
| 2/29/2020 | 55,370 | 1899.40 | 131 | 121 | 134 | 17 |
| 3/1/2020 | 55,389 | 1899.40 | 131 | 121 | 134 | 17 |
| 3/2/2020 | 55,370 | 1899.40 | 112 | 121 | 133 | 17 |
| 3/3/2020 | 55,315 | 1899.40 | 133 | 161 | 146 | 16 |
| 3/4/2020 | 55,185 | 1899.30 | 128 | 194 | 174 | 26 |
| 3/5/2020 | 55,056 | 1899.30 | 128 | 193 | 155 | 32 |
| 3/6/2020 | 54,926 | 1899.20 | 128 | 193 | 145 | 46 |
| 3/7/2020 | 54,816 | 1899.10 | 135 | 191 | 145 | 46 |
| 3/8/2020 | 54,705 | 1899.10 | 127 | 183 | 141 | 45 |
| 3/9/2020 | 54,613 | 1899.00 | 128 | 174 | 135 | 45 |
| 3/10/2020 | 54,503 | 1899.00 | 113 | 168 | 131 | 45 |
| 3/11/2020 | 54,411 | 1898.90 | 121 | 167 | 128 | 45 |
| 3/12/2020 | 54,301 | 1898.90 | 109 | 165 | 127 | 45 |
| 3/13/2020 | 54,192 | 1898.80 | 107 | 162 | 123 | 45 |
| 3/14/2020 | 54,155 | 1898.80 | 144 | 163 | 127 | 45 |
| 3/15/2020 | 54,192 | 1898.80 | 182 | 163 | 135 | 45 |
| 3/16/2020 | 54,137 | 1898.80 | 142 | 170 | 131 | 48 |
| 3/17/2020 | 54,119 | 1898.80 | 164 | 174 | 131 | 55 |
| 3/18/2020 | 54,155 | 1898.80 | 192 | 174 | 135 | 55 |
| 3/19/2020 | 54,135 | 1898.80 | 168 | 178 | 136 | 55 |
| 3/20/2020 | 54,064 | 1898.70 | 149 | 185 | 138 | 55 |
| 3/21/2020 | 53,991 | 1898.70 | 152 | 188 | 139 | 55 |
| 3/22/2020 | 53,918 | 1898.60 | 158 | 194 | 142 | 55 |
| 3/23/2020 | 53,826 | 1898.60 | 148 | 195 | 142 | 55 |
| 3/24/2020 | 53,826 | 1898.60 | 195 | 195 | 145 | 55 |
| 3/25/2020 | 53,790 | 1898.60 | 177 | 196 | 151 | 55 |
| 3/26/2020 | 53,717 | 1898.50 | 158 | 195 | 146 | 55 |
| 3/27/2020 | 53,589 | 1898.50 | 148 | 212 | 148 | 57 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/28/2020 | 53,479 | 1898.40 | 159 | 214 | 149 | 64 |
| 3/29/2020 | 53,406 | 1898.40 | 174 | 211 | 146 | 64 |
| 3/30/2020 | 53,406 | 1898.40 | 204 | 204 | 150 | 56 |
| 3/31/2020 | 53,516 | 1898.40 | 251 | 196 | 152 | 52 |
| 4/1/2020 | 53,662 | 1898.50 | 267 | 193 | 154 | 52 |
| 4/2/2020 | 53,698 | 1898.50 | 229 | 211 | 155 | 52 |
| 4/3/2020 | 53,625 | 1898.50 | 189 | 226 | 162 | 61 |
| 4/4/2020 | 53,735 | 1898.50 | 286 | 231 | 178 | 64 |
| 4/5/2020 | 54,338 | 1898.90 | 532 | 228 | 255 | 64 |
| 4/6/2020 | 54,632 | 1899.00 | 383 | 235 | 257 | 64 |
| 4/7/2020 | 54,742 | 1899.10 | 297 | 241 | 255 | 57 |
| 4/8/2020 | 54,760 | 1899.10 | 254 | 245 | 252 | 53 |
| 4/9/2020 | 54,816 | 1899.10 | 272 | 245 | 253 | 48 |
| 4/10/2020 | 54,853 | 1899.20 | 262 | 243 | 245 | 46 |
| 4/11/2020 | 54,889 | 1899.20 | 261 | 243 | 238 | 45 |
| 4/12/2020 | 55,000 | 1899.20 | 299 | 243 | 232 | 45 |
| 4/13/2020 | 55,111 | 1899.30 | 300 | 244 | 229 | 46 |
| 4/14/2020 | 55,074 | 1899.30 | 227 | 246 | 232 | 46 |
| 4/15/2020 | 55,000 | 1899.20 | 206 | 243 | 235 | 38 |
| 4/16/2020 | 54,939 | 1899.20 | 208 | 239 | 228 | 34 |
| 4/17/2020 | 54,834 | 1899.10 | 184 | 237 | 218 | 34 |
| 4/18/2020 | 54,760 | 1899.10 | 196 | 233 | 208 | 34 |
| 4/19/2020 | 54,668 | 1899.10 | 181 | 228 | 194 | 34 |
| 4/20/2020 | 54,613 | 1899.00 | 194 | 221 | 188 | 34 |
| 4/21/2020 | 54,521 | 1899.00 | 159 | 205 | 179 | 34 |
| 4/22/2020 | 54,485 | 1899.00 | 168 | 186 | 164 | 34 |
| 4/23/2020 | 54,393 | 1898.90 | 131 | 177 | 157 | 34 |
| 4/24/2020 | 54,338 | 1898.90 | 139 | 167 | 150 | 35 |
| 4/25/2020 | 54,265 | 1898.80 | 138 | 175 | 147 | 39 |
| 4/26/2020 | 54,173 | 1898.80 | 128 | 174 | 142 | 50 |
| 4/27/2020 | 54,082 | 1898.70 | 128 | 174 | 137 | 54 |
| 4/28/2020 | 53,991 | 1898.70 | 126 | 172 | 132 | 60 |
| 4/29/2020 | 53,881 | 1898.60 | 112 | 167 | 127 | 60 |
| 4/30/2020 | 53,790 | 1898.60 | 113 | 159 | 127 | 53 |
| 5/1/2020 | 53,698 | 1898.50 | 108 | 154 | 125 | 50 |
| 5/2/2020 | 53,589 | 1898.50 | 95 | 151 | 122 | 50 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/3/2020 | 53,497 | 1898.40 | 101 | 147 | 121 | 50 |
| 5/4/2020 | 53,388 | 1898.40 | 91 | 146 | 118 | 50 |
| 5/5/2020 | 53,260 | 1898.30 | 83 | 148 | 117 | 50 |
| 5/6/2020 | 53,133 | 1898.20 | 87 | 151 | 118 | 50 |
| 5/7/2020 | 53,024 | 1898.20 | 97 | 152 | 117 | 50 |
| 5/8/2020 | 52,879 | 1898.10 | 80 | 153 | 117 | 50 |
| 5/9/2020 | 52,771 | 1898.00 | 100 | 154 | 116 | 50 |
| 5/10/2020 | 52,575 | 1897.90 | 57 | 156 | 116 | 50 |
| 5/11/2020 | 52,464 | 1897.80 | 102 | 158 | 119 | 51 |
| 5/12/2020 | 52,410 | 1897.80 | 124 | 151 | 132 | 40 |
| 5/13/2020 | 52,357 | 1897.80 | 121 | 148 | 132 | 36 |
| 5/14/2020 | 52,285 | 1897.70 | 117 | 154 | 136 | 36 |
| 5/15/2020 | 52,195 | 1897.70 | 103 | 148 | 130 | 35 |
| 5/16/2020 | 52,105 | 1897.70 | 121 | 167 | 135 | 35 |
| 5/17/2020 | 52,123 | 1897.70 | 187 | 178 | 159 | 35 |
| 5/18/2020 | 53,061 | 1898.20 | 648 | 175 | 272 | 29 |
| 5/19/2020 | 53,576 | 1898.50 | 471 | 211 | 289 | 25 |
| 5/20/2020 | 53,753 | 1898.60 | 331 | 242 | 271 | 25 |
| 5/21/2020 | 53,698 | 1898.50 | 233 | 261 | 288 | 25 |
| 5/22/2020 | 53,497 | 1898.40 | 179 | 281 | 305 | 24 |
| 5/23/2020 | 53,279 | 1898.30 | 174 | 284 | 303 | 24 |
| 5/24/2020 | 52,988 | 1898.10 | 140 | 287 | 300 | 24 |
| 5/25/2020 | 52,699 | 1898.00 | 142 | 288 | 299 | 24 |
| 5/26/2020 | 52,357 | 1897.80 | 128 | 300 | 293 | 32 |
| 5/27/2020 | 52,015 | 1897.60 | 125 | 297 | 281 | 46 |
| 5/28/2020 | 51,673 | 1897.40 | 95 | 267 | 251 | 45 |
| 5/29/2020 | 51,369 | 1897.20 | 96 | 249 | 221 | 45 |
| 5/30/2020 | 51,119 | 1897.10 | 113 | 239 | 196 | 45 |
| 5/31/2020 | 50,887 | 1897.00 | 114 | 230 | 184 | 45 |
| 6/1/2020 | 50,710 | 1896.90 | 123 | 213 | 168 | 45 |
| 6/2/2020 | 50,515 | 1896.80 | 97 | 195 | 154 | 45 |
| 6/3/2020 | 50,356 | 1896.70 | 98 | 178 | 145 | 45 |
| 6/4/2020 | 50,197 | 1896.60 | 83 | 163 | 129 | 50 |
| 6/5/2020 | 50,002 | 1896.50 | 61 | 159 | 114 | 59 |
| 6/6/2020 | 49,844 | 1896.40 | 71 | 151 | 107 | 61 |
| 6/7/2020 | 49,632 | 1896.30 | 40 | 147 | 101 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/8/2020 | 49,492 | 1896.20 | 76 | 146 | 100 | 61 |
| 6/9/2020 | 49,352 | 1896.10 | 73 | 144 | 97 | 61 |
| 6/10/2020 | 49,212 | 1896.00 | 67 | 137 | 91 | 61 |
| 6/11/2020 | 49,090 | 1895.90 | 70 | 132 | 85 | 60 |
| 6/12/2020 | 48,933 | 1895.90 | 44 | 123 | 78 | 60 |
| 6/13/2020 | 48,829 | 1895.80 | 58 | 110 | 72 | 57 |
| 6/14/2020 | 48,724 | 1895.70 | 50 | 103 | 70 | 50 |
| 6/15/2020 | 48,655 | 1895.70 | 63 | 98 | 65 | 50 |
| 6/16/2020 | 48,534 | 1895.60 | 32 | 93 | 59 | 50 |
| 6/17/2020 | 48,463 | 1895.60 | 56 | 91 | 55 | 50 |
| 6/18/2020 | 48,412 | 1895.60 | 63 | 89 | 52 | 50 |
| 6/19/2020 | 48,325 | 1895.50 | 42 | 86 | 48 | 50 |
| 6/20/2020 | 48,221 | 1895.40 | 35 | 87 | 35 | 62 |
| 6/21/2020 | 48,134 | 1895.40 | 46 | 90 | 28 | 71 |
| 6/22/2020 | 48,030 | 1895.30 | 37 | 89 | 28 | 70 |
| 6/23/2020 | 47,927 | 1895.30 | 35 | 87 | 24 | 70 |
| 6/24/2020 | 47,841 | 1895.20 | 42 | 86 | 20 | 70 |
| 6/25/2020 | 47,720 | 1895.20 | 26 | 87 | 20 | 70 |
| 6/26/2020 | 47,600 | 1895.10 | 26 | 86 | 21 | 70 |
| 6/27/2020 | 47,497 | 1895.00 | 35 | 87 | 18 | 70 |
| 6/28/2020 | 47,325 | 1894.90 | 1 | 87 | 20 | 70 |
| 6/29/2020 | 47,172 | 1894.80 | 9 | 87 | 23 | 71 |
| 6/30/2020 | 47,052 | 1894.80 | 27 | 87 | 19 | 71 |
| 7/1/2020 | 46,933 | 1894.70 | 27 | 87 | 17 | 71 |
| 7/2/2020 | 46,797 | 1894.60 | 18 | 87 | 19 | 71 |
| 7/3/2020 | 46,643 | 1894.50 | 9 | 86 | 18 | 71 |
| 7/4/2020 | 46,507 | 1894.40 | 17 | 86 | 18 | 71 |
| 7/5/2020 | 46,354 | 1894.40 | 9 | 86 | 17 | 71 |
| 7/6/2020 | 46,202 | 1894.30 | 8 | 85 | 17 | 70 |
| 7/7/2020 | 46,066 | 1894.20 | 18 | 86 | 17 | 70 |
| 7/8/2020 | 45,914 | 1894.10 | 12 | 89 | 17 | 70 |
| 7/9/2020 | 45,763 | 1894.00 | 13 | 89 | 17 | 70 |
| 7/10/2020 | 45,629 | 1893.90 | 21 | 89 | 18 | 70 |
| 7/11/2020 | 45,461 | 1893.80 | 4 | 89 | 18 | 70 |
| 7/12/2020 | 45,311 | 1893.70 | 13 | 88 | 17 | 70 |
| 7/13/2020 | 45,177 | 1893.70 | 22 | 89 | 18 | 70 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/14/2020 | 45,026 | 1893.60 | 13 | 89 | 16 | 70 |
| 7/15/2020 | 44,859 | 1893.50 | 3 | 87 | 20 | 67 |
| 7/16/2020 | 44,709 | 1893.40 | 9 | 85 | 23 | 65 |
| 7/17/2020 | 44,560 | 1893.30 | 7 | 83 | 18 | 65 |
| 7/18/2020 | 44,411 | 1893.20 | 7 | 83 | 17 | 65 |
| 7/19/2020 | 44,262 | 1893.10 | 7 | 83 | 16 | 65 |
| 7/20/2020 | 44,130 | 1893.00 | 16 | 82 | 15 | 65 |
| 7/21/2020 | 43,965 | 1892.90 | -1 | 82 | 15 | 65 |
| 7/22/2020 | 43,784 | 1892.80 | -9 | 83 | 15 | 65 |
| 7/23/2020 | 43,637 | 1892.70 | 8 | 82 | 15 | 65 |
| 7/24/2020 | 43,457 | 1892.60 | -2 | 89 | 17 | 65 |
| 7/25/2020 | 43,293 | 1892.50 | 10 | 92 | 19 | 72 |
| 7/26/2020 | 43,097 | 1892.40 | -6 | 92 | 13 | 75 |
| 7/27/2020 | 42,918 | 1892.30 | 2 | 92 | 13 | 76 |
| 7/28/2020 | 42,739 | 1892.20 | 2 | 92 | 13 | 76 |
| 7/29/2020 | 42,545 | 1892.10 | -3 | 95 | 12 | 76 |
| 7/30/2020 | 42,367 | 1891.90 | 5 | 94 | 16 | 76 |
| 7/31/2020 | 42,174 | 1891.80 | -4 | 93 | 14 | 76 |
| 8/1/2020 | 41,982 | 1891.70 | -4 | 93 | 14 | 76 |
| 8/2/2020 | 41,805 | 1891.60 | 5 | 94 | 15 | 76 |
| 8/3/2020 | 41,613 | 1891.50 | -3 | 94 | 16 | 75 |
| 8/4/2020 | 41,421 | 1891.40 | -2 | 95 | 15 | 75 |
| 8/5/2020 | 41,230 | 1891.20 | 0 | 96 | 15 | 75 |
| 8/6/2020 | 41,056 | 1891.10 | 10 | 98 | 16 | 76 |
| 8/7/2020 | 40,866 | 1891.00 | 2 | 98 | 17 | 76 |
| 8/8/2020 | 40,677 | 1890.90 | 2 | 98 | 16 | 75 |
| 8/9/2020 | 40,504 | 1890.80 | 10 | 98 | 16 | 75 |
| 8/10/2020 | 40,301 | 1890.70 | -6 | 97 | 17 | 75 |
| 8/11/2020 | 40,112 | 1890.50 | 1 | 96 | 14 | 75 |
| 8/12/2020 | 39,917 | 1890.40 | -4 | 95 | 15 | 75 |
| 8/13/2020 | 39,722 | 1890.30 | -4 | 94 | 15 | 75 |
| 8/14/2020 | 39,536 | 1890.20 | 1 | 95 | 15 | 75 |
| 8/15/2020 | 39,350 | 1890.00 | 2 | 95 | 15 | 75 |
| 8/16/2020 | 39,165 | 1889.90 | 3 | 96 | 15 | 75 |
| 8/17/2020 | 38,985 | 1889.80 | 6 | 97 | 16 | 75 |
| 8/18/2020 | 38,835 | 1889.70 | 22 | 97 | 17 | 76 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/19/2020 | 38,674 | 1889.60 | 14 | 96 | 16 | 76 |
| 8/20/2020 | 38,475 | 1889.50 | -8 | 92 | 19 | 75 |
| 8/21/2020 | 38,322 | 1889.40 | 13 | 90 | 18 | 75 |
| 8/22/2020 | 38,155 | 1889.30 | 6 | 91 | 19 | 75 |
| 8/23/2020 | 37,973 | 1889.10 | -1 | 91 | 19 | 75 |
| 8/24/2020 | 37,807 | 1889.00 | 5 | 89 | 21 | 72 |
| 8/25/2020 | 37,641 | 1888.90 | 1 | 84 | 16 | 65 |
| 8/26/2020 | 37,476 | 1888.80 | 0 | 84 | 17 | 65 |
| 8/27/2020 | 37,327 | 1888.70 | 8 | 84 | 17 | 65 |
| 8/28/2020 | 37,177 | 1888.60 | 8 | 84 | 18 | 65 |
| 8/29/2020 | 37,028 | 1888.50 | 8 | 84 | 19 | 65 |
| 8/30/2020 | 36,835 | 1888.40 | -15 | 83 | 20 | 65 |
| 8/31/2020 | 36,701 | 1888.30 | 16 | 83 | 19 | 65 |
| 9/1/2020 | 36,538 | 1888.20 | 0 | 82 | 19 | 65 |
| 9/2/2020 | 36,406 | 1888.10 | 15 | 82 | 17 | 65 |
| 9/3/2020 | 36,244 | 1888.00 | 1 | 82 | 17 | 65 |
| 9/4/2020 | 36,097 | 1887.90 | 9 | 83 | 16 | 65 |
| 9/5/2020 | 35,937 | 1887.80 | 2 | 84 | 20 | 65 |
| 9/6/2020 | 35,762 | 1887.60 | -7 | 81 | 21 | 65 |
| 9/7/2020 | 35,617 | 1887.60 | 6 | 79 | 20 | 65 |
| 9/8/2020 | 35,458 | 1887.40 | -3 | 77 | 15 | 65 |
| 9/9/2020 | 35,299 | 1887.30 | -4 | 76 | 19 | 64 |
| 9/10/2020 | 35,175 | 1887.20 | 11 | 74 | 17 | 60 |
| 9/11/2020 | 35,040 | 1887.20 | 4 | 71 | 20 | 60 |
| 9/12/2020 | 34,912 | 1887.10 | 5 | 70 | 18 | 57 |
| 9/13/2020 | 34,769 | 1887.00 | -2 | 70 | 17 | 54 |
| 9/14/2020 | 34,641 | 1886.90 | 0 | 65 | 16 | 53 |
| 9/15/2020 | 34,499 | 1886.80 | -8 | 64 | 11 | 50 |
| 9/16/2020 | 34,400 | 1886.70 | 18 | 68 | 16 | 51 |
| 9/17/2020 | 34,216 | 1886.60 | -26 | 67 | 18 | 51 |
| 9/18/2020 | 34,075 | 1886.50 | -5 | 65 | 15 | 50 |
| 9/19/2020 | 33,960 | 1886.40 | 7 | 65 | 16 | 51 |
| 9/20/2020 | 33,840 | 1886.30 | 4 | 65 | 16 | 50 |
| 9/21/2020 | 33,720 | 1886.20 | 4 | 65 | 14 | 50 |
| 9/22/2020 | 33,590 | 1886.10 | -1 | 65 | 14 | 50 |
| 9/23/2020 | 33,470 | 1886.00 | 4 | 65 | 15 | 50 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/24/2020 | 33,350 | 1885.90 | 5 | 65 | 15 | 50 |
| 9/25/2020 | 33,212 | 1885.80 | 6 | 76 | 16 | 54 |
| 9/26/2020 | 33,060 | 1885.70 | 5 | 81 | 15 | 65 |
| 9/27/2020 | 32,923 | 1885.60 | 12 | 81 | 15 | 65 |
| 9/28/2020 | 32,731 | 1885.50 | -15 | 82 | 14 | 65 |
| 9/29/2020 | 32,581 | 1885.40 | 5 | 81 | 15 | 65 |
| 9/30/2020 | 32,390 | 1885.20 | 3 | 99 | 15 | 72 |
| 10/1/2020 | 32,173 | 1885.10 | 5 | 114 | 22 | 89 |
| 10/2/2020 | 31,957 | 1884.90 | 6 | 114 | 22 | 90 |
| 10/3/2020 | 31,742 | 1884.80 | 7 | 115 | 21 | 91 |
| 10/4/2020 | 31,528 | 1884.60 | 9 | 117 | 21 | 91 |
| 10/5/2020 | 31,302 | 1884.40 | 5 | 120 | 25 | 90 |
| 10/6/2020 | 31,050 | 1884.20 | -8 | 119 | 26 | 90 |
| 10/7/2020 | 30,826 | 1884.10 | 6 | 119 | 26 | 90 |
| 10/8/2020 | 30,604 | 1883.90 | 7 | 119 | 26 | 90 |
| 10/9/2020 | 30,395 | 1883.70 | 14 | 119 | 27 | 90 |
| 10/10/2020 | 30,187 | 1883.60 | 15 | 119 | 28 | 90 |
| 10/11/2020 | 29,968 | 1883.40 | 8 | 119 | 30 | 89 |
| 10/12/2020 | 29,762 | 1883.30 | 14 | 118 | 27 | 90 |
| 10/13/2020 | 29,545 | 1883.10 | 9 | 119 | 24 | 90 |
| 10/14/2020 | 29,328 | 1882.90 | 13 | 122 | 25 | 89 |
| 10/15/2020 | 29,088 | 1882.70 | 15 | 136 | 39 | 89 |
| 10/16/2020 | 28,886 | 1882.60 | 14 | 116 | 52 | 65 |
| 10/17/2020 | 28,698 | 1882.40 | 5 | 99 | 46 | 55 |
| 10/18/2020 | 28,524 | 1882.30 | 7 | 95 | 43 | 55 |
| 10/19/2020 | 28,350 | 1882.10 | 3 | 91 | 40 | 55 |
| 10/20/2020 | 28,189 | 1882.00 | 8 | 89 | 38 | 55 |
| 10/21/2020 | 28,029 | 1881.90 | 9 | 90 | 38 | 55 |
| 10/22/2020 | 27,858 | 1881.70 | 5 | 92 | 38 | 55 |
| 10/23/2020 | 27,687 | 1881.60 | 5 | 91 | 38 | 56 |
| 10/24/2020 | 27,529 | 1881.50 | 11 | 91 | 38 | 56 |
| 10/25/2020 | 27,372 | 1881.30 | 11 | 90 | 38 | 56 |
| 10/26/2020 | 27,179 | 1881.20 | -6 | 91 | 37 | 56 |
| 10/27/2020 | 27,011 | 1881.00 | 8 | 92 | 37 | 56 |
| 10/28/2020 | 26,856 | 1880.90 | 14 | 92 | 37 | 56 |
| 10/29/2020 | 26,701 | 1880.80 | 26 | 104 | 37 | 56 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/30/2020 | 26,548 | 1880.60 | 14 | 92 | 37 | 56 |
| 10/31/2020 | 26,371 | 1880.50 | 3 | 92 | 37 | 56 |
| 11/1/2020 | 26,230 | 1880.40 | 20 | 91 | 42 | 48 |
| 11/2/2020 | 26,090 | 1880.30 | 11 | 81 | 40 | 46 |
| 11/3/2020 | 25,950 | 1880.10 | 9 | 79 | 38 | 46 |
| 11/4/2020 | 25,811 | 1880.00 | 9 | 79 | 38 | 46 |
| 11/5/2020 | 25,684 | 1879.90 | 15 | 79 | 38 | 45 |
| 11/6/2020 | 25,545 | 1879.80 | 10 | 79 | 38 | 45 |
| 11/7/2020 | 25,396 | 1879.70 | 4 | 79 | 38 | 46 |
| 11/8/2020 | 25,248 | 1879.50 | 5 | 79 | 36 | 46 |
| 11/9/2020 | 25,123 | 1879.40 | 17 | 79 | 24 | 46 |
| 11/10/2020 | 24,988 | 1879.30 | 12 | 80 | 26 | 46 |
| 11/11/2020 | 24,864 | 1879.20 | 18 | 81 | 38 | 46 |
| 11/12/2020 | 24,740 | 1879.10 | 20 | 83 | 39 | 46 |
| 11/13/2020 | 24,718 | 1879.10 | 73 | 85 | 39 | 46 |
| 11/14/2020 | 24,628 | 1879.00 | 39 | 84 | 39 | 46 |
| 11/15/2020 | 24,539 | 1878.90 | 39 | 84 | 39 | 46 |
| 11/16/2020 | 24,439 | 1878.80 | 33 | 83 | 38 | 46 |
| 11/17/2020 | 24,384 | 1878.80 | 56 | 84 | 45 | 46 |
| 11/18/2020 | 24,395 | 1878.80 | 91 | 85 | 51 | 46 |
| 11/19/2020 | 24,384 | 1878.80 | 79 | 84 | 45 | 46 |
| 11/20/2020 | 24,273 | 1878.70 | 40 | 96 | 43 | 46 |
| 11/21/2020 | 24,141 | 1878.50 | 42 | 109 | 55 | 46 |
| 11/22/2020 | 23,999 | 1878.40 | 37 | 108 | 55 | 46 |
| 11/23/2020 | 23,869 | 1878.30 | 40 | 106 | 55 | 46 |
| 11/24/2020 | 23,717 | 1878.20 | 28 | 104 | 52 | 46 |
| 11/25/2020 | 23,598 | 1878.00 | 46 | 106 | 52 | 46 |
| 11/26/2020 | 23,448 | 1877.90 | 29 | 105 | 51 | 46 |
| 11/27/2020 | 23,319 | 1877.80 | 35 | 100 | 48 | 46 |
| 11/28/2020 | 23,117 | 1877.60 | -7 | 95 | 43 | 46 |
| 11/29/2020 | 23,012 | 1877.50 | 15 | 68 | 40 | 46 |
| 11/30/2020 | 22,812 | 1877.30 | 24 | 124 | 52 | 46 |
| 12/1/2020 | 22,520 | 1877.00 | 30 | 177 | 119 | 46 |
| 12/2/2020 | 22,241 | 1876.80 | 29 | 169 | 123 | 46 |
| 12/3/2020 | 21,966 | 1876.50 | 28 | 167 | 119 | 46 |
| 12/4/2020 | 21,703 | 1876.20 | 34 | 167 | 118 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/5/2020 | 21,432 | 1876.00 | 30 | 167 | 118 | 46 |
| 12/6/2020 | 21,154 | 1875.70 | 26 | 166 | 118 | 46 |
| 12/7/2020 | 20,899 | 1875.40 | 38 | 167 | 117 | 46 |
| 12/8/2020 | 20,637 | 1875.10 | 35 | 167 | 117 | 46 |
| 12/9/2020 | 20,368 | 1874.90 | 31 | 167 | 116 | 46 |
| 12/10/2020 | 20,120 | 1874.60 | 41 | 166 | 116 | 46 |
| 12/11/2020 | 19,867 | 1874.30 | 38 | 166 | 118 | 46 |
| 12/12/2020 | 19,617 | 1874.10 | 40 | 166 | 120 | 46 |
| 12/13/2020 | 19,488 | 1873.90 | 102 | 167 | 131 | 46 |
| 12/14/2020 | 19,451 | 1873.90 | 148 | 166 | 134 | 46 |
| 12/15/2020 | 19,286 | 1873.70 | 83 | 166 | 130 | 46 |
| 12/16/2020 | 19,195 | 1873.60 | 121 | 166 | 128 | 46 |
| 12/17/2020 | 19,096 | 1873.50 | 108 | 158 | 129 | 46 |
| 12/18/2020 | 18,998 | 1873.40 | 119 | 169 | 125 | 46 |
| 12/19/2020 | 18,900 | 1873.30 | 119 | 169 | 123 | 46 |
| 12/20/2020 | 18,704 | 1873.10 | 64 | 162 | 120 | 46 |
| 12/21/2020 | 18,466 | 1872.80 | 41 | 161 | 119 | 46 |
| 12/22/2020 | 18,258 | 1872.50 | 56 | 161 | 117 | 46 |
| 12/23/2020 | 18,026 | 1872.30 | 43 | 160 | 116 | 46 |
| 12/24/2020 | 17,788 | 1872.00 | 39 | 159 | 116 | 46 |
| 12/25/2020 | 17,644 | 1871.80 | 87 | 160 | 127 | 46 |
| 12/26/2020 | 17,966 | 1872.20 | 323 | 161 | 178 | 46 |
| 12/27/2020 | 17,949 | 1872.20 | 151 | 160 | 143 | 46 |
| 12/28/2020 | 17,830 | 1872.00 | 99 | 159 | 128 | 46 |
| 12/29/2020 | 17,644 | 1871.80 | 64 | 158 | 123 | 46 |
| 12/30/2020 | 17,477 | 1871.60 | 71 | 156 | 120 | 45 |
| 12/31/2020 | 17,292 | 1871.40 | 63 | 156 | 123 | 45 |
| 1/1/2021 | 17,124 | 1871.20 | 70 | 154 | 120 | 45 |
| 1/2/2021 | 16,953 | 1871.00 | 67 | 153 | 120 | 45 |
| 1/3/2021 | 16,784 | 1870.80 | 67 | 152 | 120 | 44 |
| 1/4/2021 | 17,856 | 1872.10 | 696 | 156 | 250 | 44 |
| 1/5/2021 | 18,458 | 1872.80 | 461 | 157 | 190 | 45 |
| 1/6/2021 | 18,669 | 1873.00 | 269 | 162 | 150 | 45 |
| 1/7/2021 | 18,891 | 1873.30 | 269 | 157 | 151 | 45 |
| 1/8/2021 | 19,236 | 1873.60 | 332 | 158 | 172 | 45 |
| 1/9/2021 | 19,414 | 1873.80 | 247 | 157 | 156 | 45 |

Notes:
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| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/10/2021 | 19,451 | 1873.90 | 175 | 156 | 144 | 45 |
| 1/11/2021 | 19,442 | 1873.90 | 151 | 156 | 138 | 45 |
| 1/12/2021 | 19,497 | 1873.90 | 184 | 156 | 138 | 45 |
| 1/13/2021 | 20,262 | 1874.80 | 568 | 182 | 280 | 45 |
| 1/14/2021 | 20,531 | 1875.00 | 337 | 202 | 277 | 44 |
| 1/15/2021 | 20,579 | 1875.10 | 237 | 213 | 207 | 44 |
| 1/16/2021 | 20,540 | 1875.00 | 187 | 207 | 184 | 44 |
| 1/17/2021 | 20,444 | 1874.90 | 158 | 206 | 179 | 45 |
| 1/18/2021 | 20,329 | 1874.80 | 147 | 205 | 174 | 45 |
| 1/19/2021 | 20,139 | 1874.60 | 110 | 206 | 171 | 45 |
| 1/20/2021 | 19,942 | 1874.40 | 106 | 205 | 169 | 45 |
| 1/21/2021 | 19,756 | 1874.20 | 110 | 204 | 167 | 45 |
| 1/22/2021 | 19,552 | 1874.00 | 97 | 199 | 164 | 46 |
| 1/23/2021 | 19,350 | 1873.80 | 88 | 190 | 157 | 46 |
| 1/24/2021 | 19,168 | 1873.60 | 88 | 180 | 150 | 46 |
| 1/25/2021 | 18,998 | 1873.40 | 85 | 171 | 144 | 46 |
| 1/26/2021 | 18,935 | 1873.30 | 131 | 163 | 141 | 47 |
| 1/27/2021 | 18,998 | 1873.40 | 187 | 156 | 154 | 47 |
| 1/28/2021 | 19,515 | 1874.00 | 420 | 159 | 204 | 47 |
| 1/29/2021 | 19,886 | 1874.40 | 346 | 159 | 204 | 47 |
| 1/30/2021 | 19,989 | 1874.50 | 211 | 159 | 204 | 47 |
| 1/31/2021 | 20,329 | 1874.80 | 331 | 159 | 204 | 47 |
| 2/1/2021 | 21,472 | 1876.00 | 771 | 195 | 324 | 46 |
| 2/2/2021 | 24,875 | 1879.20 | 1,918 | 202 | 681 | 46 |
| 2/3/2021 | 26,043 | 1880.20 | 794 | 205 | 425 | 46 |
| 2/4/2021 | 26,642 | 1880.70 | 507 | 205 | 348 | 46 |
| 2/5/2021 | 27,011 | 1881.00 | 391 | 205 | 287 | 47 |
| 2/6/2021 | 27,336 | 1881.30 | 369 | 205 | 246 | 47 |
| 2/7/2021 | 27,565 | 1881.50 | 322 | 206 | 230 | 47 |
| 2/8/2021 | 27,833 | 1881.70 | 341 | 206 | 217 | 47 |
| 2/9/2021 | 27,995 | 1881.90 | 287 | 205 | 206 | 46 |
| 2/10/2021 | 28,160 | 1882.00 | 289 | 206 | 199 | 46 |
| 2/11/2021 | 28,264 | 1882.10 | 260 | 207 | 220 | 46 |
| 2/12/2021 | 29,968 | 1883.40 | 1,067 | 208 | 298 | 46 |
| 2/13/2021 | 31,011 | 1884.20 | 732 | 206 | 318 | 47 |
| 2/14/2021 | 31,769 | 1884.80 | 589 | 207 | 288 | 47 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/15/2021 | 33,019 | 1885.70 | 838 | 208 | 304 | 47 |
| 2/16/2021 | 33,795 | 1886.30 | 600 | 209 | 277 | 47 |
| 2/17/2021 | 34,301 | 1886.60 | 464 | 210 | 253 | 47 |
| 2/18/2021 | 34,671 | 1886.90 | 396 | 210 | 237 | 47 |
| 2/19/2021 | 35,083 | 1887.20 | 419 | 211 | 247 | 47 |
| 2/20/2021 | 35,588 | 1887.50 | 467 | 213 | 298 | 47 |
| 2/21/2021 | 35,951 | 1887.80 | 396 | 212 | 280 | 47 |
| 2/22/2021 | 36,259 | 1888.00 | 367 | 212 | 261 | 47 |
| 2/23/2021 | 36,479 | 1888.10 | 323 | 212 | 245 | 47 |
| 2/24/2021 | 36,627 | 1888.20 | 286 | 211 | 231 | 47 |
| 2/25/2021 | 36,716 | 1888.30 | 255 | 210 | 220 | 47 |
| 2/26/2021 | 36,734 | 1888.30 | 219 | 210 | 213 | 47 |
| 2/27/2021 | 36,753 | 1888.30 | 219 | 210 | 207 | 47 |
| 2/28/2021 | 36,772 | 1888.30 | 219 | 210 | 201 | 47 |
| 3/1/2021 | 36,805 | 1888.40 | 229 | 212 | 197 | 47 |
| 3/2/2021 | 36,790 | 1888.30 | 206 | 213 | 195 | 47 |
| 3/3/2021 | 36,790 | 1888.30 | 213 | 213 | 192 | 47 |
| 3/4/2021 | 36,790 | 1888.30 | 213 | 213 | 185 | 47 |
| 3/5/2021 | 36,790 | 1888.30 | 213 | 213 | 186 | 47 |
| 3/6/2021 | 36,790 | 1888.30 | 213 | 213 | 195 | 47 |
| 3/7/2021 | 36,790 | 1888.30 | 213 | 213 | 189 | 47 |
| 3/8/2021 | 36,894 | 1888.40 | 252 | 200 | 188 | 47 |
| 3/9/2021 | 36,909 | 1888.40 | 207 | 200 | 198 | 47 |
| 3/10/2021 | 37,192 | 1888.60 | 353 | 210 | 246 | 47 |
| 3/11/2021 | 37,536 | 1888.80 | 384 | 210 | 254 | 47 |
| 3/12/2021 | 37,822 | 1889.00 | 354 | 210 | 243 | 47 |
| 3/13/2021 | 38,094 | 1889.20 | 334 | 197 | 239 | 47 |
| 3/14/2021 | 38,368 | 1889.40 | 347 | 209 | 254 | 47 |
| 3/15/2021 | 39,103 | 1889.90 | 572 | 202 | 327 | 47 |
| 3/16/2021 | 39,714 | 1890.30 | 528 | 220 | 292 | 47 |
| 3/17/2021 | 40,325 | 1890.70 | 528 | 220 | 269 | 47 |
| 3/18/2021 | 40,936 | 1891.10 | 528 | 220 | 287 | 47 |
| 3/19/2021 | 41,547 | 1891.40 | 528 | 220 | 309 | 47 |
| 3/20/2021 | 42,158 | 1891.80 | 528 | 220 | 278 | 47 |
| 3/21/2021 | 42,626 | 1892.10 | 447 | 211 | 255 | 47 |
| 3/22/2021 | 42,966 | 1892.30 | 382 | 210 | 244 | 47 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/23/2021 | 43,080 | 1892.40 | 392 | 334 | 324 | 50 |
| 3/24/2021 | 43,195 | 1892.50 | 363 | 306 | 334 | 57 |
| 3/25/2021 | 43,408 | 1892.60 | 323 | 216 | 228 | 46 |
| 3/26/2021 | 43,604 | 1892.70 | 314 | 215 | 216 | 45 |
| 3/27/2021 | 43,735 | 1892.80 | 281 | 215 | 209 | 45 |
| 3/28/2021 | 43,866 | 1892.90 | 281 | 214 | 205 | 45 |
| 3/29/2021 | 44,014 | 1893.00 | 289 | 214 | 202 | 45 |
| 3/30/2021 | 44,130 | 1893.00 | 261 | 203 | 199 | 45 |
| 3/31/2021 | 44,113 | 1893.00 | 249 | 257 | 215 | 53 |
| 4/1/2021 | 44,047 | 1893.00 | 248 | 282 | 246 | 57 |
| 4/2/2021 | 43,998 | 1892.90 | 242 | 267 | 242 | 58 |
| 4/3/2021 | 43,883 | 1892.90 | 210 | 268 | 241 | 58 |
| 4/4/2021 | 43,768 | 1892.80 | 210 | 268 | 240 | 57 |
| 4/5/2021 | 43,653 | 1892.70 | 225 | 282 | 238 | 57 |
| 4/6/2021 | 43,588 | 1892.70 | 244 | 277 | 234 | 57 |
| 4/7/2021 | 43,430 | 1892.60 | 194 | 273 | 222 | 57 |
| 4/8/2021 | 43,273 | 1892.50 | 194 | 273 | 207 | 63 |
| 4/9/2021 | 43,116 | 1892.40 | 188 | 267 | 196 | 66 |
| 4/10/2021 | 42,972 | 1892.30 | 163 | 235 | 191 | 66 |
| 4/11/2021 | 42,824 | 1892.20 | 171 | 245 | 189 | 67 |
| 4/12/2021 | 42,674 | 1892.10 | 162 | 238 | 178 | 73 |
| 4/13/2021 | 42,528 | 1892.00 | 159 | 232 | 170 | 75 |
| 4/14/2021 | 42,335 | 1891.90 | 117 | 214 | 165 | 75 |
| 4/15/2021 | 42,190 | 1891.80 | 147 | 220 | 166 | 69 |
| 4/16/2021 | 42,046 | 1891.70 | 136 | 209 | 158 | 66 |
| 4/17/2021 | 41,870 | 1891.60 | 117 | 206 | 152 | 66 |
| 4/18/2021 | 41,725 | 1891.50 | 125 | 198 | 153 | 60 |
| 4/19/2021 | 41,565 | 1891.40 | 108 | 189 | 149 | 54 |
| 4/20/2021 | 41,437 | 1891.40 | 120 | 185 | 146 | 54 |
| 4/21/2021 | 41,326 | 1891.30 | 106 | 163 | 142 | 52 |
| 4/22/2021 | 41,246 | 1891.20 | 124 | 164 | 138 | 45 |
| 4/23/2021 | 41,119 | 1891.20 | 98 | 162 | 135 | 39 |
| 4/24/2021 | 41,008 | 1891.10 | 105 | 161 | 132 | 39 |
| 4/25/2021 | 40,961 | 1891.10 | 137 | 161 | 137 | 39 |
| 4/26/2021 | 40,866 | 1891.00 | 116 | 164 | 135 | 42 |
| 4/27/2021 | 40,724 | 1890.90 | 100 | 171 | 133 | 49 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/28/2021 | 40,583 | 1890.80 | 104 | 175 | 129 | 55 |
| 4/29/2021 | 40,410 | 1890.70 | 88 | 175 | 127 | 55 |
| 4/30/2021 | 40,230 | 1890.60 | 83 | 174 | 124 | 55 |
| 5/1/2021 | 40,050 | 1890.50 | 82 | 172 | 121 | 55 |
| 5/2/2021 | 39,871 | 1890.40 | 81 | 171 | 119 | 55 |
| 5/3/2021 | 39,691 | 1890.30 | 81 | 171 | 118 | 55 |
| 5/4/2021 | 39,505 | 1890.10 | 77 | 171 | 117 | 55 |
| 5/5/2021 | 39,334 | 1890.00 | 78 | 164 | 126 | 35 |
| 5/6/2021 | 39,195 | 1889.90 | 82 | 152 | 117 | 35 |
| 5/7/2021 | 38,980 | 1889.80 | 49 | 157 | 119 | 38 |
| 5/8/2021 | 38,812 | 1889.70 | 71 | 156 | 121 | 37 |
| 5/9/2021 | 38,612 | 1889.60 | 52 | 152 | 117 | 34 |
| 5/10/2021 | 38,429 | 1889.40 | 59 | 151 | 116 | 34 |
| 5/11/2021 | 38,246 | 1889.30 | 58 | 150 | 114 | 35 |
| 5/12/2021 | 38,049 | 1889.20 | 50 | 150 | 113 | 35 |
| 5/13/2021 | 37,852 | 1889.10 | 51 | 150 | 113 | 35 |
| 5/14/2021 | 37,672 | 1888.90 | 58 | 150 | 112 | 35 |
| 5/15/2021 | 37,446 | 1888.80 | 40 | 153 | 114 | 37 |
| 5/16/2021 | 37,237 | 1888.70 | 45 | 150 | 112 | 39 |
| 5/17/2021 | 37,058 | 1888.50 | 50 | 140 | 104 | 39 |
| 5/18/2021 | 36,879 | 1888.40 | 36 | 127 | 90 | 39 |
| 5/19/2021 | 36,716 | 1888.30 | 37 | 119 | 82 | 39 |
| 5/20/2021 | 36,538 | 1888.20 | 25 | 115 | 77 | 40 |
| 5/21/2021 | 36,406 | 1888.10 | 46 | 113 | 73 | 41 |
| 5/22/2021 | 36,273 | 1888.00 | 42 | 109 | 76 | 35 |
| 5/23/2021 | 36,171 | 1887.90 | 49 | 101 | 72 | 32 |
| 5/24/2021 | 36,054 | 1887.80 | 36 | 95 | 67 | 32 |
| 5/25/2021 | 35,981 | 1887.80 | 42 | 79 | 60 | 30 |
| 5/26/2021 | 35,908 | 1887.70 | 31 | 68 | 46 | 28 |
| 5/27/2021 | 35,878 | 1887.70 | 52 | 66 | 43 | 28 |
| 5/28/2021 | 35,806 | 1887.70 | 30 | 66 | 41 | 30 |
| 5/29/2021 | 35,733 | 1887.60 | 29 | 66 | 39 | 30 |
| 5/30/2021 | 35,660 | 1887.60 | 30 | 67 | 38 | 30 |
| 5/31/2021 | 35,588 | 1887.50 | 28 | 64 | 38 | 30 |
| 6/1/2021 | 35,515 | 1887.50 | 25 | 62 | 35 | 30 |
| 6/2/2021 | 35,414 | 1887.40 | 10 | 61 | 33 | 31 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/3/2021 | 35,313 | 1887.30 | 17 | 68 | 33 | 32 |
| 6/4/2021 | 35,227 | 1887.30 | 26 | 70 | 34 | 39 |
| 6/5/2021 | 35,126 | 1887.20 | 18 | 69 | 31 | 40 |
| 6/6/2021 | 35,012 | 1887.10 | 8 | 66 | 33 | 37 |
| 6/7/2021 | 34,912 | 1887.10 | 10 | 61 | 34 | 32 |
| 6/8/2021 | 34,826 | 1887.00 | 12 | 55 | 30 | 30 |
| 6/9/2021 | 34,740 | 1886.90 | 7 | 50 | 26 | 30 |
| 6/10/2021 | 34,683 | 1886.90 | 25 | 53 | 20 | 31 |
| 6/11/2021 | 34,612 | 1886.90 | 22 | 58 | 18 | 40 |
| 6/12/2021 | 34,556 | 1886.80 | 29 | 57 | 17 | 40 |
| 6/13/2021 | 34,513 | 1886.80 | 36 | 57 | 16 | 40 |
| 6/14/2021 | 34,428 | 1886.70 | 14 | 57 | 16 | 40 |
| 6/15/2021 | 34,329 | 1886.70 | 13 | 63 | 17 | 41 |
| 6/16/2021 | 34,230 | 1886.60 | 20 | 70 | 21 | 47 |
| 6/17/2021 | 34,118 | 1886.50 | 15 | 72 | 22 | 50 |
| 6/18/2021 | 33,991 | 1886.40 | 12 | 76 | 21 | 51 |
| 6/19/2021 | 33,865 | 1886.30 | 15 | 79 | 22 | 55 |
| 6/20/2021 | 33,725 | 1886.20 | 7 | 77 | 24 | 52 |
| 6/21/2021 | 33,600 | 1886.10 | 7 | 71 | 21 | 50 |
| 6/22/2021 | 33,490 | 1886.10 | 8 | 64 | 19 | 44 |
| 6/23/2021 | 33,380 | 1886.00 | 5 | 60 | 16 | 42 |
| 6/24/2021 | 33,275 | 1885.90 | 6 | 59 | 13 | 43 |
| 6/25/2021 | 33,157 | 1885.80 | -1 | 58 | 12 | 43 |
| 6/26/2021 | 33,060 | 1885.70 | 10 | 58 | 11 | 43 |
| 6/27/2021 | 32,950 | 1885.70 | 3 | 58 | 11 | 43 |
| 6/28/2021 | 32,854 | 1885.60 | 3 | 52 | 18 | 35 |
| 6/29/2021 | 32,786 | 1885.50 | 12 | 47 | 16 | 30 |
| 6/30/2021 | 32,690 | 1885.50 | -2 | 47 | 14 | 31 |
| 7/1/2021 | 32,594 | 1885.40 | -1 | 47 | 15 | 30 |
| 7/2/2021 | 32,499 | 1885.30 | -1 | 48 | 15 | 30 |
| 7/3/2021 | 32,417 | 1885.30 | 6 | 47 | 15 | 30 |
| 7/4/2021 | 32,308 | 1885.20 | -8 | 47 | 14 | 30 |
| 7/5/2021 | 32,214 | 1885.10 | -2 | 46 | 14 | 30 |
| 7/6/2021 | 32,105 | 1885.00 | -9 | 46 | 14 | 30 |
| 7/7/2021 | 31,984 | 1884.90 | -16 | 46 | 14 | 30 |
| 7/8/2021 | 31,890 | 1884.90 | -2 | 46 | 14 | 30 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/9/2021 | 31,796 | 1884.80 | -2 | 46 | 14 | 30 |
| 7/10/2021 | 31,689 | 1884.70 | -8 | 46 | 14 | 30 |
| 7/11/2021 | 31,582 | 1884.70 | -8 | 46 | 14 | 30 |
| 7/12/2021 | 31,475 | 1884.60 | -4 | 50 | 14 | 30 |
| 7/13/2021 | 31,355 | 1884.50 | -8 | 53 | 14 | 30 |
| 7/14/2021 | 31,222 | 1884.40 | -10 | 57 | 15 | 32 |
| 7/15/2021 | 31,077 | 1884.30 | -13 | 60 | 16 | 41 |
| 7/16/2021 | 30,945 | 1884.20 | -6 | 60 | 17 | 40 |
| 7/17/2021 | 30,800 | 1884.10 | -13 | 60 | 18 | 39 |
| 7/18/2021 | 30,682 | 1884.00 | -1 | 58 | 17 | 39 |
| 7/19/2021 | 30,564 | 1883.90 | -3 | 57 | 15 | 39 |
| 7/20/2021 | 30,434 | 1883.80 | -10 | 56 | 15 | 39 |
| 7/21/2021 | 30,291 | 1883.70 | -16 | 56 | 14 | 39 |
| 7/22/2021 | 30,174 | 1883.60 | -4 | 55 | 13 | 39 |
| 7/23/2021 | 30,045 | 1883.50 | -11 | 54 | 14 | 39 |
| 7/24/2021 | 29,916 | 1883.40 | -11 | 54 | 13 | 39 |
| 7/25/2021 | 29,788 | 1883.30 | -11 | 54 | 13 | 39 |
| 7/26/2021 | 29,659 | 1883.20 | -13 | 52 | 20 | 33 |
| 7/27/2021 | 29,557 | 1883.10 | -2 | 50 | 21 | 29 |
| 7/28/2021 | 29,442 | 1883.00 | -8 | 49 | 19 | 29 |
| 7/29/2021 | 29,341 | 1882.90 | -1 | 50 | 18 | 29 |
| 7/30/2021 | 29,214 | 1882.80 | -14 | 50 | 18 | 29 |
| 7/31/2021 | 29,100 | 1882.70 | -7 | 50 | 17 | 29 |
| 8/1/2021 | 28,974 | 1882.60 | -11 | 53 | 18 | 30 |
| 8/2/2021 | 28,849 | 1882.50 | -8 | 55 | 14 | 36 |
| 8/3/2021 | 28,723 | 1882.40 | -7 | 56 | 14 | 36 |
| 8/4/2021 | 28,611 | 1882.30 | -1 | 55 | 13 | 36 |
| 8/5/2021 | 28,474 | 1882.20 | -13 | 56 | 13 | 36 |
| 8/6/2021 | 28,363 | 1882.10 | 0 | 57 | 14 | 36 |
| 8/7/2021 | 28,251 | 1882.10 | 1 | 57 | 15 | 35 |
| 8/8/2021 | 28,135 | 1882.00 | -3 | 56 | 16 | 34 |
| 8/9/2021 | 28,023 | 1881.90 | -5 | 51 | 16 | 26 |
| 8/10/2021 | 27,910 | 1881.80 | -5 | 52 | 14 | 29 |
| 8/11/2021 | 27,798 | 1881.70 | -5 | 51 | 16 | 30 |
| 8/12/2021 | 27,685 | 1881.60 | -6 | 50 | 15 | 30 |
| 8/13/2021 | 27,573 | 1881.50 | -3 | 54 | 14 | 35 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/14/2021 | 27,460 | 1881.40 | 2 | 59 | 14 | 35 |
| 8/15/2021 | 27,348 | 1881.30 | -1 | 56 | 17 | 35 |
| 8/16/2021 | 27,239 | 1881.20 | -2 | 53 | 13 | 35 |
| 8/17/2021 | 27,119 | 1881.10 | -8 | 52 | 10 | 35 |
| 8/18/2021 | 27,011 | 1881.00 | -2 | 52 | 10 | 35 |
| 8/19/2021 | 26,880 | 1880.90 | -16 | 50 | 15 | 32 |
| 8/20/2021 | 26,763 | 1880.80 | -11 | 48 | 14 | 30 |
| 8/21/2021 | 26,646 | 1880.70 | -11 | 48 | 13 | 30 |
| 8/22/2021 | 26,529 | 1880.60 | -11 | 47 | 14 | 30 |
| 8/23/2021 | 26,412 | 1880.50 | -11 | 48 | 14 | 30 |
| 8/24/2021 | 26,394 | 1880.50 | 39 | 48 | 14 | 30 |
| 8/25/2021 | 26,289 | 1880.40 | -5 | 48 | 13 | 30 |
| 8/26/2021 | 26,195 | 1880.40 | 1 | 48 | 12 | 30 |
| 8/27/2021 | 26,090 | 1880.30 | -5 | 48 | 12 | 30 |
| 8/28/2021 | 25,985 | 1880.20 | -5 | 48 | 13 | 30 |
| 8/29/2021 | 25,892 | 1880.10 | 1 | 48 | 13 | 30 |
| 8/30/2021 | 25,787 | 1880.00 | -5 | 48 | 13 | 30 |
| 8/31/2021 | 25,684 | 1879.90 | -5 | 47 | 13 | 30 |
| 9/1/2021 | 25,568 | 1879.80 | -11 | 48 | 13 | 30 |
| 9/2/2021 | 25,472 | 1879.70 | -1 | 48 | 14 | 30 |
| 9/3/2021 | 25,373 | 1879.60 | -2 | 48 | 14 | 30 |
| 9/4/2021 | 25,271 | 1879.60 | -4 | 48 | 14 | 30 |
| 9/5/2021 | 25,169 | 1879.50 | -4 | 48 | 14 | 30 |
| 9/6/2021 | 25,078 | 1879.40 | 2 | 48 | 14 | 30 |
| 9/7/2021 | 24,976 | 1879.30 | -1 | 51 | 14 | 30 |
| 9/8/2021 | 24,864 | 1879.20 | -1 | 56 | 18 | 33 |
| 9/9/2021 | 24,673 | 1879.00 | 0 | 56 | 14 | 35 |
| 9/10/2021 | 24,584 | 1878.90 | 11 | 56 | 15 | 35 |
| 9/11/2021 | 24,473 | 1878.80 | -1 | 55 | 14 | 35 |
| 9/12/2021 | 24,362 | 1878.70 | -1 | 55 | 14 | 35 |
| 9/13/2021 | 24,262 | 1878.60 | 5 | 55 | 14 | 35 |
| 9/14/2021 | 24,152 | 1878.60 | -1 | 54 | 18 | 32 |
| 9/15/2021 | 24,043 | 1878.50 | -2 | 53 | 20 | 30 |
| 9/16/2021 | 23,958 | 1878.40 | 7 | 50 | 17 | 30 |
| 9/17/2021 | 23,874 | 1878.30 | 5 | 48 | 13 | 30 |
| 9/18/2021 | 23,789 | 1878.20 | 5 | 48 | 16 | 30 |

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| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/19/2021 | 23,704 | 1878.10 | 6 | 49 | 16 | 30 |
| 9/20/2021 | 23,620 | 1878.10 | 6 | 48 | 14 | 30 |
| 9/21/2021 | 23,523 | 1878.00 | -1 | 48 | 14 | 30 |
| 9/22/2021 | 23,437 | 1877.90 | 5 | 48 | 13 | 30 |
| 9/23/2021 | 23,373 | 1877.80 | 16 | 48 | 12 | 30 |
| 9/24/2021 | 23,273 | 1877.70 | -2 | 48 | 12 | 30 |
| 9/25/2021 | 23,174 | 1877.60 | -2 | 48 | 12 | 30 |
| 9/26/2021 | 23,075 | 1877.60 | -3 | 47 | 13 | 30 |
| 9/27/2021 | 22,991 | 1877.50 | 5 | 47 | 14 | 30 |
| 9/28/2021 | 22,885 | 1877.40 | -5 | 48 | 14 | 30 |
| 9/29/2021 | 22,802 | 1877.30 | 5 | 47 | 17 | 30 |
| 9/30/2021 | 22,718 | 1877.20 | 3 | 46 | 13 | 30 |
| 10/1/2021 | 22,635 | 1877.10 | 3 | 46 | 13 | 30 |
| 10/2/2021 | 22,551 | 1877.10 | 4 | 46 | 13 | 30 |
| 10/3/2021 | 22,458 | 1877.00 | 2 | 49 | 15 | 30 |
| 10/4/2021 | 22,365 | 1876.90 | 1 | 48 | 18 | 30 |
| 10/5/2021 | 22,272 | 1876.80 | 0 | 47 | 15 | 30 |
| 10/6/2021 | 22,180 | 1876.70 | 0 | 47 | 21 | 24 |
| 10/7/2021 | 22,098 | 1876.60 | 4 | 46 | 38 | 10 |
| 10/8/2021 | 22,006 | 1876.50 | -2 | 44 | 37 | 10 |
| 10/9/2021 | 21,925 | 1876.40 | 1 | 42 | 35 | 10 |
| 10/10/2021 | 21,838 | 1876.40 | -1 | 43 | 35 | 10 |
| 10/11/2021 | 21,743 | 1876.30 | -4 | 43 | 36 | 10 |
| 10/12/2021 | 21,624 | 1876.10 | -16 | 44 | 36 | 10 |
| 10/13/2021 | 21,542 | 1876.10 | 3 | 45 | 37 | 10 |
| 10/14/2021 | 21,462 | 1876.00 | 4 | 45 | 36 | 10 |
| 10/15/2021 | 21,382 | 1875.90 | 5 | 45 | 36 | 10 |
| 10/16/2021 | 21,305 | 1875.80 | 8 | 46 | 37 | 10 |
| 10/17/2021 | 21,243 | 1875.80 | 16 | 48 | 40 | 10 |
| 10/18/2021 | 21,164 | 1875.70 | 8 | 48 | 41 | 10 |
| 10/19/2021 | 21,134 | 1875.60 | 32 | 47 | 40 | 10 |
| 10/20/2021 | 21,095 | 1875.60 | 27 | 46 | 43 | 10 |
| 10/21/2021 | 21,115 | 1875.60 | 57 | 47 | 41 | 10 |
| 10/22/2021 | 21,854 | 1876.40 | 421 | 48 | 67 | 10 |
| 10/23/2021 | 22,088 | 1876.60 | 173 | 55 | 50 | 10 |
| 10/24/2021 | 37,342 | 1888.70 | 7,778 | 87 | 1,384 | 10 |

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| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/25/2021 | 43,375 | 1892.60 | 3,191 | 149 | 797 | 30 |
| 10/26/2021 | 44,993 | 1893.50 | 1,007 | 191 | 331 | 45 |
| 10/27/2021 | 45,864 | 1894.10 | 630 | 191 | 244 | 46 |
| 10/28/2021 | 46,320 | 1894.30 | 422 | 191 | 211 | 45 |
| 10/29/2021 | 46,592 | 1894.50 | 330 | 193 | 191 | 45 |
| 10/30/2021 | 46,729 | 1894.60 | 261 | 193 | 182 | 45 |
| 10/31/2021 | 46,763 | 1894.60 | 211 | 194 | 175 | 45 |
| 11/1/2021 | 47,052 | 1894.80 | 342 | 196 | 183 | 45 |
| 11/2/2021 | 47,600 | 1895.10 | 472 | 196 | 201 | 45 |
| 11/3/2021 | 47,927 | 1895.30 | 359 | 194 | 181 | 45 |
| 11/4/2021 | 48,394 | 1895.50 | 433 | 197 | 241 | 44 |
| 11/5/2021 | 48,776 | 1895.80 | 389 | 197 | 219 | 45 |
| 11/6/2021 | 48,985 | 1895.90 | 302 | 197 | 200 | 45 |
| 11/7/2021 | 49,107 | 1896.00 | 258 | 197 | 188 | 45 |
| 11/8/2021 | 49,439 | 1896.10 | 365 | 197 | 190 | 45 |
| 11/9/2021 | 52,500 | 1897.90 | 1,747 | 203 | 644 | 45 |
| 11/10/2021 | 53,500 | 1898.40 | 704 | 200 | 345 | 45 |
| 11/11/2021 | 54,100 | 1898.70 | 502 | 200 | 261 | 45 |
| 11/12/2021 | 54,600 | 1899.00 | 451 | 199 | 229 | 45 |
| 11/13/2021 | 54,871 | 1899.20 | 335 | 199 | 210 | 45 |
| 11/14/2021 | 55,056 | 1899.30 | 292 | 199 | 198 | 45 |
| 11/15/2021 | 55,148 | 1899.30 | 245 | 198 | 191 | 45 |
| 11/16/2021 | 55,241 | 1899.40 | 245 | 198 | 185 | 45 |
| 11/17/2021 | 55,241 | 1899.40 | 199 | 199 | 182 | 45 |
| 11/18/2021 | 55,310 | 1899.40 | 234 | 199 | 179 | 45 |
| 11/19/2021 | 55,289 | 1899.40 | 189 | 200 | 184 | 45 |
| 11/20/2021 | 55,269 | 1899.40 | 188 | 199 | 179 | 45 |
| 11/21/2021 | 55,230 | 1899.40 | 179 | 199 | 175 | 45 |
| 11/22/2021 | 55,143 | 1899.30 | 156 | 200 | 174 | 45 |
| 11/23/2021 | 55,008 | 1899.20 | 135 | 203 | 175 | 45 |
| 11/24/2021 | 54,892 | 1899.20 | 145 | 203 | 175 | 44 |
| 11/25/2021 | 54,730 | 1899.10 | 118 | 199 | 172 | 43 |
| 11/26/2021 | 54,587 | 1899.00 | 121 | 194 | 164 | 45 |
| 11/27/2021 | 54,471 | 1899.00 | 125 | 184 | 155 | 46 |
| 11/28/2021 | 54,356 | 1898.90 | 112 | 170 | 143 | 46 |
| 11/29/2021 | 54,259 | 1898.80 | 107 | 156 | 130 | 45 |

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| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/30/2021 | 54,240 | 1898.80 | 143 | 153 | 123 | 46 |
| 12/1/2021 | 54,048 | 1898.70 | 57 | 154 | 125 | 46 |
| 12/2/2021 | 53,954 | 1898.70 | 107 | 154 | 124 | 46 |
| 12/3/2021 | 53,826 | 1898.60 | 89 | 154 | 124 | 46 |
| 12/4/2021 | 53,717 | 1898.50 | 101 | 156 | 124 | 46 |
| 12/5/2021 | 53,570 | 1898.50 | 85 | 158 | 125 | 46 |
| 12/6/2021 | 53,589 | 1898.50 | 168 | 159 | 126 | 46 |
| 12/7/2021 | 54,046 | 1898.70 | 389 | 158 | 126 | 46 |
| 12/8/2021 | 54,503 | 1899.00 | 389 | 158 | 125 | 46 |
| 12/9/2021 | 54,963 | 1899.20 | 391 | 159 | 125 | 46 |
| 12/10/2021 | 55,426 | 1899.50 | 393 | 160 | 126 | 46 |
| 12/11/2021 | 55,891 | 1899.70 | 394 | 160 | 125 | 46 |
| 12/12/2021 | 56,359 | 1900.00 | 400 | 165 | 267 | 46 |
| 12/13/2021 | 56,829 | 1900.20 | 405 | 168 | 393 | 46 |
| 12/14/2021 | 57,227 | 1900.40 | 649 | 449 | 671 | 46 |
| 12/15/2021 | 58,562 | 1901.10 | 1,464 | 791 | 1,224 | 46 |
| 12/16/2021 | 58,504 | 1901.10 | 2,087 | 2,116 | 2,677 | 44 |
| 12/17/2021 | 57,816 | 1900.70 | 1,009 | 1,356 | 1,826 | 44 |
| 12/18/2021 | 57,454 | 1900.50 | 629 | 811 | 1,240 | 44 |
| 12/19/2021 | 57,151 | 1900.40 | 481 | 634 | 940 | 44 |
| 12/20/2021 | 57,018 | 1900.30 | 475 | 541 | 778 | 44 |
| 12/21/2021 | 57,094 | 1900.40 | 531 | 493 | 684 | 44 |
| 12/22/2021 | 58,581 | 1901.10 | 1,856 | 1,107 | 1,351 | 44 |
| 12/23/2021 | 61,249 | 1902.50 | 5,384 | 4,039 | 4,473 | 44 |
| 12/24/2021 | 59,974 | 1901.90 | 3,334 | 3,977 | 4,444 | 44 |
| 12/25/2021 | 60,169 | 1902.00 | 3,544 | 3,446 | 4,353 | 44 |
| 12/26/2021 | 59,082 | 1901.40 | 2,151 | 2,699 | 3,355 | 44 |
| 12/27/2021 | 58,542 | 1901.10 | 1,672 | 1,944 | 2,532 | 45 |
| 12/28/2021 | 58,159 | 1900.90 | 1,233 | 1,426 | 1,985 | 44 |
| 12/29/2021 | 57,930 | 1900.80 | 1,056 | 1,171 | 1,747 | 45 |
| 12/30/2021 | 57,683 | 1900.70 | 849 | 974 | 1,524 | 45 |
| 12/31/2021 | 57,492 | 1900.60 | 725 | 821 | 1,281 | 44 |
| 1/1/2022 | 57,321 | 1900.50 | 631 | 717 | 1,087 | 44 |
| 1/2/2022 | 57,227 | 1900.40 | 599 | 647 | 964 | 44 |
| 1/3/2022 | 58,696 | 1901.20 | 1,467 | 726 | 1,129 | 44 |
| 1/4/2022 | 59,275 | 1901.50 | 2,868 | 2,576 | 2,961 | 44 |

Notes:
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| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/5/2022 | 59,159 | 1901.40 | 2,499 | 2,558 | 2,821 | 45 |
| 1/6/2022 | 58,908 | 1901.30 | 2,286 | 2,412 | 2,658 | 46 |
| 1/7/2022 | 58,869 | 1901.30 | 2,185 | 2,205 | 2,510 | 46 |
| 1/8/2022 | 58,446 | 1901.10 | 1,803 | 2,016 | 2,322 | 46 |
| 1/9/2022 | 58,045 | 1900.90 | 1,364 | 1,567 | 1,898 | 46 |
| 1/10/2022 | 57,778 | 1900.70 | 1,093 | 1,228 | 1,569 | 46 |
| 1/11/2022 | 57,607 | 1900.60 | 940 | 1,026 | 1,360 | 46 |
| 1/12/2022 | 57,492 | 1900.60 | 847 | 905 | 1,231 | 46 |
| 1/13/2022 | 57,378 | 1900.50 | 781 | 838 | 1,140 | 46 |
| 1/14/2022 | 57,302 | 1900.50 | 740 | 778 | 1,061 | 46 |
| 1/15/2022 | 57,208 | 1900.40 | 680 | 728 | 986 | 46 |
| 1/16/2022 | 57,151 | 1900.40 | 655 | 684 | 919 | 46 |
| 1/17/2022 | 57,056 | 1900.30 | 597 | 645 | 859 | 46 |
| 1/18/2022 | 57,113 | 1900.40 | 538 | 607 | 797 | 46 |
| 1/19/2022 | 57,037 | 1900.30 | 532 | 570 | 743 | 46 |
| 1/20/2022 | 56,980 | 1900.30 | 511 | 539 | 694 | 46 |
| 1/21/2022 | 56,943 | 1900.30 | 508 | 527 | 669 | 46 |
| 1/22/2022 | 56,943 | 1900.30 | 508 | 508 | 636 | 46 |
| 1/23/2022 | 56,869 | 1900.20 | 439 | 477 | 585 | 45 |
| 1/24/2022 | 56,834 | 1900.20 | 435 | 452 | 545 | 46 |
| 1/25/2022 | 56,792 | 1900.20 | 413 | 434 | 517 | 46 |
| 1/26/2022 | 56,754 | 1900.20 | 393 | 412 | 483 | 46 |
| 1/27/2022 | 56,697 | 1900.10 | 359 | 387 | 448 | 46 |
| 1/28/2022 | 56,660 | 1900.10 | 344 | 363 | 415 | 46 |
| 1/29/2022 | 56,603 | 1900.10 | 313 | 341 | 386 | 46 |
| 1/30/2022 | 56,566 | 1900.10 | 305 | 324 | 360 | 46 |
| 1/31/2022 | 56,547 | 1900.10 | 300 | 310 | 339 | 46 |
| 2/1/2022 | 56,509 | 1900.00 | 276 | 295 | 322 | 46 |
| 2/2/2022 | 56,472 | 1900.00 | 260 | 279 | 299 | 46 |
| 2/3/2022 | 56,453 | 1900.00 | 257 | 267 | 283 | 46 |
| 2/4/2022 | 56,397 | 1900.00 | 226 | 255 | 261 | 46 |
| 2/5/2022 | 56,359 | 1900.00 | 225 | 244 | 244 | 46 |
| 2/6/2022 | 56,340 | 1900.00 | 228 | 238 | 234 | 46 |
| 2/7/2022 | 56,303 | 1899.90 | 212 | 231 | 225 | 46 |
| 2/8/2022 | 56,303 | 1899.90 | 228 | 228 | 220 | 46 |
| 2/9/2022 | 56,265 | 1899.90 | 209 | 228 | 217 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/10/2022 | 56,265 | 1899.90 | 228 | 228 | 216 | 46 |
| 2/11/2022 | 56,303 | 1899.90 | 250 | 231 | 219 | 46 |
| 2/12/2022 | 56,303 | 1899.90 | 236 | 236 | 224 | 46 |
| 2/13/2022 | 56,303 | 1899.90 | 235 | 235 | 223 | 46 |
| 2/14/2022 | 56,322 | 1900.00 | 244 | 235 | 221 | 46 |
| 2/15/2022 | 56,284 | 1899.90 | 217 | 236 | 222 | 46 |
| 2/16/2022 | 56,247 | 1899.90 | 212 | 231 | 215 | 46 |
| 2/17/2022 | 56,190 | 1899.90 | 193 | 221 | 204 | 46 |
| 2/18/2022 | 56,153 | 1899.90 | 194 | 213 | 196 | 46 |
| 2/19/2022 | 56,116 | 1899.80 | 189 | 208 | 189 | 46 |
| 2/20/2022 | 56,041 | 1899.80 | 167 | 205 | 184 | 46 |
| 2/21/2022 | 56,003 | 1899.80 | 184 | 203 | 182 | 46 |
| 2/22/2022 | 55,966 | 1899.80 | 183 | 201 | 178 | 46 |
| 2/23/2022 | 55,891 | 1899.70 | 163 | 200 | 177 | 46 |
| 2/24/2022 | 55,798 | 1899.70 | 153 | 200 | 175 | 46 |
| 2/25/2022 | 55,705 | 1899.60 | 152 | 199 | 174 | 46 |
| 2/26/2022 | 55,631 | 1899.60 | 161 | 199 | 174 | 46 |
| 2/27/2022 | 55,500 | 1899.50 | 131 | 196 | 172 | 46 |
| 2/28/2022 | 55,389 | 1899.40 | 139 | 195 | 169 | 45 |
| 3/1/2022 | 55,315 | 1899.40 | 158 | 195 | 168 | 46 |
| 3/2/2022 | 55,222 | 1899.40 | 144 | 191 | 166 | 45 |
| 3/3/2022 | 55,185 | 1899.30 | 168 | 187 | 164 | 45 |
| 3/4/2022 | 55,130 | 1899.30 | 157 | 185 | 160 | 45 |
| 3/5/2022 | 55,000 | 1899.20 | 119 | 185 | 159 | 46 |
| 3/6/2022 | 54,908 | 1899.20 | 137 | 183 | 159 | 46 |
| 3/7/2022 | 54,779 | 1899.10 | 116 | 181 | 156 | 45 |
| 3/8/2022 | 54,650 | 1899.00 | 115 | 180 | 153 | 45 |
| 3/9/2022 | 54,540 | 1899.00 | 125 | 180 | 152 | 45 |
| 3/10/2022 | 54,430 | 1898.90 | 122 | 177 | 149 | 45 |
| 3/11/2022 | 54,283 | 1898.80 | 98 | 172 | 145 | 45 |
| 3/12/2022 | 54,192 | 1898.80 | 121 | 167 | 141 | 45 |
| 3/13/2022 | 54,100 | 1898.70 | 118 | 164 | 138 | 45 |
| 3/14/2022 | 54,009 | 1898.70 | 114 | 160 | 135 | 45 |
| 3/15/2022 | 54,009 | 1898.70 | 160 | 160 | 139 | 45 |
| 3/16/2022 | 53,936 | 1898.70 | 131 | 168 | 137 | 50 |
| 3/17/2022 | 53,826 | 1898.60 | 118 | 174 | 132 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/18/2022 | 53,717 | 1898.50 | 121 | 176 | 131 | 60 |
| 3/19/2022 | 53,625 | 1898.50 | 132 | 178 | 130 | 65 |
| 3/20/2022 | 53,497 | 1898.40 | 114 | 179 | 128 | 66 |
| 3/21/2022 | 53,379 | 1898.40 | 126 | 185 | 128 | 66 |
| 3/22/2022 | 53,260 | 1898.30 | 130 | 190 | 130 | 66 |
| 3/23/2022 | 53,151 | 1898.20 | 132 | 187 | 125 | 66 |
| 3/24/2022 | 53,061 | 1898.20 | 141 | 187 | 122 | 69 |
| 3/25/2022 | 52,934 | 1898.10 | 122 | 186 | 119 | 71 |
| 3/26/2022 | 52,807 | 1898.00 | 123 | 187 | 118 | 72 |
| 3/27/2022 | 52,735 | 1898.00 | 148 | 185 | 118 | 72 |
| 3/28/2022 | 52,663 | 1898.00 | 147 | 183 | 121 | 72 |
| 3/29/2022 | 52,536 | 1897.90 | 117 | 181 | 118 | 72 |
| 3/30/2022 | 52,428 | 1897.80 | 127 | 182 | 116 | 72 |
| 3/31/2022 | 52,285 | 1897.70 | 109 | 181 | 115 | 72 |
| 4/1/2022 | 52,123 | 1897.70 | 98 | 179 | 120 | 62 |
| 4/2/2022 | 51,979 | 1897.60 | 101 | 173 | 119 | 58 |
| 4/3/2022 | 51,835 | 1897.50 | 97 | 170 | 115 | 58 |
| 4/4/2022 | 51,691 | 1897.40 | 96 | 168 | 112 | 59 |
| 4/5/2022 | 51,548 | 1897.30 | 94 | 167 | 108 | 59 |
| 4/6/2022 | 51,404 | 1897.30 | 90 | 162 | 105 | 58 |
| 4/7/2022 | 51,279 | 1897.20 | 100 | 163 | 105 | 58 |
| 4/8/2022 | 51,083 | 1897.10 | 66 | 165 | 104 | 58 |
| 4/9/2022 | 50,914 | 1897.00 | 82 | 168 | 103 | 58 |
| 4/10/2022 | 50,745 | 1896.90 | 87 | 172 | 101 | 58 |
| 4/11/2022 | 50,675 | 1896.80 | 128 | 164 | 103 | 59 |
| 4/12/2022 | 50,551 | 1896.80 | 101 | 164 | 99 | 59 |
| 4/13/2022 | 50,427 | 1896.70 | 105 | 167 | 101 | 62 |
| 4/14/2022 | 50,941 | 1897.00 | 434 | 175 | 198 | 65 |
| 4/15/2022 | 51,261 | 1897.20 | 328 | 166 | 165 | 60 |
| 4/16/2022 | 52,627 | 1897.90 | 869 | 181 | 295 | 57 |
| 4/17/2022 | 53,043 | 1898.20 | 436 | 226 | 230 | 57 |
| 4/18/2022 | 53,279 | 1898.30 | 376 | 257 | 233 | 57 |
| 4/19/2022 | 53,900 | 1898.60 | 592 | 279 | 344 | 53 |
| 4/20/2022 | 54,797 | 1899.10 | 732 | 280 | 371 | 51 |
| 4/21/2022 | 57,664 | 1900.70 | 1,736 | 291 | 806 | 49 |
| 4/22/2022 | 59,082 | 1901.40 | 1,016 | 301 | 565 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/23/2022 | 59,935 | 1901.80 | 743 | 313 | 457 | 46 |
| 4/24/2022 | 60,541 | 1902.10 | 605 | 300 | 386 | 46 |
| 4/25/2022 | 60,992 | 1902.40 | 545 | 317 | 365 | 46 |
| 4/26/2022 | 61,367 | 1902.60 | 480 | 291 | 326 | 46 |
| 4/27/2022 | 61,644 | 1902.70 | 435 | 296 | 304 | 46 |
| 4/28/2022 | 61,822 | 1902.80 | 372 | 283 | 292 | 46 |
| 4/29/2022 | 61,961 | 1902.90 | 336 | 266 | 266 | 46 |
| 4/30/2022 | 62,060 | 1902.90 | 323 | 273 | 259 | 46 |
| 5/1/2022 | 62,120 | 1902.90 | 302 | 272 | 244 | 52 |
| 5/2/2022 | 62,060 | 1902.90 | 245 | 275 | 245 | 55 |
| 5/3/2022 | 62,080 | 1902.90 | 289 | 279 | 236 | 63 |
| 5/4/2022 | 61,981 | 1902.90 | 233 | 283 | 242 | 60 |
| 5/5/2022 | 61,882 | 1902.80 | 228 | 278 | 237 | 58 |
| 5/6/2022 | 61,783 | 1902.80 | 222 | 272 | 233 | 58 |
| 5/7/2022 | 61,664 | 1902.70 | 213 | 273 | 232 | 53 |
| 5/8/2022 | 61,625 | 1902.70 | 262 | 282 | 229 | 51 |
| 5/9/2022 | 61,506 | 1902.60 | 209 | 269 | 233 | 51 |
| 5/10/2022 | 61,367 | 1902.60 | 205 | 275 | 236 | 54 |
| 5/11/2022 | 61,209 | 1902.50 | 188 | 268 | 216 | 62 |
| 5/12/2022 | 61,091 | 1902.40 | 201 | 261 | 207 | 65 |
| 5/13/2022 | 60,973 | 1902.40 | 189 | 249 | 203 | 59 |
| 5/14/2022 | 60,815 | 1902.30 | 178 | 257 | 196 | 64 |
| 5/15/2022 | 60,619 | 1902.20 | 182 | 281 | 183 | 98 |
| 5/16/2022 | 60,384 | 1902.10 | 153 | 271 | 176 | 97 |
| 5/17/2022 | 60,150 | 1901.90 | 139 | 257 | 163 | 97 |
| 5/18/2022 | 59,916 | 1901.80 | 140 | 257 | 158 | 97 |
| 5/19/2022 | 59,644 | 1901.70 | 110 | 247 | 151 | 97 |
| 5/20/2022 | 59,411 | 1901.60 | 117 | 234 | 139 | 98 |
| 5/21/2022 | 59,178 | 1901.40 | 118 | 235 | 132 | 103 |
| 5/22/2022 | 58,927 | 1901.30 | 110 | 237 | 131 | 105 |
| 5/23/2022 | 58,715 | 1901.20 | 122 | 229 | 123 | 105 |
| 5/24/2022 | 58,504 | 1901.10 | 117 | 224 | 115 | 109 |
| 5/25/2022 | 58,274 | 1901.00 | 101 | 217 | 109 | 111 |
| 5/26/2022 | 58,025 | 1900.80 | 93 | 218 | 104 | 111 |
| 5/27/2022 | 57,759 | 1900.70 | 85 | 219 | 103 | 111 |
| 5/28/2022 | 57,530 | 1900.60 | 103 | 218 | 106 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/29/2022 | 57,302 | 1900.50 | 95 | 210 | 101 | 111 |
| 5/30/2022 | 57,056 | 1900.30 | 79 | 203 | 92 | 111 |
| 5/31/2022 | 56,811 | 1900.20 | 77 | 201 | 90 | 111 |
| 6/1/2022 | 56,603 | 1900.10 | 93 | 198 | 86 | 111 |
| 6/2/2022 | 56,367 | 1900.00 | 78 | 198 | 85 | 112 |
| 6/3/2022 | 56,116 | 1899.80 | 70 | 197 | 83 | 113 |
| 6/4/2022 | 56,003 | 1899.80 | 135 | 192 | 86 | 108 |
| 6/5/2022 | 56,003 | 1899.80 | 184 | 184 | 104 | 101 |
| 6/6/2022 | 55,854 | 1899.70 | 108 | 184 | 95 | 94 |
| 6/7/2022 | 55,649 | 1899.60 | 80 | 184 | 98 | 90 |
| 6/8/2022 | 55,445 | 1899.50 | 77 | 180 | 95 | 90 |
| 6/9/2022 | 55,222 | 1899.40 | 63 | 175 | 89 | 90 |
| 6/10/2022 | 55,019 | 1899.20 | 68 | 170 | 85 | 91 |
| 6/11/2022 | 54,816 | 1899.10 | 67 | 170 | 77 | 99 |
| 6/12/2022 | 54,595 | 1899.00 | 56 | 167 | 75 | 102 |
| 6/13/2022 | 54,375 | 1898.90 | 49 | 160 | 69 | 102 |
| 6/14/2022 | 54,210 | 1898.80 | 69 | 152 | 68 | 97 |
| 6/15/2022 | 54,009 | 1898.70 | 46 | 148 | 65 | 95 |
| 6/16/2022 | 53,808 | 1898.60 | 51 | 153 | 58 | 106 |
| 6/17/2022 | 53,570 | 1898.50 | 35 | 154 | 57 | 111 |
| 6/18/2022 | 53,351 | 1898.30 | 41 | 151 | 55 | 111 |
| 6/19/2022 | 53,170 | 1898.20 | 54 | 146 | 51 | 111 |
| 6/20/2022 | 52,988 | 1898.10 | 50 | 142 | 47 | 110 |
| 6/21/2022 | 52,825 | 1898.00 | 55 | 137 | 42 | 111 |
| 6/22/2022 | 52,645 | 1897.90 | 43 | 134 | 38 | 112 |
| 6/23/2022 | 52,554 | 1897.90 | 89 | 134 | 35 | 115 |
| 6/24/2022 | 52,312 | 1897.80 | 14 | 136 | 34 | 116 |
| 6/25/2022 | 52,069 | 1897.60 | 13 | 135 | 33 | 116 |
| 6/26/2022 | 51,826 | 1897.50 | 10 | 133 | 30 | 116 |
| 6/27/2022 | 51,583 | 1897.40 | 11 | 133 | 26 | 118 |
| 6/28/2022 | 51,351 | 1897.20 | 16 | 134 | 26 | 120 |
| 6/29/2022 | 51,119 | 1897.10 | 17 | 134 | 27 | 120 |
| 6/30/2022 | 50,887 | 1897.00 | 18 | 135 | 27 | 120 |
| 7/1/2022 | 50,657 | 1896.80 | 19 | 135 | 29 | 120 |
| 7/2/2022 | 50,462 | 1896.70 | 35 | 133 | 26 | 120 |
| 7/3/2022 | 50,232 | 1896.60 | 17 | 133 | 26 | 120 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/4/2022 | 50,055 | 1896.50 | 43 | 133 | 26 | 120 |
| 7/5/2022 | 49,844 | 1896.40 | 23 | 130 | 26 | 120 |
| 7/6/2022 | 49,650 | 1896.30 | 31 | 129 | 23 | 119 |
| 7/7/2022 | 49,422 | 1896.10 | 13 | 129 | 22 | 120 |
| 7/8/2022 | 49,194 | 1896.00 | 14 | 129 | 21 | 120 |
| 7/9/2022 | 48,968 | 1895.90 | 14 | 128 | 20 | 120 |
| 7/10/2022 | 48,759 | 1895.80 | 23 | 128 | 20 | 120 |
| 7/11/2022 | 48,534 | 1895.60 | 17 | 131 | 18 | 121 |
| 7/12/2022 | 48,290 | 1895.50 | 9 | 132 | 22 | 120 |
| 7/13/2022 | 48,030 | 1895.30 | 0 | 131 | 20 | 119 |
| 7/14/2022 | 47,806 | 1895.20 | 21 | 134 | 22 | 120 |
| 7/15/2022 | 47,531 | 1895.00 | -1 | 137 | 24 | 120 |
| 7/16/2022 | 47,274 | 1894.90 | 8 | 138 | 27 | 120 |
| 7/17/2022 | 47,018 | 1894.70 | 8 | 137 | 26 | 120 |
| 7/18/2022 | 46,729 | 1894.60 | -9 | 137 | 25 | 120 |
| 7/19/2022 | 46,456 | 1894.40 | -1 | 136 | 24 | 121 |
| 7/20/2022 | 46,185 | 1894.30 | 1 | 138 | 26 | 121 |
| 7/21/2022 | 45,914 | 1894.10 | 0 | 136 | 26 | 120 |
| 7/22/2022 | 45,612 | 1893.90 | -18 | 135 | 24 | 119 |
| 7/23/2022 | 45,377 | 1893.80 | 16 | 134 | 23 | 120 |
| 7/24/2022 | 45,077 | 1893.60 | -17 | 134 | 23 | 120 |
| 7/25/2022 | 44,809 | 1893.40 | -3 | 131 | 21 | 121 |
| 7/26/2022 | 44,560 | 1893.30 | 3 | 129 | 21 | 117 |
| 7/27/2022 | 44,311 | 1893.10 | 3 | 128 | 22 | 115 |
| 7/28/2022 | 44,113 | 1893.00 | -5 | 95 | 22 | 90 |
| 7/29/2022 | 43,981 | 1892.90 | 6 | 73 | 24 | 57 |
| 7/30/2022 | 43,850 | 1892.90 | 2 | 68 | 18 | 55 |
| 7/31/2022 | 43,719 | 1892.80 | 2 | 68 | 17 | 55 |
| 8/1/2022 | 43,604 | 1892.70 | 11 | 69 | 17 | 55 |
| 8/2/2022 | 43,473 | 1892.60 | 3 | 69 | 18 | 55 |
| 8/3/2022 | 43,358 | 1892.60 | 12 | 70 | 17 | 55 |
| 8/4/2022 | 43,211 | 1892.50 | -4 | 71 | 16 | 55 |
| 8/5/2022 | 43,080 | 1892.40 | 7 | 72 | 14 | 55 |
| 8/6/2022 | 42,934 | 1892.30 | -1 | 73 | 18 | 55 |
| 8/7/2022 | 42,788 | 1892.20 | -2 | 72 | 16 | 56 |
| 8/8/2022 | 42,642 | 1892.10 | -2 | 72 | 15 | 56 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/9/2022 | 42,480 | 1892.00 | -10 | 72 | 15 | 56 |
| 8/10/2022 | 42,351 | 1891.90 | 6 | 72 | 15 | 56 |
| 8/11/2022 | 42,190 | 1891.80 | -9 | 72 | 15 | 56 |
| 8/12/2022 | 42,046 | 1891.70 | -1 | 72 | 15 | 56 |
| 8/13/2022 | 41,886 | 1891.60 | -9 | 71 | 15 | 56 |
| 8/14/2022 | 41,741 | 1891.60 | 0 | 73 | 15 | 56 |
| 8/15/2022 | 41,597 | 1891.50 | 4 | 77 | 17 | 56 |
| 8/16/2022 | 41,453 | 1891.40 | 0 | 72 | 15 | 56 |
| 8/17/2022 | 41,326 | 1891.30 | 7 | 71 | 12 | 56 |
| 8/18/2022 | 41,151 | 1891.20 | -14 | 74 | 12 | 55 |
| 8/19/2022 | 40,993 | 1891.10 | -5 | 75 | 13 | 56 |
| 8/20/2022 | 40,850 | 1891.00 | 4 | 76 | 15 | 56 |
| 8/21/2022 | 40,693 | 1890.90 | -7 | 72 | 19 | 50 |
| 8/22/2022 | 40,551 | 1890.80 | -3 | 68 | 19 | 47 |
| 8/23/2022 | 40,426 | 1890.70 | 4 | 67 | 15 | 48 |
| 8/24/2022 | 40,301 | 1890.70 | 9 | 72 | 19 | 47 |
| 8/25/2022 | 40,081 | 1890.50 | 10 | 75 | 19 | 50 |
| 8/26/2022 | 39,925 | 1890.40 | 1 | 80 | 20 | 53 |
| 8/27/2022 | 39,769 | 1890.30 | 2 | 81 | 21 | 54 |
| 8/28/2022 | 39,613 | 1890.20 | 2 | 80 | 19 | 56 |
| 8/29/2022 | 39,474 | 1890.10 | 10 | 80 | 18 | 56 |
| 8/30/2022 | 39,303 | 1890.00 | -4 | 82 | 18 | 56 |
| 8/31/2022 | 39,149 | 1889.90 | 5 | 82 | 19 | 56 |
| 9/1/2022 | 38,980 | 1889.80 | -3 | 82 | 18 | 56 |
| 9/2/2022 | 38,812 | 1889.70 | -4 | 81 | 19 | 51 |
| 9/3/2022 | 38,658 | 1889.60 | 5 | 82 | 18 | 50 |
| 9/4/2022 | 38,505 | 1889.50 | 4 | 81 | 21 | 50 |
| 9/5/2022 | 38,368 | 1889.40 | 7 | 76 | 20 | 55 |
| 9/6/2022 | 38,216 | 1889.30 | -1 | 76 | 16 | 57 |
| 9/7/2022 | 38,049 | 1889.20 | -4 | 80 | 20 | 56 |
| 9/8/2022 | 37,883 | 1889.10 | 1 | 85 | 25 | 55 |
| 9/9/2022 | 37,702 | 1889.00 | -2 | 89 | 24 | 56 |
| 9/10/2022 | 37,521 | 1888.80 | -5 | 86 | 26 | 56 |
| 9/11/2022 | 37,342 | 1888.70 | -6 | 84 | 26 | 56 |
| 9/12/2022 | 37,192 | 1888.60 | 6 | 81 | 30 | 50 |
| 9/13/2022 | 37,013 | 1888.50 | -14 | 76 | 30 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/14/2022 | 36,864 | 1888.40 | -1 | 74 | 33 | 43 |
| 9/15/2022 | 36,731 | 1888.30 | 4 | 72 | 33 | 42 |
| 9/16/2022 | 36,598 | 1888.20 | 1 | 68 | 37 | 38 |
| 9/17/2022 | 36,487 | 1888.10 | 6 | 62 | 41 | 29 |
| 9/18/2022 | 36,553 | 1888.20 | 91 | 58 | 46 | 25 |
| 9/19/2022 | 36,672 | 1888.30 | 111 | 51 | 47 | 20 |
| 9/20/2022 | 36,642 | 1888.20 | 67 | 82 | 57 | 17 |
| 9/21/2022 | 36,494 | 1888.10 | 32 | 106 | 91 | 17 |
| 9/22/2022 | 36,332 | 1888.00 | 26 | 108 | 96 | 17 |
| 9/23/2022 | 36,185 | 1887.90 | 30 | 104 | 93 | 16 |
| 9/24/2022 | 36,024 | 1887.80 | 20 | 101 | 93 | 15 |
| 9/25/2022 | 35,864 | 1887.70 | 19 | 100 | 90 | 15 |
| 9/26/2022 | 35,689 | 1887.60 | 12 | 100 | 91 | 15 |
| 9/27/2022 | 35,515 | 1887.50 | 13 | 101 | 91 | 15 |
| 9/28/2022 | 35,328 | 1887.40 | 9 | 104 | 92 | 15 |
| 9/29/2022 | 35,083 | 1887.20 | 10 | 104 | 93 | 15 |
| 9/30/2022 | 34,897 | 1887.10 | 12 | 105 | 91 | 17 |
| 10/1/2022 | 34,740 | 1886.90 | 9 | 88 | 108 | 20 |
| 10/2/2022 | 34,612 | 1886.90 | 9 | 73 | 79 | 20 |
| 10/3/2022 | 34,499 | 1886.80 | 15 | 72 | 73 | 17 |
| 10/4/2022 | 34,386 | 1886.70 | 10 | 67 | 65 | 15 |
| 10/5/2022 | 34,273 | 1886.60 | 8 | 65 | 54 | 15 |
| 10/6/2022 | 34,174 | 1886.50 | 15 | 65 | 54 | 15 |
| 10/7/2022 | 34,075 | 1886.50 | 15 | 65 | 54 | 15 |
| 10/8/2022 | 33,935 | 1886.40 | 2 | 73 | 55 | 18 |
| 10/9/2022 | 33,777 | 1886.30 | -2 | 78 | 58 | 25 |
| 10/10/2022 | 33,620 | 1886.10 | 7 | 86 | 59 | 27 |
| 10/11/2022 | 33,449 | 1886.00 | 6 | 92 | 61 | 35 |
| 10/12/2022 | 33,293 | 1885.90 | 13 | 92 | 62 | 35 |
| 10/13/2022 | 33,124 | 1885.80 | 7 | 92 | 62 | 35 |
| 10/14/2022 | 32,941 | 1885.70 | 9 | 101 | 65 | 36 |
| 10/15/2022 | 32,745 | 1885.50 | 9 | 107 | 65 | 45 |
| 10/16/2022 | 32,550 | 1885.40 | 9 | 107 | 65 | 45 |
| 10/17/2022 | 32,355 | 1885.20 | 6 | 104 | 67 | 42 |
| 10/18/2022 | 32,189 | 1885.10 | 18 | 102 | 66 | 41 |
| 10/19/2022 | 31,997 | 1885.00 | 6 | 102 | 67 | 41 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/20/2022 | 31,836 | 1884.80 | 16 | 97 | 67 | 38 |
| 10/21/2022 | 31,675 | 1884.70 | 10 | 91 | 72 | 29 |
| 10/22/2022 | 31,488 | 1884.60 | -7 | 87 | 70 | 25 |
| 10/23/2022 | 31,342 | 1884.50 | 12 | 86 | 69 | 25 |
| 10/24/2022 | 31,183 | 1884.30 | 5 | 85 | 70 | 25 |
| 10/25/2022 | 31,037 | 1884.20 | 13 | 86 | 70 | 25 |
| 10/26/2022 | 30,879 | 1884.10 | 6 | 86 | 70 | 25 |
| 10/27/2022 | 30,734 | 1884.00 | 13 | 86 | 71 | 25 |
| 10/28/2022 | 30,590 | 1883.90 | 13 | 85 | 71 | 25 |
| 10/29/2022 | 30,447 | 1883.80 | 13 | 86 | 71 | 25 |
| 10/30/2022 | 30,291 | 1883.70 | 7 | 85 | 72 | 25 |
| 10/31/2022 | 30,148 | 1883.60 | 14 | 86 | 72 | 24 |
| 11/1/2022 | 30,084 | 1883.50 | 42 | 74 | 81 | 11 |
| 11/2/2022 | 29,980 | 1883.40 | 20 | 72 | 73 | 11 |
| 11/3/2022 | 29,877 | 1883.40 | 19 | 71 | 72 | 11 |
| 11/4/2022 | 29,800 | 1883.30 | 31 | 70 | 72 | 11 |
| 11/5/2022 | 29,736 | 1883.20 | 36 | 69 | 74 | 11 |
| 11/6/2022 | 29,736 | 1883.20 | 69 | 69 | 73 | 11 |
| 11/7/2022 | 29,749 | 1883.30 | 77 | 70 | 84 | 11 |
| 11/8/2022 | 29,903 | 1883.40 | 152 | 74 | 90 | 11 |
| 11/9/2022 | 29,903 | 1883.40 | 82 | 82 | 84 | 11 |
| 11/10/2022 | 29,813 | 1883.30 | 48 | 94 | 91 | 11 |
| 11/11/2022 | 29,711 | 1883.20 | 50 | 102 | 98 | 11 |
| 11/12/2022 | 29,608 | 1883.10 | 53 | 105 | 101 | 11 |
| 11/13/2022 | 29,481 | 1883.00 | 43 | 107 | 102 | 11 |
| 11/14/2022 | 29,328 | 1882.90 | 31 | 108 | 104 | 11 |
| 11/15/2022 | 29,227 | 1882.80 | 50 | 102 | 101 | 11 |
| 11/16/2022 | 29,126 | 1882.80 | 40 | 91 | 93 | 11 |
| 11/17/2022 | 29,037 | 1882.70 | 36 | 80 | 85 | 11 |
| 11/18/2022 | 28,962 | 1882.60 | 37 | 75 | 79 | 11 |
| 11/19/2022 | 28,886 | 1882.60 | 36 | 74 | 76 | 11 |
| 11/20/2022 | 28,811 | 1882.50 | 35 | 73 | 76 | 11 |
| 11/21/2022 | 28,723 | 1882.40 | 29 | 73 | 76 | 11 |
| 11/22/2022 | 28,648 | 1882.40 | 36 | 74 | 76 | 11 |
| 11/23/2022 | 28,561 | 1882.30 | 29 | 73 | 76 | 11 |
| 11/24/2022 | 28,499 | 1882.30 | 43 | 74 | 76 | 11 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/25/2022 | 28,412 | 1882.20 | 30 | 74 | 76 | 11 |
| 11/26/2022 | 28,338 | 1882.10 | 36 | 74 | 76 | 11 |
| 11/27/2022 | 28,264 | 1882.10 | 36 | 74 | 76 | 11 |
| 11/28/2022 | 28,177 | 1882.00 | 30 | 74 | 76 | 11 |
| 11/29/2022 | 27,965 | 1881.80 | -33 | 74 | 76 | 12 |
| 11/30/2022 | 27,864 | 1881.70 | 37 | 88 | 76 | 12 |
| 12/1/2022 | 28,008 | 1881.90 | 220 | 148 | 159 | 12 |
| 12/2/2022 | 27,944 | 1881.80 | 110 | 143 | 142 | 11 |
| 12/3/2022 | 27,806 | 1881.70 | 73 | 143 | 137 | 12 |
| 12/4/2022 | 27,803 | 1881.70 | 128 | 129 | 136 | 12 |
| 12/5/2022 | 28,403 | 1882.20 | 431 | 128 | 225 | 11 |
| 12/6/2022 | 28,562 | 1882.30 | 207 | 127 | 162 | 11 |
| 12/7/2022 | 28,509 | 1882.30 | 121 | 148 | 145 | 11 |
| 12/8/2022 | 28,518 | 1882.30 | 165 | 160 | 165 | 11 |
| 12/9/2022 | 28,590 | 1882.30 | 194 | 158 | 196 | 11 |
| 12/10/2022 | 30,840 | 1884.10 | 1,305 | 171 | 419 | 11 |
| 12/11/2022 | 32,130 | 1885.10 | 828 | 177 | 395 | 11 |
| 12/12/2022 | 32,438 | 1885.30 | 328 | 172 | 230 | 11 |
| 12/13/2022 | 32,530 | 1885.40 | 213 | 167 | 191 | 11 |
| 12/14/2022 | 32,540 | 1885.40 | 172 | 167 | 178 | 11 |
| 12/15/2022 | 32,510 | 1885.30 | 152 | 168 | 171 | 11 |
| 12/16/2022 | 32,438 | 1885.30 | 132 | 168 | 167 | 11 |
| 12/17/2022 | 32,367 | 1885.20 | 132 | 168 | 164 | 11 |
| 12/18/2022 | 32,296 | 1885.20 | 133 | 169 | 162 | 11 |
| 12/19/2022 | 32,184 | 1885.10 | 113 | 169 | 160 | 11 |
| 12/20/2022 | 32,086 | 1885.00 | 111 | 161 | 154 | 11 |
| 12/21/2022 | 32,056 | 1885.00 | 128 | 143 | 140 | 11 |
| 12/22/2022 | 32,174 | 1885.10 | 193 | 134 | 128 | 11 |
| 12/23/2022 | 32,415 | 1885.30 | 253 | 132 | 128 | 11 |
| 12/24/2022 | 32,712 | 1885.50 | 280 | 130 | 126 | 11 |
| 12/25/2022 | 33,010 | 1885.70 | 285 | 134 | 125 | 11 |
| 12/26/2022 | 33,643 | 1886.20 | 473 | 154 | 146 | 11 |
| 12/27/2022 | 46,862 | 1894.70 | 6,873 | 209 | 1,469 | 34 |
| 12/28/2022 | 49,643 | 1896.30 | 1,617 | 215 | 439 | 45 |
| 12/29/2022 | 52,201 | 1897.70 | 1,510 | 220 | 395 | 45 |
| 12/30/2022 | 65,043 | 1904.40 | 9,644 | 3,170 | 4,171 | 34 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/31/2022 | 60,753 | 1902.30 | 7,402 | 9,565 | 9,492 | 10 |
| 1/1/2023 | 58,843 | 1901.30 | 2,832 | 3,796 | 3,482 | 45 |
| 1/2/2023 | 58,051 | 1900.90 | 1,782 | 2,181 | 2,037 | 45 |
| 1/3/2023 | 57,514 | 1900.60 | 1,214 | 1,485 | 1,492 | 45 |
| 1/4/2023 | 59,855 | 1901.80 | 2,639 | 1,459 | 1,660 | 45 |
| 1/5/2023 | 62,430 | 1903.10 | 9,039 | 7,741 | 7,503 | 9 |
| 1/6/2023 | 60,460 | 1902.10 | 4,551 | 5,544 | 5,387 | 22 |
| 1/7/2023 | 61,469 | 1902.60 | 5,267 | 4,758 | 4,672 | 46 |
| 1/8/2023 | 62,465 | 1903.10 | 7,818 | 7,315 | 7,383 | 14 |
| 1/9/2023 | 63,461 | 1903.60 | 9,678 | 9,175 | 9,196 | 2 |
| 1/10/2023 | 62,698 | 1903.20 | 7,477 | 7,861 | 7,847 | 2 |
| 1/11/2023 | 62,858 | 1903.30 | 7,498 | 7,417 | 7,471 | 2 |
| 1/12/2023 | 61,625 | 1902.70 | 7,570 | 8,192 | 8,071 | 2 |
| 1/13/2023 | 62,878 | 1903.30 | 7,250 | 6,618 | 6,147 | 14 |
| 1/14/2023 | 63,945 | 1903.90 | 9,938 | 9,401 | 9,538 | 2 |
| 1/15/2023 | 61,427 | 1902.60 | 6,441 | 7,710 | 8,295 | 2 |
| 1/16/2023 | 59,877 | 1901.80 | 4,006 | 4,787 | 4,631 | 43 |
| 1/17/2023 | 58,927 | 1901.30 | 2,690 | 3,169 | 3,034 | 45 |
| 1/18/2023 | 58,715 | 1901.20 | 2,260 | 2,367 | 2,288 | 45 |
| 1/19/2023 | 58,293 | 1901.00 | 1,985 | 2,198 | 2,120 | 45 |
| 1/20/2023 | 57,892 | 1900.80 | 1,527 | 1,729 | 1,715 | 45 |
| 1/21/2023 | 57,645 | 1900.60 | 1,262 | 1,386 | 1,448 | 45 |
| 1/22/2023 | 57,454 | 1900.50 | 1,082 | 1,178 | 1,269 | 45 |
| 1/23/2023 | 57,321 | 1900.50 | 961 | 1,028 | 1,130 | 45 |
| 1/24/2023 | 57,170 | 1900.40 | 833 | 910 | 1,016 | 45 |
| 1/25/2023 | 57,454 | 1900.50 | 949 | 805 | 907 | 45 |
| 1/26/2023 | 57,359 | 1900.50 | 727 | 775 | 868 | 45 |
| 1/27/2023 | 57,283 | 1900.50 | 689 | 728 | 813 | 45 |
| 1/28/2023 | 57,264 | 1900.40 | 691 | 701 | 776 | 45 |
| 1/29/2023 | 57,208 | 1900.40 | 645 | 673 | 743 | 45 |
| 1/30/2023 | 57,113 | 1900.40 | 580 | 628 | 692 | 44 |
| 1/31/2023 | 57,037 | 1900.30 | 537 | 576 | 626 | 44 |
| 2/1/2023 | 56,980 | 1900.30 | 518 | 547 | 580 | 44 |
| 2/2/2023 | 56,924 | 1900.30 | 494 | 523 | 542 | 45 |
| 2/3/2023 | 56,980 | 1900.30 | 554 | 526 | 547 | 45 |
| 2/4/2023 | 57,037 | 1900.30 | 554 | 525 | 538 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/5/2023 | 57,816 | 1900.70 | 1,232 | 839 | 939 | 45 |
| 2/6/2023 | 57,549 | 1900.60 | 858 | 993 | 1,099 | 45 |
| 2/7/2023 | 57,359 | 1900.50 | 704 | 799 | 902 | 45 |
| 2/8/2023 | 57,227 | 1900.40 | 634 | 701 | 790 | 45 |
| 2/9/2023 | 57,132 | 1900.40 | 595 | 643 | 717 | 45 |
| 2/10/2023 | 57,094 | 1900.40 | 588 | 607 | 667 | 45 |
| 2/11/2023 | 57,056 | 1900.30 | 563 | 582 | 633 | 45 |
| 2/12/2023 | 56,980 | 1900.30 | 515 | 553 | 594 | 45 |
| 2/13/2023 | 56,943 | 1900.30 | 512 | 531 | 556 | 45 |
| 2/14/2023 | 56,886 | 1900.20 | 478 | 507 | 522 | 45 |
| 2/15/2023 | 56,829 | 1900.20 | 454 | 483 | 490 | 45 |
| 2/16/2023 | 56,792 | 1900.20 | 440 | 459 | 458 | 45 |
| 2/17/2023 | 56,773 | 1900.20 | 431 | 441 | 430 | 45 |
| 2/18/2023 | 56,716 | 1900.20 | 394 | 422 | 406 | 45 |
| 2/19/2023 | 56,679 | 1900.10 | 387 | 406 | 382 | 45 |
| 2/20/2023 | 56,660 | 1900.10 | 382 | 392 | 362 | 45 |
| 2/21/2023 | 56,679 | 1900.10 | 405 | 396 | 361 | 45 |
| 2/22/2023 | 56,697 | 1900.10 | 408 | 398 | 364 | 45 |
| 2/23/2023 | 56,754 | 1900.20 | 431 | 403 | 370 | 45 |
| 2/24/2023 | 56,773 | 1900.20 | 452 | 443 | 421 | 45 |
| 2/25/2023 | 56,735 | 1900.20 | 408 | 427 | 403 | 45 |
| 2/26/2023 | 56,924 | 1900.30 | 547 | 452 | 448 | 45 |
| 2/27/2023 | 57,132 | 1900.40 | 660 | 555 | 589 | 45 |
| 2/28/2023 | 57,283 | 1900.50 | 732 | 656 | 744 | 45 |
| 3/1/2023 | 57,170 | 1900.40 | 603 | 661 | 756 | 45 |
| 3/2/2023 | 57,208 | 1900.40 | 646 | 626 | 707 | 45 |
| 3/3/2023 | 57,340 | 1900.50 | 755 | 688 | 772 | 45 |
| 3/4/2023 | 57,511 | 1900.60 | 865 | 778 | 875 | 45 |
| 3/5/2023 | 57,530 | 1900.60 | 871 | 861 | 983 | 45 |
| 3/6/2023 | 57,454 | 1900.50 | 778 | 817 | 948 | 45 |
| 3/7/2023 | 57,454 | 1900.50 | 777 | 777 | 914 | 45 |
| 3/8/2023 | 57,568 | 1900.60 | 904 | 846 | 1,019 | 45 |
| 3/9/2023 | 59,178 | 1901.40 | 1,838 | 1,027 | 1,230 | 45 |
| 3/10/2023 | 60,560 | 1902.20 | 4,966 | 4,269 | 4,496 | 45 |
| 3/11/2023 | 59,586 | 1901.70 | 2,777 | 3,269 | 3,274 | 45 |
| 3/12/2023 | 62,498 | 1903.10 | 5,164 | 3,695 | 3,990 | 40 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/13/2023 | 64,066 | 1903.90 | 9,495 | 8,705 | 9,676 | 2 |
| 3/14/2023 | 67,407 | 1905.50 | 14,895 | 13,211 | 15,903 | 2 |
| 3/15/2023 | 62,718 | 1903.20 | 9,305 | 11,669 | 14,610 | 2 |
| 3/16/2023 | 60,874 | 1902.30 | 4,451 | 5,380 | 6,544 | 27 |
| 3/17/2023 | 59,877 | 1901.80 | 3,191 | 3,694 | 4,368 | 45 |
| 3/18/2023 | 59,392 | 1901.60 | 2,716 | 2,961 | 3,401 | 46 |
| 3/19/2023 | 60,462 | 1902.10 | 3,845 | 3,305 | 3,784 | 45 |
| 3/20/2023 | 60,033 | 1901.90 | 3,521 | 3,738 | 4,345 | 45 |
| 3/21/2023 | 59,877 | 1901.80 | 3,125 | 3,203 | 3,625 | 45 |
| 3/22/2023 | 59,605 | 1901.70 | 3,024 | 3,161 | 3,463 | 45 |
| 3/23/2023 | 59,140 | 1901.40 | 2,477 | 2,712 | 2,982 | 45 |
| 3/24/2023 | 58,735 | 1901.20 | 2,066 | 2,270 | 2,501 | 45 |
| 3/25/2023 | 58,408 | 1901.00 | 1,725 | 1,890 | 2,102 | 45 |
| 3/26/2023 | 58,178 | 1900.90 | 1,484 | 1,600 | 1,793 | 45 |
| 3/27/2023 | 57,987 | 1900.80 | 1,291 | 1,387 | 1,578 | 45 |
| 3/28/2023 | 58,773 | 1901.20 | 2,056 | 1,659 | 1,831 | 45 |
| 3/29/2023 | 58,715 | 1901.20 | 2,052 | 2,081 | 2,350 | 45 |
| 3/30/2023 | 58,446 | 1901.10 | 1,736 | 1,872 | 2,131 | 45 |
| 3/31/2023 | 58,255 | 1901.00 | 1,580 | 1,677 | 1,906 | 45 |
| 4/1/2023 | 58,178 | 1900.90 | 1,523 | 1,562 | 1,772 | 45 |
| 4/2/2023 | 58,159 | 1900.90 | 1,550 | 1,560 | 1,773 | 45 |
| 4/3/2023 | 57,949 | 1900.80 | 1,344 | 1,450 | 1,665 | 45 |
| 4/4/2023 | 57,759 | 1900.70 | 1,167 | 1,263 | 1,497 | 45 |
| 4/5/2023 | 57,645 | 1900.60 | 1,065 | 1,123 | 1,360 | 45 |
| 4/6/2023 | 57,664 | 1900.70 | 1,071 | 1,061 | 1,283 | 45 |
| 4/7/2023 | 58,351 | 1901.00 | 1,798 | 1,452 | 1,632 | 45 |
| 4/8/2023 | 58,217 | 1900.90 | 1,635 | 1,703 | 1,883 | 44 |
| 4/9/2023 | 58,197 | 1900.90 | 1,554 | 1,564 | 1,732 | 44 |
| 4/10/2023 | 58,542 | 1901.10 | 1,892 | 1,718 | 1,821 | 44 |
| 4/11/2023 | 58,927 | 1901.30 | 2,289 | 2,095 | 2,157 | 44 |
| 4/12/2023 | 58,715 | 1901.20 | 2,043 | 2,150 | 2,243 | 44 |
| 4/13/2023 | 58,408 | 1901.00 | 1,685 | 1,840 | 1,956 | 44 |
| 4/14/2023 | 58,178 | 1900.90 | 1,460 | 1,576 | 1,686 | 44 |
| 4/15/2023 | 58,045 | 1900.90 | 1,374 | 1,442 | 1,518 | 78 |
| 4/16/2023 | 58,006 | 1900.80 | 1,365 | 1,384 | 1,439 | 89 |
| 4/17/2023 | 57,968 | 1900.80 | 1,321 | 1,341 | 1,396 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/18/2023 | 57,816 | 1900.70 | 1,179 | 1,256 | 1,341 | 90 |
| 4/19/2023 | 57,607 | 1900.60 | 978 | 1,083 | 1,194 | 90 |
| 4/20/2023 | 57,454 | 1900.50 | 864 | 940 | 1,058 | 90 |
| 4/21/2023 | 57,530 | 1900.60 | 947 | 909 | 1,002 | 90 |
| 4/22/2023 | 57,778 | 1900.70 | 1,176 | 1,051 | 1,102 | 90 |
| 4/23/2023 | 57,968 | 1900.80 | 1,349 | 1,253 | 1,269 | 90 |
| 4/24/2023 | 57,930 | 1900.80 | 1,303 | 1,323 | 1,340 | 90 |
| 4/25/2023 | 57,835 | 1900.70 | 1,189 | 1,237 | 1,280 | 90 |
| 4/26/2023 | 57,816 | 1900.70 | 1,171 | 1,180 | 1,220 | 90 |
| 4/27/2023 | 57,873 | 1900.80 | 1,227 | 1,198 | 1,223 | 90 |
| 4/28/2023 | 57,930 | 1900.80 | 1,282 | 1,253 | 1,260 | 90 |
| 4/29/2023 | 57,854 | 1900.80 | 1,227 | 1,265 | 1,274 | 90 |
| 4/30/2023 | 57,721 | 1900.70 | 1,114 | 1,182 | 1,208 | 90 |
| 5/1/2023 | 57,645 | 1900.60 | 1,027 | 1,065 | 1,107 | 90 |
| 5/2/2023 | 57,588 | 1900.60 | 1,019 | 1,048 | 1,100 | 90 |
| 5/3/2023 | 57,359 | 1900.50 | 802 | 917 | 991 | 90 |
| 5/4/2023 | 57,454 | 1900.50 | 765 | 717 | 791 | 90 |
| 5/5/2023 | 57,511 | 1900.60 | 700 | 671 | 699 | 90 |
| 5/6/2023 | 57,568 | 1900.60 | 753 | 724 | 779 | 90 |
| 5/7/2023 | 57,492 | 1900.60 | 663 | 702 | 750 | 90 |
| 5/8/2023 | 57,492 | 1900.60 | 673 | 673 | 715 | 90 |
| 5/9/2023 | 57,416 | 1900.50 | 613 | 651 | 692 | 90 |
| 5/10/2023 | 57,378 | 1900.50 | 604 | 623 | 653 | 90 |
| 5/11/2023 | 57,378 | 1900.50 | 609 | 609 | 632 | 90 |
| 5/12/2023 | 57,435 | 1900.50 | 656 | 627 | 639 | 89 |
| 5/13/2023 | 57,492 | 1900.60 | 726 | 697 | 695 | 89 |
| 5/14/2023 | 57,378 | 1900.50 | 728 | 786 | 803 | 89 |
| 5/15/2023 | 57,245 | 1900.40 | 701 | 768 | 778 | 117 |
| 5/16/2023 | 57,132 | 1900.40 | 692 | 749 | 737 | 130 |
| 5/17/2023 | 57,056 | 1900.30 | 683 | 721 | 713 | 130 |
| 5/18/2023 | 57,018 | 1900.30 | 674 | 693 | 678 | 130 |
| 5/19/2023 | 56,962 | 1900.30 | 647 | 676 | 656 | 130 |
| 5/20/2023 | 56,905 | 1900.30 | 620 | 649 | 626 | 130 |
| 5/21/2023 | 56,829 | 1900.20 | 582 | 620 | 589 | 130 |
| 5/22/2023 | 56,735 | 1900.20 | 535 | 583 | 547 | 130 |
| 5/23/2023 | 56,660 | 1900.10 | 510 | 548 | 499 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/24/2023 | 56,566 | 1900.10 | 471 | 519 | 451 | 130 |
| 5/25/2023 | 56,434 | 1900.00 | 424 | 490 | 414 | 130 |
| 5/26/2023 | 56,359 | 1900.00 | 422 | 460 | 369 | 130 |
| 5/27/2023 | 56,265 | 1899.90 | 387 | 434 | 334 | 130 |
| 5/28/2023 | 56,209 | 1899.90 | 384 | 413 | 304 | 130 |
| 5/29/2023 | 56,116 | 1899.80 | 353 | 401 | 288 | 130 |
| 5/30/2023 | 56,041 | 1899.80 | 353 | 390 | 274 | 130 |
| 5/31/2023 | 55,947 | 1899.80 | 338 | 385 | 266 | 130 |
| 6/1/2023 | 55,929 | 1899.70 | 356 | 365 | 252 | 130 |
| 6/2/2023 | 55,798 | 1899.70 | 278 | 344 | 228 | 131 |
| 6/3/2023 | 55,686 | 1899.60 | 263 | 319 | 202 | 130 |
| 6/4/2023 | 55,575 | 1899.50 | 244 | 301 | 177 | 130 |
| 6/5/2023 | 55,500 | 1899.50 | 249 | 287 | 164 | 130 |
| 6/6/2023 | 55,426 | 1899.50 | 226 | 263 | 165 | 110 |
| 6/7/2023 | 55,370 | 1899.40 | 213 | 241 | 150 | 100 |
| 6/8/2023 | 55,315 | 1899.40 | 196 | 224 | 137 | 100 |
| 6/9/2023 | 55,352 | 1899.40 | 226 | 208 | 120 | 99 |
| 6/10/2023 | 55,333 | 1899.40 | 190 | 199 | 111 | 99 |
| 6/11/2023 | 55,315 | 1899.40 | 187 | 197 | 107 | 100 |
| 6/12/2023 | 55,426 | 1899.50 | 245 | 188 | 101 | 99 |
| 6/13/2023 | 55,408 | 1899.50 | 173 | 182 | 94 | 99 |
| 6/14/2023 | 55,408 | 1899.50 | 179 | 179 | 90 | 100 |
| 6/15/2023 | 55,389 | 1899.40 | 164 | 174 | 85 | 101 |
| 6/16/2023 | 55,315 | 1899.40 | 132 | 169 | 78 | 104 |
| 6/17/2023 | 55,241 | 1899.40 | 129 | 166 | 73 | 106 |
| 6/18/2023 | 55,130 | 1899.30 | 108 | 164 | 70 | 106 |
| 6/19/2023 | 55,037 | 1899.30 | 117 | 164 | 70 | 105 |
| 6/20/2023 | 54,945 | 1899.20 | 109 | 155 | 67 | 106 |
| 6/21/2023 | 54,908 | 1899.20 | 127 | 146 | 57 | 105 |
| 6/22/2023 | 54,853 | 1899.20 | 113 | 141 | 53 | 104 |
| 6/23/2023 | 54,779 | 1899.10 | 101 | 138 | 50 | 105 |
| 6/24/2023 | 54,724 | 1899.10 | 108 | 136 | 47 | 105 |
| 6/25/2023 | 54,668 | 1899.10 | 107 | 135 | 45 | 106 |
| 6/26/2023 | 54,613 | 1899.00 | 107 | 135 | 46 | 105 |
| 6/27/2023 | 54,558 | 1899.00 | 107 | 135 | 45 | 105 |
| 6/28/2023 | 54,485 | 1899.00 | 102 | 139 | 43 | 108 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/29/2023 | 54,411 | 1898.90 | 104 | 141 | 44 | 110 |
| 6/30/2023 | 54,301 | 1898.90 | 83 | 139 | 41 | 111 |
| 7/1/2023 | 54,192 | 1898.80 | 83 | 138 | 39 | 110 |
| 7/2/2023 | 54,064 | 1898.70 | 74 | 138 | 39 | 110 |
| 7/3/2023 | 53,918 | 1898.60 | 64 | 138 | 38 | 110 |
| 7/4/2023 | 53,771 | 1898.60 | 64 | 138 | 38 | 110 |
| 7/5/2023 | 53,625 | 1898.50 | 64 | 138 | 38 | 110 |
| 7/6/2023 | 53,461 | 1898.40 | 55 | 137 | 37 | 110 |
| 7/7/2023 | 53,297 | 1898.30 | 55 | 138 | 37 | 110 |
| 7/8/2023 | 53,115 | 1898.20 | 46 | 137 | 37 | 110 |
| 7/9/2023 | 52,952 | 1898.10 | 55 | 137 | 37 | 110 |
| 7/10/2023 | 52,789 | 1898.00 | 51 | 133 | 41 | 102 |
| 7/11/2023 | 52,645 | 1897.90 | 57 | 130 | 41 | 99 |
| 7/12/2023 | 52,536 | 1897.90 | 76 | 130 | 40 | 99 |
| 7/13/2023 | 52,357 | 1897.80 | 39 | 130 | 39 | 100 |
| 7/14/2023 | 52,213 | 1897.70 | 58 | 130 | 38 | 100 |
| 7/15/2023 | 52,051 | 1897.60 | 49 | 131 | 38 | 100 |
| 7/16/2023 | 51,871 | 1897.50 | 40 | 131 | 38 | 100 |
| 7/17/2023 | 51,691 | 1897.40 | 40 | 131 | 38 | 100 |
| 7/18/2023 | 51,494 | 1897.30 | 34 | 134 | 37 | 103 |
| 7/19/2023 | 51,369 | 1897.20 | 72 | 136 | 38 | 105 |
| 7/20/2023 | 51,172 | 1897.10 | 37 | 136 | 37 | 106 |
| 7/21/2023 | 50,976 | 1897.00 | 35 | 134 | 36 | 106 |
| 7/22/2023 | 50,763 | 1896.90 | 26 | 133 | 34 | 106 |
| 7/23/2023 | 50,568 | 1896.80 | 35 | 133 | 33 | 106 |
| 7/24/2023 | 50,339 | 1896.70 | 18 | 133 | 34 | 105 |
| 7/25/2023 | 50,109 | 1896.50 | 17 | 133 | 34 | 105 |
| 7/26/2023 | 49,914 | 1896.40 | 36 | 134 | 34 | 105 |
| 7/27/2023 | 49,703 | 1896.30 | 25 | 131 | 36 | 101 |
| 7/28/2023 | 49,474 | 1896.20 | 17 | 132 | 35 | 103 |
| 7/29/2023 | 49,247 | 1896.00 | 19 | 134 | 71 | 66 |
| 7/30/2023 | 49,020 | 1895.90 | 19 | 133 | 33 | 105 |
| 7/31/2023 | 48,811 | 1895.80 | 28 | 133 | 33 | 105 |
| 8/1/2023 | 48,568 | 1895.60 | 11 | 134 | 33 | 105 |
| 8/2/2023 | 48,342 | 1895.50 | 20 | 134 | 34 | 105 |
| 8/3/2023 | 48,100 | 1895.40 | 12 | 135 | 31 | 105 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/4/2023 | 47,806 | 1895.20 | -10 | 138 | 34 | 105 |
| 8/5/2023 | 47,565 | 1895.10 | 16 | 138 | 35 | 105 |
| 8/6/2023 | 47,342 | 1894.90 | 29 | 141 | 35 | 105 |
| 8/7/2023 | 47,103 | 1894.80 | 21 | 141 | 39 | 105 |
| 8/8/2023 | 46,865 | 1894.70 | 15 | 135 | 34 | 105 |
| 8/9/2023 | 46,626 | 1894.50 | 13 | 133 | 31 | 105 |
| 8/10/2023 | 46,422 | 1894.40 | 27 | 130 | 29 | 105 |
| 8/11/2023 | 46,202 | 1894.30 | 18 | 129 | 28 | 105 |
| 8/12/2023 | 45,982 | 1894.10 | 18 | 129 | 27 | 105 |
| 8/13/2023 | 45,763 | 1894.00 | 19 | 129 | 27 | 105 |
| 8/14/2023 | 45,545 | 1893.90 | 19 | 129 | 27 | 105 |
| 8/15/2023 | 45,377 | 1893.80 | 45 | 129 | 26 | 106 |
| 8/16/2023 | 45,160 | 1893.60 | 20 | 129 | 26 | 105 |
| 8/17/2023 | 44,926 | 1893.50 | 14 | 133 | 25 | 107 |
| 8/18/2023 | 44,693 | 1893.40 | 16 | 134 | 24 | 110 |
| 8/19/2023 | 44,477 | 1893.20 | 24 | 133 | 24 | 110 |
| 8/20/2023 | 44,295 | 1893.10 | 41 | 133 | 25 | 111 |
| 8/21/2023 | 44,080 | 1893.00 | 25 | 133 | 25 | 111 |
| 8/22/2023 | 43,866 | 1892.90 | 22 | 130 | 27 | 107 |
| 8/23/2023 | 43,653 | 1892.70 | 21 | 129 | 26 | 104 |
| 8/24/2023 | 43,457 | 1892.60 | 30 | 129 | 25 | 105 |
| 8/25/2023 | 43,211 | 1892.50 | 4 | 128 | 24 | 106 |
| 8/26/2023 | 42,999 | 1892.30 | 21 | 128 | 24 | 106 |
| 8/27/2023 | 42,771 | 1892.20 | 12 | 127 | 24 | 106 |
| 8/28/2023 | 42,545 | 1892.10 | 11 | 125 | 26 | 102 |
| 8/29/2023 | 42,303 | 1891.90 | -1 | 121 | 27 | 97 |
| 8/30/2023 | 42,094 | 1891.80 | 14 | 119 | 26 | 96 |
| 8/31/2023 | 41,886 | 1891.60 | 12 | 117 | 31 | 92 |
| 9/1/2023 | 41,677 | 1891.50 | 10 | 115 | 31 | 90 |
| 9/2/2023 | 41,429 | 1891.40 | -10 | 115 | 31 | 90 |
| 9/3/2023 | 41,237 | 1891.20 | 19 | 116 | 30 | 90 |
| 9/4/2023 | 41,047 | 1891.10 | 20 | 116 | 29 | 90 |
| 9/5/2023 | 40,866 | 1891.00 | 24 | 115 | 28 | 90 |
| 9/6/2023 | 40,646 | 1890.90 | 12 | 124 | 30 | 93 |
| 9/7/2023 | 40,410 | 1890.70 | 17 | 136 | 30 | 103 |
| 9/8/2023 | 40,159 | 1890.60 | 13 | 139 | 29 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/9/2023 | 39,909 | 1890.40 | 13 | 139 | 29 | 111 |
| 9/10/2023 | 39,660 | 1890.20 | 13 | 139 | 29 | 110 |
| 9/11/2023 | 39,412 | 1890.10 | 14 | 139 | 29 | 110 |
| 9/12/2023 | 39,165 | 1889.90 | 13 | 138 | 31 | 107 |
| 9/13/2023 | 38,919 | 1889.80 | 11 | 135 | 31 | 104 |
| 9/14/2023 | 38,674 | 1889.60 | 11 | 134 | 29 | 104 |
| 9/15/2023 | 38,429 | 1889.40 | 10 | 134 | 29 | 105 |
| 9/16/2023 | 38,231 | 1889.30 | 14 | 114 | 33 | 89 |
| 9/17/2023 | 38,049 | 1889.20 | 13 | 105 | 27 | 80 |
| 9/18/2023 | 37,852 | 1889.10 | 5 | 104 | 29 | 77 |
| 9/19/2023 | 37,687 | 1888.90 | 21 | 104 | 31 | 75 |
| 9/20/2023 | 37,506 | 1888.80 | 13 | 104 | 31 | 75 |
| 9/21/2023 | 37,297 | 1888.70 | -2 | 103 | 31 | 73 |
| 9/22/2023 | 37,118 | 1888.60 | 9 | 100 | 35 | 69 |
| 9/23/2023 | 36,939 | 1888.40 | 7 | 97 | 33 | 67 |
| 9/24/2023 | 36,775 | 1888.30 | 17 | 100 | 33 | 69 |
| 9/25/2023 | 36,627 | 1888.20 | 27 | 101 | 32 | 74 |
| 9/26/2023 | 36,465 | 1888.10 | 16 | 98 | 37 | 67 |
| 9/27/2023 | 36,317 | 1888.00 | 22 | 96 | 37 | 64 |
| 9/28/2023 | 36,185 | 1887.90 | 30 | 97 | 36 | 64 |
| 9/29/2023 | 36,024 | 1887.80 | 16 | 97 | 39 | 61 |
| 9/30/2023 | 35,893 | 1887.70 | 26 | 92 | 37 | 60 |
| 10/1/2023 | 35,762 | 1887.60 | 25 | 91 | 34 | 60 |
| 10/2/2023 | 35,602 | 1887.50 | 12 | 93 | 35 | 60 |
| 10/3/2023 | 35,472 | 1887.50 | 25 | 91 | 36 | 60 |
| 10/4/2023 | 35,328 | 1887.40 | 14 | 87 | 34 | 56 |
| 10/5/2023 | 35,213 | 1887.30 | 25 | 83 | 34 | 51 |
| 10/6/2023 | 35,083 | 1887.20 | 15 | 80 | 32 | 51 |
| 10/7/2023 | 34,969 | 1887.10 | 22 | 79 | 30 | 51 |
| 10/8/2023 | 34,840 | 1887.00 | 14 | 79 | 30 | 51 |
| 10/9/2023 | 34,740 | 1886.90 | 29 | 79 | 32 | 51 |
| 10/10/2023 | 34,655 | 1886.90 | 36 | 79 | 33 | 50 |
| 10/11/2023 | 34,527 | 1886.80 | 15 | 79 | 33 | 50 |
| 10/12/2023 | 34,414 | 1886.70 | 20 | 77 | 35 | 46 |
| 10/13/2023 | 34,315 | 1886.60 | 26 | 76 | 35 | 43 |
| 10/14/2023 | 34,216 | 1886.60 | 26 | 76 | 35 | 43 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/15/2023 | 34,118 | 1886.50 | 25 | 75 | 34 | 43 |
| 10/16/2023 | 34,019 | 1886.40 | 25 | 75 | 34 | 43 |
| 10/17/2023 | 33,935 | 1886.40 | 32 | 74 | 35 | 41 |
| 10/18/2023 | 33,739 | 1886.20 | 31 | 73 | 34 | 40 |
| 10/19/2023 | 33,656 | 1886.20 | 31 | 73 | 34 | 40 |
| 10/20/2023 | 33,544 | 1886.10 | 17 | 73 | 33 | 40 |
| 10/21/2023 | 33,433 | 1886.00 | 17 | 73 | 33 | 41 |
| 10/22/2023 | 33,475 | 1886.00 | 94 | 73 | 39 | 41 |
| 10/23/2023 | 33,447 | 1886.00 | 59 | 73 | 40 | 41 |
| 10/24/2023 | 33,364 | 1886.00 | 31 | 73 | 36 | 41 |
| 10/25/2023 | 33,281 | 1885.90 | 31 | 73 | 36 | 41 |
| 10/26/2023 | 33,198 | 1885.80 | 31 | 73 | 35 | 41 |
| 10/27/2023 | 33,102 | 1885.80 | 27 | 75 | 36 | 41 |
| 10/28/2023 | 33,005 | 1885.70 | 26 | 75 | 41 | 38 |
| 10/29/2023 | 32,909 | 1885.60 | 25 | 74 | 40 | 35 |
| 10/30/2023 | 32,813 | 1885.60 | 26 | 75 | 42 | 35 |
| 10/31/2023 | 32,717 | 1885.50 | 25 | 73 | 42 | 35 |
| 11/1/2023 | 32,608 | 1885.40 | 18 | 73 | 40 | 35 |
| 11/2/2023 | 32,526 | 1885.30 | 30 | 71 | 40 | 35 |
| 11/3/2023 | 32,471 | 1885.30 | 42 | 69 | 37 | 35 |
| 11/4/2023 | 32,390 | 1885.20 | 27 | 68 | 35 | 35 |
| 11/5/2023 | 32,322 | 1885.20 | 33 | 67 | 34 | 35 |
| 11/6/2023 | 32,308 | 1885.20 | 61 | 68 | 37 | 35 |
| 11/7/2023 | 32,322 | 1885.20 | 74 | 67 | 36 | 35 |
| 11/8/2023 | 32,241 | 1885.10 | 29 | 70 | 458 | 35 |
| 11/9/2023 | 32,173 | 1885.10 | 37 | 71 | 38 | 35 |
| 11/10/2023 | 32,105 | 1885.00 | 38 | 72 | 38 | 35 |
| 11/11/2023 | 32,038 | 1885.00 | 38 | 72 | 40 | 35 |
| 11/12/2023 | 31,957 | 1884.90 | 31 | 72 | 40 | 35 |
| 11/13/2023 | 31,890 | 1884.90 | 38 | 72 | 40 | 35 |
| 11/14/2023 | 31,863 | 1884.90 | 59 | 72 | 43 | 35 |
| 11/15/2023 | 31,850 | 1884.80 | 66 | 72 | 42 | 35 |
| 11/16/2023 | 31,796 | 1884.80 | 46 | 73 | 41 | 35 |
| 11/17/2023 | 31,782 | 1884.80 | 67 | 73 | 41 | 35 |
| 11/18/2023 | 32,308 | 1885.20 | 341 | 76 | 63 | 35 |
| 11/19/2023 | 32,758 | 1885.50 | 311 | 84 | 72 | 35 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/20/2023 | 32,758 | 1885.50 | 116 | 116 | 80 | 35 |
| 11/21/2023 | 32,649 | 1885.40 | 87 | 142 | 107 | 35 |
| 11/22/2023 | 32,512 | 1885.30 | 88 | 156 | 122 | 35 |
| 11/23/2023 | 32,268 | 1885.20 | 41 | 165 | 135 | 35 |
| 11/24/2023 | 32,051 | 1885.00 | 53 | 162 | 132 | 35 |
| 11/25/2023 | 31,836 | 1884.80 | 51 | 159 | 129 | 35 |
| 11/26/2023 | 31,622 | 1884.70 | 47 | 155 | 127 | 35 |
| 11/27/2023 | 31,462 | 1884.60 | 44 | 125 | 113 | 35 |
| 11/28/2023 | 31,342 | 1884.50 | 39 | 99 | 80 | 35 |
| 11/29/2023 | 31,236 | 1884.40 | 41 | 94 | 78 | 38 |
| 11/30/2023 | 31,183 | 1884.30 | 49 | 76 | 58 | 36 |
| 12/1/2023 | 31,037 | 1884.20 | 39 | 112 | 62 | 35 |
| 12/2/2023 | 30,932 | 1884.20 | 90 | 143 | 112 | 35 |
| 12/3/2023 | 30,945 | 1884.20 | 142 | 136 | 145 | 35 |
| 12/4/2023 | 30,918 | 1884.10 | 120 | 133 | 120 | 35 |
| 12/5/2023 | 30,826 | 1884.10 | 88 | 134 | 109 | 35 |
| 12/6/2023 | 30,774 | 1884.00 | 110 | 136 | 116 | 35 |
| 12/7/2023 | 30,984 | 1884.20 | 242 | 136 | 153 | 35 |
| 12/8/2023 | 31,077 | 1884.30 | 180 | 134 | 135 | 35 |
| 12/9/2023 | 31,011 | 1884.20 | 111 | 144 | 119 | 36 |
| 12/10/2023 | 30,892 | 1884.10 | 108 | 168 | 125 | 0 |
| 12/11/2023 | 30,761 | 1884.00 | 92 | 159 | 126 | 36 |
| 12/12/2023 | 30,617 | 1883.90 | 80 | 152 | 122 | 36 |
| 12/13/2023 | 30,460 | 1883.80 | 69 | 148 | 116 | 36 |
| 12/14/2023 | 30,317 | 1883.70 | 73 | 145 | 114 | 36 |
| 12/15/2023 | 30,161 | 1883.60 | 64 | 143 | 111 | 36 |
| 12/16/2023 | 30,006 | 1883.50 | 63 | 141 | 109 | 36 |
| 12/17/2023 | 29,865 | 1883.30 | 69 | 141 | 111 | 36 |
| 12/18/2023 | 32,159 | 1885.10 | 1,305 | 148 | 268 | 36 |
| 12/19/2023 | 38,231 | 1889.30 | 3,229 | 168 | 632 | 36 |
| 12/20/2023 | 43,129 | 1892.40 | 2,680 | 210 | 728 | 43 |
| 12/21/2023 | 44,960 | 1893.50 | 1,144 | 222 | 342 | 47 |
| 12/22/2023 | 45,780 | 1894.00 | 633 | 219 | 246 | 46 |
| 12/23/2023 | 46,219 | 1894.30 | 441 | 219 | 215 | 46 |
| 12/24/2023 | 46,456 | 1894.40 | 341 | 221 | 201 | 46 |
| 12/25/2023 | 46,592 | 1894.50 | 292 | 224 | 193 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/26/2023 | 46,643 | 1894.50 | 251 | 225 | 189 | 46 |
| 12/27/2023 | 47,240 | 1894.90 | 527 | 226 | 209 | 46 |
| 12/28/2023 | 48,100 | 1895.40 | 663 | 229 | 240 | 46 |
| 12/29/2023 | 50,515 | 1896.80 | 1,454 | 236 | 372 | 46 |
| 12/30/2023 | 53,552 | 1898.40 | 1,768 | 237 | 572 | 46 |
| 12/31/2023 | 54,963 | 1899.20 | 948 | 236 | 373 | 46 |
| 1/1/2024 | 55,817 | 1899.70 | 667 | 237 | 296 | 46 |
| 1/2/2024 | 56,886 | 1900.20 | 820 | 281 | 325 | 46 |
| 1/3/2024 | 57,530 | 1900.60 | 1,135 | 810 | 1,068 | 46 |
| 1/4/2024 | 57,321 | 1900.50 | 713 | 818 | 1,110 | 46 |
| 1/5/2024 | 57,113 | 1900.40 | 576 | 681 | 887 | 46 |
| 1/6/2024 | 57,037 | 1900.30 | 564 | 602 | 758 | 46 |
| 1/7/2024 | 56,905 | 1900.30 | 490 | 557 | 703 | 46 |
| 1/8/2024 | 56,773 | 1900.20 | 417 | 484 | 585 | 46 |
| 1/9/2024 | 56,867 | 1900.20 | 518 | 471 | 571 | 46 |
| 1/10/2024 | 57,549 | 1900.60 | 932 | 588 | 893 | 46 |
| 1/11/2024 | 57,530 | 1900.60 | 888 | 898 | 1,354 | 46 |
| 1/12/2024 | 57,227 | 1900.40 | 606 | 759 | 1,092 | 46 |
| 1/13/2024 | 60,697 | 1902.20 | 3,223 | 1,473 | 2,032 | 46 |
| 1/14/2024 | 59,489 | 1901.60 | 3,265 | 3,874 | 4,680 | 46 |
| 1/15/2024 | 58,389 | 1901.00 | 1,567 | 2,121 | 2,581 | 46 |
| 1/16/2024 | 58,504 | 1901.10 | 1,494 | 1,436 | 1,772 | 46 |
| 1/17/2024 | 58,389 | 1901.00 | 1,666 | 1,724 | 2,085 | 46 |
| 1/18/2024 | 57,949 | 1900.80 | 1,161 | 1,383 | 1,703 | 46 |
| 1/19/2024 | 57,911 | 1900.80 | 1,049 | 1,069 | 1,375 | 47 |
| 1/20/2024 | 60,697 | 1902.20 | 4,013 | 2,608 | 2,739 | 47 |
| 1/21/2024 | 61,822 | 1902.80 | 5,213 | 4,645 | 5,168 | 47 |
| 1/22/2024 | 60,756 | 1902.30 | 5,249 | 5,786 | 6,690 | 31 |
| 1/23/2024 | 59,333 | 1901.50 | 2,835 | 3,552 | 4,230 | 47 |
| 1/24/2024 | 59,430 | 1901.60 | 2,735 | 2,686 | 3,269 | 46 |
| 1/25/2024 | 58,831 | 1901.30 | 1,979 | 2,282 | 2,821 | 46 |
| 1/26/2024 | 58,370 | 1901.00 | 1,494 | 1,727 | 2,213 | 46 |
| 1/27/2024 | 58,121 | 1900.90 | 1,263 | 1,389 | 1,826 | 46 |
| 1/28/2024 | 57,892 | 1900.80 | 1,046 | 1,161 | 1,571 | 46 |
| 1/29/2024 | 57,683 | 1900.70 | 865 | 970 | 1,357 | 46 |
| 1/30/2024 | 57,511 | 1900.60 | 751 | 837 | 1,182 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/31/2024 | 59,721 | 1901.70 | 2,187 | 1,073 | 1,314 | 46 |
| 2/1/2024 | 59,527 | 1901.60 | 3,109 | 3,207 | 3,712 | 46 |
| 2/2/2024 | 58,927 | 1901.30 | 2,005 | 2,308 | 2,870 | 46 |
| 2/3/2024 | 58,466 | 1901.10 | 1,582 | 1,814 | 2,355 | 46 |
| 2/4/2024 | 59,994 | 1901.90 | 2,662 | 1,891 | 2,574 | 40 |
| 2/5/2024 | 60,541 | 1902.10 | 4,254 | 3,978 | 4,933 | 46 |
| 2/6/2024 | 59,508 | 1901.60 | 3,358 | 3,878 | 4,001 | 46 |
| 2/7/2024 | 58,927 | 1901.30 | 2,927 | 3,220 | 2,929 | 46 |
| 2/8/2024 | 58,427 | 1901.10 | 1,524 | 1,777 | 2,311 | 46 |
| 2/9/2024 | 58,083 | 1900.90 | 1,198 | 1,372 | 1,836 | 46 |
| 2/10/2024 | 57,873 | 1900.80 | 1,005 | 1,111 | 1,546 | 46 |
| 2/11/2024 | 57,702 | 1900.70 | 861 | 948 | 1,349 | 46 |
| 2/12/2024 | 57,607 | 1900.60 | 805 | 853 | 1,221 | 46 |
| 2/13/2024 | 57,511 | 1900.60 | 754 | 802 | 1,130 | 46 |
| 2/14/2024 | 58,140 | 1900.90 | 1,157 | 841 | 1,167 | 46 |
| 2/15/2024 | 58,677 | 1901.20 | 1,947 | 1,676 | 2,163 | 46 |
| 2/16/2024 | 58,351 | 1901.00 | 1,392 | 1,557 | 2,137 | 46 |
| 2/17/2024 | 60,189 | 1902.00 | 2,466 | 1,539 | 2,045 | 46 |
| 2/18/2024 | 61,328 | 1902.50 | 4,186 | 3,611 | 4,658 | 46 |
| 2/19/2024 | 63,341 | 1903.60 | 7,366 | 6,351 | 8,132 | 19 |
| 2/20/2024 | 62,798 | 1903.30 | 7,565 | 7,839 | 10,290 | 2 |
| 2/21/2024 | 60,973 | 1902.40 | 4,572 | 5,493 | 7,557 | 14 |
| 2/22/2024 | 59,857 | 1901.80 | 3,032 | 3,595 | 4,713 | 46 |
| 2/23/2024 | 59,178 | 1901.40 | 2,255 | 2,597 | 3,343 | 46 |
| 2/24/2024 | 58,715 | 1901.20 | 1,791 | 2,024 | 2,569 | 46 |
| 2/25/2024 | 58,427 | 1901.10 | 1,517 | 1,663 | 2,113 | 46 |
| 2/26/2024 | 58,197 | 1900.90 | 1,305 | 1,421 | 1,821 | 46 |
| 2/27/2024 | 57,987 | 1900.80 | 1,112 | 1,218 | 1,604 | 46 |
| 2/28/2024 | 57,835 | 1900.70 | 972 | 1,049 | 1,423 | 46 |
| 2/29/2024 | 58,351 | 1901.00 | 1,351 | 1,091 | 1,495 | 46 |
| 3/1/2024 | 60,013 | 1901.90 | 2,726 | 1,888 | 2,581 | 46 |
| 3/2/2024 | 59,624 | 1901.70 | 2,945 | 3,141 | 4,295 | 46 |
| 3/3/2024 | 59,159 | 1901.40 | 2,137 | 2,372 | 3,213 | 46 |
| 3/4/2024 | 58,869 | 1901.30 | 1,850 | 1,996 | 2,696 | 46 |
| 3/5/2024 | 60,697 | 1902.20 | 3,532 | 2,610 | 3,517 | 46 |
| 3/6/2024 | 61,130 | 1902.40 | 4,867 | 4,649 | 6,489 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/7/2024 | 60,169 | 1902.00 | 3,344 | 3,828 | 5,111 | 46 |
| 3/8/2024 | 59,547 | 1901.60 | 2,586 | 2,900 | 3,696 | 46 |
| 3/9/2024 | 59,217 | 1901.50 | 2,151 | 2,317 | 3,034 | 46 |
| 3/10/2024 | 59,237 | 1901.50 | 2,150 | 2,141 | 2,696 | 46 |
| 3/11/2024 | 59,159 | 1901.40 | 2,282 | 2,321 | 2,920 | 46 |
| 3/12/2024 | 59,198 | 1901.50 | 2,283 | 2,264 | 2,876 | 46 |
| 3/13/2024 | 58,947 | 1901.30 | 1,993 | 2,120 | 2,684 | 47 |
| 3/14/2024 | 58,696 | 1901.20 | 1,747 | 1,874 | 2,368 | 46 |
| 3/15/2024 | 58,504 | 1901.10 | 1,554 | 1,651 | 2,095 | 46 |
| 3/16/2024 | 58,331 | 1901.00 | 1,409 | 1,496 | 1,892 | 46 |
| 3/17/2024 | 58,274 | 1901.00 | 1,354 | 1,383 | 1,748 | 46 |
| 3/18/2024 | 58,236 | 1901.00 | 1,319 | 1,338 | 1,671 | 46 |
| 3/19/2024 | 58,255 | 1901.00 | 1,346 | 1,336 | 1,642 | 46 |
| 3/20/2024 | 58,197 | 1900.90 | 1,291 | 1,320 | 1,614 | 46 |
| 3/21/2024 | 58,083 | 1900.90 | 1,178 | 1,236 | 1,520 | 46 |
| 3/22/2024 | 58,523 | 1901.10 | 1,430 | 1,208 | 1,508 | 46 |
| 3/23/2024 | 59,062 | 1901.40 | 2,141 | 1,869 | 2,270 | 46 |
| 3/24/2024 | 58,754 | 1901.20 | 1,840 | 1,996 | 2,557 | 44 |
| 3/25/2024 | 58,370 | 1901.00 | 1,418 | 1,612 | 2,067 | 45 |
| 3/26/2024 | 58,140 | 1900.90 | 1,222 | 1,338 | 1,732 | 46 |
| 3/27/2024 | 58,908 | 1901.30 | 1,683 | 1,296 | 1,716 | 46 |
| 3/28/2024 | 59,159 | 1901.40 | 2,414 | 2,287 | 2,808 | 46 |
| 3/29/2024 | 58,831 | 1901.30 | 1,837 | 2,002 | 2,528 | 46 |
| 3/30/2024 | 58,638 | 1901.20 | 1,689 | 1,786 | 2,238 | 46 |
| 3/31/2024 | 58,427 | 1901.10 | 1,484 | 1,591 | 1,993 | 46 |
| 4/1/2024 | 58,197 | 1900.90 | 1,327 | 1,443 | 1,792 | 46 |
| 4/2/2024 | 58,083 | 1900.90 | 1,227 | 1,285 | 1,611 | 46 |
| 4/3/2024 | 58,083 | 1900.90 | 1,249 | 1,249 | 1,543 | 46 |
| 4/4/2024 | 58,121 | 1900.90 | 1,259 | 1,240 | 1,536 | 46 |
| 4/5/2024 | 57,949 | 1900.80 | 1,103 | 1,189 | 1,513 | 46 |
| 4/6/2024 | 57,740 | 1900.70 | 932 | 1,037 | 1,363 | 46 |
| 4/7/2024 | 57,607 | 1900.60 | 834 | 901 | 1,214 | 46 |
| 4/8/2024 | 57,492 | 1900.60 | 769 | 826 | 1,104 | 52 |
| 4/9/2024 | 57,397 | 1900.50 | 727 | 775 | 1,016 | 55 |
| 4/10/2024 | 57,416 | 1900.50 | 763 | 753 | 975 | 48 |
| 4/11/2024 | 57,473 | 1900.60 | 800 | 771 | 982 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/12/2024 | 57,549 | 1900.60 | 852 | 814 | 1,027 | 46 |
| 4/13/2024 | 57,778 | 1900.70 | 1,022 | 907 | 1,168 | 46 |
| 4/14/2024 | 57,740 | 1900.70 | 934 | 953 | 1,246 | 46 |
| 4/15/2024 | 57,607 | 1900.60 | 858 | 925 | 1,185 | 77 |
| 4/16/2024 | 57,530 | 1900.60 | 833 | 871 | 1,095 | 90 |
| 4/17/2024 | 57,492 | 1900.60 | 818 | 837 | 1,037 | 90 |
| 4/18/2024 | 57,492 | 1900.60 | 836 | 836 | 1,022 | 90 |
| 4/19/2024 | 57,492 | 1900.60 | 833 | 833 | 1,012 | 88 |
| 4/20/2024 | 57,454 | 1900.50 | 807 | 826 | 995 | 90 |
| 4/21/2024 | 57,416 | 1900.50 | 818 | 837 | 952 | 90 |
| 4/22/2024 | 57,378 | 1900.50 | 797 | 816 | 921 | 90 |
| 4/23/2024 | 57,359 | 1900.50 | 794 | 803 | 896 | 90 |
| 4/24/2024 | 57,302 | 1900.50 | 753 | 782 | 869 | 90 |
| 4/25/2024 | 57,208 | 1900.40 | 699 | 747 | 826 | 90 |
| 4/26/2024 | 57,151 | 1900.40 | 677 | 705 | 772 | 90 |
| 4/27/2024 | 57,037 | 1900.30 | 611 | 668 | 726 | 89 |
| 4/28/2024 | 56,943 | 1900.30 | 571 | 618 | 654 | 90 |
| 4/29/2024 | 56,848 | 1900.20 | 527 | 575 | 592 | 90 |
| 4/30/2024 | 56,792 | 1900.20 | 512 | 540 | 535 | 90 |
| 5/1/2024 | 56,716 | 1900.20 | 478 | 516 | 490 | 90 |
| 5/2/2024 | 56,754 | 1900.20 | 472 | 453 | 411 | 90 |
| 5/3/2024 | 56,924 | 1900.30 | 457 | 371 | 318 | 90 |
| 5/4/2024 | 57,588 | 1900.60 | 847 | 513 | 540 | 90 |
| 5/5/2024 | 57,435 | 1900.50 | 603 | 680 | 947 | 90 |
| 5/6/2024 | 57,302 | 1900.50 | 519 | 586 | 773 | 90 |
| 5/7/2024 | 57,245 | 1900.40 | 499 | 527 | 650 | 90 |
| 5/8/2024 | 57,151 | 1900.40 | 445 | 493 | 573 | 90 |
| 5/9/2024 | 57,094 | 1900.40 | 424 | 453 | 499 | 90 |
| 5/10/2024 | 57,094 | 1900.40 | 432 | 432 | 444 | 90 |
| 5/11/2024 | 57,094 | 1900.40 | 433 | 433 | 433 | 90 |
| 5/12/2024 | 57,018 | 1900.30 | 422 | 460 | 450 | 90 |
| 5/13/2024 | 56,867 | 1900.20 | 406 | 482 | 502 | 90 |
| 5/14/2024 | 56,773 | 1900.20 | 401 | 449 | 450 | 101 |
| 5/15/2024 | 56,735 | 1900.20 | 387 | 406 | 329 | 130 |
| 5/16/2024 | 56,697 | 1900.10 | 369 | 388 | 296 | 130 |
| 5/17/2024 | 56,622 | 1900.10 | 355 | 393 | 295 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/18/2024 | 56,472 | 1900.00 | 334 | 409 | 311 | 130 |
| 5/19/2024 | 56,228 | 1899.90 | 292 | 415 | 321 | 130 |
| 5/20/2024 | 56,022 | 1899.80 | 285 | 389 | 280 | 130 |
| 5/21/2024 | 55,798 | 1899.70 | 266 | 379 | 262 | 130 |
| 5/22/2024 | 55,519 | 1899.50 | 230 | 371 | 249 | 130 |
| 5/23/2024 | 55,333 | 1899.40 | 266 | 359 | 258 | 112 |
| 5/24/2024 | 55,111 | 1899.30 | 204 | 316 | 266 | 90 |
| 5/25/2024 | 54,945 | 1899.20 | 213 | 297 | 245 | 89 |
| 5/26/2024 | 54,779 | 1899.10 | 218 | 302 | 221 | 90 |
| 5/27/2024 | 54,613 | 1899.00 | 214 | 297 | 223 | 90 |
| 5/28/2024 | 54,430 | 1898.90 | 200 | 292 | 206 | 90 |
| 5/29/2024 | 54,210 | 1898.80 | 180 | 291 | 198 | 94 |
| 5/30/2024 | 54,009 | 1898.70 | 188 | 289 | 194 | 96 |
| 5/31/2024 | 53,790 | 1898.60 | 171 | 282 | 187 | 96 |
| 6/1/2024 | 53,552 | 1898.40 | 157 | 277 | 180 | 96 |
| 6/2/2024 | 53,333 | 1898.30 | 159 | 269 | 175 | 96 |
| 6/3/2024 | 53,133 | 1898.20 | 160 | 260 | 168 | 96 |
| 6/4/2024 | 52,934 | 1898.10 | 146 | 247 | 155 | 96 |
| 6/5/2024 | 52,861 | 1898.10 | 193 | 230 | 139 | 96 |
| 6/6/2024 | 52,681 | 1898.00 | 130 | 221 | 125 | 99 |
| 6/7/2024 | 52,500 | 1897.90 | 126 | 217 | 120 | 100 |
| 6/8/2024 | 52,339 | 1897.80 | 126 | 208 | 112 | 100 |
| 6/9/2024 | 52,141 | 1897.70 | 96 | 195 | 103 | 100 |
| 6/10/2024 | 51,979 | 1897.60 | 102 | 184 | 95 | 100 |
| 6/11/2024 | 51,817 | 1897.50 | 98 | 180 | 86 | 103 |
| 6/12/2024 | 51,655 | 1897.40 | 93 | 174 | 96 | 0 |
| 6/13/2024 | 51,476 | 1897.30 | 79 | 169 | 94 | 0 |
| 6/14/2024 | 51,297 | 1897.20 | 77 | 168 | 76 | 100 |
| 6/15/2024 | 51,101 | 1897.10 | 63 | 162 | 74 | 100 |
| 6/16/2024 | 50,923 | 1897.00 | 69 | 159 | 71 | 100 |
| 6/17/2024 | 50,745 | 1896.90 | 67 | 157 | 69 | 100 |
| 6/18/2024 | 50,586 | 1896.80 | 72 | 152 | 66 | 100 |
| 6/19/2024 | 50,427 | 1896.70 | 67 | 147 | 62 | 100 |
| 6/20/2024 | 50,286 | 1896.60 | 74 | 145 | 59 | 100 |
| 6/21/2024 | 50,126 | 1896.50 | 63 | 144 | 59 | 97 |
| 6/22/2024 | 49,967 | 1896.40 | 63 | 143 | 57 | 99 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/23/2024 | 49,808 | 1896.40 | 61 | 141 | 52 | 100 |
| 6/24/2024 | 49,632 | 1896.30 | 51 | 139 | 51 | 100 |
| 6/25/2024 | 49,509 | 1896.20 | 79 | 141 | 47 | 103 |
| 6/26/2024 | 49,334 | 1896.10 | 54 | 142 | 46 | 105 |
| 6/27/2024 | 49,142 | 1896.00 | 37 | 134 | 51 | 93 |
| 6/28/2024 | 49,037 | 1895.90 | 58 | 111 | 50 | 70 |
| 6/29/2024 | 48,915 | 1895.80 | 41 | 102 | 42 | 70 |
| 6/30/2024 | 48,794 | 1895.80 | 38 | 99 | 37 | 70 |
| 7/1/2024 | 48,690 | 1895.70 | 48 | 100 | 37 | 70 |
| 7/2/2024 | 48,603 | 1895.70 | 56 | 100 | 37 | 70 |
| 7/3/2024 | 48,481 | 1895.60 | 38 | 100 | 35 | 70 |
| 7/4/2024 | 48,360 | 1895.50 | 33 | 95 | 36 | 64 |
| 7/5/2024 | 48,256 | 1895.50 | 39 | 92 | 35 | 60 |
| 7/6/2024 | 48,152 | 1895.40 | 36 | 89 | 34 | 60 |
| 7/7/2024 | 48,030 | 1895.30 | 25 | 86 | 32 | 60 |
| 7/8/2024 | 47,927 | 1895.30 | 31 | 84 | 30 | 60 |
| 7/9/2024 | 47,806 | 1895.20 | 22 | 83 | 29 | 60 |
| 7/10/2024 | 47,686 | 1895.10 | 22 | 83 | 29 | 60 |
| 7/11/2024 | 47,582 | 1895.10 | 31 | 83 | 29 | 60 |
| 7/12/2024 | 47,454 | 1895.00 | 18 | 83 | 28 | 60 |
| 7/13/2024 | 47,342 | 1894.90 | 28 | 84 | 28 | 60 |
| 7/14/2024 | 47,206 | 1894.90 | 14 | 82 | 28 | 60 |
| 7/15/2024 | 47,086 | 1894.80 | 21 | 82 | 27 | 60 |
| 7/16/2024 | 46,950 | 1894.70 | 13 | 81 | 26 | 16 |
| 7/17/2024 | 46,814 | 1894.60 | 12 | 81 | 26 | 58 |
| 7/18/2024 | 46,677 | 1894.50 | 12 | 81 | 26 | 58 |
| 7/19/2024 | 46,541 | 1894.50 | 9 | 78 | 31 | 56 |
| 7/20/2024 | 46,405 | 1894.40 | 11 | 79 | 33 | 56 |
| 7/21/2024 | 46,269 | 1894.30 | 13 | 81 | 28 | 56 |
| 7/22/2024 | 46,151 | 1894.20 | 20 | 79 | 26 | 56 |
| 7/23/2024 | 46,016 | 1894.20 | 11 | 79 | 25 | 56 |
| 7/24/2024 | 45,864 | 1894.10 | 8 | 85 | 26 | 57 |
| 7/25/2024 | 45,713 | 1894.00 | 11 | 87 | 27 | 60 |
| 7/26/2024 | 45,561 | 1893.90 | 10 | 86 | 27 | 60 |
| 7/27/2024 | 45,411 | 1893.80 | 8 | 84 | 26 | 60 |
| 7/28/2024 | 45,244 | 1893.70 | 0 | 84 | 25 | 60 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/29/2024 | 45,110 | 1893.60 | 15 | 82 | 25 | 60 |
| 7/30/2024 | 44,960 | 1893.50 | 6 | 82 | 23 | 60 |
| 7/31/2024 | 44,826 | 1893.40 | 15 | 82 | 23 | 60 |
| 8/1/2024 | 44,676 | 1893.40 | 8 | 84 | 23 | 60 |
| 8/2/2024 | 44,527 | 1893.30 | 13 | 88 | 24 | 60 |
| 8/3/2024 | 44,361 | 1893.20 | 6 | 90 | 25 | 60 |
| 8/4/2024 | 44,245 | 1893.10 | 38 | 96 | 26 | 60 |
| 8/5/2024 | 44,047 | 1893.00 | 4 | 104 | 29 | 60 |
| 8/6/2024 | 43,833 | 1892.80 | -4 | 104 | 33 | 60 |
| 8/7/2024 | 43,669 | 1892.70 | 12 | 94 | 27 | 60 |
| 8/8/2024 | 43,506 | 1892.70 | 5 | 87 | 23 | 60 |
| 8/9/2024 | 43,342 | 1892.50 | 3 | 86 | 22 | 60 |
| 8/10/2024 | 43,195 | 1892.50 | 10 | 84 | 21 | 60 |
| 8/11/2024 | 43,032 | 1892.40 | 1 | 84 | 21 | 60 |
| 8/12/2024 | 42,869 | 1892.30 | 1 | 83 | 20 | 60 |
| 8/13/2024 | 42,723 | 1892.20 | 8 | 82 | 20 | 59 |
| 8/14/2024 | 42,577 | 1892.10 | 9 | 82 | 20 | 59 |
| 8/15/2024 | 42,415 | 1892.00 | 1 | 82 | 20 | 59 |
| 8/16/2024 | 42,287 | 1891.90 | 18 | 83 | 20 | 60 |
| 8/17/2024 | 42,142 | 1891.80 | 10 | 83 | 19 | 61 |
| 8/18/2024 | 41,998 | 1891.70 | 10 | 83 | 19 | 61 |
| 8/19/2024 | 41,853 | 1891.60 | 10 | 83 | 19 | 61 |
| 8/20/2024 | 41,709 | 1891.50 | 9 | 82 | 20 | 56 |
| 8/21/2024 | 41,581 | 1891.50 | 16 | 80 | 21 | 53 |
| 8/22/2024 | 41,421 | 1891.40 | -1 | 79 | 21 | 53 |
| 8/23/2024 | 41,310 | 1891.30 | 20 | 76 | 21 | 50 |
| 8/24/2024 | 41,215 | 1891.20 | 26 | 74 | 22 | 47 |
| 8/25/2024 | 41,135 | 1891.20 | 32 | 72 | 20 | 47 |
| 8/26/2024 | 41,040 | 1891.10 | 23 | 71 | 19 | 47 |
| 8/27/2024 | 40,929 | 1891.00 | 17 | 73 | 19 | 47 |
| 8/28/2024 | 40,819 | 1891.00 | 19 | 75 | 21 | 47 |
| 8/29/2024 | 40,677 | 1890.90 | 6 | 77 | 20 | 47 |
| 8/30/2024 | 40,536 | 1890.80 | 8 | 79 | 22 | 49 |
| 8/31/2024 | 40,394 | 1890.70 | 11 | 82 | 23 | 53 |
| 9/1/2024 | 40,253 | 1890.60 | 11 | 82 | 23 | 53 |
| 9/2/2024 | 40,097 | 1890.50 | 3 | 82 | 24 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/3/2024 | 39,956 | 1890.40 | 11 | 82 | 25 | 51 |
| 9/4/2024 | 39,800 | 1890.30 | 3 | 82 | 22 | 52 |
| 9/5/2024 | 39,567 | 1890.20 | -36 | 82 | 23 | 52 |
| 9/6/2024 | 39,443 | 1890.10 | 16 | 79 | 23 | 51 |
| 9/7/2024 | 39,303 | 1890.00 | 5 | 75 | 24 | 44 |
| 9/8/2024 | 39,165 | 1889.90 | 5 | 75 | 24 | 42 |
| 9/9/2024 | 39,026 | 1889.80 | 8 | 78 | 25 | 42 |
| 9/10/2024 | 38,888 | 1889.70 | 9 | 78 | 26 | 42 |
| 9/11/2024 | 38,735 | 1889.60 | 1 | 79 | 26 | 42 |
| 9/12/2024 | 38,612 | 1889.60 | 17 | 79 | 26 | 42 |
| 9/13/2024 | 38,490 | 1889.50 | 17 | 79 | 26 | 42 |
| 9/14/2024 | 38,338 | 1889.40 | 2 | 79 | 27 | 42 |
| 9/15/2024 | 38,185 | 1889.30 | 2 | 79 | 27 | 41 |
| 9/16/2024 | 38,079 | 1889.20 | 32 | 85 | 28 | 44 |
| 9/17/2024 | 37,928 | 1889.10 | 16 | 92 | 30 | 51 |
| 9/18/2024 | 37,792 | 1889.00 | 24 | 92 | 30 | 51 |
| 9/19/2024 | 37,641 | 1888.90 | 15 | 90 | 30 | 52 |
| 9/20/2024 | 37,506 | 1888.80 | 14 | 82 | 28 | 49 |
| 9/21/2024 | 37,387 | 1888.70 | 17 | 77 | 27 | 47 |
| 9/22/2024 | 37,267 | 1888.70 | 15 | 76 | 28 | 43 |
| 9/23/2024 | 37,147 | 1888.60 | 13 | 74 | 27 | 42 |
| 9/24/2024 | 37,043 | 1888.50 | 17 | 70 | 25 | 42 |
| 9/25/2024 | 36,909 | 1888.40 | 9 | 77 | 26 | 45 |
| 9/26/2024 | 36,760 | 1888.30 | 8 | 83 | 28 | 46 |
| 9/27/2024 | 36,642 | 1888.20 | 22 | 82 | 36 | 39 |
| 9/28/2024 | 36,524 | 1888.20 | 14 | 74 | 32 | 36 |
| 9/29/2024 | 36,406 | 1888.10 | 11 | 71 | 28 | 36 |
| 9/30/2024 | 36,273 | 1888.00 | 21 | 87 | 33 | 38 |

# Appendix B – Wildfire Suppression

## Analysis of Effects of Proposed Scott Dam Decommissioning

### County of Lake

Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

August 22, 2025

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# Acronyms and Abbreviations

| | |
|---|---|
| ac-ft | acre-feet |
| AEP | annual exceedance probability |
| CADSOD | California Division of Dam Safety |
| CDEC | California Data Exchange Center |
| COY | Coyote Lake Mendocino Weather Station |
| cfs | Cubic feet per second |
| DWR | Department of Water Resources |
| ELP | Eel River below Lake Pillsbury Weather Station |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| FFA | flood frequency analysis |
| ft | feet |
| HEC-RAS | Hydrologic Engineering Center's River Analysis System |
| LAT | Large Airtanker |
| LPY | Lake Pillsbury Weather Station |
| MSL | mean seal level |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NIDIS | National Integrated Drought Information System |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NOAA | National Oceanic and Atmospheric Administration |
| NMFS | National Marine Fisheries Service |
| NERF | New Eel Russian Facility |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| SCD | Scott Dam Weather Station |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| SPI | Standardized Precipitation Index |
| USACE | United States Army Corps of Engineers |
| USDM | U.S. Drought Monitor |
| USGS | United States Geologic Survey |
| VLAT | Very Large Airtanker |

| | |
|---|---|
| WSE | water surface elevation |
| WY | water year |

# Wildfire Suppression

This Appendix presents SLR Team's evaluation of the potential negative impacts of Scott Dam decommissioning on fire risk, wildfire suppression, and vegetation management and then examines as mitigation options, an alternate reservoir or water storage requirements for areal wildfire suppression in Lake County. The details of our evaluations are presented in the following sections of this Appendix with conclusions provided at the end.

# B1.0 Introduction

The proposed removal of Scott Dam and the subsequent draining of Lake Pillsbury in Lake County, California, presents significant challenges for wildfire suppression and vegetation management in the region. This section presents the preliminary analysis of the potential negative impacts of Scott Dam decommissioning on fire risk, suppression efforts, and vegetation management in the area.

## B1.1 Background on Scott Dam and Lake Pillsbury

Scott Dam, constructed in 1922, created Lake Pillsbury as part of the Potter Valley Project. The dam is located on the Eel River in Lake County, within the Mendocino National Forest. Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts (Friends of the Eel River, 2023).

The proposed decommissioning of Scott Dam is part of a broader initiative to restore the Eel River's ecosystem and improve fish passage. However, this project raises concerns about its impact on wildfire management in an area already prone to significant fire events (California Trout, 2023).

## B1.2 Historical Fire Activity in the Region

The Mendocino National Forest and surrounding areas have a long history of significant wildfires. Some notable fires in recent years include:

a) Mendocino Complex Fire (2018):
   - Burned 459,123 acres
   - Largest wildfire in California history at the time
   - Destroyed 280 structures and led to significant evacuations

b) August Complex Fire (2020):
   - Burned 1,032,648 acres
   - Largest wildfire in California history
   - Destroyed 935 structures and caused significant environmental damage

c) Ranch Fire (2018):
   - Part of the Mendocino Complex Fire
   - Burned 410,203 acres

d) Redwood Valley Complex Fire (2017):
   - Burned 36,523 acres
   - Destroyed over five hundred structures and resulted in several fatalities

These fires demonstrate the region's susceptibility to large-scale wildfire events and underscore the importance of effective fire suppression resources (Cal Fire, 2021).

## B1.3 Current Fire Suppression Resources and Strategies

### B1.3.1 Water Sources for Fire Suppression

Lake Pillsbury currently serves as a critical water source for fire suppression efforts in the region. Its importance is highlighted by the following factors:

a) Proximity to fire-prone areas: Lake Pillsbury is centrally located within the Mendocino National Forest, providing quick access to water for firefighting efforts.

b) Size and capacity: With a surface area of approximately 2,000 acres and a capacity of 80,000 acre-feet, Lake Pillsbury offers a substantial and reliable water source for firefighting (PG&E, 2022).

c) Accessibility for various firefighting methods: The lake is suitable for multiple water collection methods, including:

- Helicopter bucket operations
- Fixed-wing aircraft water scooping (e.g., Canadair CL-415 “Super Scooper”)
- Ground-based drafting for fire engines and water tenders

### B1.3.2 Aerial Firefighting Resources

The region relies heavily on aerial firefighting resources that can access Lake Pillsbury for water replenishment during firefighting operations. Firefighters depend on accessible water sources like Lake Pillsbury. Key aerial assets include:

a) Helicopters:

- Cal-Fire Howard Forest Helitack Base (19.7 miles from Lake Pillsbury)
- Boggs Mountain Helitack Base

b) Fixed-wing aircraft: Cal-Fire Ukiah Air Attack Base (23 miles from Lake Pillsbury)

### B1.3.3 Ground-based Firefighting Resources

Ground-based firefighting efforts also benefit from Lake Pillsbury's presence:

a) Fire engines and water tenders can draft water directly from the lake.

b) The lake serves as a reliable water source for establishing portable water tanks and fill sites near fire locations.

In the case of the fixed wing aircraft, these water carriers rely on a system called “Super Scooper,” whereby an aircraft literally repeatedly scoops water from a lake and drops it on fires.

### B1.3.4 Super Scooper Key Features

**Water Scooping**: The Super Scooper can scoop up to 1,412 gallons of water in approximately 10 seconds from nearby water sources.

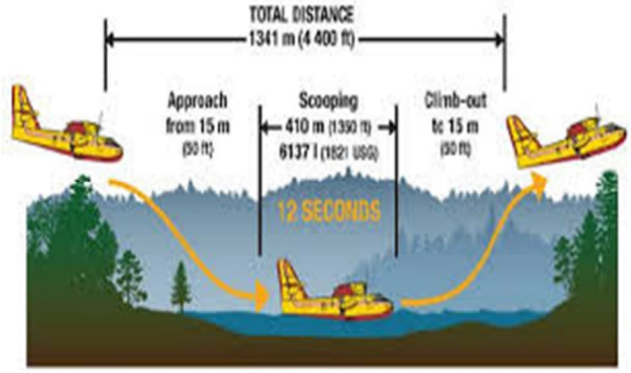
**Firefighting Efficiency**: It can remain on station for up to three hours, typically dropping nine tank loads of water on a fire.

### B1.3.5 Super Scooper Operational Benefits

**Rapid Response:** With 90% of wildfires occurring within 20 miles of a major water source, the Super Scooper can quickly refill and return to the fire, making it highly effective.

**Cost-Effective:** It provides an economical solution for delivering large quantities of water to fires without needing to return to an airport to refill.

For a Canadair CL-415, the most typical “Super Scooper” aircraft in use in California, a lake needs to be at least about 4,400 feet long to safely scoop water.



Remote areas such as the Lake Pillsbury already brings water availability problems when dealing with wildfires in the area. No longer having the lake would increase response times for firefighting efforts to relay water to affected sites.

**Table B-1: Surrounding Water Sources for Super Scooper and Helicopter use**

**East Park Reservoir**

39.34938019929924, -122.51182382180117

- Distance from Lake Pillsbury closest edges – 23.6 miles East by Southeast
- Length of waterways - 3.0 miles long

**Black Butte Lake**

39.785852302476016, -122.36262039993922

- Distance from Lake Pillsbury - 36 miles
- Length of waterways – 4.4 miles

**Stony Gorge Reservoir**

39.557787859251185, -122.52688311655206

- Distance from Lake Pillsbury closest edges twenty-three miles
- Length of waterways – 4 miles long

**Upper Letts Lake**

39.3060884576836, -122.71276408932592

- Distance from Lake Pillsbury 14.4 miles Southeast
- Length of waterways – 2400 feet ( too small to scoop)

**Clear Lake**

39.034929, -122.784312

- Distance from Lake Pillsbury closest edges – 21 miles south
- Length of waterways – 8 miles

**Indian Valley Reservoir**

39.12100732092549, -122.53998770436975

- Distance from Lake Pillsbury closest edges – 25.8 Miles
- Length of waterways – 4.5 miles

**Ruth Lake**

40.342161063068524, -123.41182091332125

- Distance from Lake Pillsbury closest edges 67.5 Miles North of Lake Pillsbury
- Length of waterways 5,279 Feet long

**Lake Mendocino**

39.21632420100953, -123.17323576134581

- Distance from Lake Pillsbury closest edges - 16.5 miles
- Length of waterways - 2.78 Miles

Helicopters can get water from a variety of spots including swimming pools, ponds, and lakes. The limitations are few- Mainly trees, wires, and cross contamination of lakes.

One notable instance of water scooping from Lake Pillsbury occurred during the **August Complex Fire** in 2020. This fire, which became the largest wildfire in California's history, required extensive firefighting efforts, including the use of water from Lake Pillsbury. For over three weeks, 4,000-gallon water tenders were drafting water from Lake Pillsbury 24/7 to support firefighting efforts. Aircraft, including helicopters, frequently scooped water from the lake to make repeated drops on the fire, helping to control its spread and protect nearby communities.

## B1.4 Potential Negative Impacts of Scott Dam Removal on Fire Suppression

The removal of Scott Dam and the subsequent loss of Lake Pillsbury would have several negative impacts on fire suppression efforts in the region as noted below:

**Reduced Water Availability**

a) Loss of a central water source: The removal of Lake Pillsbury would eliminate a significant, centrally located water source for firefighting efforts in the Mendocino National Forest and surrounding areas.

b) Increased water shuttle distances: Firefighting resources would need to travel farther to access alternative water sources, potentially increasing turnaround times and reducing operational efficiency.

c) Limited options for aerial firefighting: The loss of Lake Pillsbury would restrict options for aerial firefighting operations, particularly for large air tankers and "Super Scooper" aircraft that require substantial bodies of water for effective operations.

**Increased Response Times**

a) Extended travel for water resupply: Ground-based and aerial firefighting resources would need to spend more time traveling to and from more distant water sources, potentially allowing fires to grow larger before being contained.

b) Reduced operational efficiency: The increased time spent on water shuttling would decrease the overall efficiency of firefighting efforts, potentially requiring more resources to achieve the same level of fire suppression.

**Limitations on Firefighting Strategies**

a) Reduced flexibility in firefighting tactics: The loss of a large, centrally located water source may limit the range of firefighting strategies available to incident commanders, potentially impacting their ability to respond effectively to dynamic fire conditions.

b) Constraints on large-scale aerial operations: Without Lake Pillsbury, the ability to conduct sustained, large-scale aerial firefighting operations in the immediate area would be significantly hampered.

## B1.5 Vegetation Management Challenges

The removal of Scott Dam and Lake Pillsbury would also present several challenges for vegetation management in the area:

**Initial Vegetation Loss and Soil Exposure**

a) Immediate impact: The draining of Lake Pillsbury would expose approximately 2,000 acres of previously submerged land, leading to initial vegetation loss and soil exposure (PG&E, 2022).

b) Erosion risk: The newly exposed soil would be susceptible to erosion, potentially impacting water quality in the Eel River and its tributaries (American Rivers, 2023).

**Invasive Species Colonization**

a) Opportunistic invasion: The newly exposed reservoir bed could be quickly colonized by invasive plant species before native vegetation has a chance to establish (Tullos et al., 2016).

b) Altered fire regimes: Invasive species may have different burning characteristics compared to native vegetation, potentially altering fire behavior and intensity in the affected areas (Brooks et al., 2004).

**Changes in Fuel Moisture and Fire Risk**

a) Drying of reservoir bed: The exposed reservoir bed would initially dry out, potentially leading to lower fuel moisture content and increased fire risk in those areas until vegetation re-establishes (Ahlstrom et al., 2020).

b) Altered hydrology: The removal of the dam could change local hydrology, potentially affecting groundwater levels and vegetation moisture content in the surrounding area (Foley et al., 2017).

**Long-term Vegetation Shifts**

a) Ecosystem transformation: The area currently occupied by Lake Pillsbury would undergo a significant ecological transformation, transitioning from an aquatic to a terrestrial ecosystem over time (Bellmore et al., 2019).

b) Changes in species composition: The vegetation community in the affected area would likely shift, potentially altering the overall fire risk and behavior in the region (Pettit et al., 2020).

## B1.6 Implications for Fire Risk and Behavior

The removal of Scott Dam and Lake Pillsbury could have several implications for fire risk and behavior in the region:

**Potential Increase in Fire Risk**

a) Reduced moisture availability: The loss of a large body of water could lead to lower relative humidity in the immediate area, potentially creating more favorable conditions for fire ignition and spread (Boisramé et al., 2017).

b) Changes in wind patterns: The absence of Lake Pillsbury could alter local wind patterns, potentially affecting fire behavior and spread in the region (Syphard et al., 2019).

**Altered Fire Behavior**

a) Changes in fuel continuity: As vegetation reestablishes in the former reservoir area, it may create new patterns of fuel continuity, potentially affecting fire spread patterns (Coop et al., 2020).

b) Variation in fuel types: The introduction of new plant species and changes in vegetation structure could lead to different fire intensities and rates of spread compared to the current landscape (Pausas & Keeley, 2021).

## B1.7 Mitigation Strategies and Recommendations

The importance of maintaining water sources for firefighting is well-documented. According to the California Department of Forestry and Fire Protection (CAL FIRE), water availability is crucial for effective fire suppression (CAL FIRE, 2019). To address potential negative impacts of Scott Dam removal on wildfire suppression and vegetation management, several mitigation strategies and recommendations should therefore be considered and adopted. In the December 21, 2023, letter from the Lake Pillsbury Alliance (LPA) entitled “Comments on PG&E's Initial Draft Surrender Application and Conceptual Decommissioning Plan [Letter to Tony Gigliotti, Pacific Gas & Electric Company]” they noted significant water suppression resources were needed to provide community resilience upon removal of Lake Pillsbury. These mitigation strategies and recommendations are provided herein, along with others.

**Enhanced Water Infrastructure**

a) Develop alternative water sources: The LPA suggests creating alternative water supply sources such as ponds, pools, or smaller lakes in case the lake basin supply becomes insufficient for aerial or ground firefighting efforts. This recommendation aligns with best practices in fire management, which emphasize the importance of having multiple water sources available (National Wildfire Coordinating Group, 2021). This will partially compensate for the loss of Lake Pillsbury as a firefighting water source.

b) Improve access to remaining water sources: Enhance Road infrastructure and develop additional drafting points at nearby lakes and rivers to facilitate easier access for firefighting resources.

### B1.7.1 Augmented Firefighting Resources

a) Increase aerial firefighting capacity: Consider establishing additional helitack bases or expanding existing ones to compensate for the loss of Lake Pillsbury as a central water source.

b) Enhance ground-based resources: Increase the number of water tenders and portable water tanks available in the region to support extended water-shuttling operations. The LPA also recommend this mitigation strategy by providing equipment and water to the Lake Pillsbury Fire Protection District (LPFPD) with input from various stakeholders, including Cal Fire, the United States Forest Service, LPFPD, County of Lake, and the Lake Pillsbury communities. This collaborative approach aligns with best practices in community wildfire protection planning (Community Wildfire Planning Center, 2020).

c) Roadway access enhancements. The LPA suggests constructing or modifying roads to provide adequate emergency transport capacity throughout the lake basin, including roads connecting permanent or alternative water sources. Improved access is crucial for effective firefighting, as noted in the National Wildfire Coordinating Group's Wildland Fire Incident Management Field Guide (2021).

### B1.7.2 Proactive Vegetation Management

a) Develop a comprehensive revegetation plan: Create and implement a plan for the exposed reservoir bed that prioritizes native species and fire-resistant vegetation (Shafroth et al., 2016).

b) Implement invasive species control: Establish an aggressive invasive species monitoring and control program to prevent the establishment of fire-prone non-native vegetation (Holl & Aide, 2011).

### B1.7.3 Enhanced Fire Prevention and Detection

a) Expand fire detection systems: Implement additional fire detection technologies, such as cameras and sensors, to compensate for potentially increased fire risk in the area. The LPA recommends implementing brushfire detection and warning systems in the region, including the Mendocino National Forest. Early detection systems can significantly improve response times and effectiveness of firefighting efforts. Research has shown that early detection can reduce the spread and impact of wildfires (Alkhatib, 2014).

b) Increase public education: Enhance community outreach and education programs to promote fire safety and prevention in the affected region.

### B1.7.4 Adaptive Management Approach

a) Monitoring and assessment: Implement a robust monitoring program to track changes in vegetation, fire risk, and ecosystem health following dam removal (Foley et al., 2017).

b) Flexible management strategies: Develop adaptive management plans that can be adjusted based on observed changes in fire risk and ecosystem dynamics over time.

## B1.8 Findings

The proposed removal of Scott Dam and the subsequent loss of Lake Pillsbury present significant challenges for wildfire suppression and vegetation management in Lake County and the surrounding regions. The potential negative impacts include reduced water availability for firefighting, increased turnaround times, limitations on firefighting strategies, and complex vegetation management issues.

These challenges are particularly concerning given the area's history of large-scale wildfires and the increasing fire risk due to climate change. While the dam removal project aims to improve river ecosystem health and fish passage, it is crucial to carefully consider and mitigate the potential increased fire risks and suppression challenges.

Implementing a comprehensive set of mitigation strategies, including enhanced water infrastructure, augmented firefighting resources, proactive vegetation management, and adaptive management approaches, will be essential to maintaining effective fire suppression capabilities and managing vegetation-related fire risks in the post-dam removal landscape.

Further research and detailed planning will be necessary to fully understand and address the long-term implications of this project on wildfire management in the region. Collaboration between fire management agencies, ecological researchers, and local communities will be crucial in developing effective strategies to navigate these challenges and ensure the continued protection of the area's natural resources and human communities from wildfire threats.

# B2.0 Reservoir Requirements for Aerial Wildfire Suppression

## B2.1 Introduction

The decommissioning of Scott Dam and the potential elimination of Lake Pillsbury in Lake County, California, would result in the need to establish new water sources for aerial firefighting operations, correlating with rotary winged aircraft.

Criteria for reservoir size and area are determined by the capabilities of the helicopters that are being utilized for aerial firefighting operations.

Wildfire air resources are categorized and typed to ensure effective and efficient response during wildfires. This typing system helps determine the type of aircraft, crew, and equipment needed for specific fire situations.

## B2.2 Helicopter Water Capacities

The largest and most capable helicopters used in aerial firefighting are classified as Type 1 or heavy helicopters. These heavy-lift helicopters can carry more than 3,000 gallons of water or fire retardant. These are primarily the Boeing CH-47 Chinook (also known as CH-47D or Chinook. This enormous capacity makes the Chinook ideal for tackling large, complex wildfires. With a maximum external load capacity of up to 25,000 lbs., a top speed of around 132 mph, and a range of approximately 244 miles. The Sikorsky S-70 Firehawk is also a type-1 helicopter capable of holding 1,000 gallons of water in the belly of the aircraft making it a very reliable resource when fighting rural wildfires (Lockheed Martin, 2025).

Type 2 helicopters are very effective initial attack resources. These helicopters can support firefighters on the ground by delivering up to 300 gallons of water to the fire line in a bucket or tank.

The need for aerial water drops is based on the duration, size, and values threatened during the lifecycle of those events. This pattern is most pronounced for Type 1 helicopters (74%), multiengine scoopers (74%), Large Airtankers (LATs; 61%), and Very Large Airtankers (VLATs; 65%). LATs and VLATs are used to drop fire retardant or water on wildfires. LATs generally carry 2,000 to 4,000 gallons of retardant or water, while VLATs can carry over 8,000 gallons (U.S. Department of Agriculture 2020).

Helicopters will take water from many places including small ponds and portable water tanks with easy access. The fixed winged super scoopers primarily use large bodies of water for aerial firefighting operations. Specifications for a reservoir to be constructed for wildfire suppression, should be designed with aircraft capabilities and limitations, and the anticipated suppression needs for the region.

The Canadair CL-415 (Super Scooper) scoops at 80 knots and is on the water for approximately 10 to 12 seconds. The pickup water source should be a minimum of 1 mile long, depending on environmental conditions. The scooping path does not have to be straight, as the aircraft is maneuverable while scooping. (U.S. Department of Agriculture, 2016) Super Scoopers have limitations when it pertains to aerial firefighting. The planes need a long enough body of water to safely maneuver and scoop up over 1,000 gallons of water to drop on strategic parts of a wildfire. Super scoopers have been used most recently on the Palisades fire in Los Angeles (2025) but have also been used throughout the country, including the 2018 and 2020 fires within the Lake County.

## B2.3 Reservoir Volume

Usage and capacity planning involves analyzing factors such as water volume needs, turnover rates, anticipated scales of operation, environmental regulations, and seasonal variables. When designing ponds or reservoirs for firefighting, capacity versus utilization rates and future demand growth must be balanced. Larger dip ponds must be able to supply 100,000 gallons or more to support extensive aerial operations, while smaller ponds should provide 10,000-30,000 gallons for limited use.

Most guidelines recommend sizing capacity to hold the maximum water volume required for at least 4 hours of aerial firefighting usage, providing a safety buffer in case refilling is delayed. (BTL Liners, 2025) A reservoir or a dip pond would have to be of significant size to support a large-scale aerial helicopter operation during a wildfire. A reservoir or dip pond would also have to utilize its own water source to be able to be used throughout the year by localized natural watershed.

### B2.3.1 Reservoir Dimensions

The reservoir should have sufficient surface area to allow safe helicopter dipping maneuvers. As a rule, safety circles should be at a minimum 1.5 times the rotor diameter. In addition, separation between safety circles should be at a minimum one rotor blade(s) diameter. (National Wildfire Coordinating Group, 2019). Dimensions of the reservoir matter due to operational safety of the types and size of the helicopter utilizing the body of water. Clearance of rotary wing would play a significant role in determining dimensions for a reservoir.

The size of a pond or reservoir required to supply water for aerial wildfire operations depends on its **capacity** in gallons, which is determined by its dimensions (surface area and average depth). Below is a breakdown of the minimum size required for two scenarios: supporting **extensive aerial wildfire operations** (100,000+ gallons) and **limited use** (10,000–30,000 gallons).

***General Formula for Water Volume***

The volume of a pond or reservoir can be estimated using the formula:

**Volume (gallons) = Surface Area (square feet) × Average Depth (feet) × 7.48**

Where:

- 7.48 is the number of gallons in one cubic foot.
- Surface area is typically measured in square feet, and depth is in feet.

***Minimum Size for 100,000+ Gallons (Extensive Use)***

To support extensive aerial wildfire operations requiring **100,000 gallons or more**, the minimum size can be calculated as follows:

a) **Assumptions**:

- A small pond typically has an average depth of **6 feet** for practicality and ease of construction.
- Surface area is calculated based on the required capacity.

b) **Calculation**:
- Rearrange the formula:

**Surface Area = Volume / (Depth × 7.48)**

**Surface Area = 100,000 / (6 × 7.48) ≈ 2,230 square feet**

c) **Dimensions**:
- A pond with a surface area of **2,230 square feet** would be approximately:

**47 feet × 47 feet** (roughly square), or

**30 feet × 75 feet** (rectangular).

d) **Depth Considerations**:
- If the depth is increased to **10 feet**, the required surface area decreases:

**Surface Area = 100,000 / (10 × 7.48) ≈ 1,337 square feet**

  - **Dimensions: 37 feet × 37 feet** (square), or **25 feet × 55 feet** (rectangular).

***Minimum Size for 10,000–30,000 Gallons (Limited Use)***

For limited aerial wildfire operations requiring 10,000–30,000 gallons, the pond or reservoir size can be significantly smaller:

a) **Assumptions**:
- Average depth remains **6 feet**.

b) **Calculations**:
- For **10,000 gallons**:

**Surface Area = 10,000 / (6 × 7.48) ≈ 223 square feet**

  - Dimensions: 15 feet × 15 feet (square), or 10 feet × 23 feet (rectangular).
- For **30,000 gallons**:

**Surface Area = 30,000 / (6 × 7.48) ≈ 669 square feet**

  - Dimensions: **26 feet × 26 feet** (square), or **15 feet × 45 feet** (rectangular).

c) **Depth Considerations**:
- Increasing depth to **10 feet** reduces surface area further:
  - For **10,000 gallons**: **Surface Area ≈ 134 square feet** (12 feet × 12 feet).
  - For **30,000 gallons**: **Surface Area ≈ 401 square feet** (20 feet × 20 feet).

## B2.4 Practical Considerations

1. **Shape of the Pond**:
   - Ponds do not need to be perfect rectangles or squares. Irregular shapes are common, but the total surface area and depth should meet the volume requirement.

2. **Access for Aerial Operations**:
   - The pond must be large enough for helicopters or planes to safely scoop water. A minimum width of **50 feet** is often recommended for efficient helicopter bucket operations, even for smaller ponds.

3. **Water Replenishment**:
   - The pond or reservoir should have a reliable water source (e.g., rainfall, streams, or wells) to ensure it can be refilled during extended wildfire operations.

4. **Depth and Safety**:
   - Shallow ponds with a depth of less than **6 feet** may evaporate quickly, especially during hot and dry wildfire conditions, reducing their long-term usability.

**Table B-2: General Aeiral Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Volume (Gallons) | Average Depth (feet) | Minimum Surface Area (square feet) | Approximate Dimensions (feet x feet) |
|---|---|---|---|---|
| Extensive Use | 100,000+ | 6 | 2,230 | 47 × 47 or 30 × 75 |
| Limited Use (Low Volume) | 10,000 | 6 | 223 | 15 × 15 or 10 × 23 |
| Limited Use (High Volume) | 30,000 | 6 | 669 | 26 × 26 or 15 × 45 |

## B2.5 Constructing a Dedicated Reservoir vs. Utilizing Distant Water Sources

### B2.5.1 Constructing a Dedicated Reservoir

*Pros*:

1. Reduced response/turnaround time for aerial firefighting aircraft.
2. Higher amount of water air drops due to closer proximity of water source.
3. Beneficial placement of constructed reservoir.
4. Lower cost for fuel due to proximity of water to the fire.

*Cons*:

1. High cost for new infrastructure and the maintenance that comes with it.
2. Environmental permits/hurdles needed to be obtained to build a reservoir of this size.
3. Potential for ecosystems to be disrupted from introducing another water source.

### B2.5.2 Utilizing Distant Water Sources

*Pros*:

1. Low cost, no new infrastructure being introduced.
2. Low impact of the environment.

*Cons*:

1. Longer response times/turnaround time to travel to a nearby water source.
2. Drought conditions for the area may impact water availability near the incident.
3. Increase fuel consumption for aircraft due to longer turnaround times.
4. Longer flight times for aircraft would create more air traffic.

## B2.6 Findings

Reservoir dimensions directly correlate with the firefighting equipment being used and the capabilities that they can produce. A body of water needs to be able to support an operational period of aerial firefighting in abundance when dealing with any large-scale wildfire. To supply 100,000 gallons or more, a pond or reservoir with a surface area of 2,230 square feet (e.g., 47 feet × 47 feet) and an average depth of 6 feet is sufficient. For 10,000–30,000 gallons, much smaller ponds of about 223–669 square feet can meet the need, if they are deep enough and accessible for aerial operations. Additionally, the water source would need to be supplied by a river to replenish the water supply that was taken for aerial water drops for firefighting. A 24-hour period of aerial firefighting can use up to 80,000 to 100,000 gallons of water depending on wildfire conditions and the length of time from when the fire started to end. Maintaining existing or constructing a new reservoir or body of water to support firefighting aircraft with their needs is essential to wildland firefighting within the Lake County.

# B3.0 Conclusions

## B3.1 Fire Risk, Wildfire Suppression Efforts and Vegetation Management Aspects

The proposed removal of Scott Dam and the subsequent loss of Lake Pillsbury present significant challenges for wildfire suppression and vegetation management in Lake County and the surrounding regions. The potential negative impacts include reduced water availability for firefighting, increased turnaround times, limitations on firefighting strategies, and complex vegetation management issues.

These challenges are particularly concerning given the area's history of large-scale wildfires and the increasing fire risk due to climate change. While the dam removal project aims to improve river ecosystem health and fish passage, it is crucial to carefully consider and mitigate the potential increased fire risks and suppression challenges.

Implementing a comprehensive set of mitigation strategies, including enhanced water infrastructure, augmented firefighting resources, proactive vegetation management, and adaptive management approaches, will be essential to maintaining effective fire suppression capabilities and managing vegetation-related fire risks in the post-dam removal landscape.

Further research and detailed planning will be necessary to fully understand and address the long-term implications of this project on wildfire management in the region. Collaboration between fire management agencies, ecological researchers, and local communities will be crucial in developing effective strategies to navigate these challenges and ensure the continued protection of the area's natural resources and human communities from wildfire threats.

## B3.2 Potential Alternate Water Storage or Reservoir Requirements for Fire Suppression

SLR Team examined the reservoir requirements for aerial wildfire suppression in Lake County, California, focusing on the potential impact of decommissioning Scott Dam and Lake Pillsbury. The loss of these water sources requires the establishment of alternative reservoirs to support aerial firefighting operations, particularly for rotary-wing aircraft like the Type 1 Boeing CH-47 Chinook and Sikorsky S-70 Firehawk, which are critical resources for combating large-scale wildfires.

Reservoir design must consider the water capacity, accessibility, and operational safety for aerial firefighting. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet at an average depth of 6 feet. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223 to 669 square feet. Properly designed reservoirs must also account for environmental factors, safety clearances for helicopter dipping, and year-round water replenishment from natural watersheds.

Constructing local reservoirs offers several advantages, including reduced turnaround times, increased water availability, and lower aircraft fuel costs. However, challenges such as high construction expenses, environmental permitting, and potential ecosystem disruption must be addressed. Alternatively, relying on distant water sources minimizes infrastructure costs but increases response times, fuel consumption, and air traffic congestion.

The conclusion emphasizes the necessity of maintaining or constructing reservoirs to ensure effective wildfire suppression. A 24-hour aerial firefighting operation may require up to 100,000

gallons of water, underscoring the importance of proximity and capacity for sustained operations. Balancing reservoir design with environmental and logistical considerations is crucial for enhancing County of Lake\’s wildfire readiness.

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# Appendix C – Fish Passage Scenario Evaluation

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

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August 22, 2025

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# Acronyms and Abbreviations

| | |
|---|---|
| CDFG | California Department of Fish and Game |
| CDFW | California Department of Fish and Wildlife |
| CESA | California Endangered Species Act |
| DO | Dissolved Oxygen |
| DSOD | California Division of Dam Safety |
| DWR | Department of Water Resources |
| ESA | Endangered Species Act |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| NERF | New Eel-Russian Facility |
| NHD | National Hydrology Dataset |
| NMFS | National Marine Fisheries Service |
| NOAA | National Oceanic and Atmospheric Administration |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| RPA | Reasonable and Prudent Alternative |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| STD | Standard Deviation |
| USGS | United States Geological Survey |
| VAFS | Van Arsdale Fish Station |
| VTN | VTN Oregon, Inc. |
| WSE | water surface elevation |
| WY | water year |

# C1.0 Introduction

SLR International Corporation (SLR) was contracted by the County of Lake to conduct an analysis of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam. This Appendix outlines potential fish pathways and restoration opportunities, and evaluates conceptual infrastructure alternatives. Both the County of Lake and PG&E have identified the need for fish migration to the upper reaches of the Eel river (upstream of Scott Dam) as a benefit to the local and down stream communities as part of the Scott Dam Decommissioning project.

Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Seven alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Existing Operational Conditions Scenario covers operational conditions during the WY[1] period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft (PG&E Datum)[2] and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Surrender Application dated January 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and construction of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

The results of this study are meant to provide the following:

1. Provide a high-level overview of the current infrastructure, resident fish communities, and current barriers to fish passage;

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

2. Explore potential fish pathway opportunities within the seven (7) mitigation scenarios identified above for Scott Dam Decommissioning; and
3. Based on the high level analysis presented here for fish pathway options, offer alternate options for fish pathways for consideration by the County of Lake and PG&E as part of final site restoration.

## C1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with 2,275 acres of surface area and 31 miles of shoreline (Stillwater, 2021) (PG&E, 2023). The Lake's main flow contributor is the Eel River's upper watershed[3], located in Lake County, California. Scott Dam, which forms Lake Pillsbury, is a complete barrier to upstream fish migration, preventing access to high value habitat for federally Endangered Species Act (ESA)-listed anadromous[4] salmonids including Chinook Salmon (*Oncorhynchus tshawytscha),* Coho Salmon *(O. kisutch)* and Steelhead Trout (*O. mykiss)*.

Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam primarily provides water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley, which has been offline since July 2021 after PG&E discovered a transformer did not meet current operating standards. The Scott Dam has also historically provided water for irrigation to the Potter Valley Irrigation District (PVID) and to meet the minimum instream flow requirements in the Eel River and East Fork Russian River (Stillwater, 2021) (PG&E, 2025). Lake Pillsbury also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID are diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pilsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the Russian River. Cape Horn Dam has a fish ladder allowing fish species to migrate upstream until encountering Scott Dam.

Scott Dam currently operates with an approximate maximum capacity of 56,000 acre-feet at Lake Pillsbury (PG&E, 2025). The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs). Water can be released from the dam through two structures: a needle valve[5] located at the base of the dam and the spillway[6]. The flow capacity of the needle valve is approximately 400 cubic feet per second (cfs) but can operate at varying flows for each Water Surface Elevation (WSE). The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[7]) controls. The spillway consists of multiple sliding and radial gates that are typically fully open between November 1 and April 1 and closed the remainder of the year. Flow through the gated spillway will only occur when the WSE within Lake Pilsbury

[3] An area of land drained by a river, river system, or other body of water.

[4] Fish that are born in freshwater, migrate to the ocean to grow and mature, and then return to freshwater to spawn.

[5] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[6] A structure used to provide controlled release of water downstream of a dam or levee.

[7] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

exceeds the spillway invert elevation of 1,900 feet. The top of the spillway gates are all at elevation 1,910 feet. Slide gate #13 and one of the five (5) radial gates area SCADA controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained fully open during the whole year.

As part of the PG&E's license surrender, a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E and the Eel-Russian Project Authority (ERPA) as non-project use of project land. On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023). ERPA will be responsible for the construction, maintenance and operation of NERF after the completion of the surrender requirements to be included in a regulatory order to be issues by FERC.

## C1.2 Existing Climate and Hydrology

Lake Pillsbury has several watercourses contributing to water storage availability. These tributaries historically routed directly into the Eel River prior to the construction of Scott Dam. A majority of the tributaries to Lake Pillsbury are ephemeral[8] or intermittent[9] streams with the Eel River, Habematolel Creek, and Rice Fork identified as entirely perennial[10] by the United States Geological Survey's (USGS's) National Hydrography Dataset (NHD). Other notable watercourses with large perennial reaches and short stretches that are intermittent, specifically as they reach Lake Pillsbury include Smokehouse Creek, Habematolel Creek, Salt Spring Creek, and Salmon Creek.

The hydrology of the Eel River for the Scott Dam and Cape Horn Dam system is monitored using a network of gaging stations that track water storage and flow. There are two main gages used to monitor Scott Dam, a PG&E gage that reports average daily water storage within Lake Pillsbury and a PG&E gage that reports average daily river flows downstream of Scott Dam. The Eel River watershed experiences a Mediterranean climate, typical of California's northern coast. It is characterized by intense floods in late fall and winter, limited snowmelt and spring runoff (which may decline due to climate change), minimal summer flows, and early fall rainfall that signals the start of the high-flow winter season. Peak impaired outflows from Scott Dam have been reported to be between 0.1 and 45,300 where flows are attenuated by the dam and impoundment during the wet and dry season. The wet season is currently defined between December and April and the dry season falls between May and November. Unimpaired flows post Scott Dam removal are projected to exceed 1,500 cfs during the wet months and drop below 17 cfs during the dry season. The area is also susceptible to extreme drought and flood conditions as well as climate change. Additional details on watershed hydrology are presented in SLR's Draft Impact Analysis and Adaptation Strategies Report Appendix A - Draft Hydrology and Hydraulics Scenario Evaluation.

[8] This stream flows only briefly after rainfall events. It is usually dry most of the year and only carries water for a short duration following precipitation. Ephemeral streams do not have a continuous source of water and are highly dependent on immediate weather conditions.

[9] This type of stream flows only during certain times of the year, typically when there is enough rainfall or snowmelt to sustain flow. It may have water for weeks or months at a time, but it dries up during periods of low precipitation.

[10] A perennial stream is one that flows continuously throughout the year, having a consistent and reliable source of water, such as groundwater, springs, or sustained rainfall.

## C1.3 Aquatic Habitat and Fish Populations

SLR reviewed the existing information regarding the aquatic habitat and fish populations as relates to the Eel River and Lake Pillsbury to document the fish population of concern and understand the avenues needed for fish pathway evaluations. We present our evaluation in the following sections.

### C.1.3.1 Target Fish Species

Target fish species in the Eel River watershed identified by Cal Trout and the California Department of Fish and Wildlife (CDFW, Formerly California Department of Fish and Game) include Steelhead Trout (*Oncorhynchus mykiss*), Chinook Salmon (*Oncorhynchus tshawytscha*), Coho Salmon (*Oncorhynchus kisutch),* and the Pacific Lamprey (*Entosphenus tridentatus*) (CDFW, 2024).

### C.1.3.2 Eel River Downstream of Scott Dam

The Eel River, extending from Scott Dam to Outlet Creek (approximately 31 miles downstream of Cape Horn Dam), flows through a confined valley with limited floodplain development. An overview of the Eel River is presented in Figure C1-1 . Riparian[11] vegetation is generally restricted to narrow corridors, typically no wider than 40 feet, along the channel margins. Below the confluence[12] with the Middle Fork Eel River, the channel features extensive, nearly continuous sediment bars. Under low-flow conditions, the river typically presents as a single-thread, sinuous channel, though it may exhibit multi-thread characteristics where it bifurcates[13] around mid-channel bars. Sparse riparian vegetation along the low-flow channel and sediment bars suggests a dynamic fluvial environment[14] with frequent flood events that inhibit vegetation establishment. Further downstream, from the vicinity of the Van Duzen River confluence to the estuary[15], the Eel River transitions into a multi-threaded, anastomosing[16] channel system, with flow paths diverging around vegetated islands.

The reach immediately downstream of Scott Dam supports high-quality spawning habitat for anadromous fish, largely due to the substantial gravel supply from Soda Creek, located approximately 1.1 miles downstream of the dam (SEC, 1998). Two major tributaries, Tomki Creek (3.8 miles below Cape Horn Dam) and Outlet Creek (31 miles downstream) provide extensive spawning and rearing habitat for anadromous salmonids (SEC, 1998). In addition, several smaller, unregulated tributaries to the Eel River, including Soda Creek, Benmore Creek, Bucknell Creek, Trout Creek, Thomas Creek, and Garcia Creek, offer important habitat for fish and other aquatic species.

[11] Land and environment along the edges of rivers, streams, or other bodies of flowing water. These areas are important because they support unique plants and animals, help prevent erosion, and improve water quality.

[12] The place where two or more rivers or streams meet and flow together.

[13] Channel splits into two separate branches.

[14] A river or stream system that is constantly changing over time due to the movement of water, sediment, and the shaping of the landscape.

[15] A coastal area where freshwater from rivers and streams mixes with saltwater from the ocean.

[16] A type of river or stream system made up of multiple interconnected channels that split and rejoin around stable islands or bars.

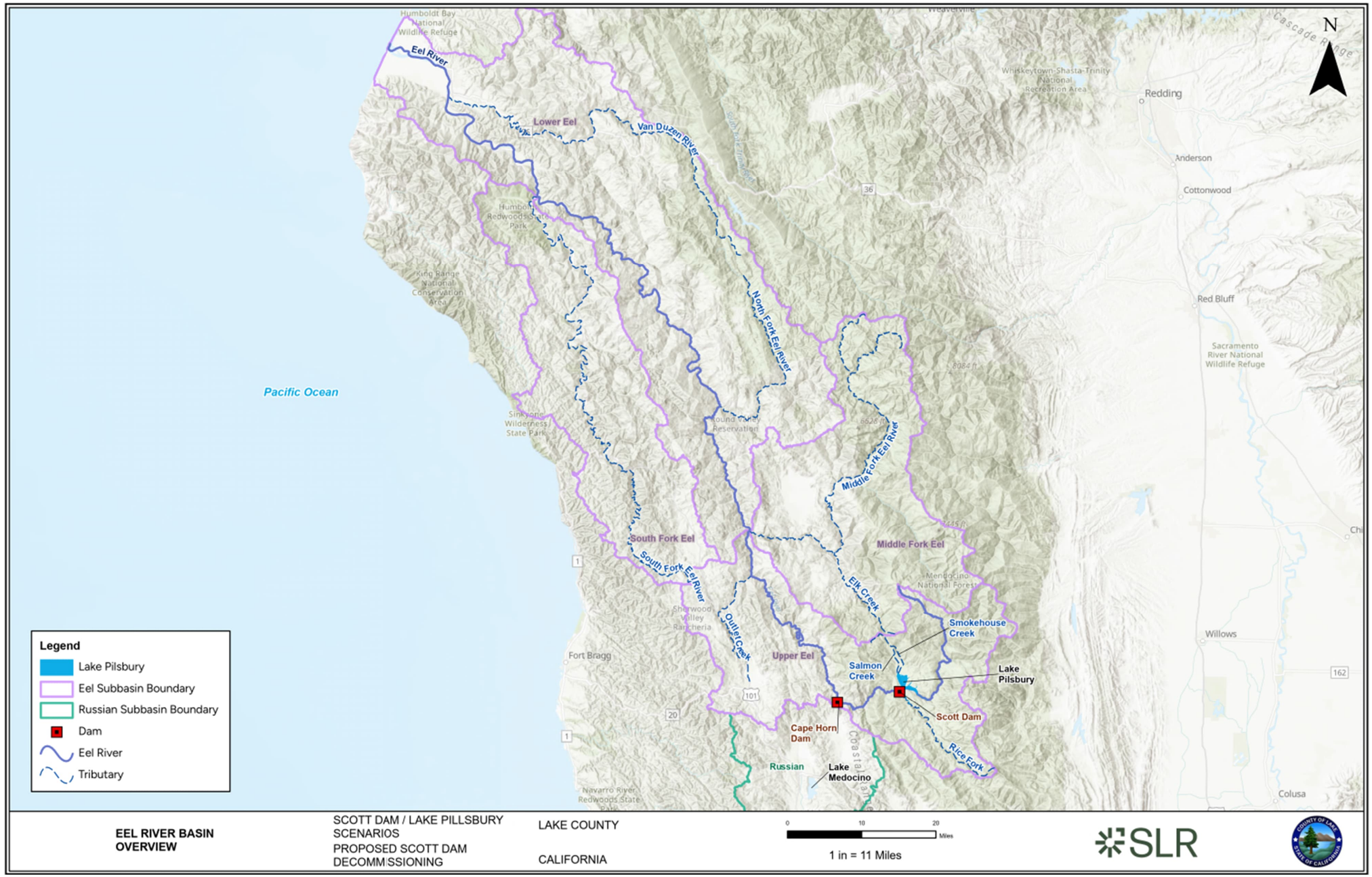


**Figure C1-1: Eel River Basin Overview**

The Eel River downstream of Scott Dam, including the lower river and estuary, provides important aquatic habitat for numerous special-status species. This includes several anadromous salmonids listed as threatened under the ESA, such as Chinook salmon (*Oncorhynchus tshawytscha*), coho salmon (*O. kisutch*), and steelhead (*O. mykiss*), as well as the threatened Pacific eulachon (*Thaleichthys pacificus*) and the endangered tidewater goby (*Eucyclogobius newberryi*). Species proposed for ESA listing, such as the northwestern pond turtle (*Actinemys marmorata marmorata*) and the longfin smelt (*Spirinchus thaleichthys*), also inhabit this area.

Other notable species include those listed under the California Endangered Species Act (CESA), like the endangered summer steelhead (*O. mykiss irideus*), the candidate species white sturgeon (*Acipenser transmontanus*), and species of special concern such as the green sturgeon (Acipenser medirostris), Pacific lamprey (*Entosphenus tridentatus and Lampetra spp.),* foothill yellow-legged frog (*Rana boylii, Northwest/North Coast clade*), coastal tailed frog (*Ascaphus truei*), southern torrent salamander (*Rhyacotriton variegatus*), California floater (*Anodonta californiensis*), western pearlshell (*Margaritifera falcata*), and the northern red-legged frog (*Rana aurora*). (CDFG, 2010), (CDFW, 2024a), (iNaturalist, 2024).

The river also supports a variety of non-listed native freshwater fish, including sculpin (*Cottus spp.),* stickleback (*Gasterosteus spp.),* and sucker (*Catostomus spp.).* Additionally, occasional runs of pink salmon (*O. gorbuscha*) and chum salmon (*O. keta*) have been documented (Yoshiyama and Moyle, 2010).

### C.1.3.3 Lake Pillsbury

Aquatic habitat within Lake Pillsbury is primarily composed of shallow shoreline and flat areas lacking vegetative cover, along with deeper pelagic zones[17]. Seasonal fluctuations in reservoir elevation typically inhibit the establishment of extensive riparian vegetation or aquatic macrophyte (vascular plant) communities. Limited riparian/wetland vegetation is present at tributary inlets and along the northern shore (Gravelly Valley), which becomes submerged at peak reservoir levels. The littoral zone—characterized by shallow, light-penetrating waters with potential for macrophyte growth—offers minimal structural cover for aquatic fauna.

Thermal stratification[18] during spring through fall separates the reservoir into a large, epilimnetic[19] layer of warm water, which supports aquatic habitat for warm-water fishes including Largemouth bass (*Micropterus salmoides*), Green sunfish (*Lepomis cyanellus*) and Bluegill (*Lepomis macrochirus)*, along with various amphibians, aquatic reptiles, and macroinvertebrates. Native species such as Rainbow trout (*Oncorhynchus mykiss*) and Sacramento sucker (*Catostomus occidentalis*) are found in the relatively limited volume of colder, deeper water in the metalimnion[20] and hypolimnion[21] layers of the reservoir (CDFW, 2016).

[17] The open water area of a lake that is not close to the bottom or the shore that provides different habitats supporting various stages of a fish's life and influence their behavior, feeding, and survival.

[18] Temperatures in a lake vary in layers based on depth and season.

[19] The top layer of warm water in a thermally stratified lake.

[20] The colder middle thermally stratified layer just below the epilimnetic layer.

[21] The bottom and coldest layer of a thermally stratified lake below the metalimnion layer.

### C.1.3.4 Upper Eel River

Upstream of Lake Pillsbury, the Eel River and Rice Fork currently offer habitat for the foothill yellow-legged frog, northwestern pond turtle, and a variety of other non-listed native fish, amphibians, aquatic reptiles, and macroinvertebrates. Historically, before construction of Scott Dam in 1921, the Upper Eel watershed supported extensive spawning and rearing habitat for Steelhead and Chinook. Anadromous fish species are blocked from accessing this habitat by Scott Dam and the Lake Pillsbury reservoir (See Section C1.4).

### C.1.3.5 Van Arsdale Fish Station (VAFS)

Since 1909, upstream and downstream fish passage for migratory species has been provided at Cape Horn Dam via the Van Arsdale pool-and-weir fish ladder, approximately 12 miles downstream of Scott Dam/Lake Pillsbury. This ladder has undergone several upgrades to enhance fish passage, with modifications made in 1915, 1962, 1987, and most recently in 2020. Additionally, experimental improvements targeting Pacific lamprey passage were implemented between 2014 and 2016. A summary of is outlined in Figure C1-2 and Figure C1-3 summarize fish passage counts at the Van Arsdale Fisheries Station (VAFS).

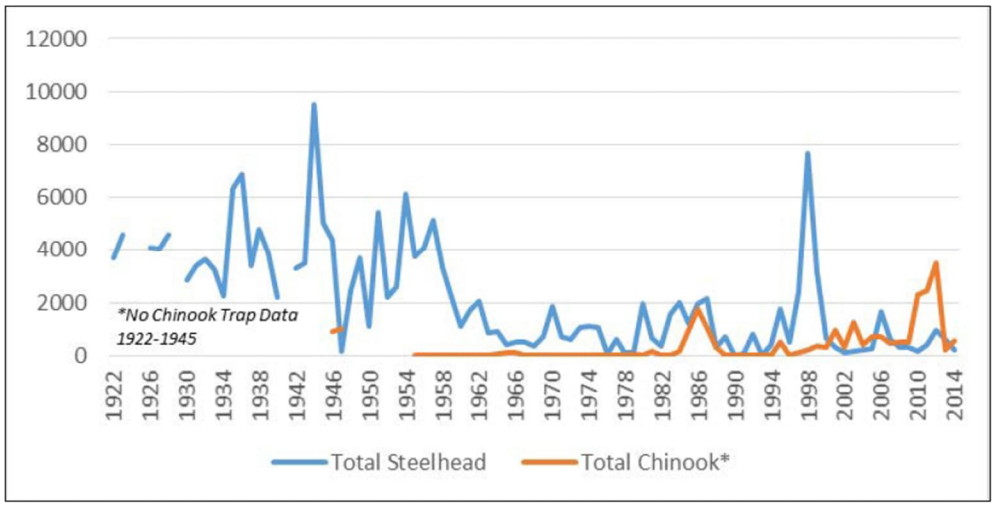


**Figure C1-2: Figure A-4 from McMillen Jacobs Potter Valley Project Capital Modifications Feasibility Study Report, 2018: Total Adult Steelhead and Chinook Trapped at Van Arsdale Fisheries Station; 1922-2014 (from CDFW counts).**

| Steelhead | | | |
|---|---|---|---|
| Season | Number Wild | Number Hatchery | Total Count |
| 1990/1991 | 19 | 12 | 31 |
| 1991/1992 | 26 | 34 | 60 |
| 1992/1993 | 52 | 771 | 823 |
| 1993/1994 | 23 | 11 | 34 |
| 1994/1995 | 116 | 318 | 434 |
| 1995/1996 | 158 | 1585 | 1743 |
| 1996/1997 | 104 | 407 | 511 |
| 1997/1998 | 175 | 2218 | 2393 |
| 1998/1999 | 355 | 7324 | 7679 |
| 1999/2000 | 189 | 2961 | 3150 |
| 2000/2001 | 250 | 391 | 641 |
| 2001/2002 | 226 | 82 | 308 |
| 2002/2003 | 99 | 3 | 102 |
| 2003/2004 | 149 | 0 | 149 |
| 2004/2005 | 234 | 0 | 234 |
| 2005/2006 | 184 | 69 | 253 |
| 2006/2007 | 492 | 1143 | 1635 |
| 2007/2008 | 423 | 199 | 622 |
| 2008/2009 | 305 | 10 | 315 |
| 2009/2010 | 324 | 0 | 324 |
| 2010/2011 | 166 | 0 | 166 |
| 2011/2012 | 423 | 0 | 423 |
| 2012/2013 | 934 | 1 | 935 |
| 2013/2014 | 609 | 0 | 609 |
| 2014/2015 | 215 | 0 | 215 |
| 10 year mean (2005-2015): | | | 550 |
| 10 year STDEV: | | | 448 |

| Chinook | | | |
|---|---|---|---|
| Season | Number Wild | Number Hatchery | Total Count |
| 1990/1991 | na | na | 0 |
| 1991/1992 | na | na | 5 |
| 1992/1993 | na | na | 4 |
| 1993/1994 | na | na | 1 |
| 1994/1995 | na | na | 21 |
| 1995/1996 | 525 | 0 | 525 |
| 1996/1997 | 26 | 0 | 26 |
| 1997/1998 | 106 | 1 | 107 |
| 1998/1999 | 141 | 59 | 200 |
| 1999/2000 | 232 | 107 | 339 |
| 2000/2001 | 223 | 80 | 303 |
| 2001/2002 | 641 | 314 | 955 |
| 2002/2003 | 268 | 61 | 329 |
| 2003/2004 | 997 | 236 | 1233 |
| 2004/2005 | 309 | 83 | 392 |
| 2005/2006 | 620 | 105 | 725 |
| 2006/2007 | 697 | 2 | 699 |
| 2007/2008 | 478 | 0 | 478 |
| 2008/2009 | 496 | 0 | 496 |
| 2009/2010 | 518 | 1 | 519 |
| 2010/2011 | 2314 | 0 | 2314 |
| 2011/2012 | 2436 | 0 | 2436 |
| 2012/2013 | 3471 | 0 | 3471 |
| 2013/2014 | 215 | 0 | 215 |
| 2014/2015 | 583 | 0 | 583 |
| 10 year mean (2005-2015): | | | 1194 |
| 10 year STDEV: | | | 1117 |

**Figure C1-3: Table A-1 from McMillen Jacobs Potter Valley Project Capital Modifications Feasibility Study Report, 2018.: Number of Upstream Migrating Adult Steelhead Oncorhynchus mykiss and Chinook O. tshawytscha Trapped annually at Van Arsdale Fisheries Station, 1990-2015 (Source: CDFW).**

## C1.4 Scott Dam as a Fish Barrier

The construction of Scott Dam in 1921 created a 130-foot-high barrier that prevents upstream fish migration. Currently, there are no facilities at the dam to allow fish passage. Estimates of the blocked river habitat and potential returning fish numbers vary widely. A 1982 VTN study identified 35.7 miles of primary spawning habitat above the dam, estimating possible returns of 1,250 Chinook salmon and 1,499 Steelhead (VTN, 1982). National Marine Fisheries Services (NMFS) (2002) referenced this and suggested that historically, 2,000 to 4,000 fall-run Chinook and winter-run steelhead may have spawned upstream. Other studies estimate 100–150 miles of blocked habitat, while Cooper et al. (2020) projected even greater totals—up to 288 miles for steelhead spawning. PG&E's 2018 relicensing efforts included aerial and field surveys to assess habitat, barriers, and temperature conditions, which identified a minimum of 50 miles of spawning and rearing habitat upstream of Scott Dam (PG&E, 2025).

There are currently no downstream fish passage facilities at Scott Dam. However, potential downstream fish passage can occur during spill events when water flows over the dam crest. When the reservoir is not spilling, water is released through the low-level outlet (needle valve). Fish movement into the low-level outlet is expected to be minimal, as studies at other reservoirs have documented very low fish densities at the depths where low-level outlets are located (Placer County Water Agency 2011; Nevada Irrigation District and PG&E 2011).

# C2.0 Fish Passage Objectives and Design Criteria

## C2.1 Objective

The functional objective when considering fish passage is to facilitate upstream and downstream fish movement at the existing Scott Dam to restore historical migration routes for anadromous fish in the Eel River watershed. Other objectives include maintaining the currently established fish habitats at Lake Pillsbury, as possible and where a supplemental water body is proposed as part of the mitigation scenarios.

## C2.2 Fish Passage Design Criteria

A wide range of design criteria were considered by SLR when evaluating fish passage alternatives, including but not limited to:

- Ability to provide volitional passage and accommodate biological design criteria for target fish species;
- Accommodating in-stream flow requirements during critical fish migration windows;
- Complexity and cost of implementation;
- Environmental impact during and after construction; and
- Long term maintenance and operations.

### C.2.2.1 Biological Design Criteria

The NOAA Fisheries West Coast Region *Anadromous Salmonid Passage Design Manual* (NMFS, 2022b) establishes engineering criteria for fishways and associated fish passage in the state of California. These guidelines are intended to ensure that selected fish passage facilities function effectively within project-specific contexts. The manual outlines quantitative design parameters, such as minimum water depth, maximum water velocity, and energy dissipation factors for fish ladders, as well as mesh size, porosity, and approach velocity requirements for fish screens.

Additional biological design criteria for the specific target fish species are summarized in Table C2-1:, below.

**Table C2-1: Biological Design Criteria for Target Fish Species (Modified from McMillen Jacobs, 2018)**

| Criteria / Target Species | Units | Value | Comments |
|---|---|---|---|
| | | | |
| **Fish Size** | | | |
| Steelhead | mm | 340 - 1050 | Fork length |
| Coho Salmon (Winter Run) | mm | 341 - 1050 | Fork length |
| Chinook Salmon (Fall Run) | mm | 342 - 1050 | Fork length |
| Sea Run Cutthroat | mm | 400 | Fork length |
| Pacific Lamprey | mm | 152 - 910 | Total length |
| | | | |
| **Average Fish Weight (Adult)** | | | |

| Criteria / Target Species | Units | Value | Comments |
|---|---|---|---|
| Steelhead | lb | 4-5 | |
| Coho Salmon (Winter Run) | lb | 4-6 | |
| Chinook Salmon (Fall Run) | lb | 18-22 | |
| Sea Run Cutthroat | lb | 1.5 | |
| Pacific Lamprey | lb | 1.0 | |
| | | | |
| **Swimming Capabilities (Sustained/Burst)** | | **Adults** | |
| Steelhead | fps | 3.0 / 20.3 | Jones et al. 1974 / Bell 1991 |
| Coho Salmon (Winter Run) | fps | 4.0 / 21.0 | Bell 1991 |
| Chinook Salmon (Fall Run) | fps | 5.0 / 14.0 | Geist et al. 2003 / Hunter and Mayor 1986 |
| Sea Run Cutthroat | fps | 3.0 / 14.0 | Bell 1991 |
| Pacific Lamprey | fps | 1.3 / 2.7 | Moursund et al. 2003 |
| | | | |
| **Migration Timing** | | | |
| Steelhead | mo. | Dec-Apr | Higgins 2010 |
| Coho Salmon (Winter Run) | mo. | Nov-Jan | Shapovalov 1954 |
| Chinook Salmon (Fall Run) | mo. | Oct-Dec | Higgins 2010 |
| Sea Run Cutthroat | mo. | Dec-Apr | Johnson and Jones 2002 |
| Pacific Lamprey | mo. | Variable | Luzier et al. 2001 |
| | | | |
| **Numbers of Fish at VAFS** | | | |
| Steelhead - mean annual (STD) | total | 550 (448) | 2005-2015 mean counts |
| Coho - mean annual (STD) | total | N/A | |
| Chinook - mean annual (STD) | total | 1194 (1117) | |
| Pacific Lamprey - mean annual | total | N/A | |

### C.2.2.2 Instream Flows during Fish Migration Periods

Target fish species in the Eel River typically migrate during the wet season, from November to May. Fall run Chinook salmon migrate upstream from November through January and downstream migration occurs primarily April through May. Steelhead migrate upstream January through May and downstream primarily February through May (Table C2-2:).

**Table C2-2: Adult Migration and Smolt Outmigration for Chinook Salmon and Steelhead from the Potential Ecosystem and Fisheries Responses to Project Alternatives Technical Memorandum (Stillwater Sciences et al., 2021)**

| Species | Life Stage | Month | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| Chinook Salmon Fall-run | Adult Migration | | | | | | | | | | | | |
| | Smolt Outmigration | | | | | | | | | | | | |
| Steelhead (Summer-run and Winter-run) | Adult Migration (Winter-run) | | | | | | | | | | | | |
| | Adult Migration (Summer-run) | | | | | | | | | | | | |
| | Smolt Outmigration | | | | | | | | | | | | |
| | Kelt (Adult) Outmigration | | | | | | | | | | | | |

Per VTN 1982, SEC 1998, CDFW, unpublished VAFS count data, Beak 1986 and Moyle et al. Per VTN (1982), SEC (1998), CDFW, unpublished VAFS count data, Beak (1986) and Moyle et al. (2017).

Minimum streamflow requirements have been established in the Eel River below Lake Pillsbury and Van Arsdale Reservoir to protect Chinook salmon and Steelhead populations, and in the East Branch Russian River to support a managed rainbow trout fishery upstream of Lake Mendocino. Before 1979, the year-round minimum flow below Van Arsdale Reservoir was just 2 cubic feet per second (cfs). Since then, more natural flow patterns have been adopted. The NMFS's 2002 Reasonable and Prudent Alternative (RPA) updated flow requirements, aiming to replicate the natural hydrograph, supporting key life stages of salmonids, including fall and winter migration, spring emigration, and summer rearing. From December to March, minimum flows from Cape Horn Dam range between a floor of 100 cfs and a cap of 140 cfs, though the minimum can drop to 25 cfs during periods of exceptionally low inflow to Lake Pillsbury (NMFS, 2002).

# C3.0 Scenario Descriptions and Evaluation

This section presents the range of alternatives considered by SLR in the Impact Analysis and Adaptation Strategies evaluation for potential modifications to the Scott Dam facilities. These alternatives span from partial dam removal and traditional fish passage methods to full decommissioning of Scott Dam (as proposed by PG&E). The following sections provide a summary of the proposed scenarios as applicable to fish migration and the SLR's findings.

## C3.1 Current Operational Conditions Scenario (WY 2019-Present)

### C.3.1.1 General Description

This option involves keeping Scott Dam in its current configuration without adding any fish passage facilities. Based on observed operational data, the maximum operational WSE for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 feet with a corresponding water storage volume of 56,322 ac-ft as depicted in Figure C3-1. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the WSE at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained.

### C.3.1.2 Evaluation Summary

#### C.3.1.2.1 Proposed Work for Fish Pathway Improvements

No work proposed under this summary other than outstanding and routine maintenance and operations.

#### C.3.1.2.2 Fish Passage

Under this scenario the dam would continue to be a barrier and block access to upstream habitat for target fish species. However resident fish populations in Lake Pillsbury, including Largemouth bass (*Micropterus salmoides*), Green sunfish (*Lepomis cyanellus*), Bluegill (*Lepomis macrochirus)*, Rainbow trout (*Oncorhynchus mykiss*) and Sacramento sucker (*Catostomus occidentalis*), would be maintained.

#### C.3.1.2.3 Instream Flows

Instream flows downstream of Scott Dam during the fish migration period would remain unchanged. A statistical summary of the observed wet season climate conditions (WYs 2019-2024) corresponding to the fish migration period is presented in Table C3-1. Median and average flows in this scenario meet the minimum in-stream flow requirements, however the minimum average daily flow of 67 cfs drops below the recommended flows of 100 cfs (NMFS, 2002).

**Table C3-1: Existing Operational Conditions – Statistics for Observed Storage within Lake Pillsbury and Observed Outflow Downstream of Scott Dam during the Wet Season Fish Migration (WY 2019-2024)**

| Statistic | Wet Season (December – April) | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 16,800 | 1870.9 | 67 |
| Median | 53,200 | 1898.4 | 207 |
| Average | 45,439 | 1893.8 | 768 |
| Maximum | 67,300 | 1905.6 | 13,200 |

#### C.3.1.2.4 Operation and Maintenance

Continued operation and maintenance with minimal capital investment. If the seismic conditions change additional capital improvement may be needed

#### C.3.1.2.5 Ecosystem

No major changes; current barriers to fish migration remain. Fish habitats within the Lake Pillsbury will be sustained.

#### C.3.1.2.6 Advantages

- Existing biological communities and habitat conditions would remain unchanged.

#### C.3.1.2.7 Disadvantages

- Scott Dam would remain a complete barrier to fish passage, preventing access to upstream habitat in the upper watershed.

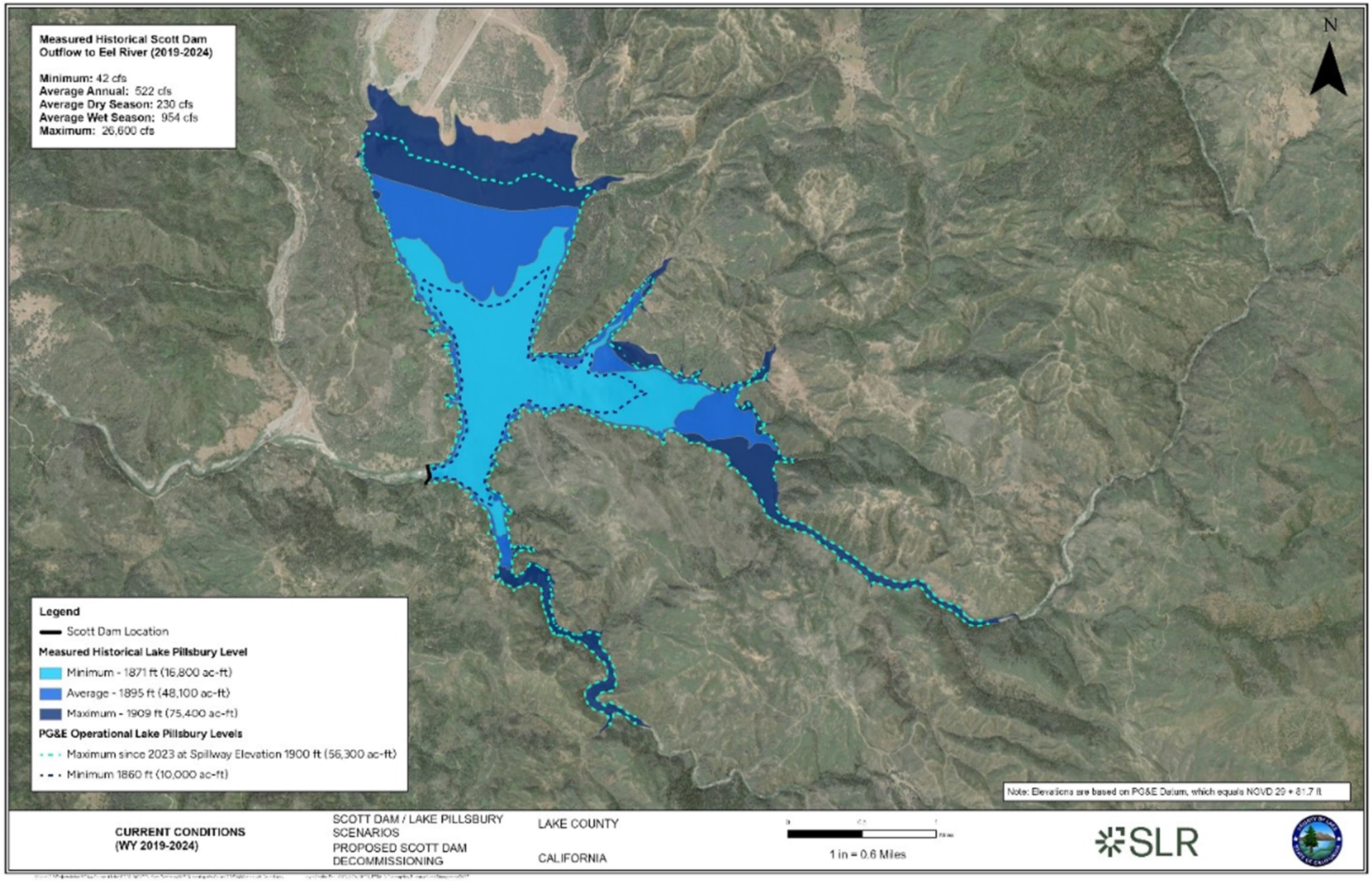


**Figure C3-1: Existing Operational Conditions (WY 2019-2024), with measured minimum average, maximum, and operational Lake Pillsbury WSE levels (Figure A7-3 from Appendix A).**

## C3.2 Scenario A1 – PG&E Proposed Removal

### C.3.2.1 General Description

Under this scenario, PG&E proposes the full decommissioning of Scott Dam. A plan view depiction of the Scott Dam and Lake Pillsbury area with projected response to Scenario A1 conditions are presented in Figure C3-2 and Figure C3-3. The Eel River and tributaries are assumed to return to their pre-dam condition as represented in the 1922 topographic and bathymetric mapping compiled by PG&E (2016).

The proposed approach is divided into three phases per PG&E's current proposed plan (PG&E, 2025), including two low-flow seasons and one high flow season:

a. Low flow season one (June 1 – October 31) - initial dewatering and removal of the upper portion of the dam;

b. High flow season one (November 1 – May 31) - initial sediment flushing; and

c. Low flow season two (June 1 – October 31) - final dam removal.

As proposed by PG&E, the sediments that have accumulated within the impoundment will be managed passively. PG&E's current proposed action under the Draft Application for Surrender of License (PG&E, 2025) includes the decommissioning and removal of Scott Dam, draining Lake Pillsbury and removing associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. PG&E proposed a Rapid Dam Removal approach estimated to take 2 years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards) downstream of the Eel River (PG&E, 2025). PG&E anticipates temporary alteration of the Eel River channel and/or floodplain morphology from sediment deposition after deconstruction of the dam until subsequent high-flow events can resuspend the sediment and transport it farther downstream, until the river channel adapts to its natural condition. Other activities proposed by PG&E as part of decommissioning are documented in the Draft Application for Surrender of License (PG&E, 2025). PG&E may propose additional responses at locations where natural fluvial processes are not mobilizing sediment to the volumes anticipated. These locations may have continued issues with bank instability and channel incision that may adversely affect habitat and fish passage.

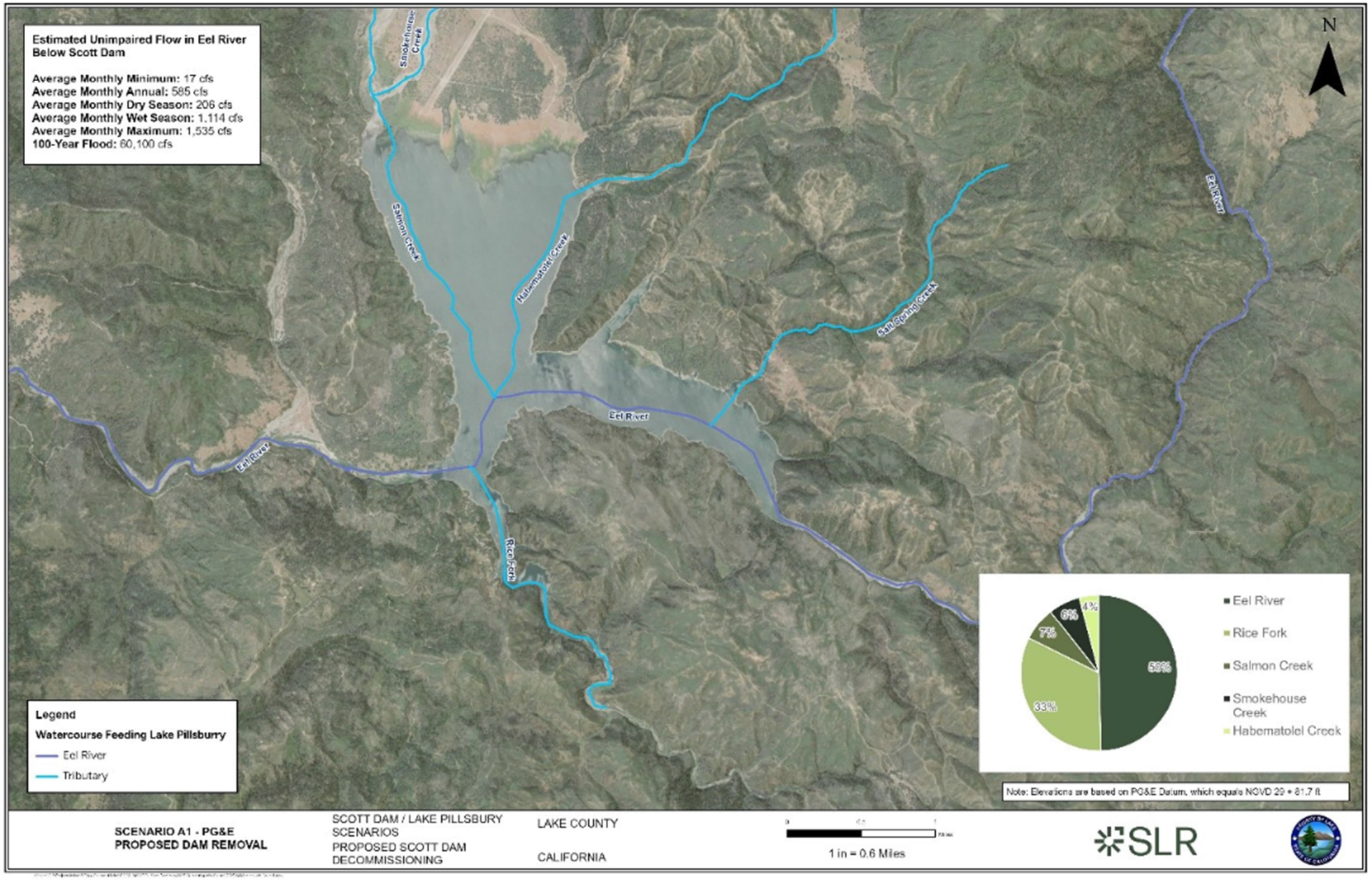


**Figure C3-2: Scenarios A1 and A2 PG&E proposed Scott Dam removal, showing Eel River and tributary watercourses originally feeding Lake Pillsbury (Figure A7-10 from Appendix A).**

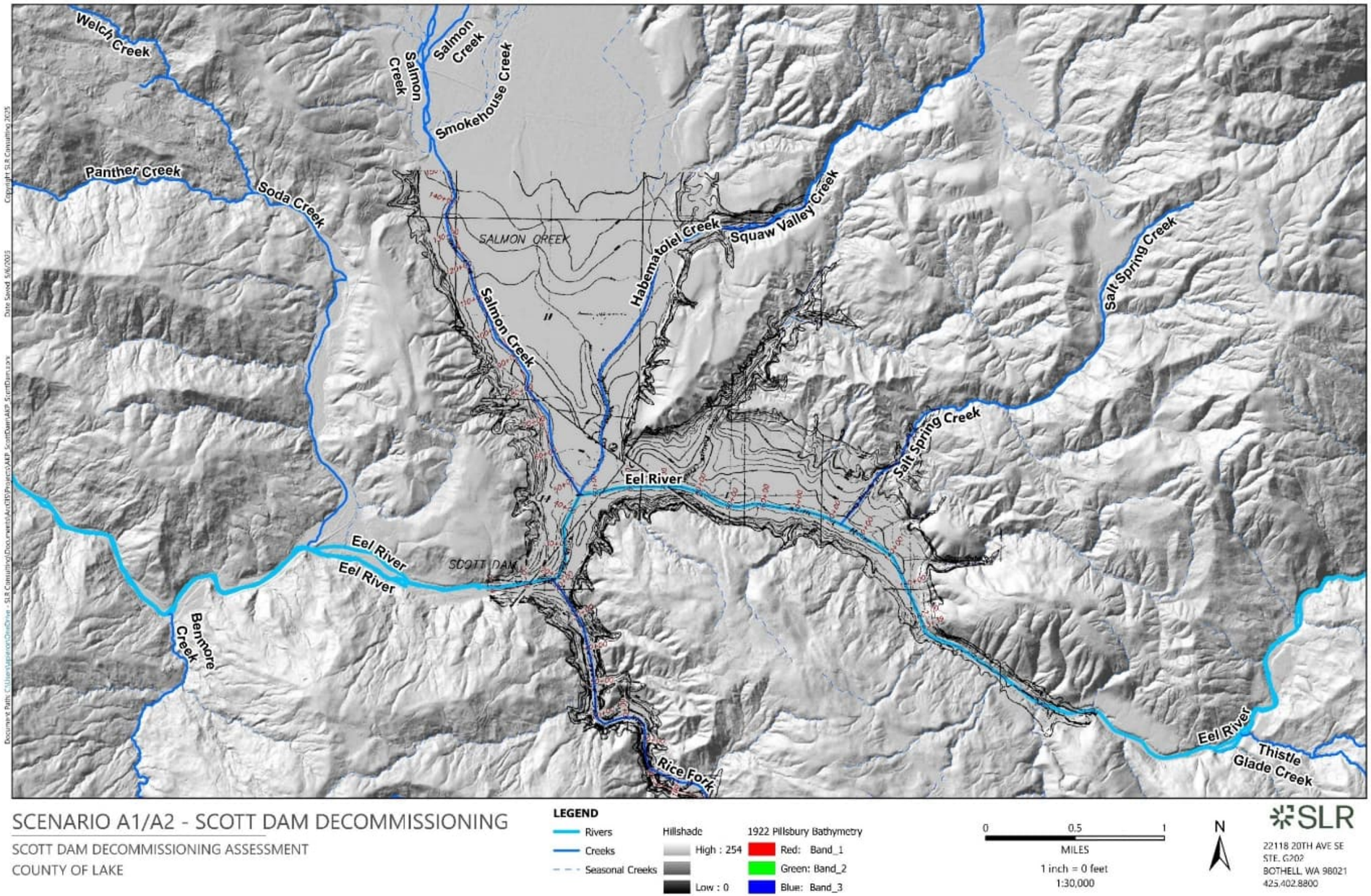


**Figure C3-3: Scenarios A1 and A2 Projected Response Post Dam Removal (Figure A7-11 from Appendix A).**

### C.3.2.2 Evaluation Summary

#### C.3.2.2.1 Proposed Work for Fish Pathway Improvements

Rapid removal of Scott Dam and associated structures per PG&E plans. No additional work is proposed.

#### C.3.2.2.2 Fish Passage

This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. Existing warmwater aquatic habitat in the reservoir would be eliminated.

#### C.3.2.2.3 Instream Flows

Instream flows in the Eel River for Scenario A1 during the wet season fish migration period are summarized in Table C3-2. The average monthly wet season flows range from 791 cfs in April to 1,535 cfs in February, representing an increase in flow during the critical fish migration period and an improvement compared to the existing operating conditions. Instream flows during the wet season remain well above the recommended floor of 100 cfs.

**Table C3-2: Dam Removal Scenario A1 Wet Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 791 |
| Median | 1,237 |
| Average | 1,216 |
| Maximum | 1,535 |

#### C.3.2.2.4 Channel Restoration and Sediment Management

PG&E has proposed a passive sediment management approach with no detailed channel or floodplain restoration plan submitted to date. The plan pledges to develop and implement a Sediment/Channel Monitoring Response and Restoration Plan, which would monitor tributary mouths and critical riffles downstream of Scott Dam for sediment accumulation and the former reservoir footprint for channel downcutting and erosion issues. Field crews and/or equipment would be dispatched to remove/reposition sediment, as required after dam removal to facilitate fish passage through the Eel River, the dam footprint and the former reservoir. Without active management, channels could become deeply incised, resulting in the loss of functional floodplains, degradation of riparian habitat, disruption of natural pool–riffle sequences, and reduction of available spawning habitat.

#### C.3.2.2.5 Ecosystem

The primary environmental impact of the rapid dam removal alternative would be the release of 12 million cubic yards of accumulated sediment from behind Scott Dam. During the initial 1-2

days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species.

The majority of clay and silt sediments are expected to be transported to the coast over time, however extremely high suspended sediment load in the days following the sediment release is expected to result in lethal effects on aquatic species inhabiting the mainstem Eel River. Significant volumes of coarser sand are expected to deposit in habitat features from Scott Dam to approximately the Middle Fork Eel River confluence. Sand deposition would diminish aquatic habitat by burying pools and coarse riffle habitat until the river can flush accumulated sediment and establish a new geomorphic equilibrium[22]. Flushing of accumulated sediment is expected to take a number of years and depends on local hydrology and storm magnitude in the season following removal. Flushing events may also pose risk to aquatic species depending on the wash load, or the fine sediment particles carried in suspension by the Eel River.

An additional impact is the elimination of warmwater aquatic habitat in Lake Pillsbury and the potential release of non-native fish species from the reservoir into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River and will require PG&E to implement a Construction Non-native Invasive Aquatic Species Management Plan to minimize the spread of non-native species.

#### C.3.2.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.2.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.

[22] The river is not gaining or losing material overall—it's stable, even though small changes may still happen.

## C3.3 Scenario A2 – Scenario A1 with Additional Mitigation

### C.3.3.1 General Description

Under this scenario, PG&E proposes the full decommissioning of Scott Dam and the County of Lake proposes additional mitigation efforts implemented to address their concerns and those of the local community. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment movement. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the County of Lake, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and would have higher ecosystem benefits to the community.

Figure C3-2 and Figure C3-3 will similarly apply when predicting the changes to the Eel River system upstream of Scott Dam upon scenario completion. Figure C3-4 shows an example of the results of sediment management and revegetation efforts in the Elwha Dam reservoir over a 6-year period. The floodplain has been recolonized and stabilized by native plant species.

**Figure C3-4: Elwha River reservoir soon after the dam removal (top) and six years later (bottom), after the implementation of a comprehensive revegetation strategy (photo credit: Gussman, 2018)**

### C.3.3.2 Evaluation Summary

#### C.3.3.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures with additional revegetation and other ecosystem enhancements in the reservoir footprint and sediment management.

#### C.3.3.2.2 Fish Passage

Similar to Scenario A1, this option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. Existing warmwater aquatic habitat in the reservoir would be eliminated.

#### C.3.3.2.3 Instream Flows

Instream flows in the Eel River for Scenario A2 would be the same as Scenario A1, as summarized in Table C3-2. The average monthly wet season flows would range from 791 cfs in April to 1,535 cfs in February, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions.

#### C.3.3.2.4 Channel Restoration and Sediment Management

This option calls for the excavation and re-establishment of the river channel through the reservoir reach. Excavated sediment removed to reconstruct the channel would be relocated to the overbank areas or removed off-site. The new river channel and overbank areas would then be stabilized through structural and vegetative techniques. Revegetation of the overbank floodplain in the former reservoir through an extensive planting plan would expedite site stabilization, reduce the volume of sediment transported downstream, and help mitigate some of the environmental impacts identified in Scenario A1 (Section C3.2). With the channel restoration and sediment management measures, Scenario A2 would have a beneficial effect on habitat for fish and aquatic habitat.

#### C.3.3.2.5 Ecosystem

Despite sediment management and mitigative measures, the short-term environmental impacts resulting from the initial sediment flush are expected to be similar to Scenario A1. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species. However, the long-term volume of mobilized sediment is expected to be less than Scenario A1 due to sediment management within the reservoir and revegetation efforts that will stabilize expose soils.

This scenario may also result in the release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile

salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River.

#### C.3.3.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.3.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.

## C3.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

### C.3.4.1 General Description

This scenario is similar to the current operational conditions described in Section C3.1, with the addition of a fish ladder, as previously evaluated by Mead and Hunt (2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, a seismic retrofitting of the dam would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules, and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures.

### C.3.4.2 Evaluation Summary

#### C.3.4.2.1 Proposed Work for Fish Pathway Improvements

The Mead and Hunt modified conventional fishway (2018) consists of a pool and weir fishway on the southern abutment of the dam to provide volitional fish passage over Scott Dam. The majority of pools would be located downstream of the dam, and the exit pools upstream of the dam, with the two sections connected by a 48-inch diameter tunnel bored through the dam (Figure C3-5). Boring a tunnel through the face of the concrete dam would introduce additional seismic concerns.

#### C.3.4.2.2 Fish Passage and Instream Flows

The fish ladder could be designed to operate over the range of flows observed under current operational conditions, as described in Section C3.1. However, facilitating volitional fish passage at the dam through the fishway would be challenging, considering the range of reservoir fluctuations. To address the fluctuation in reservoir levels, the fishway exit must operate over the full range of water surface elevations. This would be achieved through controllable gated openings through the fishway exit perimeter wall that can be opened or closed as lake levels rise and fall. The exit pool nearest to the lake level can be opened to allow access to migrating salmonids. This highly complex, mechanical fishway would increase the capital and maintenance costs of the system.

The design also relies on the upstream fish ladder for downstream fish passage, which is not a conventional or demonstrated approach. The downstream migration system lacks effective attraction velocities at the downstream passage entry to attract out-migrants to the ladder. It also introduces the risk of predation on out-migrants by adults migrating upstream through the same system.

Mead and Hunt (2018) ultimately concluded that the proposed fish ladder system would be difficult to construct, complex to manage, expensive, and potentially ineffective, primarily due to challenges in attracting out-migrating fish to the ladder and increased predation risks during migration.

#### C.3.4.2.3 Ecosystem

This scenario is expected to have relatively minimal environmental impact during construction compared to other dam removal options. However, excavation and blasting would be required on the left (south) abutment of the dam. The fishway would provide some improvement to fish migration post-construction, but the ultimate effectiveness is uncertain.

#### C.3.4.2.4 Advantages

- Existing biological communities and habitat conditions would remain unchanged.
- Potential increase in resident/anadromous fish production.
- Would provide upstream fish passage over the dam.

#### C.3.4.2.5 Disadvantages

- Uncertain fish passage effectiveness, particularly for downstream migration.

- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level would need to be maintained within a set range to support operation.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.
- Would require extensive excavation and construction activities on the left abutment of the dam.
- Tunnel through concrete dam face may introduce additional seismic concerns.

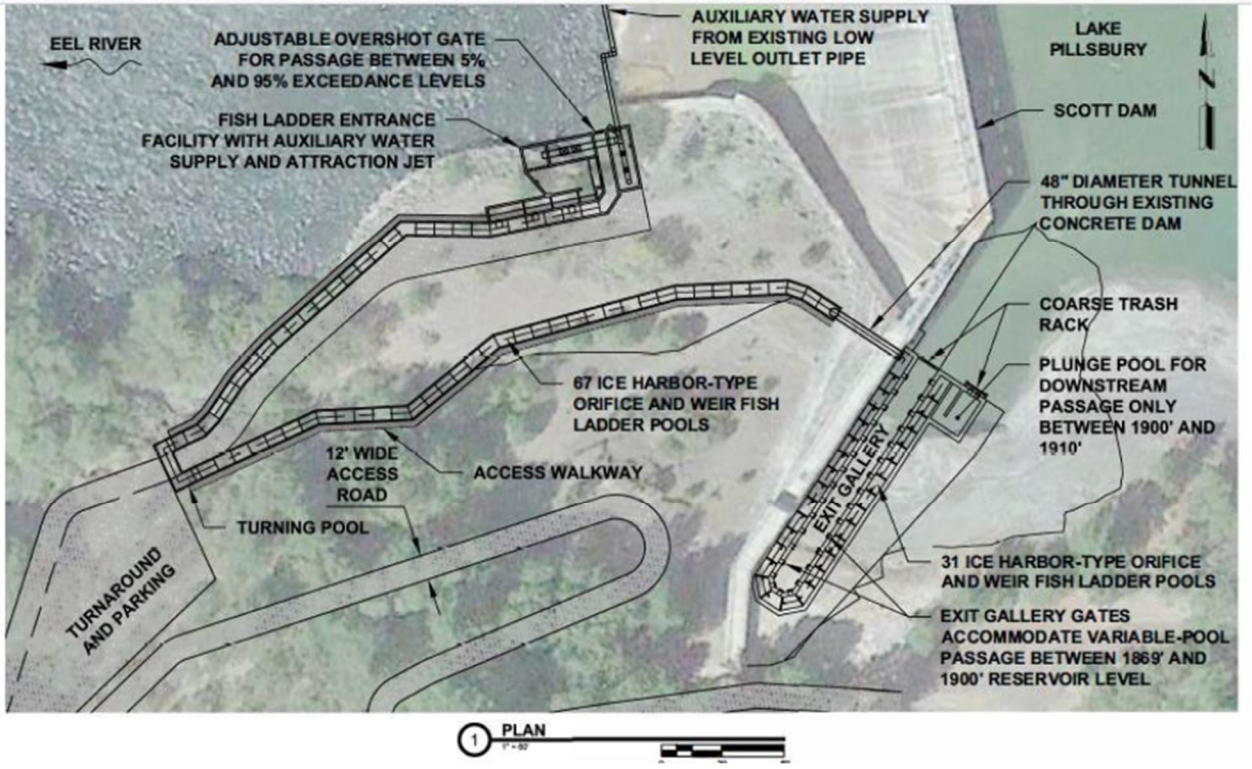


**Figure C3-5: Mead & Hunt Modified Conventional Fishway (2018)**

## C3.5 Scenario C – Lowered Scott Dam with Fish Ladder

### C.3.5.1 General Description

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 feet (PG&E datum) and the installation of a fish passage system, similar to the Mead and Hunt fishway described in Scenario B. Modifications to the spillway structure would be required to facilitate safe downstream fish passage over the dam and into the tailrace. The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 feet would create a reservoir of approximately 40 percent of the current storage volume at WSE 1,900 feet. Potentially, the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain

minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury WSEs and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage at the current maximum operational WSE of 1,900 feet, the spillway invert may be further reduced in height by 25 feet below the existing spillway invert, which would result in a new lake footprint shown in Figure C3-6. The smaller reservoir footprint would resemble Lake Pillsbury during drought conditions, as shown in Figure C37. At that reduced maximum operational WSE, the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam abutment area (where previous slides have occurred) by placing the concrete material on both or either of the downstream and upstream sides of the lowered dam. Further evaluation would be required to understand the effects on the adjacent land area and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation, and ecosystem benefits as compared to the full decommissioning.

This alternative would significantly alter project operations. Reduction of water storage in Lake Pillsbury would transition the project to a run-of-river operation, ending typical water releases and diversions from stored supplies. Despite the operational changes, the installation of an upstream fishway - along with downstream fish passage over the lowered spillway - would enhance fish migration conditions at the site.

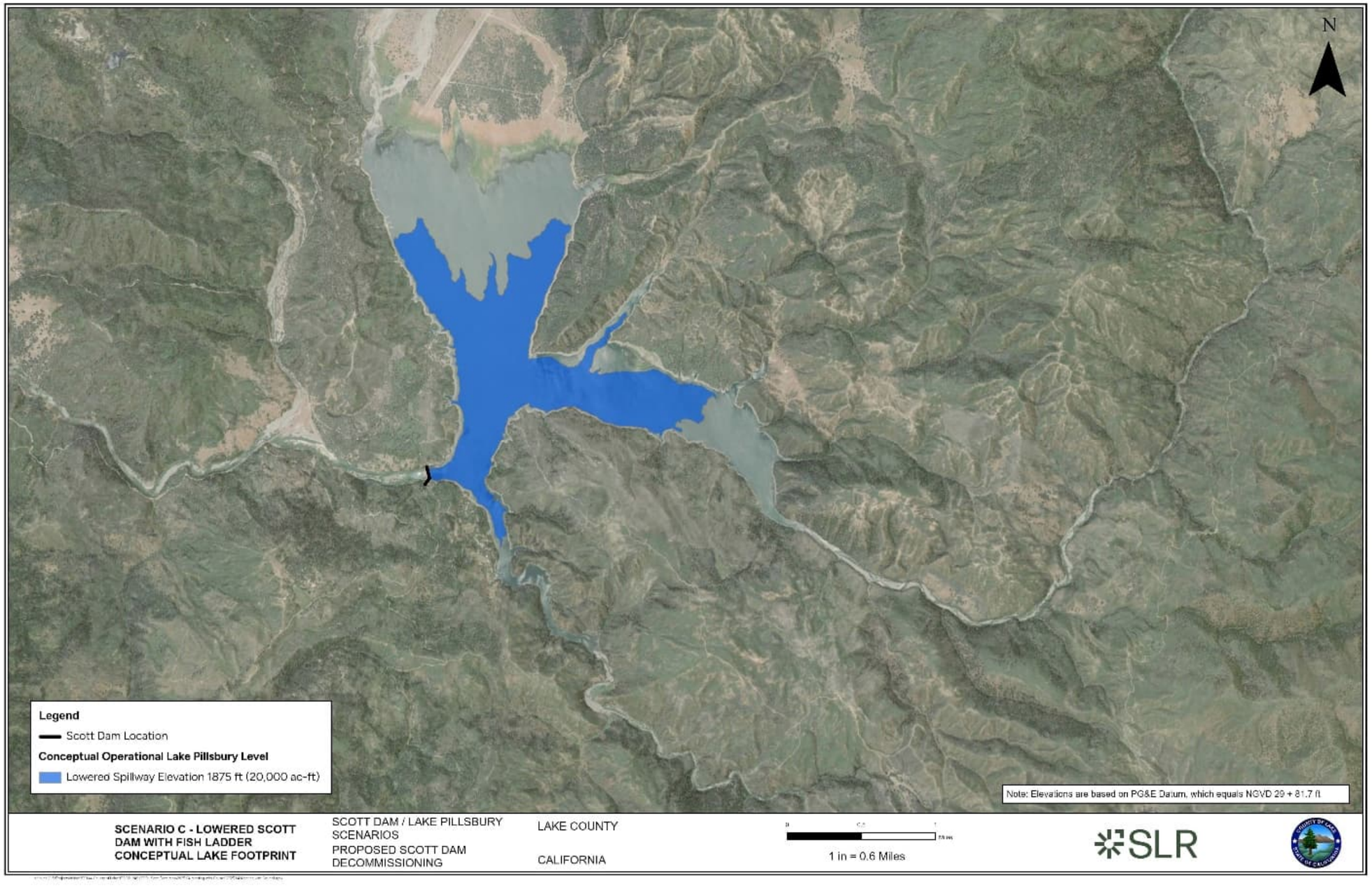


**Figure C3-6: Scenario C lowered Scott Dam with Fish Ladder: Conceptual Lake Pillsbury Footprint (Figure A7-16 from Appendix A).**

## C.3.5.2 Evaluation Summary

### C.3.5.2.1 Proposed Work for Fish Pathway Improvements

At Scott Dam, the existing spillway would be lowered to reduce the hydraulic drop, facilitating direct downstream passage of juvenile fish over the spillway and allowing for a lower height fishway structure. Reservoir operations would transition to run-of-river mode, allowing natural river flows to pass over the spillway or through the fishway. A lower fish ladder, similar to the Mead and Hunt modified conventional fishway (2018) consisting of a pool and weir fishway on the southern abutment of the dam would provide volitional fish passage. The majority of pools would be located downstream of the dam, and the exit pools upstream of the dam, with the two sections connected by a 48-inch diameter tunnel bored through the dam (Figure C3-5).

### C.3.5.2.2 Fish Passage and Instream Flows

The lowered fish ladder could be designed to operate over the range of wet season flows modelled in the hydrological assessment for the lowered dam scenario, summarized in Table C3-3. However, like Scenario B, the fishway exit must be designed to operate over the full range of reservoir elevations. This would be achieved through the same controllable gate mechanism at the fishway exit. The exit pool closest to the lake level would be opened to allow access to migrating salmonids. The lowered dam allows operation over a narrower range of water levels, but still requires a complex, mechanical fishway that would increase the capital and maintenance costs of the system. Downstream migration of juvenile fish would be possible over the crest of the lowered spillway.

**Table C3-3: Lowered Scott Dam with Fish Ladder – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam during the Wet Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 5,420 | 1850.7 | 20 |
| Average | 18,760 | 1872.7 | 1,116 |
| Maximum | 28,460 | 1882.2 | 47,470 |

### C.3.5.2.3 Ecosystem

This scenario is expected to have relatively minimal environmental impact during construction compared to other dam removal options. However, excavation and blasting would be required on the left (south) abutment of the dam. Post-construction, the fishway would ultimately provide improvements to fish migration compared to current operational conditions.

### C.3.5.2.4 Advantages

- Would provide upstream and downstream fish passage.
- Potential increase in resident/anadromous fish production.
- Restoration of historical ecosystem function and dynamics.

- Potential improvement to water quality and temperature due to reduction of reservoir.

#### C.3.5.2.5 Disadvantages

- Existing biological communities and habitat conditions will be impacted in the short term from the change in lake footprint and depth.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level has to be maintained within a set range to support operation.
- May not be fully effective during summer periods due to change in water temperature due to the surface draw of the fishway and lower level release to the river.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.

**Figure C3-7: Channel migration and downcutting and reduced reservoir footprint area at the head of Lake Pillsbury during the 2014 drought. Lowering the crest of Scott Dam will result in a permanently smaller reservoir footprint (Stillwater, 2021).**

## C3.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

### C.3.6.1 General Description

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure, consistent with the Rapid Dam Removal approach outlined in Scenario A1. A new 3,100-foot long earthen dam would be established in the Salmon/Smokehouse and Habematolel Creek watershed areas to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir. A conceptual rendering of the Salmon Creek reservoir, superimposed on imagery from the Klamath River upstream of the Iron Gate Dam

removal is provided in Figure C3-9**Error! Reference source not found.**, as an analog for visual comparison. The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction, with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within the Lake Pillsbury footprint can also be reused for the new impoundment construction if suitable. Low-permeability clay/silt sediments can be used in the core of the dam, and gravelly sediments can be used in the embankment/slopes of the new dam.

This option will support continued use of recreation facilities, ecosystem benefits and fire suppression. Similarly to Scenarios A1 and A2, this scenario would eliminate the existing fish barrier at Scott Dam and return the Eel River and Rice Fork to a free-flowing river system with unimpeded upstream and downstream fish passage. However, the construction of a new dam would prevent fish passage to Habematolel, Salmon and Smokehouse Creeks. This would result in 86% of the total flows toward Scott Dam being unimpeded, with the remaining 14% of flows blocked by the proposed earthen dam (Figure C3-8).

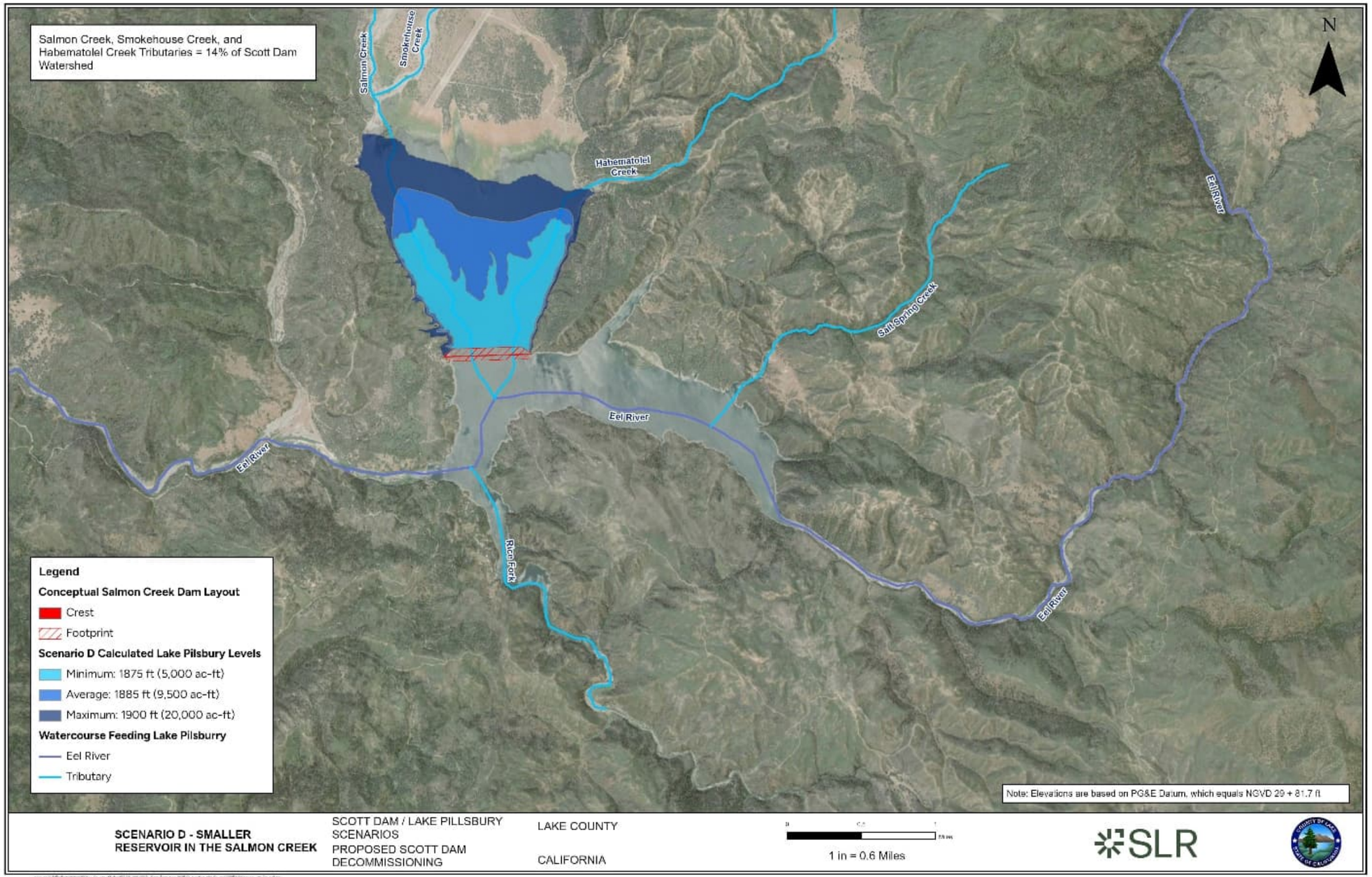


**Figure C3-8: Scenario D smaller reservoir in the Salmon Creek watershed: projected minimum, average, and maximum Lake Pillsbury WSE levels (Figure A7-19 from Appendix A).**

**Figure C3-9: Conceptual Salmon Creek Reservoir mock-up, using Klamath River imagery as an analog (Photo credit: *Shane Anderson/Swiftwater Films, 2024)*.**

## C.3.6.2 Evaluation Summary

### C.3.6.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures would allow fish passage in the upper Eel River. This scenario considers construction of a new earthen dam to retain flows from the Salmon/Smokehouse and Habematolel Creek watershed, no fish passage is proposed for the new earthen dam.

### C.3.6.2.2 Fish Passage

Similar to Scenario A1, this option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. No fish passage is proposed for the new earthen dam, nevertheless two thirds of the watershed will allow natural fish passage due to the removal of the Scott Dam.

### C.3.6.2.3 Instream Flows

Instream flows in the Eel River for Scenario D during the wet season fish migration period are summarized in Table C3-4. The average monthly wet season flows range from 1,327 cfs in February to 684 cfs in April, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions. The 14% reduction in contributing drainage from the blocked tributaries is not expected to negatively impact fish passage in the mainstem of the Eel River. Unimpaired flows during the migratory wet season remain well above the recommended floor of 100 cfs.

**Table C3-4: Scenario D Wet Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 684 |
| Median | 1,070 |
| Average | 1,051 |
| Maximum | 1,327 |

#### C.3.6.2.4 Channel Restoration and Sediment Management

At this stage it is assumed that channel restoration and sediment management in the Lake Pillsbury reservoir footprint for Scenario D will mostly be consistent with PG&E's Final Draft Surrender Application (PG&E, 2025). PG&E will develop a passive Sediment/Channel Monitoring Response and Restoration Plan - no detailed channel or floodplain restoration plans have been submitted to date. However, extensive sediment management is expected to be required in the footprint of the proposed Salmon Creek dam to provide a stable foundation capable of supporting the dams weight and the reservoir's water pressure. Once the reservoir is drained, impounded sediment will be excavated and removed into the overbank or an offsite location. Depending on the existing sediments encountered on site, imported material may be required to prevent settlement, ensure uniform bearing capacity and minimize seepage and erosion.

#### C.3.6.2.5 Ecosystem

The primary environmental impact of the rapid dam removal alternative would be the release of 12 million cubic yards of accumulated sediment from behind Scott Dam. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species.

The majority of clay and silt sediments are expected to be transported to the coast over time, however extremely high suspended sediment load in the days following the sediment release is expected to result in lethal effects on aquatic species inhabiting the mainstem Eel River. Significant volumes of coarser sand are expected to deposit in habitat features from Scott Dam to approximately the Middle Fork Eel River confluence. Sand deposition would diminish aquatic habitat by burying pools and coarse riffle habitat until the river can flush accumulated sediment and establish a new geomorphic equilibrium. Flushing of accumulated sediment is expected to take a number of years and depends on local hydrology and storm magnitude in the season following removal.

An additional impact is the potential release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River downstream.

In addition to impacts resulting from the removal of Scott Dam, Scenario D involves the construction of a 3,100 ft long earthen dam in the Salmon Creek watershed. Construction of the dam is anticipated to have adverse effects on resident fish populations; however, it is noted that removal of Scott Dam, the draining of Lake Pillsbury and the associated impacts to the resident fish population will preclude any new dam construction.

#### C.3.6.2.6 Advantages

- A portion of the existing biological communities and habitat conditions would remain unchanged or undergo short term changes while the new dam is being constructed.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat within the Eel River and Rice Fork.
- Restores historic river ecosystem functions and natural hydrologic dynamics within the Eel River and Rice Fork.
- Over time, parts of the Eel River ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.6.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Requires extensive engineering and construction of new dam.

## C3.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

### C.3.7.1 General Description

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2, but with specific mitigation added to enhance fish passage while maintaining fire suppression and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[23], the off-channel ponds would be constructed.

[23] Quasi-equilibrium in this case refers to the ability of a river to maintain its dimension, pattern and profile without either aggrading or degrading over time.

Two potential pond locations have been identified as shown in Figure C3-10 based upon the 1922 historic pre-dam topography and bathymetry (McBain Associates, 2018). The 1922 topography and bathymetry was georeferenced to the Lake County hillshade to predict how the site will look after restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table C3-5 summarizes the potential footprints and volumes of Ponds A and B assuming a 10-foot normal depth. The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design. The conceptual design would function similarly to the example provided in **Error! Reference source not found.**, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table C3-5: Potential Pond Footprint and Storage Volume of Ponds A and B under Scenario E**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

## C.3.7.2 Evaluation Summary

### C.3.7.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures with additional active sediment management, channel restoration, off-channel pond excavation and revegetation in the reservoir footprint.

### C.3.7.2.2 Fish Passage

Similar to Scenarios A1, A2 and D, this scenario would eliminate the existing fish barrier at Scott Dam and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. The ponds could serve as spawning grounds and shelter habitat for fish and other aquatic species, and the free-flowing Eel River could provide easy transportation for fish through the watershed, from ponds to streams. The combination of preserving still-water habitats and allowing for stream-based habitats makes this scenario favorable for long term ecological function.

It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage in the Eel River through the former Lake Pillsbury would occur during the fall season after the final dam removal.

### C.3.7.2.3 Instream Flows

Under this scenario, it is assumed that Pond A will be fed by all inflow from the Habematolel Creek and up to 10% of the Salmon and Eel River during normal wet and dry seasons. Pond B

assumes a contribution of an additional 10% of flows from the Eel River, meaning both ponds will take up to 10% of the Eel River flows each. This scenario will reduce the total flow contribution from the Eel River downstream of Scott Dam by up to 14% when compared to Scenarios A1 and A2 during normal wet and dry seasons.

Average monthly instream flows in the Eel River for Scenario E during the wet season fish migration period are summarized in Table C3-6. The average monthly wet season flows range from 1,324 cfs in February to 682 cfs in April, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions. The 14% reduction in flows resulting from the pond construction is not expected to negatively impact fish passage in the mainstem of the Eel River. Unimpaired flows during the migratory wet season remain well above the recommended floor of 100 cfs.

**Table C3-6: Dam Removal Scenario E Wet Season Monthly Average Statistical Unimpaired Flows Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 682 |
| Median | 1,067 |
| Average | 1,048 |
| Maximum | 1,324 |

#### C.3.7.2.4 Channel Restoration and Sediment Management

This option calls for extensive sediment management and the excavation and re-establishment of the river channel through the reservoir reach. Excavated sediment removed to reconstruct the channel would be relocated to the overbank areas or removed off-site. The new river channel and overbank areas would then be stabilized through structural and vegetative techniques. Revegetation of the overbank floodplain in the former reservoir through an extensive planting plan would expedite site stabilization, reduce the volume of sediment transported downstream, and help mitigate some of the environmental impacts identified in Scenario A1. Once vegetation in the floodplain has established and the sediments have been stabilized, the off-channel ponds would be constructed. With the channel restoration, sediment management and off channel pond construction, Scenario E would have a beneficial effect on habitat for fish and aquatic habitat.

#### C.3.7.2.5 Ecosystem

Despite sediment management and mitigative measures, the short-term environmental impacts resulting from the initial sediment flush are expected to be similar to Scenario A1. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species. However, the long-term volume of mobilized sediment is expected to be less than Scenario A1 due to sediment management within the reservoir and revegetation efforts that will stabilize expose soils.

This scenario may also result in the release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River.

After the removal of Scott Dam and associated sediment flushing, environmental impacts resulting from construction of the off-channel ponds in the former reservoir are anticipated to be minor, assuming best management practices can be followed. Implementation of the ponds would have beneficial effects on critical habitat for target fish species and other aquatic organisms, including amphibians, benthic invertebrates and algae.

#### C.3.7.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat. This enhancement is increased from scenario A1 by adding ponds to support spawning and shelter for fish and aquatic species.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.7.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal. This would be of a lesser degree with active sediment management and restoration efforts than scenario A1.

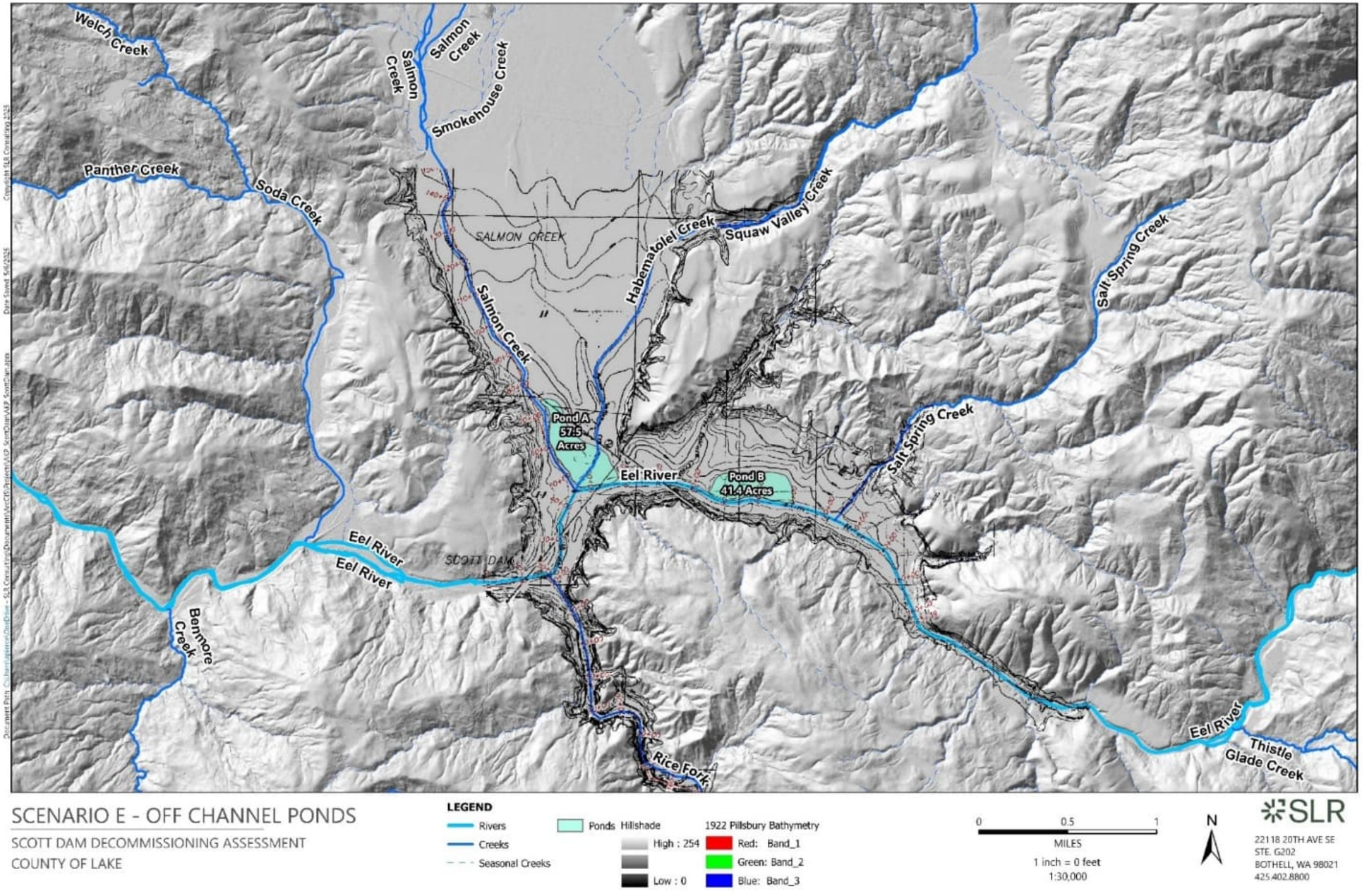


**Figure C3-10: Figure A7-24 from Appendix A: Scenario E - Dam Removal with Off-Channel Pond Restoration.**

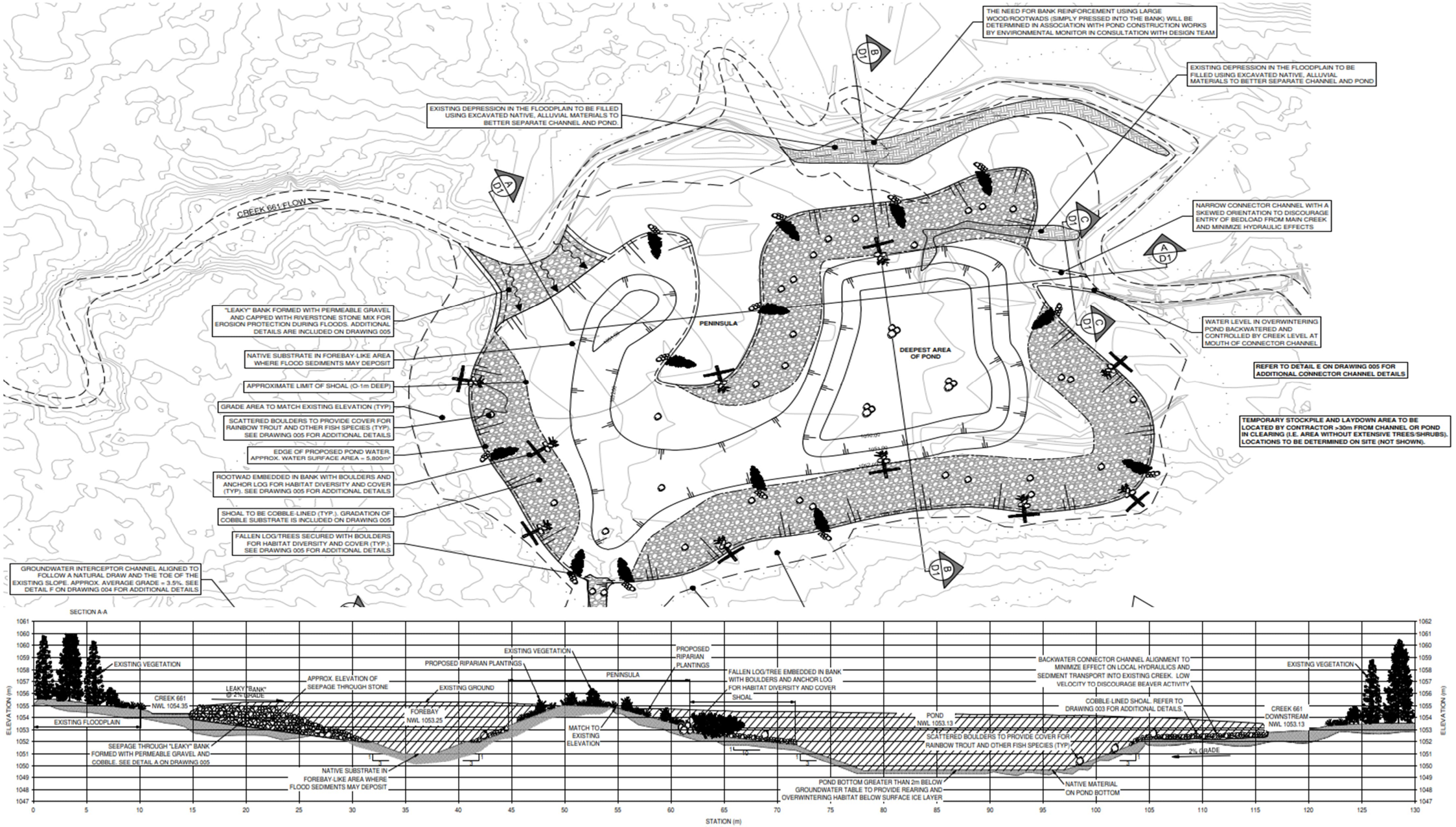


**Figure C3-11: Example design of an off-channel pond from a fish habitat offsetting project in central British Columbia, Canada (Palmer/SLR, 2023)**

# C4.0 Conclusions

The purpose of this fish passage evaluation completed by SLR is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of mitigation strategies required under the Department of Water Resources (DWR) grant. Specifically, this Appendix outlines how potential decommissioning may affect fish passage in the Eel River system and identifies potential restoration and alternate mitigation opportunities within the Lake Pillsbury area for different scenarios. A summary of all scenarios is presented in Table C4-1.

This Appendix presents the initial qualitative analyses conducted to assess the feasibility of these alternatives, with a focus on fish passage in the Eel River. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource.

This analysis does not represent a final decision or commitment for any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

The results present here provides an initial step in a broader, ongoing process. Further studies, analyses, and synthesis will be required, ultimately informing decisions by the County of Lake and other stakeholders.

**Table C4-1: Summary of Fish Passage Evaluation Criteria for Current and Proposed Scenarios**

| Scenario | Proposed Work | Fish Passage | Instream Flows | Ecosystem |
|---|---|---|---|---|
| Current Operational Conditions (WY 2019-2024) | Maintain current operational conditions. | No | Unchanged | No major impacts; current fish barrier remains |
| Scenario A1 – PG&E Proposed Removal | Rapid removal of Scott Dam and associated structures. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flows during fish migratory period. | Significant short term sediment release from reservoir;<br>Long term sediment impacts;<br>Release of non-native species from Lake Pillsbury. |
| Scenario A2 - Scenario A1 with Additional Mitigation | Rapid removal of Scott Dam and associated structures. Sediment management and revegetation in the reservoir footprint. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flows in Eel River during fish migratory period. | Significant short term sediment release from reservoir;<br>Long term sediment impacts partly mitigated by sediment management;<br>Release of non-native species from Lake Pillsbury. |
| Scenario B - Maintain the Current Operations of Scott Dam but Retrofit the Dam with Fish Ladder | Fish ladder on southern dam abutment. | Upstream passage through fish ladder. Uncertain downstream passage. | Mechanical gates at fishway exit to accommodate reservoir fluctuations. | Minor |
| Scenario C - Lowered Scott | Lowered dam and fish ladder on | Upstream passage through | Mechanical gates at fishway | Minor |

| Scenario | Proposed Work | Fish Passage | Instream Flows | Ecosystem |
|---|---|---|---|---|
| Dam with Fish Ladder | southern dam abutment. | fish ladder. Downstream passage over lowered spillway. | exit to accommodate reservoir fluctuations. | |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Rapid removal of Scott Dam and associated structures. Construction of a new earthen dam in the Salmon Creek watershed. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flow in Eel River during fish migratory period | Significant short term sediment release from reservoir;<br>Long term sediment impacts;<br>Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. |
| Scenario E – Dam Removal with Off-Channel Pond Restoration | Rapid removal of Scott Dam and associated structures with additional sediment management, off-channel pond excavation and revegetation in the reservoir footprint. | Unimpeded upstream and downstream fish migration through restored Eel River. Additional fish and aquatic habitat provided in off-channel ponds. | Improved. Increased flow in Eel River during fish migratory period | Significant short term sediment release from reservoir;<br>Long term sediment impacts partly mitigated by sediment management and revegetation;<br>Release of non-native species from Lake Pillsbury. |

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# Appendix D – Climate Vulnerability

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

August 22, 2025

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# Appended Exhibits

Exhibit D-1 Primary and Secondary Climate Stressors

# Acronyms and Abbreviations

| | |
|---|---|
| APG | Adaptation Planning Guide |
| ARPA | American Rescue Plan Act |
| GHG | Greenhouse Gas |
| ERPA | Eel-Russian Project Authority |
| ICARP | Integrated Climate Adaptation and Resiliency Program |
| IPCC | Intergovernmental Panel on Climate Change |
| NERF | New Eel-Russian Facility |
| NIDIS | National Integrated Drought Information System |
| NOAA | National Oceanic and Atmospheric Administration’s |
| PG&E | Pacific Gas and Electric Company’s |
| RCPs | Representative Concentration Pathways |
| SPI | Standardized Precipitation Index |

# D1.0 Introduction

Pacific Gas and Electric (PG&E) proposes to decommission the Potter Valley Hydroelectric Project (PVP; Federal Energy Regulatory Commission [FERC] Proceeding No. 77). On July 25, 2025, PG&E submitted a Final Application for Surrender of License and Application for Non-Project Use of Project Lands to the FERC. PG&E's overall project goal is the decommissioning of the PVP as the hydropower generation is no longer economically viable. Scott Dam, which forms Lake Pillsbury in the Eel River Watershed, is one of two dams of the PVP.

The proposed PVP surrender, removal of Scott Dam and the subsequent draining of Lake Pillsbury present complex challenges for climate resilience in Lake County, California. While dam decommissioning is intended to support ecological restoration and fish passage, it also raises concerns about water supply reliability, wildfire risk, and community vulnerability under future climate conditions.

SLR International Corporation (SLR) conducted an evaluation of PG&E Final Application for Surrender of License to assess its alignment with the Lake County Climate Vulnerability Analysis and conducted a gap analysis to provide PG&E with suggestions on how to improve their climate vulnerability assessment (PG&E, 2025; County of Lake, 2025b). Additionally, in response to PG&E's proposed Scott Dam removal and as part of a broader technical evaluation, SLR developed seven (7) mitigation scenarios to explore alternative approaches to dam removal and restoration that reflect a broader range of operational, ecological, and economic outcomes for the benefit of the county and the local stakeholders.

The climate vulnerability analysis presented in this appendix evaluates both PG&E's Proposed Action and the developed alternatives for how well they respond to the climate stressors identified in the Lake County Climate Vulnerability Analysis (County of Lake, 2025b). These stressors include both primary drivers such as temperature and precipitation changes, and secondary impacts such as drought, wildfire, flooding, and public health hazards.

SLR's analysis uses the Historical Operational Conditions (WY 1922–2019) as the baseline scenario, representing long-term operations of Scott Dam prior to recent seismic restrictions. This baseline provides a reference point for evaluating how climate stressors have historically impacted Lake Pillsbury and the surrounding region.

The seven mitigation scenarios are:

1. Existing Operational Conditions Scenario covers current operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 follows PG&E's proposal per the PG&E Surrender Application dated January 2025, for the complete removal of Scott Dam.
3. Scenario A2 follows PG&E's proposed removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggestion for additional mitigation (including active sediment management).

4. Scenario B assuming Scott Dam to remain as operated currently (i.e. Spillway gates open at elevation 1,900 ft [PG&E datum[1]] and operation of the needle valve to provide water supply downstream) and retrofitted with a fish passage structure.
5. Scenario C assumes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device, and the inclusion of a fish ladder structure.
6. Scenario D assumes Scott Dam is removed and a new earthen dam is placed to restrict flows from only the Salmon and Habematolel Creek watersheds for a smaller reservoir footprint. The spillway is assumed to be at an elevation of 1,885 ft.
7. Scenario E assumes Scott Dam is removed and one or two small off-channel ponds are built along the Eel River mainstem allowing for water storage and natural fish passage.

SLR's evaluation summarizes how Scott Dam and Lake Pillsbury have historically buffered climate vulnerabilities and assesses how each mitigation scenario is projected to respond to future climate stressors, using the historic baseline as a reference. These findings are intended to support Lake County's decision-making and guide collaborative planning efforts to strengthen long-term climate resilience in the region.

## D1.1 Background on Scott Dam and Lake Pillsbury

Lake Pillsbury is a man-made reservoir located in Lake County, California, within the Mendocino National Forest. It was created in the early 1920s with the construction of Scott Dam, as part of the Potter Valley Hydroelectric Project. This project was designed to divert water from the Eel River to generate electricity in Potter Valley and support agricultural use in the Russian River watershed.

Over time, the lake and dam also became important for recreation and regional water supply. However, due to safety concerns and changing priorities, there are ongoing efforts to transition the system away from hydroelectric power generation and toward locally managed water uses that reflect evolving environmental, community, and regulatory goals. These efforts are occurring alongside increasing climate-related challenges, such as more variable precipitation, reduced snowpack, and heightened wildfire risk, which add complexity to long-term water and infrastructure planning.

In 2023, PG&E published an Initial Draft Surrender Application and Conceptual Decommissioning Plan for the PVP, which includes the planned decommissioning of Scott Dam. The proposed decommissioning is part of a broader initiative to restore the Eel River's ecosystem and improve fish passage (California Trout, 2023). However, this project raises concerns for vulnerable populations in the area and the impact it may have on future climate resiliency. While environmental and tribal organizations largely support the removal as a step toward ecological restoration and salmon recovery, local governments and residents have voiced concerns about the potential loss of water storage, increased wildfire risk, and reduced water reliability for agriculture and rural communities (County of Lake, 2025b; LaFever, 2025). These discussions unfolded in the context of intensifying climate stressors—such as prolonged

[1] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1,821.2 feet (NAVD88 datum).

drought, reduced snowpack, and more frequent wildfires—which heighten the need for coordinated adaptive water management strategies.

## D1.2 Appendix Organization

This Appendix is organized into several sections and organized to follow the approach taken to analyze the impacts of decommissioning Scott Dam on the County of Lake regarding climate vulnerability.

**D1.0 Introduction:** This section provides an overview of the appendix's purpose and scope and a brief background on the project.

**D2.0 Climate Vulnerability:** This section provides an overview of the climate adaptation resources and planning frameworks available in California. These resources were also used to assess PG&E's Proposed Action.

**D3.0 Historical Climatology:** This section outlines the historical changes in average surface temperature from 1922 to 2019 and important historical events and policies that occurred during this timeframe.

**D4.0 Lake County Climate Vulnerability Analysis:** This section summarizes the County of Lake's Climate Vulnerability Analysis released in April 2025 and how the climate stressors will be affected by the removal of Lake Pillsbury.

**D5.0 Cumulative Effects Identified by PG&E:** This section summarizes Volume II, Section 2.2.5 "Cumulative Effects" of the Final Application for Surrender of License by PG&E and evaluates the completeness of this analysis.

**D6.0 Baseline Scenario - Historical Operational Conditions from 1922-2019:** This section summarizes current conditions and the baseline scenario used to compare the mitigation scenarios.

**D7.0 Mitigation Scenarios – Climate Vulnerability Comparison:** This section compares each proposed mitigation scenario against the baseline with respect to climate vulnerability and adaptation capacity.

**D8.0 Conclusions:** This section summarizes the key findings and insights derived from the analysis.

**D9.0 References:** This section provides a list of sources cited in the report.

# D2.0 Climate Vulnerabiity

The following section provides an overview of the climate adaptation resources and planning frameworks available in California. These tools are used by local governments, including Lake County, to assess climate vulnerability and inform long-term resilience strategies. The summaries below offer context for the climate stressors and planning approaches referenced throughout this document. A comparison of these resources to PG&E's Proposed Action is provided in Section D5.5.

## D2.1 Climate Vulnerability Over

Climate change is defined by the Intergovernmental Panel on Climate Change (IPPC, 2023) as "a change in the state of the climate that can be identified (e.g., by using statistical tests) by changes in the mean and/or the variability of its properties and that persists for an extended period, typically decades or longer." Climate change may result from natural processes (e.g. volcanic activity) or anthropogenic influence such as greenhouse gas (GHG) emissions and land use changes (IPPC, 2023).

Recent observations show that climate change has led to increased frequency and severity of natural disasters, global warming, sea level rise, and loss of polar ice. Human activities have raised global surface temperatures by approximately 2 degrees Fahrenheit (1.1 degrees Celsius) above pre-industrial or 1850-1900 levels (IPCC, 2023).

In response, communities across California have adopted climate adaptation planning to identify vulnerabilities and prepare for future conditions. These efforts include climate action plans, GHG reduction strategies, hazard mitigation planning, and integrated water resource management, etc. (California Governor's Office of Emergency Services [Cal OES], 2020).

According to the U.S. Climate Resilience Toolkit, climate vulnerability is defined as the predisposition of assets to be adversely affected by hazards, which encompasses exposure, sensitivity, potential impacts and adaptive capacity (U.S. Climate Resilience Toolkit, n.d.).

California provides a robust suite of tools and programs to support climate-resilient decision making. These resources include Cal-Adapt, the Integrated Climate Adaptation and Resiliency Program (ICARP), and the California Adaptation Planning Guide (APG) (Cal-Adapt, 2025; ICARP, 2025; Cal OES, 2020).

## D2.2 California Adaptation Planning Guide (APG)

The California APG is a resource developed by the Governor's Office of Emergency Services (Cal OES) to help local governments to incorporate best practices and current science into their adaptation efforts. The APG outlines a four-phase planning process:

- **Phase 1 - Define, Explore, and Initiate**: Scoping the project, identifying climate effects, assets, and stakeholders.
- **Phase 2: Assess Vulnerability**: Evaluating exposure, sensitivity, and adaptive capacity of populations, infrastructure, and ecosystems. Where:
    - **Exposure Reduction**: The extent to which a scenario reduces the likelihood or severity of climate-related hazards (e.g., flooding, drought, extreme heat, wildfire).
    - **Sensitivity Reduction**: The degree to which a scenario reduces the vulnerability of people, ecosystems, and infrastructure to those hazards.

    - **Adaptive Capacity**: The ability of a system to adjust to climate impacts, respond to emergencies, and recover from disruptions through operational flexibility, infrastructure, or ecological resilience.
- **Phase 3: Define Adaptation Framework and Strategies**: Developing strategies to address identified vulnerabilities.
- **Phase 4: Implement, Monitor, Evaluate, and Adjust**: Executing the plan and refining it over time through feedback and learning.

The APG emphasizes inclusive community engagement throughout all phases and encourages cross sector collaboration to build resilience (Cal OES, 2020).

## D2.3 Integrated Climate Adaptation and Resiliency Program (ICARP)

ICARP is a California statewide initiative housed within the Governor's Office of Planning and Research. Its mission is to advance equitable, integrated climate adaptation and resilience solutions across local, regional, and state efforts. ICARP provides:

- **Policy and Programmatic Alignment**: Coordinating adaptation strategies across all levels of government.
- **Climate Services**: Providing technical assistance, decision-support tools (like Cal-Adapt and the Adaptation Clearinghouse), and guidance for local and regional adaptation planning.
- **Actionable Climate Science**: Supporting climate assessments and research to inform policy.
- **Direct Investments**: Administering climate resilience grants and aligning funding with federal programs

ICARP's Vulnerable Communities Platform helps visualize climate vulnerability across California and supports equity-focused planning (ICARP, 2025).

## D2.4 Cal-Adapt Climate Modeling

Cal-Adapt is California's primary platform for accessing localized climate projections. Developed by the California Energy Commission and UC Berkeley's Geospatial Innovation Facility, Cal-Adapt provides downscaled data that helps communities understand how climate change may affect their region. This tool is especially valuable for Lake County as it evaluates the long-term implications of PG&E's proposed decommissioning of Scott Dam and the resulting changes to Lake Pillsbury (Cal-Adapt, 2025).

Cal-Adapt uses Representative Concentration Pathways (RCPs) developed by the IPCC to model future climate conditions:

- RCP 4.5: A moderate-emissions scenario where global emissions peak around 2040.
- RCP 8.5: A high-emissions scenario with continued growth throughout the century.

Projections are provided for two future periods:

- Mid-century (2035–2064)
- End-of-century (2070–2099)

Table 1 summarizes projected changes in Lake County's annual average temperature and precipitation compared to the historical climate baseline (1961-1990) (Cal-Adapt, 2025).

**Table 1: Projected Long-Term (30-year) Annual Average Changes from the Historical Baseline**

| Climate Variable | Mid Century RCP 4.5 (2035–2064) | Mid Century RCP 8.5 (2035–2064) | End of Century RCP 4.5 (2070–2099) | End of Century RCP 8.5 (2070–2099) |
|---|---|---|---|---|
| Max Temperature (Fahrenheit) | +3.8° | +4.7° | +5.0° | +8.0° |
| Min Temperature (Fahrenheit) | +3.3° | +4.3° | +4.4° | +7.5° |
| Precipitation (inch/day) | +0.005 | +0.007 | +0.009 | +0.015 |

These projections suggest that Lake County will experience significantly higher temperatures, more extreme heat days, and greater variability in precipitation. Warmer winters are expected to reduce snowpack and shift runoff timing—factors that directly affect water availability and storage.

# D3.0 Historical Climatology

The National Oceanic and Atmospheric Administration's (NOAA) Global Average Surface Temperature Map was used to contextualize climate variations from 1922-2019 at Scott Dam. NOAA used the 1901-2000 global average temperature as their ±0°F benchmark (NOAA, n.d.). The County of Lake's Climate Vulnerability Study and Cal-Adapt both use 1961-1990 as their historical benchmark for average temperature (County of Lake, 2025b; Cal-Adapt, 2025).

Between the years 1922 and 1961, the global change in average surface temperature went from -0.41°F in 1922 to +0.14°F in 1961, indicating a slight warming trend and the beginnings of climate change (NOAA, 2025). At this time, the greenhouse effect was known, but evidence for a global warming trend had not yet been proven or widely studied. At this time, California suffered from droughts, floods, heat waves, and fires (Library of Congress, 1999), but not as frequently or intensely as seen today.

From 1961 to 1990, the global change in average surface temperature went from +0.14°F in 1961 to +0.81°F in 1990, showing a consistent warming trend. By this time, climate change was well-known and studied, and the scientific community agreed on climate change's existence and future impacts on both global and local scales. More severe weather events took place during this time compared to previous years, including the most severe drought-of-record (at the time) from 1976-1977, and several wildfires. California started to introduce new policies intended to prevent and mitigate fire damage, but experienced difficulties implementing these policies and managing burns successfully (Laney, 2017). California's growing population and urbanization during this time required better flood and drought management, which is still a problem in California today (Barbour, 2002).

From 1990 to 2019, the global change in average surface temperature went from +0.81°F to +1.78°F, showing a consistent and increasing warming trend. The severity of weather events and temperature extremes continued to grow at an accelerating rate. California experienced its two most devastating droughts in living memory, from 2012-2016 and 2020-2022, which were brought on by a persistent lack of rainfall and warmer temperatures. California also experienced dozens of severe wildfires, larger, more dangerous and harder to extinguish than previous fires. The rapid succession in which these events took place shows the present-day impacts of climate change. In response to these events, California passed ten significant climate change laws between 2002 and 2018, and additional regulations (UC Berkeley Law, n.d.). Lake County experienced some of the most devastating fires in California State history, which burned through 70% of Lake County (County of Lake, 2023, see also Appendix B). The 2012-2016 and 2020-2022 droughts, 2006 and 2017 floods also affected Lake County, stressing the water supplies, forests, habitats, and communities (County of Lake, 2025b).

# D4.0 Lake County Climate Vulnerability Analysis

Lake County has experienced a series of severe climate-related disasters over the past century, including wildfires that have burned through approximately 70% of the county, prolonged droughts that have stressed water supplies, and storms that have caused flooding, power outages, and community isolation. These events have resulted in loss of life, destruction of homes, reduced tourism, and economic hardship (County of Lake, 2025b).

In response to these impacts, and in alignment with California's climate planning mandates, the County initiated its Climate Vulnerability Analysis in 2023. This effort was part of a broader update to its General Plan and Safety Element and was supported by the American Rescue Plan Act (ARPA) funding and an Adaptation Planning Grant (County of Lake, June 2023).

The analysis was developed in alignment with the APG (see Section D2.3). It incorporates data from Cal-Adapt and other state supported climate tools to assess future climate scenarios and hazards exposure. The County's approach also aligns with principles from the ICARP, particularly in its emphasis on equity, cross sector planning, and resilience building across natural built systems.

In the Lake County Climate Vulnerability Analysis, the County defines a climate hazard as "an event or physical condition that has the potential to cause fatalities, injuries, physical damage, or other types of harm or loss." The vulnerability assessment examined a wide range of systems and populations

The most significant climate hazards identified in the analysis include:

- Wildfires, which pose the greatest threat to health, safety, and infrastructure
- Severe weather, including atmospheric rivers and winter storms that disrupt transportation and utilities
- Flooding, which is increasing in frequency and severity due to changing precipitation patterns
- Drought, which is intensifying because of rising temperatures and reduced snowpack

The most vulnerable assets include:

- Energy and communication infrastructure
- Residential structures
- Transportation systems
- Agricultural operations

To support the assessment of drought and flood conditions, the County utilized data from the National Integrated Drought Information System (NIDIS), which applies hydrologic models and historical climate records to classify wet and dry periods. A key tool used in this analysis is the Standardized Precipitation Index (SPI), which quantifies precipitation anomalies by expressing them in terms of standard deviations from the historical mean. Negative SPI values indicate below-average precipitation (drought), while positive values suggest above-average precipitation (wet conditions). SPI classifications range from D0 (abnormally dry) to D4 (exceptional drought), and W0 (abnormally wet) to W4 (exceptional wetness). This framework helped the County identify and contextualize historic and projected climate stressors, particularly those related to hydrologic extremes. Further SPI data and visualizations are provided in Appendix A.

## D4.1 Climate Stressors

To better understand the underlying drivers of these vulnerabilities, Lake County's Climate Vulnerability Analysis (2025b) identified a range of climate stressors that interact with local vulnerabilities and compound risks to both natural and human systems. These stressors are categorized as primary[2] and secondary[3]. Lake Pillsbury plays a critical role in buffering many of these stressors due to its water storage, seasonal flow regulation, and support for fire suppression, agriculture, and public health. More detailed information on these climate stressors and their impact on Lake County is provided in Exhibit E-1 (appended).

### D4.1.1 Primary Climate Stressors

Primary climate stressors are the direct physical changes in climate that drive cascading environmental and social impacts. In Lake County, the most significant primary stressors include rising temperatures and shifting precipitation patterns (County of Lake, 2025b). These changes influence water availability, wildfire risk, ecosystem health, and infrastructure resilience. Understanding how these stressors are projected to evolve is essential for evaluating the vulnerability of Lake Pillsbury and surrounding communities, especially in the context of dam removal and long-term climate adaptation planning.

#### D4.1.1.1 Changes in Temperature

Lake County is projected to experience significant increases in annual average temperatures through the end of the century. According to Cal-Adapt, as referenced in Table 1, maximum temperatures may rise to 8˚F under high emissions scenarios, showing a consistent upward trend in countywide annual average temperatures through the end of the century. These increases are expected to:

- Intensify extreme heat events
- Exacerbate drought conditions and wildfire risk
- Place additional stress on ecosystems, agriculture, and public health
- Contribute to greater variability in seasonal and year-to-year climate patterns

Lake Pilsbury acts as a thermal buffer, moderating local temperature extremes. Its presence provides cooling relief for people and wildlife during heat waves. Removal of the lake may increase exposure to extreme temperatures in the surrounding area.

#### D4.1.1.2 Changes in Precipitation Patterns

According to projections from Cal-Adapt, referenced in Table 1, Lake County is expected to experience fewer rainy days but more intense storm events, with greater year-to-year variability in precipitation totals. While annual precipitation may increase, it is expected to be concentrated in shorter, more intense periods. These changes heighten the risk of:

- Flooding and erosion

[2] Primary climate stressors are direct effects of the rising global temperatures due to climate change (County of Lake, 2025b)

[3] Secondary climate stressors are "events or physical conditions that can cause harm to people, property, and natural systems." (County of Lake, 2025b)

- Landslides and debris flow
- Water supply instability, especially during dry years
- Stress on stormwater infrastructure and natural systems

Lake Pillsbury provides flood storage and seasonal flow regulation. Without it, downstream areas may face increased flood risk during high-intensity storms and reduced water availability during dry periods.

### D4.1.2 Secondary Climate Stressors

Secondary climate stressors are the indirect consequences of primary climate changes, such as rising temperatures and altered precipitation patterns. These stressors emerge through interactions with ecological, social, and economic systems, and often compound existing vulnerabilities. In Lake County, secondary stressors include drought, flooding, wildfire, extreme temperatures, public health hazards, and disruptions to agriculture and infrastructure (County of Lake, 2025b). Lake Pillsbury plays a critical role in buffering many of these stressors. Understanding how these stressors manifest and intensify under future climate scenarios is essential for evaluating the long-term resilience of the region and the implications of dam removal.

#### D4.1.2.1 Agriculture, Forestry Pests and Disease

Tree mortality increased by approximately 333% from 2019 and 2024, driven by warmer temperatures and favorable conditions for pests and diseases. This trend is expected to worsen as climate change extends growing seasons and reduces winter die-off. Historically, pest-related damage surged during the 2012–2016 drought, affecting both crops and forests (County of Lake, 2025b).

Environmental consequences include declining agricultural productivity, forest dieback, and habitat loss, which in turn increase wildfire fuel loads and reduce ecosystem resilience. Vulnerable populations, particularly farmers, rural communities, and low-income households—may face income loss, food insecurity, and mental health stress.

Lake Pillsbury supports irrigation and agricultural resilience. Its removal could hinder pest management and disease control efforts, compounding risks for both natural systems and human livelihoods.

#### D4.1.2.2 Drought

Lake County has experienced multiple droughts, notably in 1928-1934, 1976-1977, 2012-2016 and 2020-2022. The 1976-1977 drought was previously considered the most severe, but recent droughts have been intensified by persistent warming and reduced snowpack. In 2021, the U.S. Drought Monitor National Integrated Drought Information System (NIDIS) classification ranged between extreme and exceptional in Lake County (NIDIS, 2025). See section A.3.2.2 (in Appendix A) for more information on the classification system and how droughts have been classified in the past.

Future droughts are expected to be more frequent, longer in duration, and harder to recover from. These conditions threaten water supply reliability, agricultural productivity, and habitat integrity. Vulnerable populations such as tribal communities, low-income households, and those dependent on agriculture face heightened risks.

Lake Pillsbury has historically acted as a drought buffer. Its removal would eliminate this capacity and increase vulnerability across multiple sectors.

#### D4.1.2.3 Extreme Temperatures

Heat events have become more frequent and intense in Lake County. Notable spikes occurred in June 2019 (record-breaking temperatures), July 2021 (100°F+ temperatures), September 2022 (prolonged heat wave), July 2023 (100°F+ temperatures), and July 2024, which was the hottest month ever recorded in California. Projections show extreme heat days increasing from 4 days to 12 by 2030, 20 days by midcentury, and 42 days by late century. Warm nights are expected to increase from 5 to 77 days annually by late century (County of Lake, 2025b).

These conditions stress ecosystems, reduce agricultural yields, and increase energy demand. Elderly individuals, outdoor workers, and those with chronic illnesses are at higher risk of heat-related illness.

Lake Pillsbury acts as a thermal buffer and provides cooling relief for people and wildlife. Without the lake, surrounding areas may experience more intense heat stress and reduced climate resilience.

#### D4.1.2.4 Flooding

Flooding has caused significant damage in Lake County. Floods on record occurred in 1937, 1982-1983, 1995, 1998, 2006, 2014, and 2017. The 1998 flood resulted in more than 60 homes being inundated and caused approximately $5 million in damages. The 2006 flood led to a federal disaster declaration, the 2014 flood forced a state of emergency, and the 2017 flood event forced evacuations and nearly $7 million in road damages. Climate change is expected to increase precipitation intensity, leading to more frequent and severe flooding (County of Lake, 2025b).

In addition to these impacts, floodplain boundaries are projected to expand in extent, and flood flows are expected to occur with greater frequency. These changes will increase the risk to infrastructure, homes, and natural systems, particularly in low-lying and downstream areas.

Flooding can damage infrastructure, erode landscapes, contaminate water supplies, and disrupt transportation. Residents in flood prone areas, especially those with limited mobility or financial resources, may face displacement and reduced access to services.

Lake Pillsbury provides flood storage and seasonal flow regulation. Its removal would heighten downstream flood risks during high-intensity storms and rapid snowmelt.

#### D4.1.2.5 Human Health Hazards

Lake County has faced public health threats from both infectious diseases and environmental hazards. Notable examples include the emergence of the West Nile virus (identified in 2004) and the COVID-19 pandemic, which resulted in 13,860 cases and 192 deaths from COVID-19 as of February 2024. Harmful algal blooms, especially in Clear Lake, are becoming more frequent due to warming and nutrient loading (County of Lake Health Services, n.d.).

Climate change is expected to increase the risk of vector-borne diseases such as West Nile virus and Lyme disease, as well as waterborne illnesses. These hazards strain emergency services and disproportionally affect children, the elderly, and immunocompromised individuals.

Lake Pillsbury supports water supply for health emergencies. Its loss could reduce water quality and increase disease risk, especially during droughts or contamination events.

#### D4.1.2.6 Landslides and Debris Flow

Landslides occurred in Lake County between 1993-1999 and again in 2017. These events were often triggered by intense rainfall following wildfires (County of Lake, 2025b). Future landslides are expected to become more frequent due to overlapping stressors such as drought followed by heavy precipitation.

Landslides can damage infrastructure and property, disrupt transportation, and degrade water quality. Residents in steep terrain or near burn scars may face displacement, injury, and isolation during emergencies.

Lake Pillsbury helps stabilize upstream sediment. Dam removal may increase erosion and slope instability, threatening both natural systems and human safety.

#### D4.1.2.7 Severe Weather

Lake County has experienced damaging winter storms and atmospheric river events. These include the 1986 winter storms, 1998 El Nino floods, the 2005-06 winter storms which caused $5 million in damage, the 2017 winter storms which created two separate federal disaster declarations, the 2019 atmospheric river events, and 2022-23 winter storms (County of Lake, 2025b). More extreme precipitation events are expected in the future with greater year-to-year variability.

Severe weather can cause flash flooding, power outages, and transportation disruptions. Rural and isolated communities may lack access to emergency services and recovery resources.

Lake Pillsbury buffers runoff and mitigates stormwater impacts. Without it, severe weather may lead to greater infrastructure strain and reduced community resilience.

#### D4.1.2.8 Wildfire and Smoke

Since 2015, Lake County has endured ten significant wildfires. These include the Rocky, Jerusalem, and Valley Fires in 2015, the Clayton Fire in 2016, the Sulphur Fire in 2017, the Pawnee and Mendocino Complex Fire in 2018, the August Complex Fire in 2020, the Cache Fire in 2021, and Boyles Fire in 2024. Projections show an estimated 35% increase in acres burned by midcentury and 50% increase in acres burned by late century (County of Lake, 2025b).

Wildfires destroy habitat, degrade air and water quality, and increase erosion. Smoke exposure poses serious health risks, especially for individuals with respiratory conditions, children, and low-income households.

Lake Pillsbury provides water for aerial firefighting and supports vegetation moisture retention. Its removal would reduce fire suppression capacity and increase smoke-related health risks.

# D5.0 Cumulative Effects Identified by PG&E

In Volume 2, Section 2.2.5 “Cumulative Effects” of the Final Application for Surrender of License, PG&E evaluates the cumulative effects of the proposed Scott Dam decommissioning and restoration plans. PG&E defines cumulative effect as “the effect on the environment that results from the incremental effect of the action when added to other past, present, and reasonably foreseeable future actions.” (PG&E, 2025)

## D5.1 Water Supply

The cumulative effect of the removal of Scott Dam and climate change would reduce the reliability of the water supply. PG&E has stated that unavoidable adverse effects on water temperatures and flow regimes in the Eel River and East Branch Russian River are expected, particularly during spring and summer, due to the loss of cold-water storage. This warming will contribute to vegetation loss in high-burned areas and increasing sediment loading in waterways. In conjunction with more frequent and sever wildfires, could result in the loss of habitat for sensitive species and cultural and Tribal resources.

The proposed decommissioning will increase fish passage to the upstream Eel River of Lake Pillsbury, potentially enhancing fish population resiliency and restoring access to over 100 miles of historic habitat. PG&E and the Eel-Russian Project Authority (ERPA)[4] also propose the construction of the New Eel-Russian Facility (NERF) to maintain diversion flows to the Russian River watershed following dam removal. However, there are not enough studies to support that the NERF will be a viable long-term substitute for Scott Dam’s storage and flow regulation functions. Historically, Scott Dam was constructed because the Cape Horn Dam and associated diversion infrastructure failed to reliably divert sufficient water to the Russian River, particularly during drought conditions.

Moreover, PG&E’s hydrologic and hydraulic analysis of unimpaired flows was developed using impaired flow data and did not incorporate precipitation data. This methodological limitation calls into question the validity of their assumptions regarding future water supply reliability. Without incorporating precipitation variability and climate projections, the analysis cannot accurately reflect the full range of potential outcomes—water supply could be better or worse than estimated, depending on future climate conditions.

PG&E acknowledges that the combination of climate change and the decommissioning of the Potter Valley Project would have adverse effects on the water supply and hydrology in the East Branch Russian River, which would be partially mitigated by the construction of the NERF. However, PG&E does not evaluate the potential adverse effects on the water supply, hydrology, or climate resilience for Lake County, despite Lake Pillsbury’s critical role in regional water storage and drought mitigation. This omission represents a significant gap in the cumulative effects analysis.

Additionally, PG&E does not present quantitative modeling of short- or long-term water supply impacts for Lake County, nor does it evaluate the potential loss of Lake Pillsbury as a drought buffer or its role in regional water security. No analysis is provided to estimate future water availability, seasonal variability, or groundwater recharge impacts under climate change scenarios.

[4] ERPA is a joint powers authority formed by a joint exercise of powers agreement between the County of Sonoma, Sonoma County Water Agency (Sonoma Water) and the Mendocino County Inland Water and Power Commission.

## D5.2 Storm Frequency and Intensity

PG&E acknowledges that climate change is expected to increase the frequency and intensity of storms. These changes could increase the magnitude of peak flows for flood events while decreasing their duration. In conjunction with the proposed removal of Scott Dam, PG&E predicts the restoration of unimpaired hydrology could lead to increased sediment mobilization, channel bar and bank deposits, erosion, and changes in riparian vegetation. PG&E concludes that these are natural geomorphic processes and that the cumulative effects of climate change and decommissioning are expected to be neutral in terms of geomorphic function.

However, PG&E does not assess how future high flows may affect the longitudinal profile of the Eel River and its tributaries, sediment supply and capacity, or the resilience of existing infrastructure. Nor does it evaluate the time required for the river to reach a new sediment transport balance, which could take decades to resolve.

While the processes described—such as sediment redistribution and channel adjustment—are indeed natural in a general geomorphic sense, PG&E does not demonstrate whether these processes are consistent with the historical geomorphic conditions of the Eel River prior to the construction of Scott Dam. The assumption of geomorphic neutrality lacks supporting evidence, as PG&E does not provide quantitative sediment modeling, historical channel morphology data, or reference conditions to validate this claim.

To ensure that restoration efforts support long-term ecological and geomorphic resilience, PG&E should evaluate the pre-dam geomorphic state of the Eel River and use that baseline to guide restoration targets. Without this context, the conclusion that geomorphic effects will be neutral is overly generalized and unsupported and may overlook potential long-term instability or unintended consequences in sediment dynamics, channel form, and floodplain function.

## D5.3 Droughts

PG&E acknowledges that climate change will exacerbate droughts, reduce water levels, and adversely affect aquatic habitat quality and recreation opportunities. As a cumulative effect, this will negatively impact recreation value, species and habitat integrity, and local economies. PG&E also notes that socioeconomic impacts may include effects on population stability, housing, recreation-based income, and economic opportunity, which could be further worsened by increased wildfire risk.

While PG&E states that with the dam removal will result in an increase in watershed resiliency, this term is not clearly defined. It is unclear whether PG&E is referring to hydrologic resilience to drought, ecological recovery, or some other system function. Given that Lake Pillsbury currently provides critical water storage capacity, it is not evident how its removal would improve drought resilience without alternative water management strategies. No quantitative analysis or modeling is provided to support this claim or to assess whether such resiliency would offset the project’s adverse effects.

## D5.4 Carbon Emissions

Using conservative estimates, implementation of the proposed Potter Valley Project decommissioning would emit a total of approximately 10,564.52 metric tons of carbon dioxide (PG&E, 2025). However, PG&E does not provide a comparison to the baseline emissions under existing conditions, nor does it evaluate the long-term carbon sequestration potential of restored ecosystems.

## D5.5 Comparison to Lake County's Climate Vulnerability Analysis

PG&E's assessment of climate vulnerability, as presented in its Final Application for Surrender of License, is notably less comprehensive than the Lake County Climate Vulnerability Analysis (2023). While PG&E identifies several climate-related stressors, its evaluation lacks the structure, depth, and geographic specificity necessary to support informed decision-making for Lake County.

Lake County's analysis follows the California APG, which outlines a four-step framework: exposure, sensitivity, adaptive capacity, and vulnerability. The County integrates Cal-Adapt climate modeling, identifies vulnerable populations and critical infrastructure, and evaluates cumulative and compounding risks across environmental, economic, and public health domains.

In contrast, PG&E's assessment:

- Does not follow a standardized vulnerability assessment framework.
- Omits key geographic areas, including Lake County, from its hydrologic and socioeconomic analysis.
- Fails to incorporate Cal-Adapt or other state-supported climate modeling tools.
- Provides limited evaluation of social vulnerability and equity impacts.
- Focuses primarily on fish passage and geomorphic restoration, without assessing broader resilience strategies.

To strengthen its climate vulnerability assessment, PG&E should:

1. Adopt a structured framework such as the APG to evaluate exposure, sensitivity, and adaptive capacity.
2. Use Cal-Adapt and other state-supported tools to model future climate conditions and stressors.
3. Include Lake County in its hydrologic and socioeconomic impact analysis, given Lake Pillsbury's role in regional water storage and drought mitigation.
4. Evaluate impacts on vulnerable populations, including Tribal communities, low-income residents, and those dependent on agriculture or recreation.
5. Quantify and compare greenhouse gas emissions under existing and proposed conditions, including long-term carbon sequestration potential.
6. Assess recovery timelines and resilience for geomorphic and ecological systems.
7. Explore alternative water storage or conservation strategies to offset the loss of reservoir capacity.
8. Engage with local stakeholders and Tribal communities to co-develop adaptation and resilience strategies.

Without these additions, PG&E's climate vulnerability assessment remains incomplete and insufficient to address the full scope of risks associated with dam removal and climate change in Lake County.

# D6.0 Baseline Scenario: Historical Operational Conditions from 1922-2019

From 1922 to 2019 the Scott Dam operations evolved in response to changing climate conditions and regulatory requirements. During this period, County of Lake experienced increasingly variable temperature and precipitation patterns, which influenced water availability, runoff timing, and seasonal extremes. The presence of Scott Dam and Lake Pillsbury provided a measure of control over these fluctuations, helping to stabilize water supply and mitigate hydrologic volatility (Cal-Adapt, 2025; County of Lake, 2023). This section contains additional information about how the past climate conditions affected Lake Pillsbury and Scott Dam, adding on to the past and present climate stressors identified by the Lake County Vulnerability Analysis (2023).

Operational practices supported regional resilience by regulating seasonal flows, maintaining minimum downstream releases for agriculture, and preserving water availability for fire suppression and recreation. These functions became more critical as climate variability intensified and as environmental regulations began to emphasize water reliability and public safety. Historically, these operations contributed to climate resilience by buffering drought conditions, mitigating flood risks, and supporting fire suppression efforts.

However, while Scott Dam buffered climate stressors by regulating water supply and mitigating hydrologic extremes, it also introduced long-term vulnerabilities through ecological tradeoffs. Sedimentation within the reservoir gradually reduced its storage capacity, weakening its ability to absorb flood waters and retain drought reserves. The absence of fish passage disrupted upstream aquatic ecosystems, limiting biodiversity and reducing the resilience of native species to climate-induced stressors such as warming temperatures and altered flow regimes. At the same time, cold-water releases from the dam have historically supported downstream aquatic habitats (Inland Water and Power Commission, 2025), particularly during warmer months when elevated stream temperatures can threaten fish populations. This dual role—providing thermal refuge downstream while constraining upstream connectivity—illustrates the complex interplay between engineered infrastructure and ecological resilience[5]. These factors must be considered together when evaluating the dam's contribution to long-term climate adaptation and vulnerability reduction.

## D6.1 Scott Dam's Response to Historical Climate Stressors

Scott Dam's operational history from 1922 to 2019 reflects evolving responses to climate stressors, particularly drought and flood conditions. While early operations prioritized hydroelectric generation, later decades saw a shift toward more climate-responsive management, informed by seasonal hydrology and community needs (PG&E, 2025).

A long-term series of outflow and storage data from 1922 to 2022 (Figure A7-2, Appendix A) illustrates how Scott Dam buffered hydrological extremes. The graph shows seasonal fluctuations in reservoir storage and outflow, with notable operational adjustments in 1957, 1986, and 2005. These changes likely reflect evolving regulatory requirements, infrastructure upgrades, and updated bathymetric surveys of Lake Pillsbury's storage potential.

[5] SLR did not identify specific studies for the Eel River, though these effects have been documented for the South Fork McKenzie River upstream and downstream of Cougar Reservoir in Oregon. https://www.usgs.gov/water-science-school/science/temperature-and-water

The figure reveals several key patterns in climate stressor response regarding flooding, drought, and season adaptation. These operational patterns align closely with the primary and secondary climate stressors identified in Lake County's Climate Vulnerability Analysis, including increased drought frequency, flood risk, and temperature extremes. Scott Dam's ability to buffer these stressors has historically reduced exposure and sensitivity while enhancing adaptive capacity—key components of the County's resilience framework. As upstream runoff became less predictable due to climate change, the need for real-time monitoring and flexible operations grew. Scott Dam's historical performance demonstrates how infrastructure can evolve to meet changing climate demands, even when initial design priorities were not climate focused. Its role in buffering both drought and flood extremes underscore its importance in regional climate resilience planning. For additional context and data, see NIDIS drought and flood classifications and SPI trends in Appendix A (Figure A3-6).

### D6.1.1 Flood Response

During wet years, such as the early 1980s and mid-1990s, the dam absorbed large inflows and regulated outflows to prevent downstream flooding. For example, in January 1983, peak outflows reached 15,400 cfs yet did not exceed the projected 2-year flood level, indicating effective attenuation. The dam's ability to store excess runoff and release it gradually helped protect infrastructure and agricultural lands downstream (see more details in Appendix A).

### D6.1.2 Drought Response

Extended periods of low storage, particularly during the 1976–1977, 2012–2016, and 2020–2022 droughts, show how the reservoir retained water to support regional needs. Despite reduced inflows, Scott Dam operations maintained minimum releases for agriculture, fire suppression, and ecological stability. These droughts were brought on by a lack of precipitation and hot temperatures. NIDIS classified them between severe and exceptional (NIDIS, 2025). They were marked by prolonged precipitation deficits and elevated temperatures. See Appendix A, Section A3.2.2 for more information on the classification system and how droughts have been classified in the past. The dam's capacity to buffer these conditions was critical for sustaining water availability (NIDIS, 2025).

### D6.1.3 Seasonal Adaptation

Figure A7-2 (in Appendix A) shows consistent seasonal cycling, with storage peaking in spring and early summer due to snowmelt and precipitation, followed by drawdown in late summer and fall. This pattern reflects adaptive management aligned with hydrologic conditions and community demand.

## D6.2 Scott Dam Observed Vulnerabilities to Climate Stressors

In addition to its buffering capacity, Scott Dam exhibited vulnerabilities to several climate stressors during its operational history, specifically wildfire and smoke, extreme temperatures, human health hazards, landslide and debris flow, and agriculture, pests, and disease (County of Lake, 2023).

### D6.2.1 Wildfire and Smoke

Lake Pillsbury has historically supported aerial firefighting operations, particularly during major wildfire events such as the Mendocino Complex Fire (2018) and August Complex Fire (2020). The reservoir reliably supported suppression efforts throughout its operational history, even

during drought years. In addition to fire suppression, wildfire events produced significant smoke exposure across Lake County, impacting public health and air quality. The reservoir's role in supporting emergency response indirectly contributed to mitigating smoke-related health risks by enabling faster containment of fires.

### D6.2.2 Extreme Temperatures

Lake Pillsbury acts as a thermal buffer, moderating local temperature extremes. During heat waves in July 2021 and July 2024 (the hottest month ever recorded in California), the lake provided cooling relief for residents and wildlife. Throughout its operational history, Scott Dam maintained adequate water levels to support regional needs, including fire suppression and recreation. While evaporation is a natural process in open reservoirs, there is no documented evidence that it significantly reduced available storage or stressed water supply systems.

### D6.2.3 Landslides and Debris Flow

Scott Dam is located adjacent to the Bartlett Springs Fault and is partially built on a known landslide feature, making it geologically vulnerable to seismic activity and slope instability. Historically, the dam helped reduce downstream debris flow risk by regulating peak flows and trapping sediment. However, its interruption of sediment transport contributed to sediment starvation in the Eel River downstream of the dam, which may have led to channel incision and bank erosion. These geomorphic changes can increase susceptibility to slope instability and landslides in steep terrain. According to the California Geological Survey's Landslide Susceptibility Map (Map Sheet 58), the area between Scott Dam and Van Arsdale Reservoir is classified as Class IX and X for deep-seated landslide susceptibility. Class IX indicates moderate to very steep slopes in weak rocks with high likelihood of landslides, while Class X represents mapped active landslides or very steep slopes in weak rocks—the highest level of susceptibility. These classifications highlight the elevated risk of deep-seated landslides in the region, particularly in areas affected by sediment starvation and channel degradation.

### D6.2.4 Human Health Hazards

During drought years, Scott Dam played a key role in maintaining water availability for sanitation, drinking water, and emergency services. Its ability to retain water during extended dry periods helped mitigate risks associated with waterborne diseases and contamination. Additionally, stagnant water in shallow reservoir margins may have contributed to mosquito habitat, increasing the risk of vector-borne diseases such as West Nile virus.

### D6.2.5 Agriculture, Pests, and Disease

Scott Dam provided reliable water supply for agricultural irrigation and forest health, especially during drought years. This support helped reduce pest outbreaks and tree mortality by maintaining vegetation vigor and resilience. During the 2012–2016 drought, pest-related damage increased across Lake County, but areas supported by irrigation from Lake Pillsbury were better buffered. The dam's role in sustaining water availability contributed to regional resistance against climate-driven pest pressures.

# D7.0 Mitigation Scenarios – Climate Vulnerability Comparison

This section compares each proposed mitigation scenario against the baseline (historic conditions) with respect to climate vulnerability and adaptation capacity. The analysis is based on the Lake County Climate Vulnerability Analysis (County of Lake, 2025b) and compares each scenario's ability to respond to both primary stressors, such as changes in temperature and precipitation, and secondary stressors, including drought, wildfire, flooding, and public health hazards. The goal of this comparison is to help clarify the differences in resiliency under future climate conditions. Each scenario is evaluated in terms of its ability to reduce exposure, sensitivity, and enhance adaptive capacity relative to historic operations at Scott Dam as defined in the California APG and in Section D2.2 of this appendix (Cal OES, 2020). For consistency, each scenario is described using the terms 'maintained,' 'reduced,' or 'increased' to indicate how its performance compares to the baseline scenario across the three dimensions with appropriate modifiers such as *slightly*, *moderately*, or *significantly* to describe the degree of change from the baseline condition, summarized in Table 2.

**Table 2: Definition of Terms Used for Comparison of Each Scenario to the Baseline/Historical Condition**

| Term | Exposure Reduction | Sensitivity Reduction | Adaptive Capacity | Overall Resilience |
|---|---|---|---|---|
| Eliminated | No ability to buffer climate stressors; system is fully exposed. | Vulnerability is maximized across systems; no protective functions remain. | All adaptive functions are lost; no operational flexibility or resilience mechanisms. | Climate resilience is eliminated. System cannot respond to or recover from climate stressors. |
| Slightly Lessened / Increased | Minor change in exposure to one or two stressors. | Minor change in vulnerability for select systems. | Some functions are retained, but reliability or flexibility is slightly reduced. | Climate resilience is slightly reduced or improved. System retains most functions but is more vulnerable to stressors. |
| Moderately Lessened / Increased | Noticeable change across multiple stressors. | Vulnerability changes across several systems. | Reduced ability to respond to climate stressors; flexibility or infrastructure is constrained. | Climate resilience is moderately reduced or improved. System can still adapt but with limitations. |
| Significantly Lessened / Increased | Major change in exposure; system is highly exposed. | Vulnerability is substantially different across most systems. | Adaptive functions are largely lost or enhanced; system lacks or gains resilience tools. | Climate resilience is significantly reduced or improved. System is either highly vulnerable or highly robust. |

## D7.1 Current Operational Conditions Scenario (WY 2019-Present)

Following seismic safety concerns in 2019, Scott Dam operations were modified to lower the reservoir elevation by 10 feet, reducing overall storage capacity. Despite this reduction, the dam continues to provide critical water supply, seasonal flow regulation, and emergency support functions (see more information in Appendix A). This scenario reflects the most recent operational configuration and serves as a transitional state between historical operations and future mitigation alternatives.

### D7.1.1 Exposure Reduction

Compared to the baseline, current operations project a **slightly lessened ability to reduce exposure** to climate stressors, though with diminished capacity:

- **Flooding**: The dam continues to regulate inflows, but reduced storage and permanently open spillway gates limit flood attenuation, increasing downstream exposure during high-intensity storms.
- **Drought**: Exposure to drought has increased due to lowered reservoir elevation and reduced reserve capacity, which limits the ability to buffer extended dry periods.
- **Extreme temperatures**: Lake Pillsbury still acts as a thermal buffer, offering cooling relief during heat waves.
- **Landslides and debris flow**: Without sediment management landslides and debris flows are at higher risk after the dam is removed. Risk increases further with extreme events without buffering of the dam.
- **Wildfire**: Water availability is slightly lowered but still provides firefighting support under most conditions.

### D7.1.2 Sensitivity Reduction

Current operations project a **slightly increased overall sensitivity or vulnerability** to climate stressors when compared to the baseline across multiple systems:

- **Fire suppression**: Water remains available for aerial firefighting, but reduced storage volume may limit long-term reliability during extended wildfire seasons.
- **Public health**: Water supply supports sanitation and emergency services, but diminished reserve capacity may reduce reliability during prolonged droughts or emergencies.
- **Agriculture**: Irrigation support continues, though with less flexibility and reliability during dry years due to lower reservoir levels.
- **Flood risk**: Sensitivity is increased due to reduced flood storage and passive discharge, which heighten vulnerability of downstream infrastructure.
- **Evaporation**: While reduced surface area may lessen total evaporation, shallower water may warm more quickly, potentially increasing localized evaporation rates during prolonged heat events.
- **Landslides and Debris Flow**: Landslide risk remains due to known geologic hazards adjacent to the dam and local wetting and drying of steep slopes.

### D7.1.3 Adaptive Capacity

This scenario projects a **moderately lessened** adaptive capacity with some key functions retained but long-term resilience constrained when compared to the baseline:

- **Operational flexibility**: The dam continues to support seasonal flow adjustments through the needle valve, but reduced storage limits the range and duration of response options.
- **Multi-use support**: The system still provides water for agriculture, recreation, fire suppression, and public health, though with less reliability due to reduced reservoir volume and reserve capacity.
- **Monitoring and maintenance**: Ongoing oversight enables adaptive management, but aging infrastructure and seismic constraints limit future adaptability and investment potential.

### D7.1.4 Comparison to Baseline

Relative to the historical baseline (1922–2019), current operations project a **slightly less resilient configuration** across climate vulnerability metrics:

- **Exposure reduction** is **lessened** for flooding and drought due to reduced reservoir elevation and passive spillway discharge, though buffering for extreme temperatures is maintained.
- **Sensitivity reduction** is **slightly lessened,** with increased vulnerability across multiple systems including fire suppression, public health, and agriculture due to reduced reliability.
- **Adaptive capacity** is **moderately lessened,** as operational flexibility is retained but long-term resilience is constrained by aging infrastructure and reduced storage volume.

This scenario reflects a transitional state between historical operations and future alternatives. While it continues to provide essential climate buffering functions, its overall resilience is diminished due to structural and operational limitations.

## D7.2 Scenario A1 – PG&E Proposed Removal

Scenario A1 reflects PG&E's proposed full removal of Scott Dam, as outlined in its July 2025 Application for Surrender of License. This action would eliminate Lake Pillsbury as a managed reservoir, restoring unimpaired flow to the Eel River and enhancing fish passage. While ecologically beneficial in some respects, the removal introduces significant climate vulnerability for Lake County due to the loss of water storage, flow regulation, and emergency response capacity.

### D7.2.1 Exposure Reduction

Scenario A1 exhibits a **significantly lessened exposure buffering**[6] compared to the baseline scenario (1922–2019), due to the complete removal of Lake Pillsbury and Scott Dam.

[6] Exposure buffering refers to the ability of a system, infrastructure, or natural feature to reduce the intensity, frequency, or impact of climate-related stressors—such as drought, flooding, wildfire, or extreme temperatures—on surrounding communities, ecosystems, and assets.

- **Flooding**: The absence of regulated outflows from the reservoir increases downstream flood exposure, particularly during high-intensity precipitation events.
- **Drought**: Without stored water reserves, the system loses its seasonal buffering capacity, resulting in heightened exposure to prolonged dry periods.
- **Extreme Temperatures**: The removal of the lake eliminates a thermal buffer, increasing exposure to heat stress in the surrounding environment.
- **Wildfire**: Water availability for aerial firefighting is eliminated, removing a critical emergency response resource and increasing exposure to wildfire impacts.
- **Landslides and Debris Flow**: The loss of sediment control and peak flow attenuation increases exposure to slope instability and debris movement, especially in steep or recently burned terrain.

### D7.2.2 Sensitivity Reduction

Scenario A1 projects **significantly increased sensitivity** when compared to the baseline:

- **Ecological benefits**: Restores access to over 100 miles of upstream habitat, improving resilience for native fish populations.
- **Public health and safety**: Increased sensitivity due to loss of water for sanitation, emergency services, and cooling during heat waves.
- **Infrastructure**: Greater sensitivity to flood damage, erosion, and sediment mobilization without managed flow control.
- **Wildfire**: Loss of aerial firefighting water source increases wildfire sensitivity.

### D7.2.3 Adaptive Capacity

This scenario **eliminates adaptive capacity** when compared to the baseline:

- **No operational control**: Eliminates ability to manage seasonal flows, respond to droughts, or mitigate flood risks.
- **No storage buffer**: Removes capacity to retain water for agriculture, health emergencies, or fire suppression.
- **Restoration uncertainty**: While ecological recovery is anticipated, PG&E does not provide a clear framework for long-term climate adaptation or resilience-building in Lake County.

### D7.2.4 Comparison to Baseline

Compared to historical operations (1922–2019), Scenario A1:

- **Exposure reduction** is **significantly lessened** across all major stressors.
- **Sensitivity reduction** is **significantly increased**, especially for human systems.
- **Adaptive capacity** is **eliminated**, with no retained operational flexibility or emergency support.

This scenario represents a **significantly lessened overall resilience** compared to the baseline and the lowest resilience among all alternatives, particularly for Lake County's vulnerable

populations and critical infrastructure. While it supports ecological restoration goals, it requires substantial mitigation to offset the loss of climate buffering functions historically provided by Scott Dam.

## D7.3 Scenario A2 – Scenario A1 with Additional Mitigation

Scenario A2 builds on PG&E's proposed full removal of Scott Dam (Scenario A1) by incorporating additional mitigation measures suggested by Lake County. These measures are not guaranteed but could be recommended to reduce the climate vulnerabilities introduced by dam removal. Potential actions include managing impounded sediment, stream and floodplain restoration, infrastructure upgrades, and wildfire risk reduction. While these strategies may improve ecological and geomorphic resilience, the loss of Lake Pillsbury's water storage and operational control continues to pose significant challenges for climate adaptation.

### D7.3.1 Exposure Reduction

Compared to the baseline (1922–2019), Scenario A2 results in **significantly lessened exposure reduction**:

- **Flooding**: Floodplain restoration may absorb runoff but lacks the scale and control of the historical reservoir.
- **Drought:** The absence of stored water reserves increases exposure to prolonged dry periods.
- **Extreme Temperatures:** Loss of the lake removes a thermal buffer, increasing exposure to heat stress.
- **Landslides and debris flow**: Sediment management and restoration efforts will reduce risk of landslides and debris flow resulting from the dam removal. However, without the buffering and flow regulation provided by Lake Pillsbury, exposure to landslide and debris flow hazards is increased during extreme events.
- **Wildfire**: Water availability for aerial firefighting is eliminated, removing a critical emergency response resource and increasing exposure to wildfire impacts. Fuel reduction strategies (e.g., prescribed burns, thinning) may help lower exposure to high-intensity fire events, but do not replace the reservoir's support for suppression operations.

### D7.3.2 Sensitivity Reduction

Scenario A2 exhibits a **moderately lessened sensitivity reduction** compared to the baseline scenario (1922–2019), with ecological improvements offset by increased vulnerability in human systems:

- **Ecosystems:** Restored riparian zones and sediment management may improve habitat stability and connectivity, resulting in slightly lessened sensitivity relative to the baseline.
- **Infrastructure**: Upgrades to river crossings and infrastructure will reduce vulnerability to erosion, sediment transport, and extreme flooding.
- **Public Health & Agriculture**: Sensitivity is moderately increased compared to the baseline due to reduced water reliability for sanitation, irrigation, and emergency services.

- **Landslides and Debris Flow**: Sediment management and restoration efforts will help reduce slope instability and post-fire erosion, which would lessen sensitivity compared to the baseline. However, without upstream flow regulation, vulnerability remains during extreme events

### D7.3.3 Adaptive Capacity

Scenario A2 exhibits a **moderately lessened adaptive capacity** compared to the baseline scenario (1922–2019). While the removal of Scott Dam eliminates operational control and water storage functions, the proposed mitigation measures introduce some compensatory mechanisms that support long-term resilience.

- **Operational Control**: No reservoir or flow regulation mechanisms are retained, limiting flexibility in managing droughts, floods, and wildfire emergencies.
- **Emergency Response**: Water availability for aerial firefighting is eliminated, reducing emergency response capacity. However, wildfire risk reduction strategies—such as prescribed burns and vegetation thinning—may improve preparedness and reduce the need for reactive suppression.
- **Infrastructure Resilience**: Upgrades to culverts, bridges, and roads enhance the system's ability to withstand variable flows and sediment transport, contributing to adaptive capacity in transportation and access.
- **Ecological Stability**: Sediment management and floodplain restoration support geomorphic stability and habitat connectivity, which may improve long-term ecological resilience under changing climate conditions.

These mitigation measures help offset some of the lost adaptive functions and provide a foundation for future climate adaptation planning.

### D7.3.4 Comparison to Baseline

Scenario A2 exhibits a **moderately lessened climate resilience** compared to the baseline scenario (1922–2019).

- **Exposure buffering** is **significantly lessened** due to the elimination of Lake Pillsbury's water storage and flow regulation, which previously mitigated flooding, drought, wildfire, and landslide risks.
- **Sensitivity** is **moderately lessened overall**, with **slight improvements** in ecological systems offset by **increased vulnerability** in infrastructure, public health, and landslide-prone areas.
- **Adaptive capacity** is **moderately lessened**, as operational flexibility is lost, but mitigation measures—such as **sediment management**, **infrastructure upgrades**, and **wildfire risk reduction**—provide partial support for long-term resilience.

## D7.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

Scenario B assumes Scott Dam remains in place and continues operating under current conditions (spillway gates open at elevation 1,900 ft and needle valve releases downstream), with the addition of a fish ladder to improve upstream aquatic connectivity. This scenario preserves Lake Pillsbury's water storage and flow regulation functions while addressing

ecological concerns related to fish passage. Compared to the baseline scenario (1922–2019), Scenario B slightly lessens most climate resilience functions.

### D7.4.1 Exposure Reduction

This scenario maintains the **slightly lessened exposure buffering** compared to baseline:

- **Flooding**: Modified flow schedules may enhance downstream flood attenuation, but slightly reduced reservoir capacity limits peak flow absorption during high-intensity storms.
- **Drought**: Seasonal water storage is retained, but reduced volume slightly limits drought buffering during prolonged dry periods.
- **Extreme Temperatures**: The lake continues to provide thermal relief, maintaining exposure buffering for heat stress.
- **Wildfire**: Water remains available for aerial firefighting, though reduced volume may constrain long-duration suppression efforts.
- **Landslides and Debris Flow**: Continued sediment retention and flow regulation help reduce exposure to slope instability and debris movement.

### D7.4.2 Sensitivity Reduction

Scenario B exhibits **slightly lessened sensitivity** compared to the baseline:

- **Ecosystems**: Improved flow and temperature regimes may enhance habitat stability and reduce ecological sensitivity.
- **Infrastructure**: Continued flow regulation and sediment retention reduce stress on downstream infrastructure.
- **Public Health & Agriculture**: Water reliability is generally maintained, though slightly reduced reserve capacity may affect reliability during extended droughts.
- **Landslides and Debris Flow**: Sensitivity remains low due to sediment stabilization and peak flow attenuation.

### D7.4.3 Adaptive Capacity

Scenario B exhibits a **slightly lessened adaptive capacity** compared to the baseline:

- **Operational Control**: Flow regulation is retained, but reduced reservoir volume limits flexibility during extreme events.
- **Emergency Response**: Water storage supports firefighting and drought response, though with slightly reduced reliability.
- **Infrastructure Resilience**: Continued investment in dam safety and sediment management supports long-term adaptability.
- **Ecological Stability**: Operational modifications and fish passage improvements enhance ecological resilience.

### D7.4.4 Comparison to Baseline

Scenario B maintains most climate resilience functions but exhibits **slightly lessened performance** compared to the baseline scenario (1922–2019):

- **Exposure buffering** is slightly lessened due to reduced reservoir volume.
- **Sensitivity** is slightly lessened, particularly for ecosystems and infrastructure.
- **Adaptive capacity** is slightly lessened due to constrained operational flexibility during extreme events.

## D7.5 Scenario C – Lowered Scott Dam with Fish Ladder

Scenario C assumes Scott Dam remains in place but is modified to a reduced height. The spillway elevation would be lowered from 1,900 ft to 1,875 ft (25 feet), and a fish ladder would be added to improve upstream aquatic connectivity. The needle valve would be utilized or modified to achieve low-level flow control and manage releases. This scenario reduces the size and storage capacity of Lake Pillsbury while improving fish passage. It introduces new vulnerabilities due to reduced water availability and flood buffering but retains some climate resilience functions.

### D7.5.1 Exposure Reduction

Exposure reduction is **moderately lessened** compared to the baseline (1922–2019). While the lowered dam retains some flood and drought buffering capacity, the reduced reservoir volume limits its effectiveness during extreme events.

- **Flooding**: Moderately lessened due to reduced peak flow absorption.
- **Drought**: Moderately lessened; seasonal storage is retained but insufficient for prolonged dry periods.
- **Extreme Temperatures**: Slightly lessened; thermal buffering continues but with reduced surface area.
- **Wildfire**: Slightly lessened; water remains available but may be insufficient for extended suppression efforts.
- **Landslides and Debris Flow**: Moderately lessened. Reduced reservoir volume may increase peak flow variability; elevating slope instability risks downstream and within the impoundment.
- **Evaporation**: Slightly lessened. Smaller surface area may reduce total evaporation, but shallower water may warm faster, increasing localized evaporation.
- **Sedimentation**: Moderately lessened. Continued sediment accumulation may reduce reservoir capacity and increase downstream sediment transport during high flows.

### D7.5.2 Sensitivity Reduction

Sensitivity reduction is **moderately lessened** compared to the baseline. While ecological sensitivity improves due to fish passage, human systems face increased vulnerability.

- **Aquatic ecosystems**: Improved due to fish ladder.

- **Public health and agriculture**: Slightly increased sensitivity due to reduced water reliability.
- **Infrastructure**: Moderately increased sensitivity to flood damage and sediment transport.
- **Landslides and Debris Flow:** Moderately increased sensitivity due to reduced reservoir volume and increased peak flow variability, which may elevate downstream slope instability and debris flow potential, even though sediment remains impounded.
- **Evaporation**: Slightly increased sensitivity to water loss during heat waves.
- **Sedimentation**: Moderately increased sensitivity as sediment buildup may reduce operational flexibility and affect water quality within the impoundment.

### D7.5.3 Adaptive Capacity

Adaptive capacity is **moderately lessened**. Operational flexibility is retained through flow control devices, but reduced reservoir volume limits long-term resilience.

- **Operational flexibility**: Flow control devices allow for seasonal and emergency water releases.
- **Multi-use support**: The reservoir continues to serve agriculture, recreation, fire suppression, and public health needs.
- **Ecological adaptation**: Fish passage improves species resilience to climate stressors.
- **Sediment Management**: No new sediment control measures are proposed, limiting long-term adaptability and consistency in capacity.

### D7.5.4 Comparison to Baseline

Compared to historical operations (1922–2019), Scenario C:

- **Exposure reduction** is **moderately lessened** due to reduced reservoir size and buffering capacity.
- **Sensitivity reduction** is **moderately lessened**, with ecological improvements offset by increased vulnerability in human systems.
- **Adaptive capacity** is **moderately lessened**, retaining some flexibility but constrained by reduced volume and lack of sediment management.

Scenario C represents a **moderately lessened** climate resilience scenario. It improves fish habitat while retaining some water management functions but introduces new vulnerabilities that would require careful monitoring and mitigation.

## D7.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

Scenario D assumes Scott Dam is removed and replaced with a smaller earthen dam that restricts flows from only the Salmon and Habematolel Creek watersheds. The reservoir footprint would be significantly reduced, and the spillway elevation would be set at 1,885 ft. Flows from the Eel River and Rice Fork would no longer be stored and would return to natural, unimpaired conditions. This scenario includes sediment management and control efforts within the

impoundment but limited flow control. Compared to historical operations, this scenario reduces water storage availability by approximately 80% and introduces new vulnerabilities related to drought, flood control, and water reliability, while improving fish passage and ecological connectivity.

### D7.6.1 Exposure Reduction

Compared to historical operations, this scenario **significantly lessened** exposure reduction benefits:

- **Flooding**: The smaller reservoir has limited capacity to buffer high-intensity storm events or rapid snowmelt, increasing downstream flood risk.
- **Drought**: Water storage is reduced by approximately 80%, limiting the ability to provide water during dry periods.
- **Extreme temperatures**: The smaller reservoir offers less thermal buffering than Lake Pillsbury.
- **Wildfire**: Reduced water availability may limit aerial firefighting capacity and increase wildfire vulnerability.
- **Landslides and Debris Flow**: Significantly lessened. Minimal sediment retention and increased peak flows elevate exposure to slope instability and debris movement.
- **Evaporation**: Slightly lessened. Smaller surface area may reduce total evaporation, but vulnerability remains during heat waves.
- **Sedimentation**: Significantly lessened. Loss of upstream sediment control increases downstream sediment loads and erosion risk.

### D7.6.2 Sensitivity Reduction

Sensitivity reduction is **significantly lessened**. While ecological sensitivity improves, human systems face increased vulnerability.

- **Aquatic ecosystems**: Restoring unimpaired flows from the Eel River and Rice Fork improves fish passage and habitat connectivity.
- **Agriculture**: Unimpaired flows may support downstream irrigation, but reliability is lower during droughts.
- **Public health**: Less water is available for sanitation and emergency services, especially during dry years.
- **Infrastructure**: Reduced flood buffering increases sensitivity to damage from high flows.
- **Landslides and Debris Flow**: Significantly increased sensitivity due to lack of sediment buffering and flow regulation.
- **Groundwater**: Lower reservoir volume and surface area may reduce groundwater recharge, potentially affecting the regional water table.
- **Evaporation**: Smaller reservoirs are less vulnerable to evaporation but still require monitoring during heat waves.
- **Sedimentation**: Increased sensitivity for infrastructure and aquatic habitat due to uncontrolled sediment transport.

### D7.6.3 Adaptive Capacity

Adaptive capacity is **significantly lessened.** The scenario lacks operational flexibility and sediment management infrastructure.

- **Operational flexibility:** The proposed configuration includes minimal flow control, limiting responsiveness during extreme events.
- **Water supply:** The reduced reservoir size limits drought buffering and emergency response capacity.
- **Ecological adaptation:** Improved. Restored sediment transport and natural flow regimes support habitat complexity, spawning conditions, and long-term resilience for native fish species.
- **Infrastructure:** No specific upgrades are proposed to offset increased flood or sediment risks.
- **Sediment Management**: No sediment control infrastructure is proposed, reducing geomorphic resilience.

### D7.6.4 Comparison to Baseline

Relative to the 1922–2019 baseline:

- **Exposure reduction** is **significantly lessened** due to reduced reservoir size and loss of flow regulation.
- **Sensitivity reduction** is **significantly lessened**, with ecological gains offset by increased vulnerability in human systems.
- **Adaptive capacity** is **significantly lessened**, with minimal operational control and no sediment management.

Scenario D represents a **significantly lessened climate resilience scenario**, prioritizing ecological restoration but introducing major vulnerabilities for water reliability, flood control, and emergency response.

## D7.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E assumes full removal of Scott Dam and the creation of one or more small off-channel ponds along the Eel River mainstem. These ponds are designed to provide limited water storage and support natural fish passage, with specific functionality for aerial firefighting via helicopter access. The scenario also includes sediment management and restoration efforts to reestablish natural geomorphic processes. While this approach supports long-term ecological restoration and wetland habitat creation, it eliminates Lake Pillsbury's reservoir and flow regulation functions, significantly reducing climate resilience for human systems.

### D7.7.1 Exposure Reduction

Exposure reduction is **significantly lessened** due to the loss of large-scale water storage and flow control when compared to the baseline condition.

- **Flooding**: Significantly lessened. Without Lake Pillsbury, floodwaters are no longer stored. Off-channel ponds offer minimal buffering, though restored floodplains may absorb some runoff.

- **Drought**: Significantly lessened. The ponds have very limited storage capacity and cannot provide meaningful drought resilience.
- **Extreme Temperatures**: Slightly lessened. Ponds may offer localized cooling for wildlife but lack the scale to buffer regional heat stress.
- **Wildfire**: Moderately lessened. While overall water availability is reduced, the ponds are specifically designed to support aerial firefighting via helicopters, providing some emergency response capacity.
- **Landslides and Debris Flow**: Significantly lessened. No sediment retention or flow regulation increases exposure to slope instability and debris movement.
- **Evaporation**: Slightly lessened. Ponds may reduce total evaporation, but shallow water remains vulnerable to heat stress.
- **Sedimentation**: Moderately lessened. Sediment management is proposed to restore geomorphic function to a quasi-equilibrium state, which can improve sediment mobilization and reduce long-term instability across the reach. While this enhances ecological resilience, effectiveness may be limited during extreme flow events or debris flows.

### D7.7.2 Sensitivity Reduction

Relative to the baseline, sensitivity reduction is **moderately lessened.** Ecological systems benefit from restoration, but human systems remain vulnerable.

- **Habitat health**: Improved. Wetlands are resilient to flooding and support biodiversity. Sediment restoration helps stabilize the Eel River and its tributaries.
- **Public health**: Increased sensitivity. Water quality may improve through natural filtration, but supply is unreliable during emergencies. Standing water may increase mosquito habitat and algal growth.
- **Agriculture**: Increased sensitivity. Without a large reservoir, irrigation support is minimal and sensitive to seasonal variability.
- **Infrastructure**: Increased sensitivity. Reduced flood buffering and sediment control heighten vulnerability to damage.
- **Landslides and Debris Flow**: Significantly increased sensitivity due to loss of upstream sediment control and flow regulation.
- **Evaporation**: Slightly increased sensitivity to water loss and algal blooms.
- **Sedimentation**: Moderately lessened sensitivity. If restoration efforts are robust and sustained, sediment transport processes may stabilize, supporting habitat complexity and reducing vulnerability to channel degradation. However, sensitivity remains elevated during extreme events.

### D7.7.3 Adaptive Capacity

Adaptive capacity is **moderately lessened** when compared to the baseline condition. While operational control is lost, ecological restoration introduces long-term resilience benefits.

- **Water supply**: Significantly reduced. The ponds cannot support regional water needs during droughts or emergencies.

- **Flood control**: Absent. No operational control exists to manage flows or buffer runoff.
- **Ecological adaptation**: Improved. Wetland creation and sediment restoration support habitat resilience and fish passage.
- **Emergency response**: Moderately supported. Ponds provide targeted support for aerial firefighting, but broader emergency water needs remain unmet.
- **Sediment Management**: Proposed restoration aims to reestablish natural sediment dynamics, which can improve ecological resilience and support long-term geomorphic stability. However, the system lacks operational control to manage sediment transport during high-intensity events, limiting adaptive capacity under extreme conditions.

### D7.7.4 Comparison to Baseline

Relative to the 1922–2019 baseline:

- **Exposure reduction** is **significantly lessened** due to the elimination of Lake Pillsbury's buffering functions.
- **Sensitivity reduction** is **moderately lessened**, with ecological gains offset by increased vulnerability in human systems.
- **Adaptive capacity** is **moderately lessened**, with long-term ecological benefits but limited operational flexibility.

Scenario E represents a **significantly lessened climate resilience scenario**. It prioritizes ecological restoration and sediment transport recovery, which may enhance long-term ecosystem stability, but introduces substantial vulnerabilities for water supply, emergency response, and infrastructure protection.

# D8.0 Conclusions

Maintaining a safe and reliable water supply is essential for climate resilience in Lake County. The historical operations of Scott Dam and Lake Pillsbury provided strong protection against drought, flooding, wildfire, and extreme temperatures. As climate stressors intensify, the ability to store and manage water becomes increasingly critical for public health, agriculture, infrastructure, and emergency response.

This climate vulnerability analysis shows that scenarios which retain or enhance water storage and operational control offer the highest resilience. In contrast, scenarios involving full dam removal without replacement introduce significant vulnerabilities unless paired with robust mitigation strategies.

The table below summarizes how each scenario compares to the historical baseline in terms of **exposure reduction, sensitivity reduction, and adaptive capacity**.

**Table 3: Summary Matrix comparing The Historical Baseline Scenario to The Mitigation Scenarios**

| SCENARIO | EXPOSURE REDUCTION | SENSITIVITY REDUCTION | ADAPTIVE CAPACITY | Overall Rating |
|---|---|---|---|---|
| **Current Operations** | **Slightly lessened** – Reduced reservoir elevation limits flood and drought buffering | **Slightly lessened** – Increased vulnerability in fire suppression and agriculture | **Moderately lessened** – Flexibility retained but constrained by aging infrastructure | **Slightly Lessened** |
| **Scenario A1 – Full Dam Removal** | **Significantly lessened** – No water storage; system is fully exposed to climate stressors. | **Significantly increased** – loss of water supply, fire suppression, and infrastructure protection | **Eliminated** – no operational control or emergency water management | **Significantly lessened (lowest rating)** |
| **Scenario A2 – Dam Removal + Suggested Mitigation** | **Significantly lessened** – Loss of reservoir; mitigation improves ecological exposure. | **Moderately lessened** – ecological gains, but human systems remain vulnerable | **Moderately lessened** – suggested actions may help, but no guaranteed water management | **Moderately lessened** |
| **Scenario B – Current Operations + Fish Ladder** | **Slightly lessened** – similar to historical flood, drought, and heat buffering | **Slightly lessened** – adds fish passage but reduction in capacity | **Slightly lessened** – reduced reliability to support water supply needs | **Slightly lessened** |
| **Scenario C – Lowered Dam + Fish Ladder** | **Moderately lessened** – smaller reservoir limits flood and drought buffering | **Moderately lessened** – improves fish habitat, but reduces water reliability | **Moderately lessened** – retains control, but with diminished capacity | **Moderately lessened** |
| **Scenario D – Reduced Reservoir (Salmon Creek)** | **Significantly lessened** – small reservoir offers minimal buffering | **Significantly lessened** - Improved for fish, but increased vulnerability for agriculture and public health | **Significantly lessened** – limited flow control and reduced water supply | **Significantly lessened** |
| **Scenario E – Dam Removal + Off-Channel Ponds** | **Significantly lessened** – ponds offer limited buffering, but restored floodplain may absorb runoff | **Moderately lessened** – wetland habitat and sediment management enhance ecological resilience; human systems remain vulnerable | **Moderately lessened** – ecological restoration and sediment control improve long-term river stability, but water supply and emergency response remain limited | **Moderately lessened** |

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# Exhibit D-1 Primary and Secondary Climate Stressors

## Appendix D – Climate Vulnerability

Impact Analysis and Adaptation Strategy Evaluation Report

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

## Exhibit D-1

**Table E-1 Primary and Secondary Climate Stressors**

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| **Primary Climate Stressors** | | | |
| Changes in Temperature | **Past**: Historical (1961 to 1990) annual average minimum temperature was 37.9°F countywide. The annual average maximum temperature was 67.7°F countywide.<br><br>**Present and Future**: Annual average minimum temperatures are projected to increase to 40.9°F by 2030, 42.3°F by midcentury (2035 to 2064), and 45.9°F by late century (2070 to 2099). Annual average maximum temperatures are projected to increase to 70.8°F by 2030, 72.4°F by midcentury, and 75.8°F by late century. | | |
| Changes in Precipitation Patterns | **Past**: Historical annual average precipitation was approximately 42 inches countywide.<br><br>**Present and Future**: Precipitation is projected to increase to 47 inches by midcentury and 51 inches by late century. Though overall rainfall will increase, it is likely to occur during fewer, intense storm events with longer dry periods between storms. | | |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| **Secondary Climate Stressors (Hazards)** | | | |
| Agriculture and Forestry Pests and Diseases | **Past**: Tree mortality increased from 3,000 dead trees in 2019 to over 1,000,00 by 2024. Sudden oak death was identified in southwestern portions of the county and Mediterranean Oak Borer detected in 2022 in Middletown moving northward into the Mendocino National Forest by 2024<br>**Present and Future**: Warmer temperatures and changing precipitation patterns will create more favorable conditions for agricultural and forestry pests and diseases. | Agricultural Productivity Decline<br> Habitat Loss<br> Power Delivery Disruption<br> Structure & Property Damage | Income Loss<br> Food & Water Insecurity<br> Mental & Behavioral Health |
| Drought | **Past**: Significant droughts in 1976–1977, 2014, and 2020–2021.<br>**Present and Future**: More frequent and severe droughts likely, with longer durations and shorter recovery periods. County reached D3 level (Extreme Drought) in 2020 and fluctuated between D3 and D4 (Exceptional Drought) through 2021. | Water Supply/Quality Reduction<br> Agricultural Productivity Decline<br> Habitat Loss | Food & Water Insecurity<br> Income Loss<br> Mental & Behavioral Health<br> Public Health Risks |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| <br>Extreme Temperatures | **Past**: Multiple heat events included June 2019 (record-breaking temperatures), July 2021 (100°F+ temperatures), September 2022 (prolonged heat wave), July 2023 (temperatures over 100°F) and July 2024, the hottest month ever recorded in California<br><br>**Present and Future**: Extreme heat days projected to increase from 4 days annually to 12 days by 2030, 20 days by midcentury, and 42 days by late century. Warm nights projected to increase from 5 to 77 days annually by late century. | <br>Power Delivery Disruption<br> <br>Agricultural Productivity Decline<br> <br>Structure & Property Damage<br> <br>Emergency Services Strain | <br>Physical Injury<br> <br>Public Health Risks<br> <br>Mental & Behavioral Health |
| <br>Flooding | **Past**:<br>• February 1998 (60+ homes flooded, $5 million in damages)<br>• January 2006 (federal disaster declaration)<br>• December 2014 (state of emergency)<br>• February 2017 (forced evacuations, $5 to 7 million in road damage)<br><br>**Present and Future**: More intense precipitation events likely to cause more frequent and severe flooding, particularly around Clear Lake and along waterways. | <br>Structure & Property Damage<br> <br>Transportation Disruption<br> <br>Water Supply/Quality Reduction<br> <br>Emergency Services Strain | <br>Displacement<br> <br>Income Loss<br> <br>Access to Services Reduction<br> <br>Physical Injury |



Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| <br>Human Health Hazards | **Past**: West Nile virus first detected in 2004; COVID-19 pandemic (2020–present) with 13,860 total cases and 192 deaths in Lake County as of February 2024.<br><br>**Present and Future**: Ongoing concerns with vector-borne diseases and harmful algal blooms in Clear Lake. Warmer temperatures expected to expand ranges and seasons for disease vectors. | <br>Emergency Services Strain<br> <br>Water Supply/Quality Reduction<br> <br>Agricultural Productivity Decline | <br>Physical Injury<br> <br>Public Health Risks<br> <br>Access to Services Reduction |
| <br>Landslides and Debris Flows | **Past**: Spring Valley Landslide (Cache Creek Slide) showed considerable movement between 1993 and 1999. Anderson Springs landslide (~4 acres) and Ettawa Springs landslide (~2 acres) discovered during 2017 winter storms.<br><br>**Present and Future**: More frequent landslides and debris flows likely due to increased wildfire activity followed by intense precipitation events. | <br>Structure & Property Damage<br> <br>Transportation Disruption<br> <br>Power Delivery Disruption<br> <br>Emergency Services Strain | <br>Displacement<br> <br>Physical Injury<br> <br>Access to Services Reduction<br> <br>Income Loss |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| Severe weather | **Past:** Lake County has received 13 federal disaster declarations for severe winter storms since 1950. Notable events include winter storms in 1986, 1998 El Niño floods, 2005 to 2006 winter storms ($5 million in damages), 2017 winter storms (two separate federal disaster declarations), 2019 atmospheric river events, and 2022 to 2023 winter storms.<br><br>**Present and Future:** More extreme precipitation events are expected, with increased frequency and intensity of atmospheric rivers. Though damage directly associated with severe weather has historically been limited, secondary hazards like floods, landslides, and power outages will become more significant. | Structure & Property Damage<br> Power Delivery Disruption<br> Transportation Disruption<br> Emergency Services Strain | Displacement<br> Physical Injury<br> Access to Services Reduction<br> Mental & Behavioral Health |
| Wildfires | **Past**:<br>• 2015 Rocky Fire and Jerusalem Fire (69,438 acres, 43 homes destroyed)<br>• 2015 Valley Fire (76,067 acres, 1,280 homes destroyed, 4 deaths)<br>• 2016 Clayton Fire (3,929 acres, 189 homes destroyed)<br>• 2017 Sulphur Fire (2,200 acres, 138 homes destroyed)<br>• 2018 Pawnee Fire (15,185 acres, 12 homes destroyed)<br>• 2018 Mendocino Complex (459,123 acres, 157 homes destroyed in Lake County)<br>• 2020 August Complex (1,032,648 acres)<br>• 2021 Cache Fire (83 acres, 100+ structures destroyed)<br>• 2024 Boyles Fire (81 acres, 33 structures destroyed) | Structure & Property Damage<br> Power Delivery Disruption<br> Habitat Loss<br> Emergency Services Strain | Displacement<br> Mental & Behavioral Health<br> Physical Injury<br> Income Loss |



Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| | **Present and Future**: Annual average acres burned projected to increase by 35 percent by midcentury and 50 percent by late century. More extreme fire behavior expected. | Transportation Disruption Water Supply/Quality Reduction | Public Health Risks Access to Services Reduction |

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Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

# Appendix E – Permitting Process

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

Prepared by:

**SLR International Corporation**

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SLR Project No.: 102.021249.00001

August 22, 2025

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# Acronyms and Abbreviations

| | |
|---|---|
| CDFG | California Department of Fish and Game |
| CDFW | California Department of Fish and Wildlife |
| CEQA | California Environmental Quality Act |
| CESA | California Endangered Species Act |
| DSOD | California Division of Dam Safety |
| DWR | Department of Water Resources |
| EIR | Environmental Impact Report |
| ESA | Endangered Species Act |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| ITP | Incidental Take Permit |
| MND | Mitigated Negative Declaration |
| ND | Negative Declaration |
| NEPA | National Environmental Policy Act |
| NERF | New Eel-Russian Facility |
| NFS | National Forest System |
| NHD | National Hydrology Dataset |
| NMFS | National Marine Fisheries Service |
| NOAA | National Oceanic and Atmospheric Administration |
| NOD | Notice of Determination |
| NOP | Notice of Preparation |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| RPA | Reasonable and Prudent Alternative |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| USFWS | United States Fish and Wildlife Service |
| USGS | United States Geological Survey |
| USFS | United States Forest Service |
| VAFS | Van Arsdale Fish Station |
| WSE | water surface elevation |
| WY | water year |

# E1.0 Introduction

SLR International Corporation (SLR) was contracted by the County of Lake to conduct an analysis of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam. SLR has prepared this appendix (Appendix E) to explain the regulatory process for the decommissioning of Scott Dam (Proposed Action) and alternative Scenarios developed by SLR that will follow the submittal of the final Surrender Application. PG&E plans to submit the final Surrender Application and Decommissioning Plan to the Federal Energy Regulatory Commission (FERC) by July 29, 2025. Following the filing and distribution of the Surrender Application FERC and PG&E will consult with Resource Agencies and Tribes to develop Plans and Measures.

A large volume of information has been prepared for the Surrender Application and will be further developed in the resource reports. This information will be used to prepare federal, state, and local permit applications. Existing Conditions Report previously prepared by SLR (SLR, 2025) have outlined the National Environmental Policy Act (NEPA) and California Environmental Quality Act (CEQA) processes and the following focuses on the major federal and state environmental permits. FERC will be the lead federal agency for the NEPA process and compliance with the Federal Endangered Species Act. FERC will coordinate with the United States Fish and Wildlife Service (USFWS) for compliance with the Section 404 of the Clean Water Act (wetlands) and with the United States Forest Service (USFS) for a Special Use Permit.

PG&E plans to decommission the Potter Valley Hydroelectric Project (PVP; *Federal Energy Regulatory Commission [FERC] Proceeding No. 77*). PVP facilities are shown in Exhibit 1 (Main Report). PG&E's overall project goal is the decommissioning of the PVP for the reason being that the hydropower generation is no longer economically viable. The PVP consists of the following:

- Scott Dam which forms Lake Pillsbury in the Eel River Watershed;
- Cape Horn Dam, which forms Van Arsdale Reservoir in the Eel River Watershed;
- The Van Arsdale diversion, including tunnels and conveyance systems, which diverts water from the Eel River Watershed to the Russian River Watershed;
- The Potter Valley Power house, no longer operational, in the Russian River Watershed.

Based on our impact evaluation we compared a total of seven scenarios for the Scott Dam/Lake Pillsbury area, including existing conditions and the PG&E's proposal. Seven alternate scenarios were added to address more appropriate mitigation alternatives for the future of the Scott Dam/Lake Pillsbury area. The seven scenarios include:

1. Existing Operational Conditions Scenario covers operational conditions during the WY[1] period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft (PG&E Datum)[2] and operation of the needle valve to provide water supply downstream).

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is

2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Surrender Application dated January 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and construction of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

## E1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with 2,275 acres of surface area and 31 miles of shoreline (Stillwater, 2021, PG&E, 2025). The Lake's main flow contributor is the Eel River's upper watershed[3], located in Lake County, California. Scott Dam, which forms Lake Pillsbury, is a complete barrier to upstream fish migration, preventing access to high value habitat for federally Endangered Species Act (ESA)-listed fish.

Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam primarily provides water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley, which has been offline since July 2021 after PG&E discovered a transformer did not meet current operating standards. The Scott Dam has also historically provided water for irrigation to the Potter Valley Irrigation District (PVID) and to meet the minimum instream flow requirements in the Eel River and East Fork Russian River (Stillwater, 2021) (PG&E, 2025). Lake Pillsbury also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID are diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pilsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the

78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

[3] An area of land drained by a river, river system, or other body of water.

Russian River. Cape Horn Dam has a fish ladder allowing fish species to migrate upstream until encountering Scott Dam.

Scott Dam currently operates with an approximate maximum capacity of 55,948 acre-feet at Lake Pillsbury (PG&E, 2025). The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs). Water can be released from the dam through two structures: a needle valve[4] located at the base of the dam and the spillway[5]. The flow capacity of the needle valve is approximately 400 cubic feet per second (cfs) but can operate at varying flows for each Water Surface Elevation (WSE). The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[6]) controls. The spillway consists of multiple sliding and radial gates that are typically fully open between November 1 and April 1 and closed the remainder of the year. Flow through the gated spillway will only occur when the WSE within Lake Pilsbury exceeds the spillway invert elevation of 1,900 feet. The top of the spillway gates are all at elevation 1,910 feet. Slide gate #13 and one of the five (5) radial gates area SCADA controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained fully open during the whole year.

As part of the PG&E's license surrender, a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E and the Eel-Russian Project Authority (ERPA) as non-project use of project land. On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023). ERPA will be responsible for the construction, maintenance and operation of NERF after the completion of the surrender requirements to be included in a regulatory order to be issues by FERC.

[4] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[5] A structure used to provide controlled release of water downstream of a dam or levee.

[6] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

# E2.0 Permitting Evaluation

The permitting process will consist on a Federal Process, anticipated to be led by FERC, and a California Process, anticipated to be led by the California State Water Resources Control Board.

## E2.1 Federal Process

### E.2.1.1 Endangered Species Act Section 7 Consultation

The Endangered Species Act (ESA) of 1973 directs all federal agencies to ensure that any action they authorize, fund, or carry-out does not jeopardize the continued existence of an endangered or threatened species or designated or proposed critical habitat. The US Fish and Wildlife Service (USFW) and National Marine Fisheries Service (NMFS), collectively referred to as the Services, are the agencies primarily responsible for overseeing the ESA. The Services follow a structured process to authorize projects involving potential impacts to endangered species. This includes:

1. **Informal Consultation**

   Informal consultation is defined within the ESA regulations (50 CFR 402.02) as a "process that includes all discussions, correspondence, etc., between the Service and the Federal agency (FERC) or designated non-Federal representative (PG&E) prior to formal consultation.

   During informal consultation, an agency, in the early stages of project planning, approaches USFWS or NMFS and requests an informal consultation when a listed species may be affected by the action. If the agency, after discussions with the USFWS determines that the proposed action is not likely to affect any listed species in the project area, and if USFWS concurs, the informal consultation is complete.

   If the agency, after discussions with the USFWS determines that the proposed action is not likely to affect any listed species in the project area, and if USFWS determines that the proposed action may affect a listed species, that agency may then prepare a Biological Assessment (BA) to assist in its determination of the project's effect on a species.

2. **Biological Assessment**

   A biological assessment (BA) is a study that evaluates the potential impacts of a proposed project or action on biological resources, particularly threatened or endangered and their habitats. It is a crucial component of the ESA consultation process ensuring that federal agencies consider the impact of their actions on protected species.

3. **Formal Consultation**

   If it is determined that the proposed action may affect any listed species or critical habitat, then the agency must begin formal consultation with USFWS and/or NMFS. Formal consultation may last up to 90 days, after which the agency will prepare a biological opinion.

4. **Conclusion of the Biological Opinion**

   The biological opinion will state whether the federal agency has ensured that its action is not likely to jeopardize the continued existence of a listed species or result in the destruction or adverse modification of critical habitat. A biological opinion may include

reasonable and prudent measures necessary or appropriate to minimize the impact of incidental take on the species.

The USFWS and/or NMFS have 45 days after completion of formal conservation to write the biological opinion.

### E.2.1.2 Section 404 of the Clean Water Act

Section 404 of the Clean Water Act regulates the discharge of dredged or fill materials into "waters of the United States, including wetlands". Dredged and fill material refers to materials like soil, sand, rock, or other materials that are excavated or placed in waters or wetlands. Section 404 is administered by the United States Army Corps of Engineers (the Corps) and the Environmental Protection Agency (EPA). The goal is to protect water quality by controlling activities that can impact aquatic ecosystems.

The following steps outline the application process:

1. **Pre-Application coordination**:

   PG&E will start by consulting with the appropriate USACE District office in California to discuss your project. This would be the USACE Sacramento District office.

2. **Application form and project information**:

   PG&E will need to complete and sign the standard individual permit application form (ENG Form 4345). This includes providing a detailed project description, outlining activities involving dredged or fill material, and submitting site plans and drawings. PG&E will also need to list adjoining property owners and other required authorizations.

3. **Environmental impact analysis and mitigation**:

   This involves preparing a wetland delineation and jurisdictional determination. PG&E must demonstrate efforts to avoid and minimize impacts and propose a compensatory mitigation plan for any unavoidable effects. An alternatives analysis and an environmental questionnaire may also be necessary.

4. **Coordination with other agencies**:

   PG&E must also obtain a Water Quality Certification under Section 401 from the California State Water Resources Control Board or a relevant Regional Water Quality Control Board. Consultation with agencies regarding endangered species and historic properties may also be required if applicable.

5. **Additional requirements**:

   These can include a pre-filing meeting request with the Water Board for Section 401 certification and a public notice and comment period. conducted by the USACE.

### E.2.1.3 USFS Special Use Authorization

The Forest Service requires a special use permit for all uses and occupancies on National Forest System (NFS) lands. Examples of authorized land uses include road rights-of-way. The primary permit application form required for road access on USFS lands is Form FS-7700-40 titled "Application for Permit for Use of Roads, Trails, or Areas Restricted by Regulation or Order". PG&E will need to fill out and submit forms FS-7700-40a Commercial Use Attachment and FS-7700-40b Oversized or Overweight.

Additionally, USFS will be a cooperating agency actively involved with the NEPA process. Under NEPA, a cooperating agency is any Federal, State, Tribal, or local agency designated by the lead agency to participate in the environmental review process because of its jurisdiction by law or special expertise related to a proposed action's environmental impacts. As the primary land owner for which PG&E's recreational facilities are located surrounding the Scott Dam and Pillsbury reservoir, USFS is considered a cooperating agency. As a cooperating agency they must participate early in the process and scoping. USFS may also develop information and prepare environmental analyses at the lead agency's request.

### E.2.1.4 National Historic Preservation Act (Section 106)

Dam removal projects must assess their potential impact on historic properties, potentially including the dam itself and archaeological sites. Section 106 compliance refers to the process outlined in Section 106 of the National Historic Preservation Act (NHPA). This section requires federal agencies to consider the effects of their undertakings on historic properties. The process involves identifying historic properties, assessing potential effects, and consulting with stakeholders to find ways to avoid, minimize, or mitigate any adverse effects.

The following are key aspects of Section 106 compliance:

1. **Historic Properties**:

   These are defined as properties included in or eligible for the National Register of Historic Places.

2. **Undertaking**:

   This refers to any project, activity, or program funded in whole or in part under the jurisdiction of a federal agency.

3. **Consultation**:

   The Section 106 process emphasizes consultation with various parties, including the State Historic Preservation Officer (SHPO), Tribal Historic Preservation Officer (THPO), Indian tribes, and other interested parties.

4. **Four-Step Process**:

   The core of Section 106 compliance involves a four-step process: initiation, identification, assessment, and resolution.

5. **Adverse Effects**:

   If a project is found to have an adverse effect on historic properties, the agency must work with consulting parties to develop measures to avoid, minimize, or mitigate those effects.

6. **Agreement Documents**:

   The agreed-upon measures are often documented in a Memorandum of Agreement (MOA) or Programmatic Agreement which are legally binding.

7. **Advisory Council on Historic Preservation (ACHP)**:

   The ACHP plays a role in providing guidance and commenting on the Section 106 process, particularly in cases of significant or controversial projects.

   In essence, Section 106 compliance ensures that federal agencies consider the impact of their actions on the nation's cultural heritage and work to protect historic properties.

These are defined as properties included in or eligible for the National Register of Historic Places.

## E2.2 California Process

The decommissioning of Scott Dam will involve coordination with multiple federal and California agencies due to its potential impact on water resources, environmental factors, and public safety. The following discusses the major California regulations and permitting requirements for the project. As discussed under the Federal Process, a Clean Water Act Section 401 Water Quality Certification will be required from the California State Water Resources Control Board prior to issuance FERC must license change.

### E.2.2.1 Section 401 Water Quality Certification

The California State Water Resources Control Board acts as the water quality certification agency during FERC licensing. The certification confirms that the proposed dam removal will comply with state water quality standards and may include conditions to protect water quality during and after the removal process. The State Water Board may also require specific actions, such as water quality monitoring during reservoir drawdown and sediment sampling.

PG&E must file a complete application with the Division of Water Rights, including project details, fees, and information demonstrating compliance with the California Environmental Quality Act (CEQA).

A "pre-filing meeting" with the Water Board is often required at least 30 days before submitting the application.

The application includes information to allow the State Water Board to determine that water is available for the proposed project, it wouldn't deprive anyone of higher priority water rights, wouldn't harm public trust resources, is in the public interest, and impacts on water quality and the environment under CEQA have been considered. The regulatory process for obtaining a Water Quality Certification is outlined below:

1. **Application Submittal:**

   The project proponent submits a Water Quality Certification application to the Regional Water Quality Control Board (Water Board).

2. **Environmental Review Determination:**

   The Water Board evaluates the project to determine if it may result in significant environmental impacts under CEQA.

3. **CEQA Document Selection:**

   If the Water Board is the lead agency, it determines whether to prepare an Environmental Impact Report (EIR) or a Negative Declaration (ND)/Mitigated Negative Declaration (MND).

4. **Notice of Preparation (NOP):**

   If an EIR is required, the Water Board issues a Notice of Preparation and distributes it to responsible agencies and interested parties for input.

5. **Draft EIR Development:**

   The Water Board prepares the Draft EIR, incorporating input received during the NOP comment period.

6. **Public Notice and Review:**

   A Notice of Completion is filed with the Office of Planning and Research, and the Draft EIR is made available for public review, typically for a minimum of 30 days.

7. **Final EIR Preparation:**

   The Water Board reviews and responds to public comments, then prepares the Final EIR with necessary revisions.

8. **Project Consideration and Certification:**

   The Water Board certifies the Final EIR, makes CEQA findings, and decides whether to approve the project.

9. **Notice of Determination (NOD):**

   Following project approval, the Water Board files a Notice of Determination with Office of Planning and Research, finalizing the CEQA process.

### E.2.2.2 Division of Safety of Dams

The Division of Safety of Dams (DSOD) in California plays a crucial role in the dam removal process, ensuring public safety throughout the project. The process generally involves site reconnaissance, feasibility studies, community engagement, final engineering, and construction oversight. DSOD's involvement ensures that dam removals are conducted safely and in accordance with regulations.

The DSOD ensures that the following steps are completed:

1. **Site Reconnaissance and Feasibility Study**:

   This step involves a series of planning and evaluation prior to dam removal. For example:

   - Gathering Information - Collecting data on the dam's history, age, legal rights, infrastructure, biological resources, sediment, and community interests.
   - Engineering Analysis - A feasibility study assesses the dam's hydrology, hydraulics, sediment movement, and potential impacts on the surrounding environment.
   - Conceptual Plans - Developing conceptual plans for dam removal, sediment management, and stream restoration.
   - Regulatory Compliance - Permitting requirements are identified and coordinated with relevant agencies.
   - Cost Estimation - Financial feasibility is assessed, including potential funding sources.

2. **Community Engagement**:

   The community must be involved throughout the dam removal process to ensure the affected communities are informed. This can be demonstrated through (**1) Stakeholder meetings**, where community members and stakeholders are engaged to address concerns and **(2) Pre-permitting Meetings**, where a consultation with local, state, and federal agencies is conducted to streamline the permitting process.

3. **Final Engineering and Permitting:**

   The final engineering and permitting phase involves the following:

   - Detailed Plans - Final engineering plans are developed for dam removal, stream restoration, and other related construction activities.
   - Permitting - All necessary permits are obtained from relevant agencies.
   - Addressing Agency Comments - Any concerns or requirements raised by regulatory agencies are addressed.
   - Cost Estimation - A final cost estimate is developed.

4. **Construction Oversight**:

   To ensure successful execution of the project, the contractor employed to complete the project shall be subject to the following:

   - Contractor Selection - A qualified contractor is selected to carry out the removal work.
   - Construction Oversight - The engineer oversees the construction process, ensuring it aligns with approved plans and specifications.
   - Addressing Issues - Any issues that arise during construction are addressed promptly.

5. **DSOD's Role**:

   The role of DSOD for dam removal is as described below:

   - Reviewing and Approving Plans - DSOD reviews and approves plans for dam removals, ensuring they meet safety standards.
   - Independent Analysis - DSOD conducts independent analyses of dam performance and stability.
   - Construction Oversight - DSOD oversees construction to ensure compliance with approved plans.
   - Annual Inspections - DSOD inspects dams annually to ensure they are safe and functioning as intended.
   - Emergency Action Plans - DSOD reviews and approves dam owners' Emergency Action Plans (EAPs).

6. **Post-Construction**:

   Following removal of the dam the following activities can be expected:

   - Final Inspections - DSOD conducts a final inspection after dam removal is complete.
   - Cost Affidavit - Dam owners submit an affidavit detailing the actual construction costs.
   - Filing Fees - Additional fees may be required if the final cost exceeds the estimate.

### E.2.2.3 California Department of Fish and Wildlife (CDFW):

Lake or Streambed Alteration Agreement: Notification to the CDFW and a subsequent agreement are required for any activity that may substantially divert or obstruct the natural flow of, or substantially change or use any material from the bed, channel, or bank of any river, stream, or lake. Information developed for the Surrender Application will be used to prepare the agreement.

### E.2.2.4 California Endangered Species Act (CESA)

Projects must comply with CESA to protect listed species and their habitats, potentially requiring permits or consultations. PG&E will need to consult with the California Department Fish and Service to ensure that no California listed threatened or endangered species are affected by the project.

California ESA compliance permitting, involves obtaining authorization to "take" harass, harm, pursue, hunt, shoot, wound, kill, trap, capture, collect, or attempt to engage in any such conduct) a species listed as threatened or endangered under CESA. This typically involves an Incidental Take Permit (ITP) from the California Department of Fish and Wildlife (CDFW) and can be triggered by projects that may impact listed species.

Key aspects of California ESA compliance permitting include:

- **Listing of Species** – CESA lists various plant and animal species as threatened or endangered.
- **“Take”** – The definition of “take” under CESA is broad and includes various actions that could harm or disturb listed species.
- **Incidental Take Permits (ITPs)** - ITPs are the primary mechanism for authorizing the “take” of listed species when it is incidental to an otherwise lawful activity. If a project has already secured federal ESA authorization (e.g., a federal ITP or Section 7 consultation), a Consistency Determination from CDFW may be sufficient for CESA compliance.
- **Application Process** – Applications for CESA permits must be submitted to the appropriate Regional Manager at CDFW and include detailed information about the project, the species potentially impacted, and measures to minimize and mitigate those impacts.
- **Fees** – There are application fees associated with CESA permits.
- **Impact Analysis** – Permit applications require an analysis of potential impacts to listed species and whether the project would jeopardize their continued existence.
- **Minimization and Mitigation** – Permittees must implement measures to minimize and fully mitigate the impacts of the authorized take.

In essence, compliance with California’s ESA involved a comprehensive process of assessment, permitting, and mitigation to ensure that projects minimize harm to listed species and contribute to their conservation.

# E3.0 References

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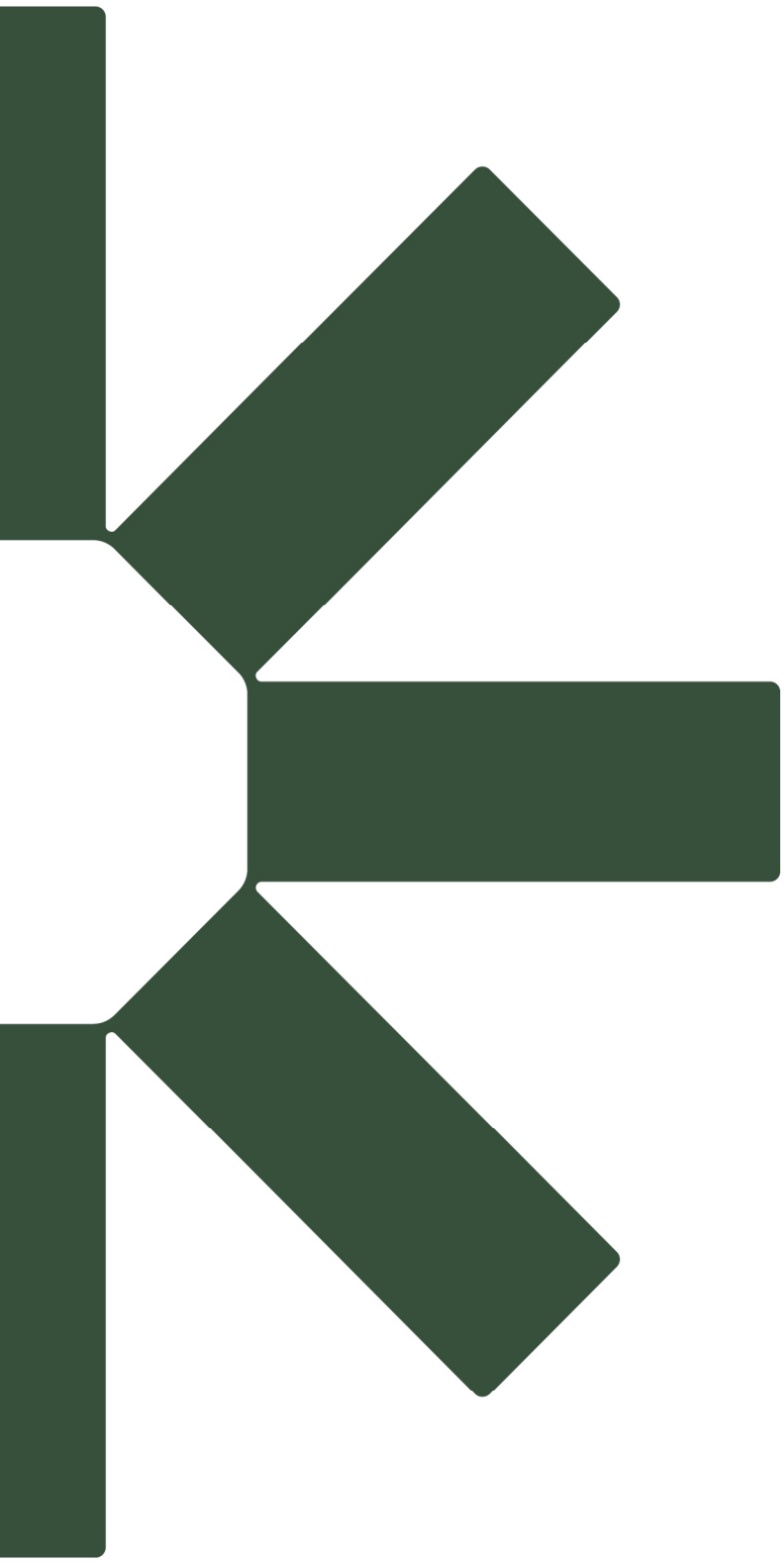

Making Sustainability Happen

# ATTACHMENT 3

**SLR International Corporation**
2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

August 21, 2025

County of Lake
255 N. Forbes Street
Lakeport, CA 95453

SLR Project No.: 102.021249.00001

**RE: SLR Comments on PG&E's Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 2025**

# 1.0 Introduction

SLR International Corporation (SLR) has been contracted by the County of Lake to analyze the effects of the proposed Scott Dam decommissioning, including providing comments on the Final Application for Surrender of License prepared by Pacific Gas & Electric Company (PG&E).

PG&E has submitted a Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 25, 2025 (*Final Application for Surrender of License; PG&E, 2025b*) for the Potter Valley Hydroelectric Project (*Federal Energy Regulatory Commission [FERC] Proceeding No. 77*) to FERC. The Final Application for Surrender of License includes information on the decommissioning process for the Potter Valley Hydroelectric Project, and for the construction of a New Eel Russian Facility, which is identified as non-project use of project lands by the Eel-Russian Project Authority (ERPA). The publicly available contents were published in five electronic documents, consisting of Volume IA, Volume IB, Volume II Parts 1 to 3. P&E also submitted two Volumes (III and IV) with privileged information to the FERC. PG&E hosted a virtual town hall to present the contents of the Final Surrender Application on August 11, 2025.

This letter report summarizes SLR's comments on PG&E's 2025 Final Application for Surrender of License. The comments provided only refer to the Application for Surrender of License aspects mainly included in Volume IA, and Volume II Parts 1 and 2, and not the Non-Project Use of Project Lands by the ERPA. It is SLR's understanding that the County of Lake intends to submit this comment letter to FERC during their official comment period.

Although FERC has primary regulatory authority in the Decommissioning Proceeding filed under FERC Docket P-77, the County would like SLR to provide recommendations on environmental mitigation and economic impacts to be considered by both FERC and PG&E in connection to the Environmental Impact Statement (EIS) process required by the National Environmental Policy Act (NEPA) and the Environmental Impact Report (EIR) process required by the California Environmental Quality Act (CEQA).

The focus of SLR's comments is on the existing known or documented conditions and effects within the County of Lake; impacts outside of the County may likewise be noted, where relevant to the County of Lake's concerns.

The position of the County of Lake Board of Supervisors is that PG&E has not demonstrated the full effects of dam removal, proposed environmental mitigation measures or addressed potential economic impacts to the County[1].

**Organization of the Comment Letter:**

This letter is organized as follows:

**1.0 – Introduction:** Provides an overview of the Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 25, 2025; background information on work conducted by PG&E and others for the Potter Valley Hydroelectric Project, and Interventions by the County of Lake.

**2.0 – General Comments by SLR:** This section lists general comments on the Final Application for Surrender of License, and the general decommissioning process.

**3.0 – Comments on PG&E Application for Surrender of License:** This section is subdivided into 17subsections that correlate with the Affected Environment resource areas identified by PG&E. In particular, the resource areas directly match those listed in *Volume II, Section 2.2.3 Affected Environment* of the Final Application for Surrender of License. A summary correlation table is included for the reader.

## 1.1 Background Information

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in PG&E's Potter Valley Hydroelectric Project. Scott Dam is located on the Eel River in the County of Lake, within the Mendocino National Forest.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The Final Application for Surrender of License considers removal of Scott Dam and draining Lake Pillsbury. The proposed decommissioning of Scott Dam is also considered by some interested parties as part of a broader initiative to restore the Eel River's ecosystem and improve fish passage[2].

Scott Dam is a concrete, gravity-type, ogee-shaped structure. It is 130 feet high and 805 feet long. The spillway crest elevation is 1,900 feet[3]. There are five radial gates and 26 steel slide gates surmounting the spill crest that allow storage of water 10 feet above the spill elevation.

At its maximum capacity, Lake Pillsbury has a surface area of approximately 2,275 acres with an elevation of 1,910 feet. The initial design capacity of the lake is 76,876-acre feet at the elevation of 1,910 feet. PG&E reduced this capacity in 2023 due to concerns of dam instability, to a maximum elevation at the spillway crest of 1,900 feet. Therefore, the reservoir is currently operated for a storage capacity of 66,876 acre-feet (ac-ft).

1) On January 25, 2019, PG&E filed a notice of withdrawal with FERC, discontinuing its efforts to relicense or sell/transfer the Potter Valley Hydroelectric Project. FERC issued a notice soliciting applications, which did not result in a viable new applicant[4].

[1] County of Lake Comments on Pacific Gas & Electric Company's (PG&E's) Draft Application for Surrender of License and Application for Non-Project Use of Project Lands dated March 3, 2025.
[2] Friends of the Eel River, n.d. https://eelriver.org/about-us/
[3] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. Elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.
[4] FERC Accession No. 20190125-5100 and FERC Accession No. 20190301-3038.

2) Since July 2021, the Potter Valley Powerhouse has been offline. PG&E determined a transformer did not meet operating standards (PG&E, 2025). No plans were made to return the Potter Valley Powerhouse to service.
3) The Project License expired April 14, 2022. The default license term for most hydroelectric projects under FERC jurisdiction is 40 years. Currently PG&E operates the project under annual licenses (PG&E, 2025).
4) FERC issued a Notice of Authorization for Continued Project Operation, on April 21, 2022.
5) On July 8, 2022, PG&E proposed to file an application for surrender of license by January 2025, which was acknowledged by Commission staff on July 29, 2022.
6) In 2023, the Scott Dam spillways were permanently opened due to seismic risk with the approval of the Division of Safety of Dams (DSOD, a Division of the California Department of Water Resources). Lake Pillsbury reservoir had 20,000 AF of reduced capacity. PG&E submitted a long-term flow variance request to FERC. A Water Quality Certification for the lower flow is pending with the State Water Resources Control Board[5].
7) On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the Eel-Russian Project Authority (ERPA)[6].
8) On June 6, 2024, PG&E documented progress in developing a surrender plan among the consulting parties.
9) Prior to filing their final application with FERC, PG&E provided a Draft Application for Surrender of License to interested parties and agencies in January 2025, for a 30-day comment period.

Several non-governmental organizations (NGOs), governmental agencies, and working groups have an interest in the Decommissioning process.

Further, a Conservation Easement for 5,660 acres of PG&E-owned land, of which approximately 2,234 acres are within the FERC Project boundary, was recorded on June 28, 2022, to permanently protect beneficial public values on lands owned by PG&E (PG&E, 2025, PG&E, 2022a). The conservation easement is held by Mendocino Land Trust. The easement permanently restricts development and protects conservation values (habitat, recreation, cultural resources) regardless of PG&E's future use. If PG&E ceases power operations, the land could be sold or transferred, but any new owner must comply with the conservation easement. Mendocino Land Trust is to enforce conservation restrictions; they do not own the land but can limit certain uses.

## 1.2 Interventions and Comments by the County of Lake

On July 31, 2023, the County of Lake filed a motion to intervene on the Potter Valley Hydroelectric Project Decommissioning with FERC using FERC Rule 214 subdivision (b). The County's interests in the proceedings include:

[5] State Water Resources Control Board (SWRCB), 2017. Study Requests and Comments on the Pre-Application Document and Scoping Document 1 for Potter Valley Hydroelectric Project, FERC Project No. 77, Mendocino and Lake Counties. August.
[6] The Agreement is available here: https://www.sonomawater.org/media/PDF/Water%20Resources/Potter%20Valley%20-%20ERPA/for%20signature_%20JEPA%20FINAL%20-%20Eel%20Russian%20Project%20Auth%20-%20signed.pdf

1) The Potter Valley Project (PVP) does not just generate power but also provides water supply for agriculture, recreation, and wildfire suppression.
2) The County has an interest in this matter as this project creates Lake Pillsbury on the Eel River. This lake has 31 miles of shoreline, used for recreational activities such as fishing, boating, sailing, and waterskiing.
3) The County opposes Decommissioning of Scott Dam, in part, because PG&E has not outlined mitigation measures that would be put in place to address environmental and safety concerns raised by the County.
4) If FERC provides approval for the Decommissioning of Scott Dam, the County of Lake requests the following mitigation measures:
    a. Establishing fire protection measures that directly address wildfire risks resulting from removal of Scott Dam;
    b. Establishing water rights for the County of Lake and its residents on Scott Dam, and implementing water storage measures for the County of Lake;
    c. Financial consideration of lost County of Lake property tax collections, and any and all revenue losses to result from the removal of Scott Dam;
    d. Additional monetary consideration for loss of economic development opportunity in the County of Lake;
    e. Full restoration and revegetation of the area, restoring flora and fauna in areas due to the loss of Scott Dam and Lake Pillsbury; and
    f. Constructing road and other infrastructure improvements necessary to safely complete the decommissioning plan.
5) The County asserts that any financial burden of these mitigation measures should be the responsibility of PG&E, as PG&E should remain responsible for the PVP and all consequences throughout the project lifecycle including Decommissioning.
6) Additionally, FERC should maintain PG&E's Licensee status until these mitigation efforts are addressed. For example, FERC required PacifiCorp (the project owner) to stay Co-Licensee surrounding Decommissioning of the Klamath Dams.

On December 23, 2023, the County of Lake issued a comment letter to PG&E regarding the PVP Initial Draft Surrender Application. This letter expressed concern regarding the lack of sufficient consideration for the effects of decommissioning on the County of Lake. These effects include loss of property, environmental concerns, regional loss of recreation, loss of water supply for wildfire suppression, and more. Additionally, the rapid timeline proposed by PG&E and lack of risk mitigation measures with respect to the County of Lake was an issue. The County is concerned about the need to manage large amounts of sediment and the lack of communication between PG&E and the County. Lake Pillsbury is an important source of economic activity as well as local government tax revenues.

PG&E documents several meetings with the County of Lake in their Final Application for Surrender of License; nevertheless, no substantive formal response to comments has been provided by PG&E to address the County of Lake's concerns, including the most recent County of Lake Comments on PG&E's Draft Application for Surrender of License and Application for Non-Project Use of Project Lands dated March 3, 2025.

## 1.3 Regulatory Process

PG&E has summarized the process to be followed by FERC and the State Water Resources Control Board as required by NEPA and CEQA. The process described is similar to what has been followed in other decommissioning processes.

A final approval of the Application for Surrender of License would be documented in a FERC Order with conditions as specified in the EIS (*led by the FERC*) and EIR (*led by the State Water Resources Control Board*), Management Plans (*prepared by PG&E and others*), and any other conditions.

# 2.0 General Comments by SLR

In accordance with 18 Code of Federal Regulations Ch. I § 6.1, an application for the Surrender of a license for a dam must include a detailed plan outlining the disposition of project works and any required restoration measures. The plan should address environmental, public safety, and operational concerns associated with cessation of the project's use. The Application for Surrender of License filed with FERC has not yet included all the necessary environmental documentation and stakeholder consultations to ensure compliance with federal regulations. Based on SLR's independent review:

1) PG&E has acknowledged but not fully addressed or requested an evaluation of Federal and California Environmental review processes. The County of Lake intends to participate in both processes.
2) PG&E's Final Application for Surrender of License states Scott Dam Decommissioning in the form of Rapid Dam Removal as the Proposed Action. The only Alternative considered was Phased dam removal. For the benefit of the County of Lake as well as other stakeholders no other alternatives were presented in the Final Application for Surrender of License. FERC needs to request PG&E to evaluate and include other decommission options such as partial removal of Scott Dam to best benefit the community and minimize environmental impacts.
3) PG&E has included the request for the use of NPUPL for FERC to approve the construction NERF in their final application, along with a comparison of outflows under existing and proposed conditions. However, the statement made by PG&E that they will be able to meet their flow requirements post-removal is overly optimistic. PG&E needs to consider what keeping Scott Dam in its existing state would do to meet minimum flow requirements compared to proposed action. Under existing conditions, PG&E sometimes falls out of compliance with FERC. Therefore, removing Scot Damm would only make it more challenging to meet minimum flows in the future, which is consistent with PG&E's modeling.
4) Specific studies are not included in the Final Application for Surrender of License even though PG&E acknowledged in the Draft Surrender Application additional studies would be conducted as part of the final Decommissioning per the future FERC order to be issued.
5) In the Final Application for Surrender of License, there are references to facilities and features that will be left in place (*including piezometers and block building*) and there is no detailed discussion with respect to the plan to leave in place/restore.
6) The County of Lake has not been engaged in meaningful stakeholder consultation, and therefore the stakeholder consultation has not been adequate from the County's perspective.

7) PG&E has stated in the Final Application for Surrender of License that various impacts resulting from the Scott Dam Decommissioning are unavoidable adverse impacts. However, the SLR review indicates these impacts are not sufficiently evaluated to be considered unavoidable.

8) PG&E should more thoroughly evaluate these impacts to establish that they are unavoidable and identify and evaluate options for minimizing unavoidable impacts. The County proposes PG&E include additional restoration scenarios that will likely minimize the unavoidable impacts to the County and enhance recreation, ecosystem, and fire suppression benefits.

9) To the benefit of stakeholders, including the County, PG&E should make all relevant data available to stakeholders and firms providing contracted technical support. Scott Dam has been in place since 1921, and there is limited publicly available data. A majority of relevant data is managed by PG&E. The various reports and studies completed reference a variety of PG&E supplied data not made available for this review.

Unavoidable adverse impacts are defined in both NEPA and CEQA. The definitions include:

- NEPA states "Effects that can not be avoided due to constraints in alternatives. These effects do not have to be avoided by the planning agency, but they must be disclosed, discussed, and mitigated, if possible [40 CFR 1500.2(e)]" [7].
- CEQA states "A project impact is considered significant and unavoidable if it would result in a substantial adverse change in the environment that cannot be feasibly avoided or mitigated to a less-than-significant level if the project is implemented. If a lead agency proposes to approve a project with significant unavoidable impacts, it must adopt a statement of overriding considerations to explain its actions (CEQA Guidelines, Section 15093(b))"

PG&E has indicated in the final application that several impacts are unavoidable; nevertheless, PG&E has failed to identify mitigation strategies and offsets. PG&E would need to comply with NEPA and CEQA with respect to defining the unavoidable impacts.

Clear understanding of the impacts expected to result from the proposed decommissioning is important to the County, as following the decommissioning process, the County of Lake will need to revise its General Plan and the Local Area Plan.

The Conceptual Decommissioning Plan does not specify how Scott Dam and the Eel River flows will be managed if the first high-flow season does not occur the November following the initial low-flow season (*i.e. if it is a low-flow winter that does not produce much precipitation*).

It is unclear how PG&E will determine when a wet season with enough precipitation is anticipated to conduct the sediment flushing needed during the first high-flow season described in the Conceptual Decommissioning Plan. PG&E should consider using the definition of the historic precipitation data in forecasting expected wet and dry seasons. In the absence of a high precipitation season, a long slow bleed of sediments may occur for many years, and PG&E would need to address these scenarios in the Sediment Management Plan.

Several plans to manage and mitigate environmental effects are briefly described in the application. PG&E needs to provide more detailed information on how proposed mitigation measures will be implemented. Additional comments are anticipated to be provided when these details become available. PG&E should also provide a schedule for when Management Plans will

[7] US Forest Service, nd. NEPA Glossary. https://www.fs.usda.gov/Internet/FSE_DOCUMENTS/stelprdb5298903.pdf

become available for public review and explain how the measures proposed in different management plans will be related to each other.

# 3.0 Comments on PG&E Surrender Application

SLR comments are presented in the following subsections, organized by the resource areas in the draft PG&E application. For the reader's reference, this table outlines the corresponding sections in the PG&E application and this comment letter. Any comments that were not related to a specific resource area in the PG&E Final Application for Surrender of License are presented in Section 3.19.

| Topic | PG&E Vol IA | PG&E Vol II | SLR Comments |
|---|---|---|---|
| Water Use and Hydrology | 6.2.1 | 2.2.3.1<br>2.2.4.2 | 3.1 |
| Water Quality | 6.3.1.2 | 2.2.3.2<br>2.2.4.3 | 3.2 |
| Fish and Aquatic Resources | 6.3.1.3 | 2.2.3.3<br>2.2.4.4 | 3.3 |
| Botanical Resources | 6.3.1.4 | 2.2.3.4<br>2.2.4.5 | 3.4 |
| Wildlife Resources | 6.3.1.5 | 2.2.3.5<br>2.2.4.6 | 3.5 |
| Geology and Soils | 6.3.1.6 | 2.2.3.6<br>2.2.4.7 | 3.6 |
| Geomorphology | 6.3.1.7 | 2.2.3.7<br>2.2.4.8 | 3.7 |
| Land Use | 6.3.1.8 | 2.2.3.8<br>2.2.4.9 | 3.8 |
| Recreation Resources | 6.3.1.9 | 2.2.3.9<br>2.2.4.10 | 3.9 |
| Aesthetic Resources | -- | 2.2.3.10<br>2.2.4.11 | 3.10 |
| Cultural Resources | 6.3.1.10 | 2.2.3.11<br>2.2.4.12 | 3.11 |
| Tribal Resources | 6.3.1.11 | 2.2.3.12<br>2.2.4.13 | 3.12 |
| Socioeconomic Resources | 6.3.1.12 | 2.2.3.13<br>2.2.4.14 | 3.13 |

| Topic | PG&E Vol IA | PG&E Vol II | SLR Comments |
|---|---|---|---|
| Air Quality | -- | 2.2.3.14<br>2.2.4.15 | 3.15 |
| Noise | 6.3.1.13 | 2.2.3.15<br>2.2.4.16 | 3.16 |
| Traffic | -- | 2.2.3.16<br>2.2.4.17 | 3.17 |
| Marine Resources | 6.3.1.14 | 2.2.3.17<br>2.2.4.18 | 3.18 |
| Others | -- | -- | 3.19 |

## 3.1 Water Use and Hydrology

- PG&E needs to discuss the local beneficial uses of Lake Pillsbury water, including drinking, recreational, and others, and how loss of this water supply for the various beneficial uses will be mitigated. PG&E notes in the application there are no expected effects on water reliability or cost in the Scott Dam Area because there are no known surface or authorized water diversions, but PG&E did not acknowledge the potential recharge to groundwater from Lake Pillsbury, which will result in reduced water reliability and increased cost for well owners, even though it refers to this effect as part of impacts on campgrounds/recreational facilities. Additional related comments are provided as part of the Geology and Soils, and Socioeconomic Resources sections herein.
- Water from Lake Pillsbury is currently used for fire suppression, and removing the lake will affect this resource and water use. Additional related comments are provided as part of the Land Use section.
- PG&E should evaluate the water supply and the effects on groundwater recharge as part of the final surrender application. Water supply insecurity has been a concern during drought conditions in recent years even with Scott Dam in place. As part of the NEPA Process, PG&E may be required to prepare a Water Supply Management Plan and Public Drinking Water Management Plan. The Final Application for Surrender of License does not include any details on these plans. It is currently unknown what the anticipated groundwater levels will be in the vicinity of Lake Pillsbury. Drinking water should be made available to well owners that will be affected by the reduced groundwater recharge resulting from draining the lake. Long-term effects are not expected to be known until several years have passed. PG&E needs to address in more detail than currently presented in Section 2.2.4.7.2 the groundwater table changes anticipated post-construction.
- PG&E notes "removal of the dam would restore flows in the Eel River to unimpaired natural conditions. High flows are similar under existing conditions (*i.e. No-Action*) and the Proposed Action (refer to Section 2.2.3.1, Figure 2.2.3.1-18), but low flows would vary substantially. […] Because there are no water users in the Eel River between Scott Dam and the Van Arsdale Diversion, the Proposed Action would have no effect on water users in the river reach." PG&E needs to evaluate the current beneficial uses of Lake Pillsbury to include recreational, ecosystem, and fire suppression.

- The mass balance approach PG&E used to determine unimpaired flows and flood frequency is unconventional per standard industry practice. PG&E needs to develop a flood flow frequency analysis based on the USGS Guidelines for Determining Flood Flow Frequency – Bulletin 17C[8] and perform an uncertainty analysis to account for the raw hydrology data and unimpaired natural flow assumptions. Active USGS gages along the Eel River and within the County of Lake are limited and restricted in the information provided. Streamflow gages near Scott Dam only record periods where flow is controlled or impaired by dam operations and there are no known or active precipitation gages within the entire Eel River watershed. The streamflow gages used in the hydrologic analyses are likely to incur levels of uncertainty based on unimpaired data available. PG&E should further analyze the raw hydrology data and assumptions to perform an uncertainty analysis.
- The January 2025 Final Application for Surrender of License does not provide any insight on validating or calibrating the hydrology assumptions for unimpaired flows. PG&E needs to add additional streamflow gages upstream of the Scott Dam to understand inflows to the impoundment, validate and calibrate unimpaired natural flows, and predict future climate change flows. Note that, the California State Water Resources Control Board, as part of the 2022 California Stream Gaging Prioritization Plan, have highlighted the Smokehouse Creek-Lake Pillsbury, Lake Pillsbury-Eel River, and Rice Creek-Rice Fork watersheds that supply Lake Pillsbury as high-priority watersheds with significant gaps in their stream gaging network.[9] The Plan also highlights that the Eel River reach between Scott Dam and Cape Horn Dam are not defined as well-gaged but as almost well-gaged or un-gaged. PG&E needs to justify their assumptions on what unimpaired flows are expected to look like with technical references and provide rationale for their assumptions.
- PG&E should conduct further analysis into the operations, storage, and streamflow gage relationship to better understand the flow relationship to operations and assumptions for unimpaired flows and water supply. Further analysis over the entire period of record will also provide a level of confidence in the data and assumptions made for the hydrology and hydraulics. Operations of Scott Dam have changed due to varying regulations and restrictions over the life of the dam. Storage capacity has likewise changed, due to the accumulation of sediment within Pillsbury Reservoir and permanently opening the spillway gates due to seismic concerns. SLR recommends verifying the assumption of no sediment management or removal of sediment within the Lake Pillsbury Reservoir. Changes in operations and storage can affect the stage discharge relationship.
- PG&E needs to further develop the hydrology, building on past analyses to evaluate future climate change scenarios as part of the environmental analysis for dam removal.
- PG&E should develop a hydraulic model to better define the proposed action during varying seasons and recurrence intervals. This would also support quantifying the sediment yield.
- PG&E needs to analyze sediment grain size distribution and determine the grain size and flow correlation for mobility (*i.e., At what flows will sediment move?*) to support changes in the longitudinal profile as well as core sampling the entire depth of sediment within Lake Pillsbury. Tributaries upstream and downstream of Scott Dam are likely sediment sources

[8] https://pubs.usgs.gov/tm/04/b05/tm4b5.pdf
[9] SB 19 Stream Gaging Plan | California State Water Resources Control Board

that account for and will continue to transport sediment into the Eel River system under all conditions.

- PG&E needs to develop a risk and uncertainty analysis for the proposed Scott Dam removal to better understand potential consequences and articulate the level of uncertainty or confidence in the data assumptions. The hydrology informs the hydraulics and the function of the existing and proposed infrastructure and the assumptions under proposed conditions. If hydrologic information is limited or inconclusive, there is a lower level of confidence in the models and assumptions made to fill in gaps within the data.
- PG&E notes "The Flood Monitoring Plan would establish protocols for monitoring water levels and flood risks, enabling proactive measures to protect infrastructure and surrounding areas." The details of the proposed plan will need to accommodate the findings of the additional hydraulic modelling for defining the monitoring requirements.
- PG&E needs to reevaluate claims made that the proposed removal of the dam will not harm their ability to meet downstream flow requirements. Under existing conditions, with operational control of the needle valve and spillway, they have reported noncompliance to FERC, most often during the dry season. Under proposed conditions, they would have no operational control or storage. During dry seasons, and especially droughts, the likelihood of noncompliance is higher under proposed conditions for a variety of hydrologic and climate-related factors. PG&E's modeling results concur with this: graphs of flow projections under proposed conditions show flows hovering near and going below 10 cubic feet per second (cfs) during the dry season every year. Currently, 20 cfs is their all-season minimum flow requirement. Therefore, a more thorough assessment of the proposed dam removal's impact on flows is necessary.

## 3.2 Water Quality

- PG&E notes they expect a short-term adverse effect due to the suspended sediment and turbidity following removal of the dams. Based on the level of uncertainty of the models, and lack of grain size analysis data, it appears that the anticipated period of several days up to several months, and additional effects with remobilization for 1 to 3 years may be optimistic. PG&E should conduct further studies to improve the sediment grain-size distribution dataset and update the sediment transport models with improved data. PG&E needs to validate their claim that "suspended sediment concentrations might be less than modeled" with their own modeling, an explanation, and references.
- PG&E should consider the above suggested additional data collection in development of short term and long-term sediment mitigation measures, including engineering controls and maintenance activities.
- PG&E acknowledges there will be impacts to water temperatures during and following the removal of Scott Dam. Given the uncertainty of the existing models, these may be underestimated, and additional evaluation with improved data should be conducted by PG&E in order to facilitate the methodologies for temperature monitoring. PG&E should update the Summer Water Temperature Monitoring Plan from 2005 due to operational, climate, and water use changes that have taken place since that time.

## 3.3 Fish and Aquatic Resources

- The Final Application does not include the estimated current population of all fish species in Lake Pillsbury. A list of the fish species that can be found in Lake Pillsbury can be found in Table 2.2.3.3-16 (Volume II) of the Final Application. For the fish species in Lake

Pillsbury, PG&E includes the 2018 fish assemblage data in Figure 2.2.3.3-22 of the Final Application, showing the percentage composition of each fish species in a graph. However, PG&E only provides population numbers in Lake Pillsbury for rainbow trout (stocking data). PG&E needs to include the lake-related fish population for estimating the ecological impacts due to the decommissioning of the Lake.

## 3.4 Botanical Resources

- PG&E anticipates long-term changes in habitat conditions due to altered hydrology and land use, some impacts to sensitive habitats and species may be unavoidable despite mitigation. The information in the Restoration Plan will be key to better understand if PG&E is proposing adequate mitigation to prevent avoidable impacts.

## 3.5 Wildlife Resources

- The information PG&E provides in the numerous wildlife management plans will be key to assess if adequate mitigation is established to prevent and mitigate avoidable impacts. Stakeholder input will be needed on PG&E plans.

## 3.6 Geology and Soils

- PG&E needs to evaluate impacts on surface and groundwater interactions following dam removal and its effects on groundwater wells as part of the Surrender Process. It is anticipated stored groundwater will be used, and without surface water replenishment from the reservoir, groundwater levels will be lower and affect existing wells. As a mitigation, PG&E should establish a well monitoring program pre- and post-dam removal, and funding to support well owners to repair or deepen existing wells to allow access to groundwater. PG&E acknowledges the adverse impact, nevertheless there are mitigation options that should be considered and not just deemed unavoidable.
- PG&E should conduct additional hydrogeological assessments and evaluate the radius of impact of the groundwater level drawdown. Once the system is better understood, PG&E should contact residents and landowners in the impacted radius to notify of these potential impacts, and request information on existing wells in the affected area. A detailed groundwater monitoring plan should be developed prior to decommissioning of Scott Dam. In the absence of detailed groundwater studies, all well owners identified in Appendix 2.2.3.6-A should be notified of the potential impacts.
- Suggested mitigation strategies similar to those established for the Lower Klamath Projects should be considered by PG&E. For example, in that instance, the Water Board required the following conditions to be met by the Renewal Corporation. Note that the distances or number of locations listed below are for information only, and site-specific criteria applicable to this area need to be developed based on further studies:
  a) Monitor groundwater levels within a 2.5-mile range of the high-water marks, before, during and after reservoir drawdown;
  b) Contact all residents and landowners within 1,000 feet of the reservoirs;
  c) Identify potentially affected groundwater wells;
  d) Monitor groundwater levels at a minimum of 10 locations within the 1,000 feet radius for at least two months before drawdown activities, and continue monthly monitoring for at least two years; and

e) Mitigate groundwater impacts.

- The removal of the abutment structure will require more detailed evaluations including abutment removal sequence and phasing plan, temporary excavation support systems for worker safety, slope stability analysis for each stage of the proposed removal, permanent slope support systems, and ground movement monitoring and alarm systems. During removals, there is a high risk of triggering landslides. Proper restoration and re-vegetation of these slopes to their original condition may involve complete removal of the abutment.
- Upon removal, the existing slopes all around the formally inundated areas will be subject to higher risk of landslides. This risk may be heightened during or soon after water release due to rapid drawdown conditions. The slope conditions of currently submerged areas need to analyzed and remedial grading should be developed accordingly.
- Alternative options should be further evaluated, as they can provide a well-balanced remedy to many of the above-mentioned geotechnical and geologic concerns.
- PG&E needs to conduct targeted and dam-specific subsurface investigations as a crucial part of this process in the absence of existing documentation to support the post-removal conditions.
- Debris management needs to be considered as part of the removal process, including any disposal or reuse of the demolished concrete of the dam, other structures, material found at recreational facilities, and any material (*including solid waste*) discovered during sediment removal. Land disposal of waste in California is regulated under California Code of Regulations Title 27, CEQA and County ordinances. PG&E should detail their planned permitting process for disposal of non-hazardous waste, especially if they plan to dispose on PG&E lands as it had been described in their January 2025 Draft Application for Surrender of License.
- PG&E needs to evaluate risks associated with triggering slides when removing the dam and when cleaning out sediments. The Final Application to Surrender License does not provide details on evaluating these risks, it notes the exposed reservoir bed would be susceptible to erosion and proposes to define specific mitigation measures in management plans that will be prepared in the future.
- PG&E acknowledges Lake Pillsbury is designated as mercury-impaired on the State of California's 303(d) list because of elevated mercury concentrations in fish. Geosyntec[10] conducted an environmental sediment characterization and found no indication of significant mercury, or other contamination. Since the sediment studies were only conducted for shallow sediments, additional studies are recommended to evaluate metal concentrations and mobility, including but not limited to mercury, for the deeper sediments impounded.
- Additional investigations and planning are needed to verify and protect against a potential release of Naturally Occurring Asbestos (NOA) which is known to occur within the Lake Pillsbury watershed. A NOA management and mitigation plan would need to be developed to mitigate the potential impacts during the dam decommissioning process. The Final Application for Surrender of License cites prior studies on Lake Pilsbury sediments, nevertheless, does not provide specific references. A Geosyntec[11] study was conducted, but it only tested shallow sediments in the lakebed as the target depth of 20 feet could not

[10] Geosyntec, 2020. Lake Pillsbury and Van Arsdale Reservoir Sediment Characterization. April 1.
[11] Geosyntec, 2020. Lake Pillsbury and Van Arsdale Reservoir Sediment Characterization. April 1.

be met. Any available study should be properly cited, and additional studies should be conducted.

## 3.7 Geomorphology

- PG&E's proposed conceptual approach lacks sufficient baseline studies on the amount and quality of sediments impounded in Lake Pillsbury. The proposed Sediment Management Plan needs to include such additional studies needed.
- PG&E needs to complete a comprehensive sediment risk analysis for the proposed action. The United States Bureau of Reclamation recommends an iterative, risk-based approach for evaluating and managing sediment impacts from dam removal projects in its *Dam Removal Analysis Guidelines for Sediment* (Bureau of Reclamation, 2017).
- PG&E plans to develop a Scott Dam Slope Stability Monitoring Plan. That plan should include measures to address and reduce the potential for reactivating the landslide and to protect worker safety. PG&E should provide the details to better understand the proposed plan.
- The application notes "Water surface elevations would continue to decline after the dam is removed. Without the impoundment, the denuded slopes surrounding Lake Pillsbury and the exposed reservoir bed would be susceptible to water and wind erosion, especially as the soils dry." PG&E should provide the details to better understand the proposed plan.
- The PG&E Final Application for Surrender of License generally acknowledges sediment impacts to water quality, erosion and transport following dam removal. Nevertheless, no clear mitigation alternatives such as sediment stabilization or removal are proposed.
- To analyze the effects of sediment transport, PG&E relies on models by Stillwater Sciences (2021a,b)[12], though the model assumptions differ from the Proposed Action. Because the method of dam removal heavily affects sediment erosion and transport modelling, PG&E should model the final removal approach and determine sediment-related impacts.
- PG&E will need to evaluate sediment grain size distribution. Additional field studies would be needed to properly evaluate the relative proportions of coarse and fine sediment impounded behind Scott Dam, which will affect the transport of sediments. If the lake is drained, there would be increased landslide hazards of the currently submerged slopes. These risks would be increased in a similar fashion to lowered safety factors for any slope under drawdown conditions. This may involve long-term maintenance efforts in the wet seasons. Such slope failures or slumps would increase the volume of materials that have to be managed, in addition to the already inordinate amounts of sediments that must be mitigated. Other geotechnical issues to be looked at include the stability of submerged slopes when the lake is drained.
- Potential scouring at or near the toes of future slopes needs to be addressed as it can be a triggering factor for landslides.

[12] Stillwater Sciences. 2021c. Hydraulic model development and potential flood implications following the proposed Scott Dam removal, Eel River, California. Technical memorandum. August 2021. As referred to in PG&E, 2025.

## 3.8 Land Use

- PG&E noted that "The Proposed Action may result in unavoidable adverse effects to local fire suppression to properties near Lake Pillsbury due to the replacement of Lake Pillsbury with the Eel River or other sources as a water source, resulting in potentially longer fire response times." The loss of Lake Pillsbury water availability would have significant wildfire suppression adverse impacts on infrastructure, property, and human life. Below is an analysis of the key impacts that PG&E need to include in their analysis:
    - Keeping small wildfires small is vital in the County of Lake due to the history of wildfires in the area. Federal, state and local fire resources have all worked together in the past to combat the many large campaign fires that have brought stress to the communities that have been affected.
    - Dry lightning around the Lake Pillsbury area poses the greatest risk for large scale wildfires in the area due to their unpredictability. Lightning fires can spark a large-scale wildfire in very remote areas of the Mendocino National Forest and have the potential to grow rapidly with fuel and topography alignment. Dry Lightning is a natural occurring event in California and is always a threat to the wildland every year.
    - The County is already prone to having water availability problems due to the wildland rural community interface that affects the populace that call the area home. Water availability directly correlates with fire department resources efficiency to fight fire in a rural area. Large fires that develop near Lake Pillsbury can grow (and historically have grown) exceptionally large due to the difficult access and topography aspects that promote fire growth.
    - Since its creation, Lake Pillsbury has always been utilized as a water resource because it reduces flight or drive time to resupply water to combat fires in the surrounding area. Extended fire attack operations that take place around Lake Pillsbury benefit from and rely on the lake as a primary water source because of reduced drive time for ground resources to refill their apparatus with water. Additionally, not having a body of water available to fire resources will always change fire strategies. Mitigating fire threats in rural locations always makes fire attack operations difficult for firefighter safety.
    - In the Lake Pillsbury area, the first suppression resource on scene of a fire will most likely be a fire engine. There exists a direct mutual aid agreement allowing both US Forest Service and CAL FIRE to respond regardless of jurisdiction. The first responder could be one of the helicopters from CAL FIRE. Because of the limited resources in the more rural areas, the helicopter resource becomes a critical initial attack resource. The closest fire station to Lake Pillsbury is a USFS station 2.6 miles/9 min drive to Lake Pillsbury. The closest CAL FIRE helicopter air support is in Willits, Ca. The estimated flight time for a Sikorsky S-70 Firehawk helicopter from Willits to Lake Pillsbury is approximately 8 to 10 minutes, depending on specific conditions and flight paths. CAL FIRE's helicopter attack (Helitac) philosophy incorporates specific goals for air resources, enhancing their effectiveness in wildfire management. Their number one goal is a rapid response. Specifically, it is used to achieve a quick initial attack on wildfires, keeping most fires at 10 acres or less. Helicopters can reach remote fires in approximately 20 minutes, allowing for timely intervention.
    - The distance from Clear Lake to Lake Pillsbury is approximately 20 miles. Given that the Sikorsky S-70 Firehawk has a cruise speed of around 150 to 160 knots (approximately 173 to 184 mph), the estimated flight time for a Sikorsky S-70 Firehawk

from Clear Lake to Lake Pillsbury is approximately 6 to 7 minutes, depending on specific conditions and flight paths. Given the spacing between fire stations and the windy roads, the next fire engine to respond to a fire in the surrounding areas adjacent to Lake Pillsbury will be 20+ minutes away. Rotary wing and fixed wing aircraft play a pivotal role in helping control the spread of a fire. Helicopters can get water from many sources, sources as small as a swimming pool. However, smaller water sources may only allow for one or two dips.

- Another issue with water sources for the fire service is cross-contamination. In California, aircraft used for firefighting must adhere to strict guidelines to prevent cross-contamination of water from different sources, such as lakes or reservoirs. Firefighting aircraft are required to identify and use designated water sources that have been approved for firefighting purposes. To prevent contamination, aircraft must avoid collecting water from multiple sources in a single mission. This means they should not dip from different lakes or reservoirs in one operation. For CAL FIRE's fixed wing resources, the CL-415 Super Scooper is utilized, and these aircraft are able to scoop water from a larger water source. The scoopers response time is a little slower than the Helicopters, but they can dump more water on a fire than a helicopter. It would take about 20-25 minutes to fly from a nearby airbase to wildfires adjacent to Lake Pillsbury. For fires north of Lake Pillsbury there are fewer larger lakes and greater distances between fire stations.
- Removing Scott Dam, eventually leading to Lake Pillsbury ceasing to exist, would increase fire behavior in the area due to the exposed lakebed. Invasive plant species are already well established in the area that can contribute to erratic fire behavior. Previous fires like the August Complex and the Mendocino Complex have already burned an exceptionally sizable portion of the Mendocino National Forest. Additionally, invasive brush and grass species will grow in these burn scars and create a bigger problem for wildfire, and in return make wildfire difficult to mitigate with ground and aerial resources. Ecosystems will be affected when taking a water source from the area, which means drier fuel moisture content that can increase fire behavior with certain types of fuel models. The largest fire in the State of California was the Mendocino Complex Fire. Without the water from Lake Pillsbury, the potential exists for a similar situation. This makes Lake Pillsbury an important resource for the fire service and its loss will have long-term adverse wildfire suppression impacts.

- There are additional Wildfire Suppression-related adverse Impacts on infrastructure, property, and human life, as noted below (these should be addressed by PG&E):
  - Increased Wear on Aviation Equipment: Helicopters would need to fly an additional 40 miles to access water, leading to increased operational wear and tear on rotors, engines, and other critical components. This could shorten equipment lifespan and increase maintenance costs, reducing overall readiness.
  - Strain on Fuel Infrastructure: The additional flight distance increases fuel consumption, which could strain local aviation fuel supplies and necessitate more frequent refueling, delaying firefighting operations.
  - Delay in Other Emergency Responses: With helicopters occupied for longer periods, there would be reduced availability for other critical missions, such as evacuations or medical response.
  - Delays in Fire Containment: The additional round-trip distance to collect water translates to slower response times for suppressing fires, increasing the likelihood of

wildfire spreading. This could result in greater destruction of homes, businesses, agricultural lands, and critical infrastructure (*e.g., power lines, roads*).

- Increased Costs of Fire Damage: Larger, uncontrolled wildfires typically lead to greater property losses and more extensive rebuilding efforts, which can strain local and state resources.
- Increased Risk to Residents: Delayed fire suppression increases the danger to residents, especially in rural areas where evacuation routes are limited. Lives could be lost or disrupted due to a lack of timely intervention.
- Helitac Crew Safety: Longer flight distances increase fatigue for Helitac crews, raising the risk of accidents during firefighting operations. The additional flight time also increases exposure to hazardous conditions, such as smoke, turbulence, and heat.
- Health Impacts from Smoke: Slower containment of wildfires leads to prolonged exposure to wildfire smoke for residents, exacerbating respiratory and cardiovascular issues, particularly among vulnerable populations.
- Loss of Natural Resources: Delayed fire suppression can lead to the loss of forests, watersheds, and wildlife habitats, which are critical for local ecosystems and the area's economy.
- Economic Hardship: The destruction of property and infrastructure could result in significant economic hardship for affected communities, including loss of jobs, reduced tourism, and increased insurance premiums.

- The decommissioning project will require several road improvements to ensure safe execution during the project and to maintain accessibility for emergency wildfire response, recreation, and other purposes afterward. Below is a breakdown of the necessary road improvements that PG&E will need to incorporate in future road improvements:
  - Fire Access Road accessibility around Lake Pillsbury is a concern for emergency response and must be upgraded to allow rapid emergency response. Roads always pose concern for emergency vehicles due to the difficulty in turning fire engines, if needed. Constructing or upgrading fire access roads with all-weather surfaces will ensure year-round usability. Adding turnouts and passing lanes on long, narrow roads will improve emergency vehicle safe passage.
  - Encroaching vegetation will also need to be cleared from high priority areas to reduce fire risk and improve visibility for emergency responders.
  - Recreational access roadways post-decommissioning, to include trailhead and parking access may be changed or require modifications to parking lots and trailheads. Improvements may need to be made, including creating or improving parking facilities and access points for hiking, fishing, and other activities, and installing clear signage to direct visitors to new recreation areas.
  - If the decommissioning affects current road alignments (*e.g., roads become submerged or rerouted due to reservoir changes*), new road sections may need to be built. Any unpaved or poorly maintained roads in the area must be upgraded to meet CAL FIRE standards for emergency access. The decommissioning project will require significant upgrades to roads and related infrastructure to ensure safety during construction and maintain accessibility afterward. Improvements such as road widening, weight-bearing capacity increases, emergency access enhancements, erosion control, and vegetation management will be essential. These upgrades will not

only support the decommissioning project but also ensure the long-term usability of the area for wildfire response, recreation, and other community needs.

- The removal of Lake Pillsbury as part of the PG&E Scott Dam Decommissioning Project raises valid concerns about wildfire spread, particularly the loss of the Lake's role as a barrier to prevent small fires from becoming larger events, according to United States Forest Service and Lake Pillsbury Fire Protection District personnel. The lake has frequently acted as a natural firebreak, slowing or stopping the spread of wildfires. Without the lake, vegetation would replace the reservoir basin, potentially increasing fuel loads and creating a continuous pathway for fire spread. Below is an analysis of the technical considerations to address wildfire concerns and the feasibility of retaining or reconfiguring the lake to meet both wildfire mitigation and seismic safety goals that must be considered by PG&E:
    - Vegetation Management: Implementing rigorous fuel reduction measures (*e.g., controlled burns, mechanical thinning*) in the lake basin and surrounding areas to reduce fire risk.
    - Establishing permanent firebreaks where the lake currently acts as a barrier.
    - Strategic Water Storage: Constructing smaller reservoirs, ponds, or water tanks within the region to provide accessible water for firefighting.
    - Installing dry hydrants and ensuring sufficient water storage for aerial and ground crews.
    - Infrastructure Upgrades: Setting up permanent firefighting staging areas with water access near the former lakebed.
    - Enhanced Detection Systems: Installing advanced fire detection systems, such as remote cameras, infrared sensors, and drones, to monitor lightning-related ignitions.

## 3.9 Recreation Resources

- PG&E is planning to remove several of the existing camping facilities. This would result in a significant recreational impact. To reduce recreation impacts, PG&E should also include recreational restoration in the Restoration Plan they intend to develop. The plan should be developed to offset the loss of recreational facilities, such as swimming opportunities, hiking, picnicking, off-road vehicle paths, camping and fishing. This plan needs to be developed in coordination with both USFS/Mendocino National Forest (MNF) staff and the County of Lake.
- The recreational features surrounding Lake Pillsbury that will continue to operate are:
    - Pogie Point Campground
    - Sunset Point Campground
    - Oak Flat Campground
    - Navy Camp Campground
    - Trout Creek Campground (Cape Horn Dam Area)

    These recreational features can be found in Map 2.2.3.9-2, though PG&E should provide additional details on any improvements or changes to the facilities to maintain its functionality if Lake Pillsbury is drained.

- PG&E's Final Application and environmental reports need to address loss of revenue generated by visitor usage and similar losses that will be experienced in the area.
- Recreation needs to be quantified as value revenue loss. PG&E should provide compensation to the County of Lake for the loss of recreation. Other locations offering similar lake-based recreational opportunities are a 45-minute to 2-hour drive from Lake Pillsbury, and include Clear Lake, Lake Sonoma, and Lake Mendocino. PG&E should discuss the potential for overcrowding at these other facilities due to closure of recreational facilities at Scott Dam.
- Recreation opportunities in the Project vicinity are concentrated around Lake Pillsbury, which inundates land primarily owned by PG&E but is surrounded by public land managed by the US Forest Service.
- Lake Pillsbury is the largest waterbody in the MNF and provides a variety of reservoir-based recreation opportunities such as boating, wind surfing, fishing, and swimming. There are a variety of developed recreation facilities in the immediate area of Lake Pillsbury, including family campgrounds, group campgrounds, and day-use opportunities.
- All Project recreation facilities (including campgrounds and day-use areas), ancillary facilities, and associated access roads in the Scott Dam area would be removed and the sites would be restored.
- PG&E has stated in the Final Application that recreation will shift from lake-based to riverine-based and acknowledges the "unavoidable adverse effects" due to the loss of these recreation facilities. However, they have not provided details on how they plan to assist in this transition and the support they will provide the County of Lake.

## 3.10 Aesthetic Resources

- PG&E needs to incorporate the various proposed restoration activities to improve the aesthetic aspects of the decommissioning project. This consideration needs to be addressed within the NEPA and CEQA process. PG&E did not detail specific restoration plans or activities, once prepared the plans should be available for stakeholder comment.
- SLR recommends that PG&E include photo modelling/artist renderings of visual impacts following dam removal.
- PG&E will need to prepare an ecosystem restoration plan that considers management of invasive species, and reintroduction of native and endangered species, along with management and stabilization of the lake sediments. Monitoring should be considered to evaluate the progress of revegetation, and any needed adaptation until vegetation is established in the areas affected by the Project.
- PG&E would need to properly address the restoration of all areas and items referenced in their preliminary restoration plan to the landowners' and the County's agreement.
- PG&E does not describe in this section how the Scott Dam area recreation facilities would be restored such as the campground, day-use areas, and associated roads and trails.
- Removing water bodies has a significant visual impact. Anything that is restored should be visually appealing. Small ponds and hiking trails could help offset these visual impacts and should be included in a restoration plan.
- PG&E argues that no adverse effects to visual resources have been identified for Phase 2 of the Proposed Action. During Phase 2, visual resources will be impacted until final

restoration is implemented. PG&E should note this impact and propose offsets. PG&E argues the overall impact would be temporary adverse effects during construction and long-term benefits as the rivers return to natural conditions and vegetation recovers. PG&E needs to provide a better justification for the offsets indicated with respect to the aesthetic aspects of the decommissioning project over a sufficient timeframe to allow for seed collection and responsible revegetation, as well as mitigation of invasive species.

## 3.11 Cultural Resources

- PG&E's proposed Historic Properties Management Plan will need to be reviewed for compliance and consistency with Federal and State environmental review requirements.

## 3.12 Tribal Resources

- PG&E's proposed Tribal coordination will need to be reviewed for compliance with and meeting Federal and State environmental review requirements.

## 3.13 Socioeconomic Resources

- PG&E has not quantified Operation and Maintenance costs associated with areas that will be restored, where sediments will be disposed of, and where dam debris will be contained.
- PG&E states that the community way of life for residents surrounding Lake Pillsbury will likely be adversely affected both during and post construction. PG&E should identify proper compensation for current residents that may be impacted.
- PG&E states that river-based recreation, in the long term, may provide similar economic opportunity to reservoir-based recreation. PG&E also provided a study that "estimated that decommissioning of the Project would extend the rafting and kayak season on the Eel River by approximately 6 weeks…", which appears to be for a portion of the Eel River below Cape Horn Dam, and not for the Scott Dam area. With the absence of road infrastructure to access the river, any potential rafting and kayaking activity near the Scott Dam area will be further reduced. Given that the loss of Lake Pillsbury will cause lost revenue from reservoir-based recreation for the County of Lake, and that the potential increase in river-based recreation may happen outside of county limits, PG&E will need to work with the County of Lake to identify the potential revenue loss for the County caused by the change in recreational activities from reservoir-based to river-based and conduct additional studies to identify mitigation measures associated with recreational activities for the watershed.
- PG&E states that "for recreationists who prefer lake-based activities", Lake Mendocino or Clear Lake "can provide substitute recreation opportunities." As many lake-based recreationalists from higher population centers currently travel to Lake Pillsbury from the west, most will likely go to Lake Mendocino instead. This would result in a loss of tourism and revenue to the County of Lake. Furthermore, Lake Pillsbury appeals to recreationists due to its remoteness, which these alternative lakes may not provide. PG&E has not accounted for this revenue loss.
- PG&E also states that "potential adverse effects can be offset with restoration of vegetation and site aesthetics (see the Restoration Plan) and with investment in river-based recreation infrastructure by local businesses." PG&E has not yet provided a Restoration Plan and appears to assume that restoration will involve investment by local communities. River based recreation infrastructure as part of the Restoration Plan should

be prepared and set up funded by PG&E. Furthermore, PG&E has not indicated if the lands will remain under PG&E ownership or if it will be transferred to other parties, therefore it is not clear who could or should develop the river-based recreation infrastructure.

- PG&E ties the hypothetical fish abundance to an increase in recreational activities by stating that if "fish abundance in the Eel River were to improve in response to the Proposed Action and return to natural conditions, then economic opportunity associated with commercial and recreational fishing could also increase." This statement should be supported by additional studies to be performed to identify whether restoring the river is going to lead to additional fishing and if this would overcome the loss of lake fishing at Lake Pillsbury.
- On Volume II page 2.2.4.14-26, PG&E details there are approximately 300 recreational homes and cabins surrounding Lake Pillsbury. The study provides an average assessed value of the properties at $253,000 and then goes on to apply 1.0 to 1.112% annual property tax rate or "up to approximately $285 annually per home". There is math problem where 1.112% of $253,000 is $2,833 and the annual taxes paid is nearly $850,000 not the $85,000 value presented by PG&E.
- In Volume II Section 2.2.3.13.6 of the application - Local Government Finance, PG&E have identified current revenue from property tax, lodging tax, sales tax and licenses, but have not evaluated how the lodging tax, sales tax, and licenses will be affected long-term. PG&E should consult with the County of Lake to obtain the correct tax information and incorporate it into the analysis.
- In Volume II Table 2.2.3.13-3 of the application, PG&E shows that the population of Lake County has a higher unemployment rate, poverty rate, and lower median income than the state average. PG&E has failed to quantify the effect of dam removal on these key economic indicators. These impacts need to be quantified, with mitigation measures identified and implemented by PG&E.
- PG&E should evaluate if this is an Environmental Justice community and recommend strategies to mitigate impacts on this group. This evaluation should be focused on the population in the region impacted by the Proposed Action, and not on County averages.
- In Volume II Sections 2.2.4.7 and 2.2.4.10.2, PG&E acknowledges the removal of Lake Pillsbury reservoir could reduce groundwater levels in the vicinity of the reservoir, thereby affecting local groundwater wells; and that lower groundwater levels may result in "higher pumping costs; the need for newer, stronger pumps; and in worst-case scenarios, the need to drill deeper wells." PG&E fails to account for this economic impact in the socioeconomic resources (Volume II Section 2.2.4.14) and incorrectly states "there are no expected effects on water reliability or cost in the Scott Dam Area" because the evaluation was only related to surface water and not the main source for the area which is groundwater. PG&E should list the potential effects on water reliability and cost as an adverse effect and proposed mitigation or funding options for the affected users.

## 3.14 Air Quality

- The construction plans proposed by PG&E should address emissions from construction vehicles and other greenhouse gas aspects. No specific plans or statements on addressing emissions from construction were identified in the application.

- Additional details on the monitoring of airborne NOA and other asbestos will be provided due to the presence of serpentinite rock in project construction areas and potential building demolition.

## 3.15 Noise

- The construction plans proposed by PG&E should address noise and vibration from construction equipment and vehicles.
- Demolition activities associated with the Project would generate ground borne vibration from the operation of heavy equipment. As with noise, vibration levels would vary depending on the type of equipment used and the distance to receptors.

## 3.16 Traffic

- PG&E documented existing road infrastructure, transit and aviation facilities in the vicinity of Scott Dam and Lake Pillsbury area. However, the impact on access road infrastructure was limited to three access roads. PG&E should include all access roads and other infrastructure to facilitate the future restoration and use of the facilities.
    - Scott Dam Area and Cape Horn Dam Roads documented and listed, but there were no specific plans on how the infrastructure would be restored.
    - The Transportation and Circulation Element of the Lake County General Plan also includes goals and policies that may be applicable to the Project (Lake County 2008). The relevant policies are listed below:
        - Policy T-1.8. Level of Service. County maintained roadways should be improved and maintained to provide an adequate peak period Level of Service (LOS) of "C" or better for existing and anticipated traffic volumes if roadway upgrades are feasible, such as roadway widening, addition of lanes via re-striping, and other safety and operational improvements. The County shall allow a limited number of County roadway segments to operate at a level of service of "E" or better where improving the segments to LOS C are deemed infeasible due to cost, negative community and/or environmental impacts, and/or constructability issues. This "E" level of service for certain roadways shall not include any State Highway unless approved by Caltrans.
        - Policy T-1.9. Truck Routes. To reduce heavy truck traffic in residential areas and near noise sensitive land uses, the County shall ensure truck routes are designated in a manner such that traffic noise impacts are minimized.
        - Policy T-1.10. Construction Methods. The County shall utilize road construction methods that seek to reduce air, water, and noise pollution associated with road and highway development.
- Regarding the impacts of dam removal, PG&E will need to prepare inundation maps with respect to releases during dam drainage and removal, and evaluate the potential effects on road infrastructure, including road easements, culverts, bridges and other utilities.
- Once these effects are sufficiently evaluated, road and other infrastructure improvements deemed necessary to provide for consistent opportunity for ingress/egress by emergency services (*including United States Forest Service personnel*) and residents of/visitors to the area would need to be conducted prior to removal of Potter Valley Project Dams and Equipment. The application notes "[*sediment*] deposition can reduce the capacity of the

river and floodways to transport water during periods of high rainfall, which can potentially cause localized flooding in areas that were previously unaffected. These changes can heighten the risk of flooding at downstream structures, such as bridges and road crossings. A Sediment/Channel Monitoring and Response Plan is proposed to monitor problematic levels and interventions.

- Roadwork improvements and modifications will be needed to accommodate the anticipated increased construction traffic and heavy equipment movement on local roads during the dam decommissioning. The general construction measures will include speed limits and dust control. PG&E proposes preparing a Construction Transportation Management Plan and following the General Construction Measures to mitigate transportation network effects. The construction transportation plan needs to include non-construction-related traffic that is expected to be present in the vicinity of Lake Pillsbury. The general maps presented in the Application do not provide the level of detailed required for assessment. Roadwork improvements or modifications to accommodate heavy equipment need to be specified.
- There is no detailed assessment of road capacity, load-bearing limits, or required upgrades to support increased construction traffic.
- There is no detailed assessment of road capacity, load-bearing limits, or required upgrades to support increased construction traffic.

## 3.17 Marine Resources

- Impacts on Marine Resources are outside of the scope of this review.

## 3.18 Others

**Cumulative Effects due to Climate Change**

PG&E's assessment of climate vulnerability in the Final Application for Surrender of License identifies several climate-related stressors. However, its evaluation lacks the structure, depth, and geographic specificity necessary to support informed decision-making. Specifically, PG&E's assessment:

- Does not follow a standardized vulnerability assessment framework.
- Omits key geographic areas, including Lake County, from its hydrologic and socioeconomic analysis.
- Fails to utilize Cal-Adapt or other state-supported climate modeling tools.
- Provides limited evaluation of social vulnerability and equity impacts.
- Focuses primarily on fish passage and geomorphic restoration, without assessing broader resilience strategies.

To strengthen its climate vulnerability assessment and the overall cumulative effects of the Proposed Action, PG&E should:

1) Adopt a structured framework such as the APG to evaluate exposure, sensitivity, and adaptive capacity.
2) Use Cal-Adapt and other state-supported tools to model future climate conditions and stressors.

3) Include the County of Lake in its hydrologic and socioeconomic impact analysis, given Lake Pillsbury's role in regional water storage, drought mitigation, fire suppression and the economic benefits it provides.
4) Evaluate impacts on vulnerable populations, including Tribal communities, low-income residents, and those dependent on agriculture and recreation.
5) Quantify and compare greenhouse gas emissions under existing and proposed conditions, including long-term carbon sequestration potential.
6) Assess recovery timelines and resilience for geomorphic and ecological systems.
7) Explore alternative water storage or conservation strategies to offset the loss of reservoir capacity.
8) Engage with local stakeholders and Tribal communities to co-develop adaptation and resilience strategies.

Without these additions, PG&E's climate vulnerability assessment remains incomplete and insufficient to address the full scope of risks associated with dam removal and climate change in Lake County. We request FERC to include the climate vulnerability assessment as required plan in the order to be issued by the FERC.

**Power System Impacts:**

- PG&E will need to discuss the existing power grid in the area and provide mitigation measures for any impact on the power grid due to Scott Dam and Pillsbury Lake removal. PG&E notes that the project powerhouse has not generated power since May 2021, and the decommissioning would permanently eliminate generation capabilities. The Final Application for Surrender of License does not explicitly state whether the vicinity of Lake Pillsbury is connected to the electrical grid.

# 4.0 Closure

SLR appreciates the opportunity to support the County of Lake on this important project. Please contact the undersigned if you have questions.

Regards,

**SLR International Corporation**

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# ATTACHMENT 4

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STATE OF CALIFORNIA
CALIFORNIA NATURAL RESOURCES AGENCY
DEPARTMENT OF WATER RESOURCES
DIVISION OF SAFETY OF DAMS

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101 County Lake
Type of Dam Gravity Type of Spillway Gated overpour
Water is 13.6 feet below the spillway crest at elevation 1900', and 38.18 feet below the dam crest.
WSE = 1886.4
Weather Conditions Sunny and warm
Contacts Made Matthew Colwell (PG&E)
Reason for Inspection Maintenance Inspection

## Important Observations, Recommendations or Actions Taken

In a letter dated March 17, 2023, PG&E submitted the results of a simplified seismic stability analysis of the dam to DSOD. Based on the analysis, the dam may become structurally unstable when subjected to seismic loading resulting from ground motions with an estimated return period of 850 years. The analysis also indicated that the potential for seismic instability is lower when the reservoir level is at or below the spillway crest elevation. As an interim risk reduction measure, PG&E, with concurrence from DSOD, has implemented a [redacted] foot restriction on the maximum reservoir operating level. [redacted]

PG&E's reported long-term plan for this facility is to decommission the dam as a part of surrendering the FERC license for the Potter Valley Hydroelectric Project. Details of the decommissioning are being developed.

## Conclusions

From the known information and visual inspection, the dam, reservoir, and the appurtenances are judged safe for continued use.

## Observations and Comments

**Dam**

The visible portions of the upstream and downstream faces of the concrete appeared weathered but in acceptable condition. PG&E recently made surface repairs to areas of poorly consolidated concrete on both downstream training walls and the downstream face as a part of routine maintenance (Photo 2).

Vegetation control along the downstream face, toe, and abutments is generally satisfactory. PG&E continues to remove vegetation annually and no concerning vegetation was observed.

There were no apparent signs of fresh movement on the upstream left abutment, which is continually monitored by survey monuments and inclinometers. The most recent instrumentation report, which was for calendar year 2022, indicated that the landslide continues to creep gradually and parallel with the dam.

The lower seepage [redacted] was inspected during this inspection. [redacted] The upper seepage [redacted] was not inspected during this inspection.

**Spillway**

The spillway approach and exit channel were clear of obstructions, and the log boom located several hundred feet upstream was in place.

All [redacted] gates were cycled during this inspection. [redacted] gates [redacted] were cycled [redacted], which performed satisfactorily (Photo 3). [redacted] gate [redacted] was cycled [redacted] which is scheduled to be retired sometime in the future. The [redacted] gates were cycled

Photos taken? Yes X No
cc for Owner/Book

Inspected by M. Holmes MH 11/1/2023
Date of Inspection October 18, 2023
Date of Report November 1, 2023 RG 11/1/2023

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STATE OF CALIFORNIA
CALIFORNIA NATURAL RESOURCES AGENCY
DEPARTMENT OF WATER RESOURCES
DIVISION OF SAFETY OF DAMS

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102 County Mendocino
Type of Dam Earth / Gravity Type of Spillway Overpour
Water is 0.27 feet above the spillway crest at elevation 1490.5 and 28.23 feet below the dam crest.
WSE = 1490.77
Weather Conditions Sunny and cool
Contacts Made Matthew Colwell (PG&E)
Reason for Inspection Maintenance Inspection

## Important Observations, Recommendations or Actions Taken

PG&E's reported long-term plan for this facility is to decommission the dam as a part of surrendering the FERC license for the Potter Valley Hydroelectric Project. Details of the decommissioning are being developed.

The aquatic vegetation on the water side of the retaining wall needs to be removed as a part of annual maintenance (Photo 1).

## Conclusions

From the known information and visual inspection, the dam, reservoir, and the appurtenances are judged safe for continued use.

## Observations and Comments

**Dam**

***Embankment***: The visible portions of the upstream and downstream faces, the crest, and the abutment contact remain in satisfactory condition with no indication of surficial distress or instability. The rock riprap protecting the lower reaches of the downstream face is in good condition (Photo 4). The concrete retaining wall appeared in satisfactory condition; the aquatic vegetation on the water side of the retaining wall needs to be removed as a part of annual maintenance (Photo 1). Vegetation control on both upstream and downstream faces is satisfactory. Rodent control is satisfactory, and no burrowing activity was noted.

***Concrete Gravity***: The downstream face was not visible due to water flowing over the spillway. Past inspection reports note that the stepped chute on the downstream face has erosion and deep pits in the concrete; this is not an immediate dam safety concern. The visible portions of the left abutment contact remain in satisfactory condition with no indication of surficial distress or instability (Photo 3).

**Spillway**

The spillway approach, control section, and channel were clear of obstructions. The reservoir was full and spilling at the time of inspection (Photos 2 & 3).

PG&E, along with their engineering consultant, GEI, have completed an engineering study to analyze and evaluate potential improvement alternatives for the spillway at Van Arsdale. The study was completed as part of PG&E's Spillway Assessment and Improvement Program (SAIP). PG&E has prioritized long-term spillway improvements at this dam within their overall dam portfolio. PG&E will provide results for the evaluation and provide a plan and schedule for further actions to improve the spillway. PG&E will also provide a plan and schedule for developing, implementing, and maintaining interim risk-reduction measures, as necessary, until long-term improvements are completed.

**Outlet**

[redacted] gates were fully cycled during the February 8, 2023, inspection and were found to be in satisfactory operating condition.

Photos taken? Yes X No
cc for Owner/Book

Inspected by M. Holmes MH 11/1/2023
Date of Inspection October 18, 2023
Date of Report October 31, 2023 RCB 11/1/2023

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

**Observations and Comments**

The original [redacted] valve is reportedly buried under silt and is no longer operable. [redacted].

Seepage Two 90-degree v-notch weirs, designated E3A (lower elevation) and E3B (upper elevation), are located at the toe of the embankment. Weir E3B is nearest the right groin and captures runoff and flow from groundwater springs within the adjacent hillside (Photo 5). Weir E3A also captures precipitation and may intercept spring runoff or high tailwater. It is not known if any of the flow to Weir E3B is true seepage or what percentage of the flows at E3A are seepage.

Flow at E3A was approximately 6 gpm, and flow at E3B was approximately 52 gpm. All seepage was clear and within historical ranges. Continue to remove vegetation surrounding the weirs to ensure flow through the weirs are unobstructed and accurate weir measurements can be taken.

Instr. Instrumentation consists of the following:

- Five (5) open standpipe piezometers located on the embankment portion of the dam.
- Six (6) survey monuments along the crest of the concrete retaining wall, installed in 2022.
- Two (2) seepage-monitoring weirs. The two 90-degree v-notch weirs are designed to monitor seepage through the downstream face and left groin.
- One (1) reservoir staff gauge.
- One (1) automatic reservoir water level monitoring [redacted]

The most recent combined DSSMP/DSSMR for record year 2022 was submitted in March 2023. I reviewed the report and am generally in agreement with the owner's evaluation of the data/plots as described below:

*Piezometers*: In 2022, all piezometer readings were within the historical range and at or below their respective Threshold and Action levels. Piezometers designated 1-2, 1-3, and 2-2 appear to be unreliable; 1-2 because it reads above the reservoir elevation; 1-3 because the tip is located at elevation 1498.5 ft, which is above the normal max water surface elevation at 1494.3 ft, and the readings are above the tip elevation; and 2-2 because it reads below its tip elevation and should be recorded as dry.

*Survey Monuments*: The six survey monuments were installed along the crest of the retaining wall in December 2022 in response to the 2020 Part 12D recommendation R-2. An initial survey was completed on December 28, 2022, with the initial positions. Movements will be plotted upon completion of a second survey.

*Weirs*: Overall, the weir seepage readings were within the historical range. On January 12, 2022, weir E3B exceeded the Threshold Level during a rain event. It is believed that subsurface flow from the adjacent hillside is the major contributor to the weir flow and that the reading is within normal range.

The owner's conclusion from their March 2023 submittal were that "Based on the instrumentation evaluations included in this DSSMR and the visual inspections by PG&E, FERC, and DSOD, the dam is judged to be performing satisfactorily. The project is suitable for continued safe and reliable operation. The dam surveillance and monitoring program is adequate for purposes of monitoring the normal performance of the dam." Based on the data submitted, I agree with the owner's conclusion.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 1: Vegetation on the water side of the concrete retaining wall needs to be removed as a part of annual maintenance.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 2: View of the concrete overpour, retaining wall, and embankment as seem from the left abutment.

Photo 3: View of the concrete overpour as seen from the concrete retaining wall.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 4: View of the downstream face of the embankment as seen from the toe.

Photo 5: View of weir E3B (upper); continue to clear vegetation and debris surrounding the weir.

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

[redacted]. All gates were cycled with no issues.

PG&E is operating the dam and gates with the year-round [redacted] reservoir restriction. [redacted] The gate configuration is in compliance with the Certificate of Approval.

Outlet

There is no upstream control for this dam. [redacted] valve [redacted] valve were last successfully cycled in DSOD's presence on April 27, 2022, with no apparent issues.

A lower sluice blocked on the upstream end [redacted] gate has been abandoned with the controls removed because it is buried under approximately 14-feet of silt. The sluice outfalls at the center base of the spillway chute.

Seepage

Seepage is monitored at three drainpipes located below the left abutment, labeled E1A, E1B, and E1C. E1A was dry, and E1B and E1C were estimated to be flowing less than 1 gpm. All flows were clear and within historical limits.

Minor historical seepage has been reported on the upper left downstream face at two locations along construction joints. No live seepage was observed at either joint and no standing water was visible, which is typical for the current reservoir elevation.

Seepage in the [redacted] measured through a v-notch weir [redacted]. PG&E routinely inspects and clears the sump and weir of sludge buildup. Mr. Colwell removed sludge buildup on the weir and tested the sump pump during this inspection, which was found to be in satisfactory operating condition. The weir reads accurately during low flows but reads erratically during high flows. There is currently no staff gage, and the weir cannot be read manually.

Instr.

Instrumentation consists of the following:

- Thirty (30) piezometers, located within the dam and left abutment.
- Three (3) drainage pipe type weirs located at the left abutment.
- One (1) v-notch weir [redacted].
- Sixty-two (62) survey monuments on the dam and abutment areas for measurement of vertical and horizontal movement. Ten (10) of the monuments were installed in the slide area near the left abutment.
- Eleven (11) inclinometers located in the dam and left abutment area (six are no longer read).
- Two (2) seismographs
- Five (5) tiltmeters; one on the left abutment and four [redacted]
- Three (3) sets of crack pins installed on the outside face of the dam to measure cracking.
- Two (2) crack meters, located in the upper [redacted]
- One (1) bubbler-type gage to monitor reservoir level.
- One (1) tailwater sensor
- One (1) automatic reservoir water level monitoring [redacted]

The most recent combined DSSMP/DSSMR was submitted in May 2023 for record year 2022. In October 2020 the monitoring data management system was replaced, creating inconsistencies

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

between some old and new instrumentation data set. I reviewed the report and am generally in agreement with the owner's evaluation of the data/plots as described below:

Piezometers:

- Piezometers 4C, 5F, 6C, and 6F ([redacted] were replaced in July 2020. A new vibrating wire sensor was installed for piezometer 4C.
- Piezometer 2C [redacted] has been reading above its threshold level by 2 feet since the dam's remote data collection system was replaced in October 2020. PG&E plans to re-validate the readings of these sensors throughout 2023.
- Piezometer 6C [redacted] exceeded the threshold and action level in 2022; erratic readings in the winter and spring were caused by an inconsistent valve closure schedule. PG&E plans to re-validate the readings throughout 2023.
- All other piezometers were below the Threshold Levels, Action Levels, and within the historical range.

Seepage:

- 2022 [redacted] weir readings continued to exceed the temporary action level of 4 gpm, which was established on limited data since the installation of the weir in 2014. Flow readings are unreliable during high flows due to the intermittent operation of the [redacted] sump pump. During low flows, the weir trends with the reservoir elevation and have been consistent since 2017. PG&E plans to evaluate potential alternatives for improving the reliability of the weir data for 2023.
- Readings from Weir E-1A reached threshold level on February 8, 2022. Readings from E-1B and E-1C were below Threshold levels in 2022 and within historical trends.

Survey Monuments:

- In 2016, PG&E completed their initial comparative LiDAR survey of the left abutment hillside, and it shows that the downstream hillside is creeping. The movement appears to be surficial in the upper ten to twenty feet of the slope; this matches what other instruments have indicated over recent history.
- Horizontal movements for the survey monuments located on the dam crest were within the Threshold Levels. Between 2021-2022, survey points L, M, N, P, Q, and S did not move. Survey point 218 moved slightly downstream (to the West and North), about 0.01 feet.
- Survey point L, located near the right abutment, has the largest downstream movement since 1969, equal to 0.092 feet.
- In 2022, the vertical movements were in the upward direction between 0.006 to 0.017 feet. Since 1970, the vertical movements range between 0.035 feet downward, to 0.031 feet upwards.
- The maximum horizontal movement of the crib wall since 1993 is approximately 0.21 feet.
- In 2022, the monuments within the left abutment landslide moved an average of 0.95 feet horizontally. The maximum downslope movement between 2021 and 2022 was approximately

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

0.02 feet at Monument 702. Overall, the movement within the slide is parallel with the dam.

Inclinometers

- Inclinometer SCTD, located in the left abutment slope above the crib wall, indicates movement of 0.5 inches in the uphill direction from 1986 to 2022. However, the upper 10 to 20 feet indicates surficial movement in the downhill direction. Surveys on monument 41, near SCTD, show apparent downhill movement. The uphill trend indicates the inclinometer is founded in the upper portion of the deep-seated slide.
- Inclinometer SCTE, located in the left abutment slope, indicates movement of 2.5 inches to the north (downhill) from 1986 to 2022. This movement is consistent with nearby survey monuments. The greatest deflection is seen in the upper 10 feet, indicating surficial movement downhill.
- Inclinometer SCTH, located upstream of the dam in the left abutment, show approximately 0.4 inches of downhill movement since 2014. The instrument is relatively shallow, with most of the movement within the top 20 feet of fill material.
- Erratic readings for SCTH and SCTG began occurring in July 2019. PG&E removed the automated inclinometers in July 2022.

Seismograph:

- There was no significant seismic activity that triggered seismic monitoring within the vicinity of Scott Dam during the 2022 record year.

Tiltmeters:

- There are four tiltmeters [redacted]. Three of the four tiltmeters are measuring transverse (upstream/downstream) movement. Tiltmeters 1T and 2T continue to respond to reservoir water surface fall by tilting upstream and reservoir rise by tilting downstream, as expected. Tiltmeter 2L measuring longitudinal (north/south) movement, did not measure any apparent change during 2022.
- There is one tiltmeter installed in the left abutment hillside. The hillside tiltmeter was determined to be not functional during a July 2021 site visit. A new hillside tiltmeter was installed at the same location in July 2022. The hillside tiltmeter continues to exhibit the apparent trends of cyclic year-to-year movement along the axis of the dogleg portion of the dam.

Crack Pins and Crack Meters:

- The three (3) sets of crack pins have deteriorated and no longer provide reliable measurements of change in crack width and were abandoned in place in 2017.
- Both [redacted] crack meters continue to exhibit a marked response to reservoir rise and fall. Both crack meters record a positive displacement (opening of the cracks) as the reservoir rises and a negative displacement (closing of the cracks) as the reservoir empties. CM-2 positive displacement was approximately 0.012 inches and CM-1 was approximately 0.011 inches.

The owner's conclusion from their May 2023 submittal were that "Based on the instrumentation evaluations included in this DSSMR and the visual inspections by PG&E, FERC, and DSOD, the dam is judged to be performing satisfactorily. This dam needs to be closely monitored under normal, flood,

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

and seismic conditions to detect any unusual instrumentation readings and trends relative to past behavior. The project is suitable for continued safe and reliable operation. The dam surveillance and monitoring program is adequate." Based on the data submitted, I agree with the owner's conclusion.

Photo 1: View of the upstream face as seen from the right abutment.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

Photo 2: View of the downstream face as seen from the left abutment.

[redacted]

# ATTACHMENT 5

# Technical Memorandum

| | | | |
|---|---|---|---|
| **To:** | The County of Lake | **From:** | Jason Fernet, PE, LEP<br>Dharme Rathnayake, Ph.D., PE |
| **Company:** | SLR International Corporation | | |
| **Project No.:** | 102.021249.00001 | **Date:** | November 24, 2025 |

**RE: Cost Estimate Scott Dam Rapid Removal**
**Potter Valley Hydroelectric Project, FERC Project No. 77**
**The County of Lake Resource Assessment, Impact Analysis, and adaptation Strategy Evaluation Project**

Pacific Gas and Electric (PG&E) proposes to decommission the Potter Valley Hydroelectric Project (PVP; Federal Energy Regulatory Commission [FERC] Proceeding No. 77). On July 25, 2025, PG&E submitted a Final Application for Surrender of License and Application for Non-Project Use of Project Lands to the FERC. PG&E's overall project goal is the decommissioning of the PVP as the hydropower generation is no longer economically viable. Scott Dam, which forms Lake Pillsbury in the Eel River Watershed, is one of two dams of the PVP.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. Scott Dam is located on the Eel River in the County of Lake, within the Mendocino National Forest. The decommissioning process considers removal of Scott Dam and draining Lake Pillsbury.

SLR has prepared an Association for the Advancement of Cost Engineering (AACE) Class 5 estimate, suitable for the rapid removal Alternative of Scott Dam based on the information provided in the following documents:

- Pacific Gas and Electric Company (PG&E) – Final Application for Surrender of License and Application for Non-Project Use of Project Land (Surrender Application) dated July 2025
- *Scott Dam and Cape Horn Dam Removal Alternatives* (Feasibility Study) prepared by McMillen Jacobs Associates dated November 2021.
- **AACE International Recommended Practice No. 56R-08**
  Cost Estimate Classification System – As Applied in Engineering, Procurement, and Construction for the Building and General Construction Industries
  AACE International, August 7, 2020.)

The AACE Class 5 estimate is the most conceptual level of cost estimating defined by the AACE International. It is typically used during the earliest stages of project development, such as feasibility studies or strategic planning, when only 0% to 2% of project definition is available. This estimate relies on high-level assumptions, historical data, and parametric models rather than detailed engineering or site-specific information. Due to its limited scope and high uncertainty, the expected accuracy range is broad—typically from −20% to +100%. Class 5 estimates are not

suitable for final budgeting or procurement but are valuable for initial decision-making and project screening.

The documentation listed above for Scott Dam provides only a high-level conceptual definition of the proposed removal project, accompanied by a broad timeline. No detailed construction means, methods, sequencing, engineering work packages or execution strategies were available to SLR for review. This limited scope of definition is consistent with the parameters outlined by AACE International for a Class 5 estimate, which is typically prepared during early feasibility phases and intended for order-of-magnitude cost forecasting rather than definitive budgeting. The Rapid Removal Alternative proposed by PG&E prioritizes speed, and the key components of this approach are as follows:

- **Adit Tunnel Construction**: A 15-foot diameter tunnel is built through the base of the dam's spillway to allow controlled sediment flushing and water release.
- **Dam Lowering via Barge Equipment**: Specialized floating equipment is used to dismantle the dam structure from the reservoir side.
- **Sediment Flushing**: Sediment trapped behind the dam is flushed downstream during high-flow events, reducing the need for mechanical dredging.
- **Final Dam Removal & Channel Restoration**: Remaining dam components are removed, and the river channel is rehabilitated to support ecological recovery.

Refer to Exhibit A for figures depicted Scott Dam removal plan overview and construction area from the Surrender Application.

The rapid removal plan assumes a sequenced approach that could be completed in a compressed timeframe, potentially within 2 to 3 years, depending on permitting, environmental conditions, and funding. Year 1 considers the initial mobilization, all permitting and design will be completed prior to Year 1, as presented in the Final Surrender Application, based on current timeline Year 1 is not earlier than 2028. The assumed high-level time is as follows:

- Year 1: Planning & Mobilization
  - Final engineering design and permitting (NOT INCLUDED IN THIS ESTIMATE)
  - Environmental compliance and stakeholder coordination
  - Mobilization of equipment and construction of access roads
  - Assembly and launch of floating plant and barges
- Year 2: Tunnel Construction & Initial Dam Lowering
  - Excavation of the 15-foot diameter adit tunnel through the spillway base
  - Installation of temporary tunnel supports and dredging at the portal
  - Begin lowering dam crest using barge-mounted equipment
  - Prepare for sediment flushing during high-flow season[1]
- Year 3: Sediment Flushing & Final Removal
  - Controlled sediment flushing through the tunnel during peak flows

[1] Sediment flushing costs are not included, pending PG&E's development of detailed sediment management plans.

- Continued dam lowering and removal of remaining structure
- Channel rehabilitation and turbidity control measures
- Demobilization and site restoration

The SLR cost estimate was prepared based on the above timeline. This estimate excludes costs for permitting, engineering design, legal services, and administrative overhead, focusing solely on construction-related expenses with respect to Scott Dam demolition. The unit prices used in the estimate were obtained from a combination of RS Means[2], published online sources, SLR estimates based on experience or estimates from the Feasibility Study inflated to 2025 dollars. Unit rates utilized from the Feasibility Study were reviewed and considered by SLR to be acceptable and consistent with industry standards. Inflation factor 20% from 2021 to 2025 based on Consumer Price Index inflation calculator from the U.S. Bureau of Labor Statistics. Table 1 provides a summary of the cost estimate in 2025 US dollars for the Project. The key assumptions used included in this estimate are as follows:

- Initial low flow season activities
  - Drawdown
    - Dewatering is expected to drawdown 40,000 acre-feet of water from the reservoir in approximately 150 days, requiring a minimum drawdown of approximately 135 cubic feet per second (cfs) without considering inflow.
    - Assume average inflow of 100 cfs.
    - The listed capacity of the needle valve is 400 cfs at high elevation with some decrease with drawdown.
    - Flow from needle valve appears sufficient to achieve drawdown without supplemental flow.
    - An estimate for management of drawdown activities was provided assuming a 2-person crew onsite for two 10-hour shifts per day.
    - Cost does not consider management of potential downstream impacts.
  - Adit Tunnel
    - Assume 75 linear feet of tunnel required, leaving 6 feet of tunnel on lake side as a plug.
    - Demolition of channel at toe of dam approximately 20 feet wide by 82 feet long by 15 feet tall for a total of approximately 911 cubic yards removed.
    - Tunnel equipment is mounted on barge.
    - Channel dredging completed with hydraulic dredging machine to remove approximately 3,900 cubic yards of sediment.
    - Sediment disposal offsite not included.
    - Debris from the toe demolition and tunnel construction crushed and used onsite.
    - Turbidity curtain changed monthly.

[2] RSMeans is a construction estimating database widely used in government projects.

- Refer to Exhibit B for depiction of proposed tunnel from the Feasibility Study.

o Road Improvements and Staging Areas

- New temporary road construction assumed to be 26 feet wide and 2,275 feet long. Road will require clearing grubbing, rough grading and placement of 12 inches of base material.
- Existing utility road and Scott Dam Road improvements assumed to be 40 feet wide and 4,245 feet and 6,400 feet long respectively. Roads will be regraded and an average of six inches of gravel-based material will be installed.
- Clearing and grubbing of 20 acres for the proposed staging areas.
- Refer to Exhibit A – Map 2-8a and Map 2-8b for a depiction of the above noted roads and staging areas.

o Demolition

- A barge mounted crane will be used to remove large appurtenances from top of dam.
- It is assumed the initial lowering of the dam will be done using an excavator equipped with a hydraulic hammer from a floating barge assembly.
- Assumed 16,000 cubic yards of concrete will be removed as part of this stage.
- Removed concrete will be crushed and reused onsite.
- Refer to Exhibit A Figure 2-2 for depiction of initial low flow season dam demolition.

- First High Flow Season
  - o Sediment Flushing
    - Only costs assumed here are the plug removal.
    - Cost associated with management of sediment flushing is not included as level of effort is unknown.
- Second low flow season
  - o Dewatering
    - Limited dewatering assumes given the dam has been lowered and the audit tunnel completed.
    - Costs assume one supplemental pump will be needed to manage water.
  - o Demolition
    - Demolition will be conducted from top of dam no barge is needed.
    - A total of 92,000 cubic yards will be removed, abandoning portions of the dam abutment per the Surrender Application.
    - Removed concrete will be crushed and reused onsite.
    - Refer to Exhibit A – Figure 2-3 for depiction of final demolished dam with portions remaining.

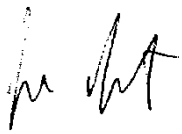
- Restoration
  - The following linear feet of river channel from the three rivers are assumed to require some level of contouring/restoration.
    - Eel River – 13,000 linear feet(LF)
    - Rice Fork – 9,000 LF
    - Salmon Creek - 15,000 LF
  - Assumed former lakebed will require soil stabilization. Estimate assumes up to 1,600 acres will require stabilization using wildflowers.
  - Temporary roads assumed to remain.
  - Grading of former lakebed not included.
  - Import of topsoil or soil amendments not included.

Indirect costs such as, general conditions, mobilization, overhead, profit, engineering, project oversight, bonding, and sales tax are applied to the direct construction costs. A **40% contingency** is included to account for unknowns related to geology, hydrology, and construction logistics. The offsite disposal of debris and sediment constitutes the most significant potential cost escalation factor, primarily due to the considerable volume of material requiring removal. The current estimates assume all material will be crushed and reused onsite.

The total estimated cost in 2025 dollars is approximately **$179 million**, with a potential accuracy range, as explained in the introductory paragraph, from **$143 million to $358 million**, depending on actual site conditions and implementation challenges.

Based on PG&E's 2027 General Rate Case prepared testimony dated May 15, 2025, the projected costs for decommissioning of the overall PVP project are $ 532 million. PG&E anticipates incurring physical decommissioning costs for the PVP during 2027, with work beginning in 2028. PG&E considered the expenses tied to PVP include not only decommissioning but also permitting, licensing, and maintenance costs. The PVP decommissioning includes not only the Scott dam removal and draining Lake Pillsbury but also the removal of Cape Horne Dam, some structures associated with diversion structures at Cape Horn, restoration around Cape Horn, and removal of the Potter Valley Powerhouse. Based on SLR's understanding of the potential scoped activities as listed in the PG&E's July 2025 Surrender Application and considering our Scott Dam removal estimate presented in this memo, PG&E's projected cost of $532 million is likely underestimated. In the next round of PVP decommissioning updates PG&E should provide a breakdown of costs and assumptions for each PVP component for better understanding of the total decommissioning costs.

Regards,



**Jason Fernet, PE, LEP**
Senior Principal

**Dharme Rathnayake, Ph.D., PE**
Senior Principal

Attachments Table 1 – Scott Dam Removal Cost Estimate Summary

Exhibit A – Figures from the Surrender Application

Exhibit B – Figures from Feasibility Study Depicting Adit Tunnel

# Table 1– Scott Dam Removal Cost Estimate Summary

**Table 1 - Scott Dam**
**Removal Cost Estimate Summary**

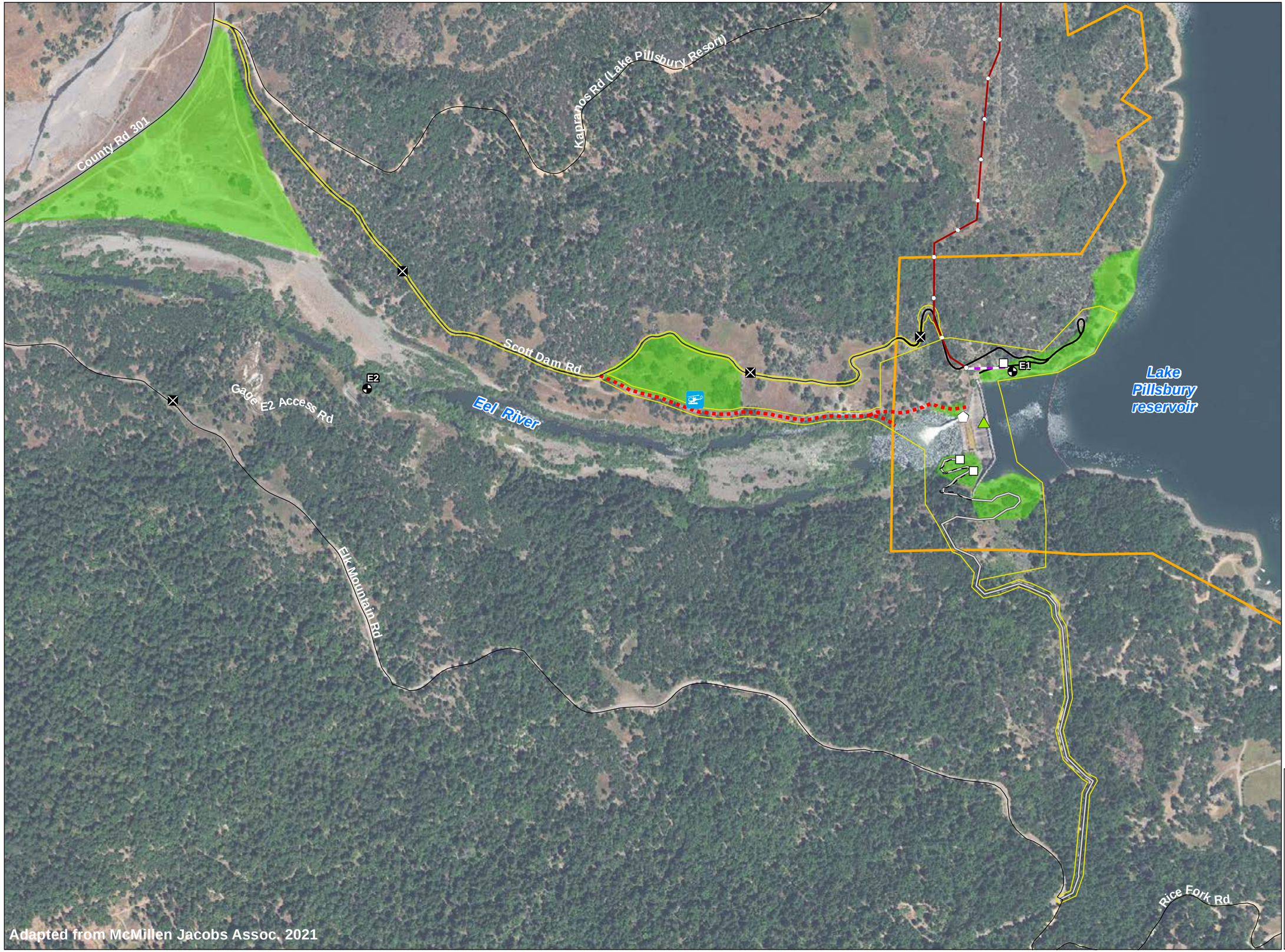
| NOTES | ITEM AND DESCRIPTION | ESTIMATED QUANTITY | UNIT | ESTIMATED UNIT COST | ESTIMATED TOTAL | REFERENCE |
|---|---|---|---|---|---|---|
| | **Initial Low Flow Season Activities** | | | | | |
| 1 | **Drawdown** | | | | | |
| | Management of Drawdown | 150 | day | $10,000 | $1,500,000 | SLR Estimate |
| 2 | **Adit Tunnel** | | | | | |
| | Tunnel Excavation | 75 | LF | $12,500 | $937,500 | RS Means 317119100220 and 317413100530 |
| | Toe Demolition | 911 | CY | $600 | $546,667 | Assumed per cubic yard cost as dam concrete removal. |
| | Channel Dredging | 3,900 | CY | $21 | $81,900 | RS Means 352413131100 |
| | Barge Launch | 1 | LS | $240,000 | $240,000 | $200K from Feasibility Study inflated from 2021 to 2025 |
| | Barge Rental | 5 | month | $20,000 | $100,000 | Assumes 5 months at $20K per month |
| | Turbidity curtain | 1,500 | LF | $32 | $48,000 | RS Mean 312514163010 Assume replacement on monthly basis |
| 3 | **Road Improvements and Staging Areas** | | | | | |
| | Temporary Road Construction - Clearing Grubbing | 1.4 | acre | $11,000 | $14,937 | RS Mean 311110100350 |
| | Temporary Road Construction - Grading and compaction native | 1.4 | acre | $42,000 | $58,800 | SLR Experience |
| | Temporary Road Construction - Base Material Purchase (12 inches) | 2,259 | CY | $55 | $124,227 | SLR Experience |
| | Temporary Road Construction - Base grading and compaction | 1.4 | acre | $42,000 | $58,800 | SLR Experience |
| | Existing Utility Road - Improvements | 3.9 | acre | $42,000 | $163,719 | SLR Experience |
| | Existing Utility Road - Base Material Purchase (6 inches) | 3,144 | CY | $55 | $172,944 | SLR Experience |
| | Existing Utility Road - Base grading and compaction | 3.9 | acre | $42,000 | $163,800 | SLR Experience |
| | Scott Dam Road Improvements | 6,400 | LF | $120 | $768,000 | Length determine based on Google Earth - cost from utility road improvements converted to LF. |
| | Staging Areas - Clearing and Grubbing | 20 | acre | $11,000 | $220,000 | RS Mean 311110100350 |
| | Staging Areas - Grading and compaction native | 20 | acre | $42,000 | $840,000 | SLR Experience |
| | **Demolition** | | | | | |
| | Barge Launch | 1 | LS | $240,000 | $240,000 | Potter Valley Feasibility Study plus 20% inflation |
| | Floating Plant Assembly and Rental | 1 | LS | $3,000,000 | $3,000,000 | Potter Valley Feasibility Study plus 20% inflation |
| | Demolition of Concrete and Crushing | 16,000 | CY | $600 | $9,600,000 | Potter Valley Feasibility Study plus 20% inflation, includes crushing |
| | Transport Concrete | 16,000 | CY | $6 | $96,000 | RS Mean 312323208190 |
| | Reuse of Concrete | 16,000 | CY | $15 | $240,000 | SLR Experience |
| | **First High Flow Season** | | | | | |
| | **Adit Tunnel** | | | | | |
| | Plug Removal | 1 | LS | $250,000 | $250,000 | SLR Estimate |
| | **Final Dam Removal Second Low Flow Season** | | | | | |
| 4 | **Dewatering** | | | | | |
| | Pumps (diesel fired rental 16,000 gpm each) | 1 | per | $90,000 | $90,000 | Sunbelt Rentals Online Price |
| | Piping | 1,000 | LF | $36 | $36,000 | RS Mean 331413253030 - Water Supply piping |
| | Fuel (13 GPH per unit) | 46,800 | gal | $5 | $234,000 | Average online pricing |
| 5 | **Demolition** | | | | | |
| | Demolition of Concrete and Crushing | 92,000 | CY | $600 | $55,200,000 | Potter Valley Feasibility Study plus 20% inflation, includes crushing |
| | Transport Concrete | 92,000 | CY | $6 | $552,000 | RS Mean 312323208190 |
| | Reuse of Concrete | 92,000 | CY | $15 | $1,380,000 | SLR Estimate |
| 6 | **Restoration** | | | | | |
| | Bank Restoration/Contouring | 37,000 | LF | $100 | $3,700,000 | Assumes distance of entire length of lake and plunge pool. Cost estimate |
| | Bank Stabilization | 1,600 | acre | $2,083 | $3,332,800 | RS Mean 329219145800 (47.79 per MSF) |
| | | | | | | |
| | **Subtotal** | | | | **$83,991,000** | |
| | **Component Taxed @ 7.875%** | | | | $6,614,291 | |
| | General Conditions and Mobilization | | | 20% | $16,798,200 | |
| | Overhead and Profit | | | 10% | $8,399,100 | |
| | Engineering Costs | | | 15% | $12,599,000 | |
| | Project Oversight Costs | | | 15% | $12,599,000 | |
| | Bonding | | | 3% | $4,230,018 | |
| | Contingency | | | 40.0% | $33,596,000 | |
| | **TOTAL** | | | | **$178,827,000** | |
| | | Low End Accuracy Range | | -20% | $143,060,000 | |
| | | High End Accuracy Range | | 100% | $357,650,000 | |

**Assumption Notes:**

1. This will require a flow of approximately 135 cfs not accounting for inflow. If we assume an average inflow of 100cfs over duration of drawdown period flow required is 235 cfs. The capacity of the needle valve is noted as 400 cfs at high elevation, with some decrease in flow as elevation drops. Therefore we are assuming no additional pumping needed.

2. Adit Tunnel assumes 75 feet of tunnel below the dam. Demolition of channel at toe of dam approximately 20 feet wide by 82 feet long by 15 feet tall for a total of approximately 911 cubic yards removed Some costs for this taken from Technical Memorandum, November 2021 - Scott Dam and Cape Horn Dam Removal Alternative and inflated from 2021 to 2025 dollars (~20% based on CPI inflation factor).
3. Road improvements assume 26 feet wide for temporary road and 40 foot wide for improvements to existing utility road and Scott Dam Road.
4 Significantly less dewatering required assumes only one pump needed toe supplement flow.
5. Assumed to be land demolition no barge required.
6. Assumes minimal contouring of river bank and soil stabilization of lakebed through hydroseeding of native wildflower mix.

# Exhibit A Figures from Surrender Application




## PG&E Project Facilities

- Dam
- Water Conveyance Feature
- Ancillary and Other Facilities
- Communication Line
- FERC Project Boundary

NOTE: Refer to Table 3.1 for the long-term fate of project facilities and features

## Other Features

- Access Road
- Access Road (proposed)
- General Access Road
- Other Road (existing)
- Access Trail
- Distribution or Powerline (Non-project)
- Gate
- Helicopter Landing Zone
- Construction Area
- Staging Area

Pacific Gas and Electric Company®

PG&E Potter Valley Project
FERC Project No. 77

**Map 2-8a**

**Scott Dam removal plan overview.**

0 125 250 500 Feet

Projection: UTM Zone 10N
Datum: NAD 83

Date: 7/15/2025

0566

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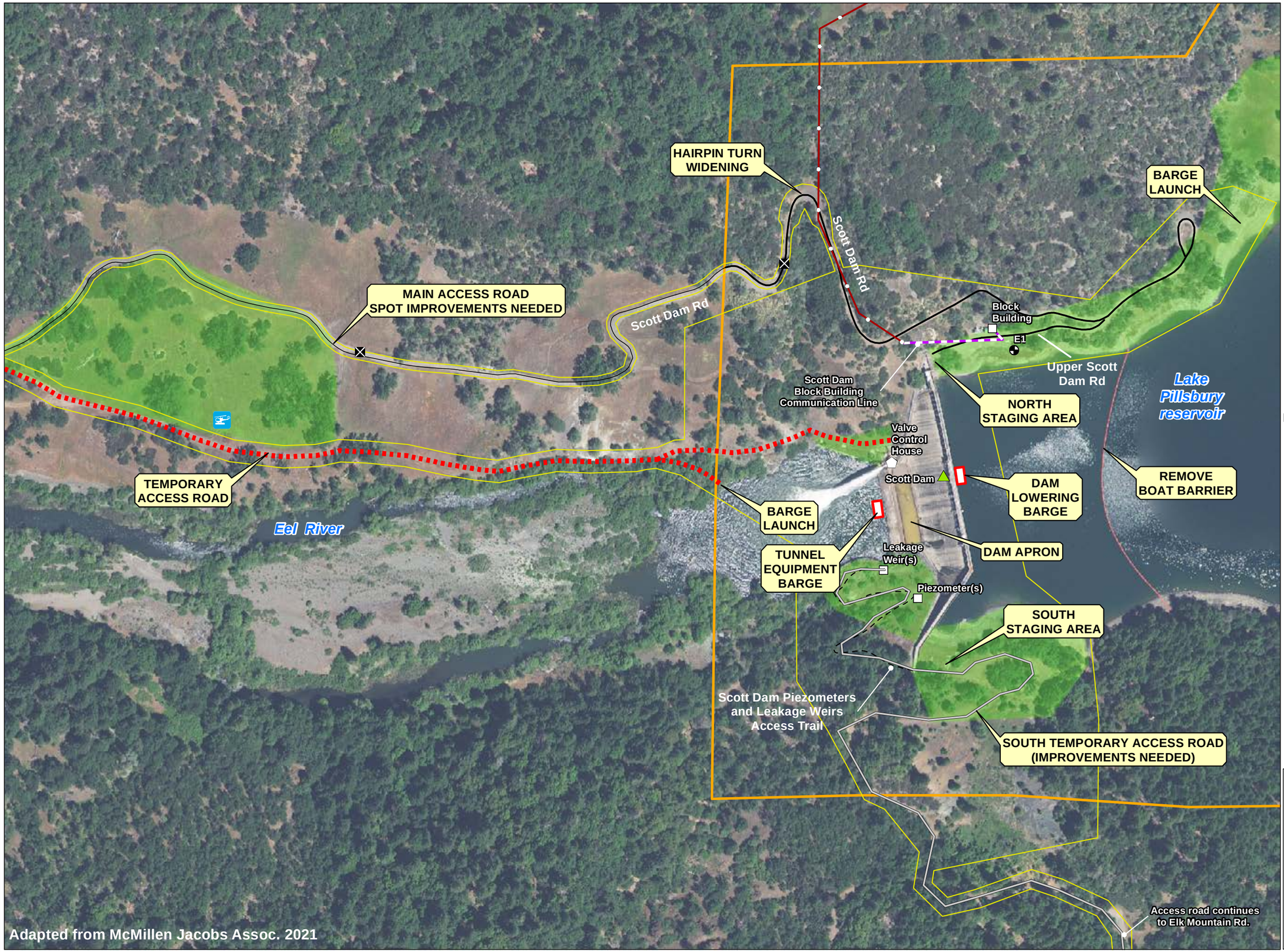


## PG&E Project Facilities

- Dam
- Water Conveyance Feature
- Ancillary and Other Facilities
- Communication Line
- FERC Project Boundary

NOTE: Refer to Table 3.1 for the long-term fate of project facilities and features

## Other Features

- Access Road
- General Access Road
- Access Trail
- Access Road (proposed)
- Distribution or Powerline (Non-project)
- Gate
- Helicopter Landing Zone
- Construction Area
- Staging Area

## Proposed Action Features Labels

Surrender of License Proposed Action @ Scott Dam

Pacific Gas and Electric Company®

PG&E Potter Valley Project
FERC Project No. 77

**Map 2-8b**

**Scott Dam removal plan construction area.**

0 50 100 200 Feet

Projection: UTM Zone 10 N
Datum: NAD 83

Date: 7/15/2025



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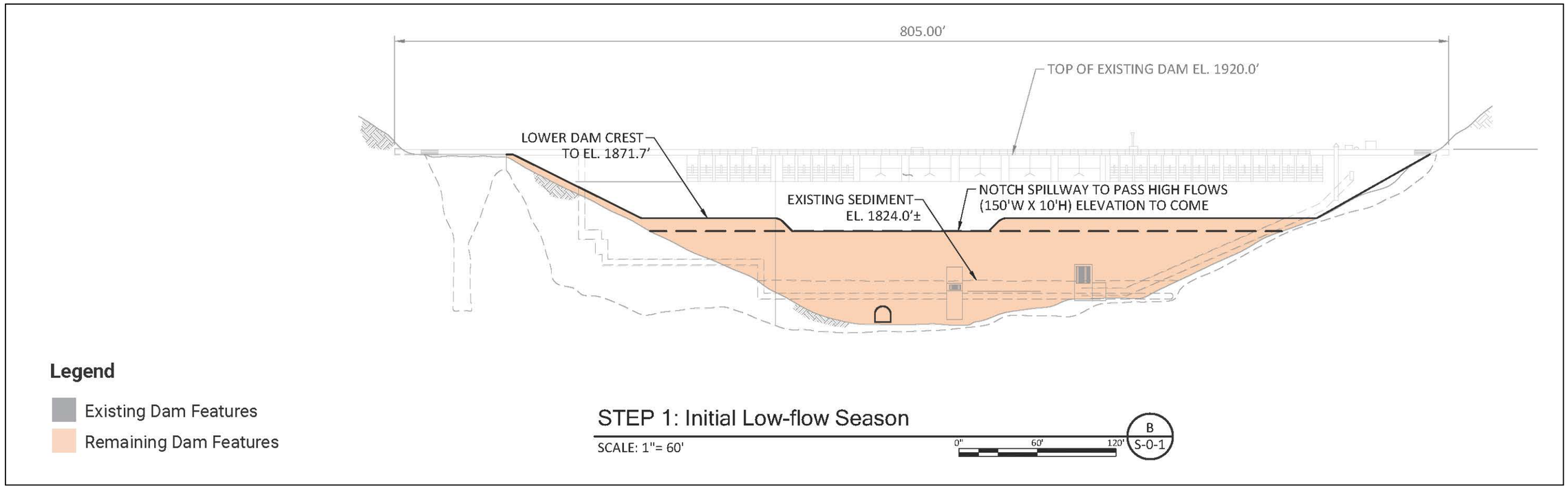



**Figure 2-2. Scott Dam removal—initial dam removal cross section (cross section—initial low-flow season).**


©2025, Pacific Gas and Electric Company

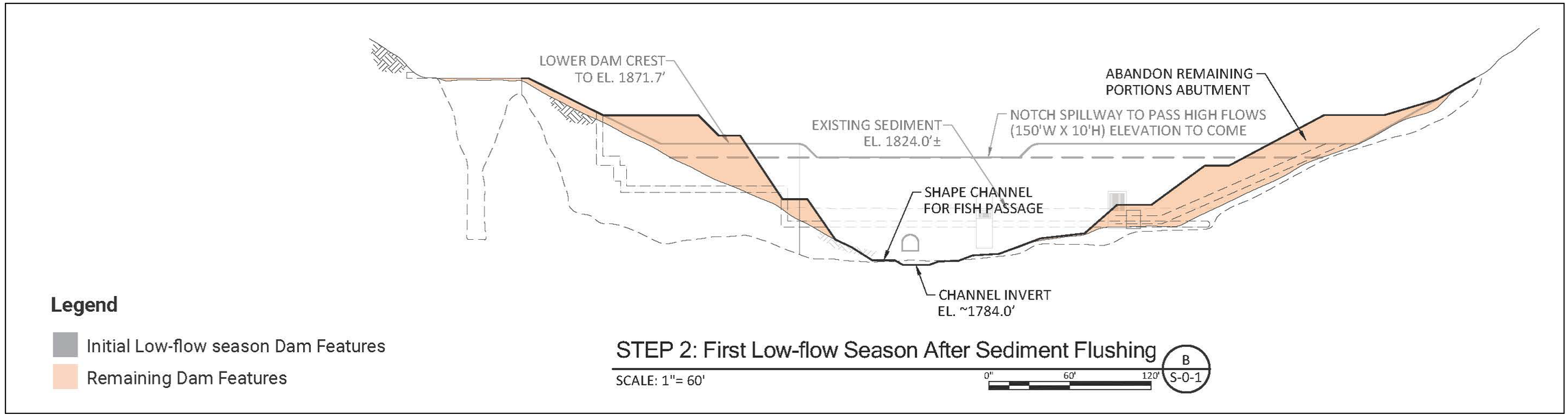


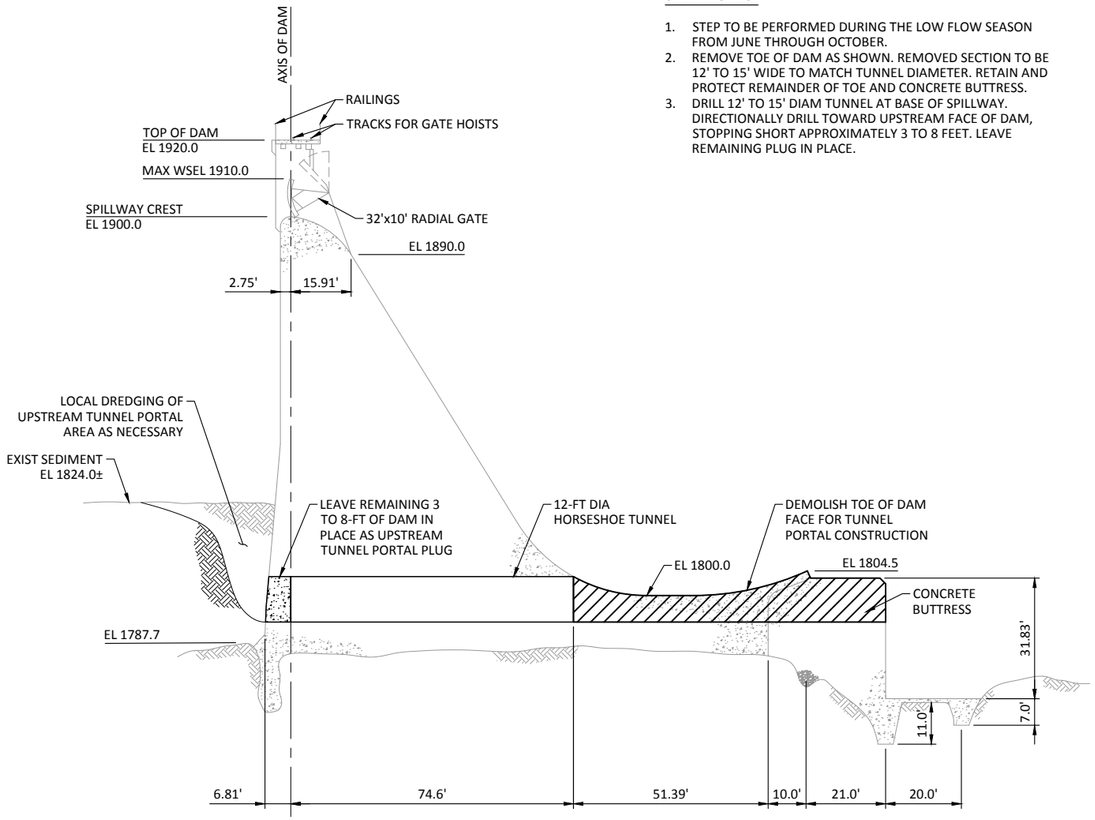

**Figure 2-3. Scott Dam removal—final dam removal cross section (cross section—first low-flow season after sediment flushing).**


©2025, Pacific Gas and Electric Company

# Exhibit B Figures from Feasibility Study Depicting Adit Tunnel

## STEP NOTES:

1. STEP TO BE PERFORMED DURING THE LOW FLOW SEASON FROM JUNE THROUGH OCTOBER.
2. REMOVE TOE OF DAM AS SHOWN. REMOVED SECTION TO BE 12' TO 15' WIDE TO MATCH TUNNEL DIAMETER. RETAIN AND PROTECT REMAINDER OF TOE AND CONCRETE BUTTRESS.
3. DRILL 12' TO 15' DIAM TUNNEL AT BASE OF SPILLWAY. DIRECTIONALLY DRILL TOWARD UPSTREAM FACE OF DAM, STOPPING SHORT APPROXIMATELY 3 TO 8 FEET. LEAVE REMAINING PLUG IN PLACE.




STEP 1 PROFILE
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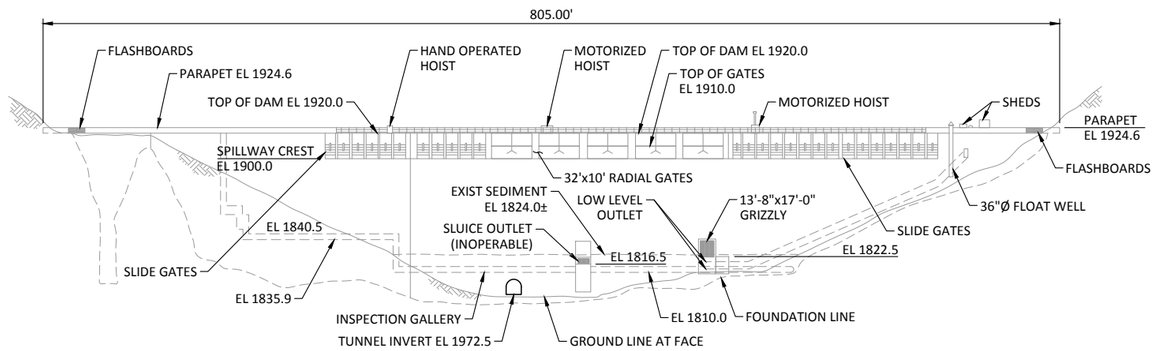
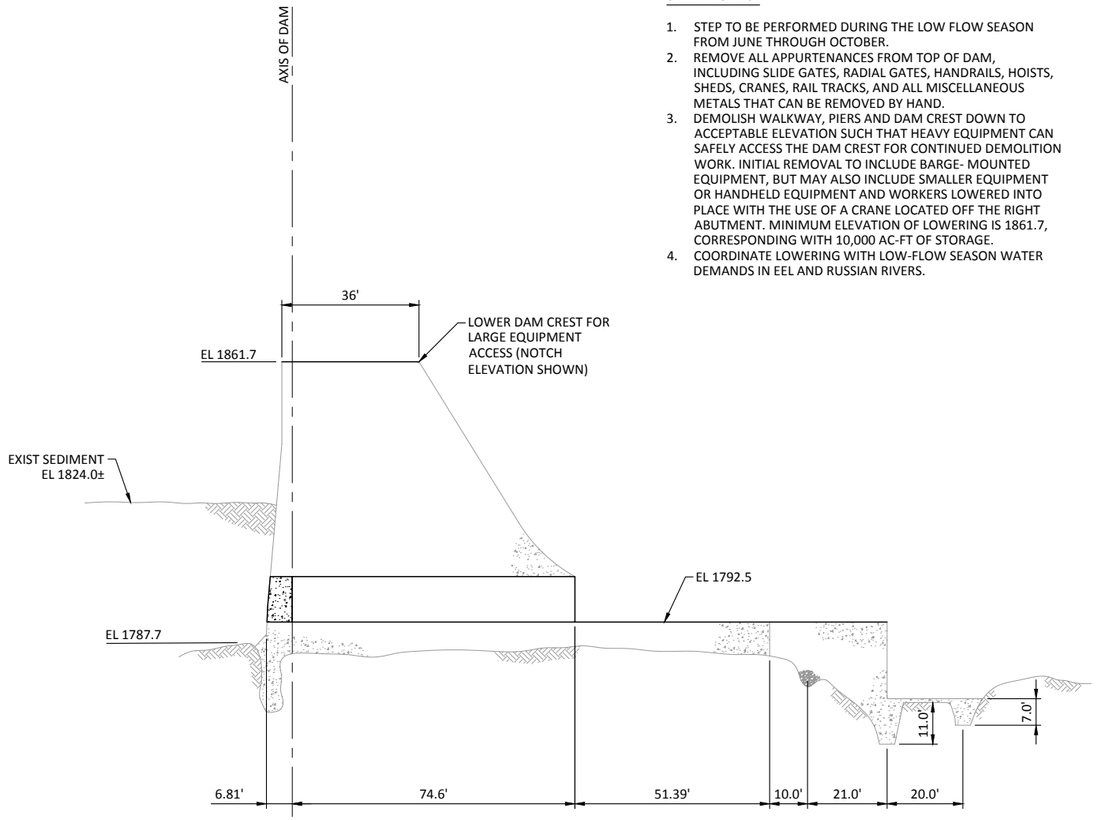
A S-0-1




STEP 1 SECTION
SCALE: 1"= 60'
B S-0-1

## STEP NOTES:

1. STEP TO BE PERFORMED DURING THE LOW FLOW SEASON FROM JUNE THROUGH OCTOBER.
2. REMOVE ALL APPURTENANCES FROM TOP OF DAM, INCLUDING SLIDE GATES, RADIAL GATES, HANDRAILS, HOISTS, SHEDS, CRANES, RAIL TRACKS, AND ALL MISCELLANEOUS METALS THAT CAN BE REMOVED BY HAND.
3. DEMOLISH WALKWAY, PIERS AND DAM CREST DOWN TO ACCEPTABLE ELEVATION SUCH THAT HEAVY EQUIPMENT CAN SAFELY ACCESS THE DAM CREST FOR CONTINUED DEMOLITION WORK. INITIAL REMOVAL TO INCLUDE BARGE- MOUNTED EQUIPMENT, BUT MAY ALSO INCLUDE SMALLER EQUIPMENT OR HANDHELD EQUIPMENT AND WORKERS LOWERED INTO PLACE WITH THE USE OF A CRANE LOCATED OFF THE RIGHT ABUTMENT. MINIMUM ELEVATION OF LOWERING IS 1861.7, CORRESPONDING WITH 10,000 AC-FT OF STORAGE.
4. COORDINATE LOWERING WITH LOW-FLOW SEASON WATER DEMANDS IN EEL AND RUSSIAN RIVERS.




STEP 2 PROFILE
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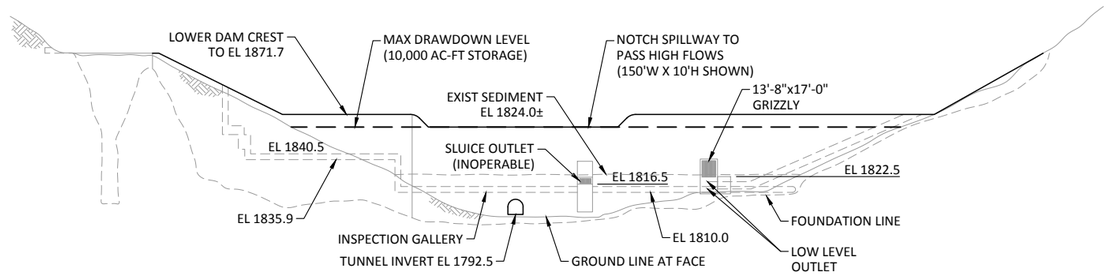
A S-0-1




STEP 2 SECTION
SCALE: 1"= 60'
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WARNING
IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE.

McMILLEN JACOBS ASSOCIATES

CALTROUT
POTTER VALLEY PROJECT FEASIBILITY STUDY
SCOTT DAM REMOVAL
RAPID REMOVAL SECTIONS 1

DESIGNED K. JENSEN
DRAWN R. GUERRERO
CHECKED M. McMILLEN
PROJECT DATE 07/14/21

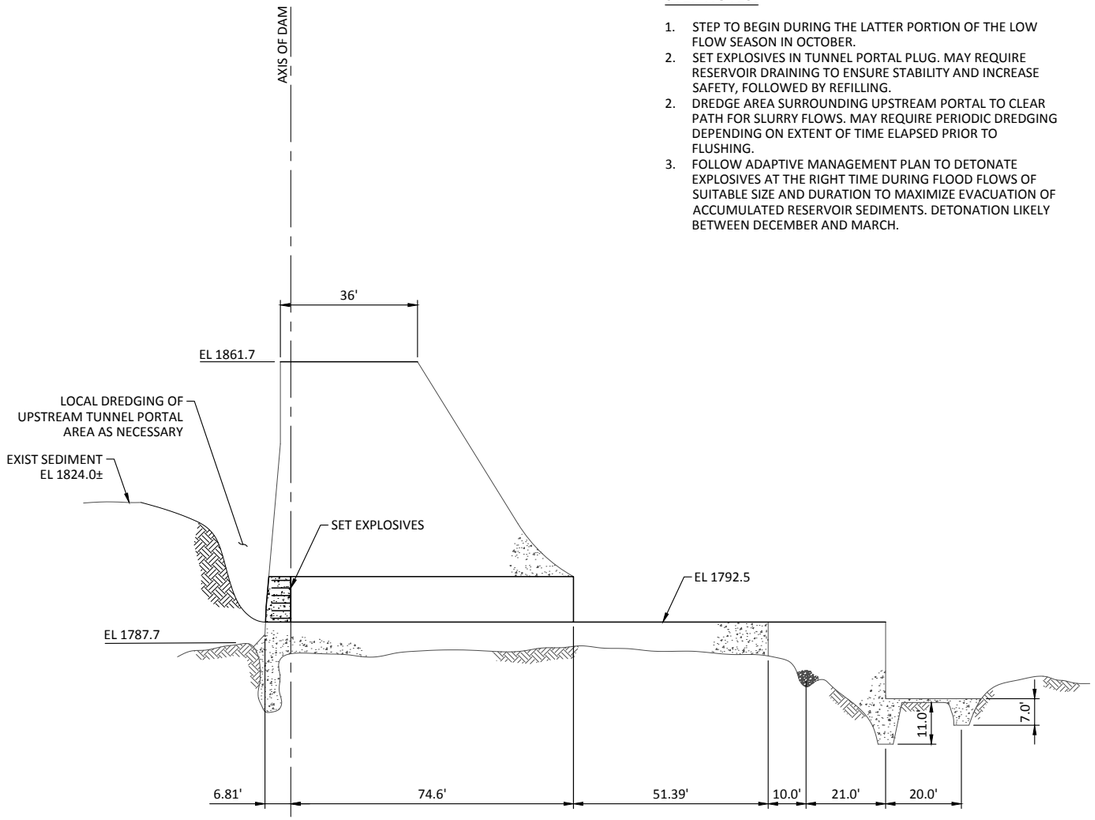
DRAWING
S-1-1
0571

JOB NO: 000000

Path: C:\Vault20\Sonoma County Water Agency\Potter Valley\S-1-1.dwg Plot date: Jul 12, 2021 08:23am, CAD User: Guerrero

STEP NOTES:

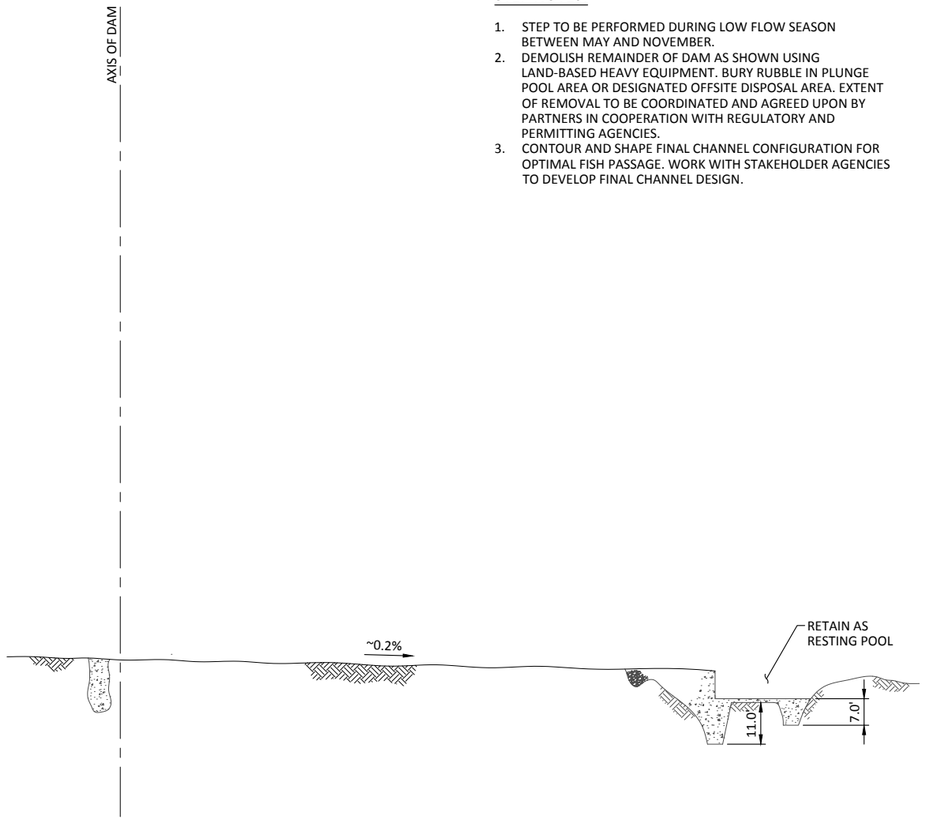
1. STEP TO BEGIN DURING THE LATTER PORTION OF THE LOW FLOW SEASON IN OCTOBER.
2. SET EXPLOSIVES IN TUNNEL PORTAL PLUG. MAY REQUIRE RESERVOIR DRAINING TO ENSURE STABILITY AND INCREASE SAFETY, FOLLOWED BY REFILLING.
2. DREDGE AREA SURROUNDING UPSTREAM PORTAL TO CLEAR PATH FOR SLURRY FLOWS. MAY REQUIRE PERIODIC DREDGING DEPENDING ON EXTENT OF TIME ELAPSED PRIOR TO FLUSHING.
3. FOLLOW ADAPTIVE MANAGEMENT PLAN TO DETONATE EXPLOSIVES AT THE RIGHT TIME DURING FLOOD FLOWS OF SUITABLE SIZE AND DURATION TO MAXIMIZE EVACUATION OF ACCUMULATED RESERVOIR SEDIMENTS. DETONATION LIKELY BETWEEN DECEMBER AND MARCH.




STEP 3 PROFILE
SCALE: 1"= 20'
A S-0-1

STEP NOTES:

1. STEP TO BE PERFORMED DURING LOW FLOW SEASON BETWEEN MAY AND NOVEMBER.
2. DEMOLISH REMAINDER OF DAM AS SHOWN USING LAND-BASED HEAVY EQUIPMENT. BURY RUBBLE IN PLUNGE POOL AREA OR DESIGNATED OFFSITE DISPOSAL AREA. EXTENT OF REMOVAL TO BE COORDINATED AND AGREED UPON BY PARTNERS IN COOPERATION WITH REGULATORY AND PERMITTING AGENCIES.
3. CONTOUR AND SHAPE FINAL CHANNEL CONFIGURATION FOR OPTIMAL FISH PASSAGE. WORK WITH STAKEHOLDER AGENCIES TO DEVELOP FINAL CHANNEL DESIGN.




STEP 4 PROFILE
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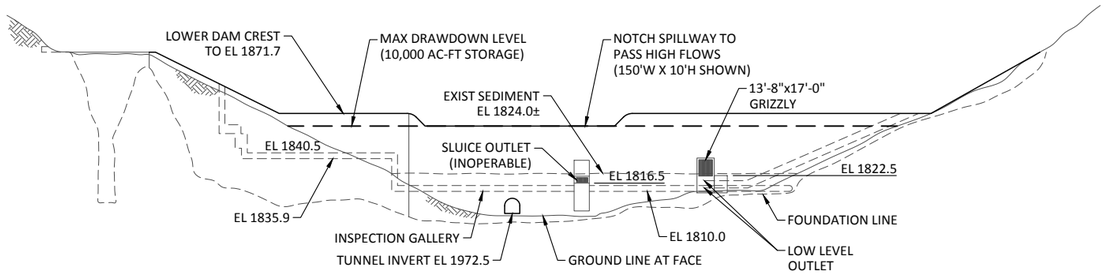
A S-0-1




STEP 3 SECTION
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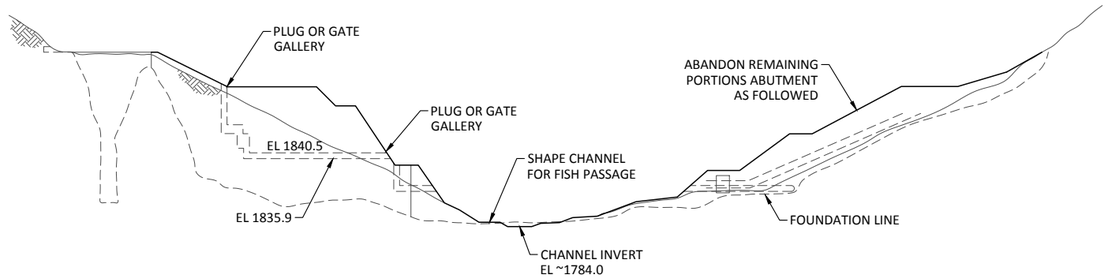


STEP 4 SECTION
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McMILLEN JACOBS ASSOCIATES

CALTROUT

POTTER VALLEY PROJECT FEASIBILITY STUDY

SCOTT DAM REMOVAL
RAPID REMOVAL SECTIONS 2

DESIGNED K. JENSEN
DRAWN R. GUERRERO
CHECKED M. McMILLEN
PROJECT DATE 07/14/21

DRAWING
S-1-2
0572

JOB NO: 000000

Path: C:\Vault20\Sonoma County Water Agency\Potter Valley\S-1-2.dwg Plot date: Jul 12, 2021 08:23am, CAD User: Guerrero

# ATTACHMENT 6

LLOYD C. GUINTIVANO
County Counsel

Office of
**COUNTY COUNSEL**
County of Lake

Courthouse – 255 N. Forbes Street
Lakeport, CA 95453
Telephone: (707) 263-2321
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Nicole Johnson
Carlos Torrez
Joseph Wankmueller
Pamela Sandberg
Jackson Berumen

RISK ANALYST:
Rick Ensley
(707) 263-2532

November 3, 2025

**By E-Mail Only *(wr401program@waterboards.ca.gov*)**

Honorable Chair Esquivel and Members of the Board
State Water Resources Control Board
Division of Water Rights – Water Quality Certification Program
1001 I Street, 15th Floor
Sacramento, CA 95814
c/o Ms. Wilhelmina Chon, Hydroelectric Project Manager

Re: Potter Valley NOP Comments – the County of Lake

Honorable Chair Esquivel and Members of the Board:

The County of Lake appreciates the State Water Resources Control Board's (State Water Board's) solicitation of comments from the community concerning environmental review for the Potter Valley Hydroelectric Project Surrender and Decommissioning project (Proposed Decommissioning) that is proposed by Pacific Gas and Electric Company (PG&E), and the State Water Board's particular interest in the community's views regarding the potential environmental impacts that the forthcoming Environmental Impact Report (EIR) will evaluate pursuant to the California Environmental Quality Act (CEQA). The County of Lake is pleased to provide its views in relation to the various environmental factors that will potentially be implicated by the Proposed Decommissioning and that should therefore be included in the scope of the EIR.

In particular, the County wishes to highlight the State's policy under CEQA that "public agencies should not approve projects as proposed if there are feasible alternatives or

feasible mitigation measures available which would substantially lessen the significant environmental effects of such projects," pursuant to Public Resources Code Section 21002. To achieve this objective and the other objectives of CEQA, an EIR must "[]identify the significant effects on the environment of a project, [] identify alternatives to the project, and [] indicate the manner in which those significant effects can be mitigated or avoided." (Pub. Resources Code § 21002.1, subd. (a).) Significant impacts must be mitigated "whenever it is feasible to do so." (Pub. Resources Code § 21002.1, subd. (b).) The EIR the State Water Board prepares must set forth *all significant effects* of the Proposed Decommissioning, indicating separately which effects cannot be avoided and which effects would be irreversible, as well as describing both mitigation measures to minimize such impacts and alternatives to the Proposed Decommissioning. (Pub. Resources Code § 21100.)

The presence of Scott Dam and Lake Pillsbury has been a vital resource for local counties and the state of California for over one hundred years. PG&E has not considered the true loss to the community in its planning of the Scott Dam decommissioning, and that is not acceptable to the County of Lake, local communities, and other agencies. The County of Lake recommends the EIR address the County's concerns regarding all foreseeable impacts to the physical environment as part of conducting environmental review for the Proposed Decommissioning. In particular, the County implores the State Water Board to consider how the Proposed Decommissioning would threaten water supply, life and property for at least 600,000 Californians, and to evaluate emergency management considerations. This is of crucial concern due to the multiplicity of massive wildfires in Northern California. The County urges the inclusion of alternatives or certain mitigation procedures proposed by the County in order for such measures to be incorporated into final decision making regarding the Proposed Decommissioning, so that all lost values can be replaced with new options. PG&E's application makes repeated references to unavoidable adverse effects, yet investment in preparation of mitigating alternatives is not in strong evidence. It is difficult to fully define the scope required for an Environmental Impact Report surrounding the proposed Decommissioning Project, when the anticipated 41 required Management Plans have not been made available for public inspection. Yet, it is the multitude of complex issues that require robust investigation that necessitates the State Water Resources Control Board acting early, and we value and appreciate your earnest consideration of each of the substantive points raised herein. Whether as part of the CEQA review process or otherwise, including meaningful financial mitigations for financial losses to the County resulting from the Proposed Decommissioning is another key concern for the County.

The County also requests and looks forward to an opportunity to meet with State Water Board staff in person regarding issues of greatest local concern to be evaluated in the EIR. We invite staff to visit the County in the near future to view the setting for the Proposed Decommissioning, which will more fully inform preparation of the EIR. Please be in touch with County of Lake Chief Deputy County Administrative Officer Matthew Rothstein, at Matthew.Rothstein@lakecountyca.gov or (916) 544-8150, to coordinate potential meeting times.

**The County of Lake and its History**

Located in Northern California, Lake County is home to Clear Lake, the largest natural freshwater lake in California with 68 square miles of surface area. There are three major watersheds in the County: Clear Lake/Cache Creek, Putah Creek, and the Eel River. Agriculture is a prominent industry in the County, including the Lake Pillsbury Area formed by Scott Dam, as are healthcare and local government. Other important local industries include tourism/hospitality and outdoor recreation, both of which are found in the Lake Pillsbury area.

It is important for the State Water Board to consider the region's history and the long-enduring caution that what is past is prologue. The Potter Valley Project's water diversion from the Eel River to the Russian River basin became operational in 1908, with the construction of its first dam, Cape Horn Dam. Legal efforts to halt this diversion failed in ensuing years, as the utility argued that the impounded water was utilized to provide the public with both electricity and necessary water. The Court of Appeal held that: "In view of these facts, and of the fact that [plaintiff] waited four years after the public use began, and until thousands of people had become dependent upon it for light and power and necessary water for irrigation, [plaintiff's] right to an injunction is barred by this intervening of the public interest." (*Holmes v. Snow Mountain Water & Power Co.* (1918) 36 Cal.App. 394, 397.)

In 1922, PG&E and its predecessors created Lake Pillsbury by the construction of Scott Dam, the second of two dams included in PG&E's Potter Valley Hydroelectric Project. The Lake County community of Hullville was destroyed when it was submerged under 125 feet of impounded Eel River water to create Lake Pillsbury. At that time, it was investors from San Francisco who set this transformation into motion, already seeing the need for hydroelectric production to be used to power the city of Ukiah, and facilitating the diversion of Eel River water from Lake County into Mendocino and Sonoma Counties.

In 1977, regional representatives presented testimony to Congress on the need for further strengthening of the water supply by constructing the Warm Springs Dam to create Lake Sonoma. Lake Pillsbury, Lake Mendocino, and Lake Sonoma have all come to form the backbone of the Russian River's ability to meet an ever-growing water demand. At that Congressional hearing, it was noted that prior to 1908, very little water flowed in the Russian River during the summer months and, in dry years, the river was usually dry. It was the creation of Lake Pillsbury and the Van Arsdale diversion that permitted water to flow in the Russian River throughout the summer months, allowing agriculture and recreational uses to flourish along the Russian River in Potter Valley, Southern Mendocino County, and Sonoma County. PG&E's comments to the Federal Energy Regulatory Commission (FERC) in 1978 acknowledged that "diversion of Eel River water into the Russian River has been responsible

to a large measure for extensive resort developments along the Russian River that would not otherwise be possible."[1]

Given the history of the region, serious consideration must be given to the potential impacts of removing those very elements that created Lake Pillsbury and also made the Russian River a consistently viable water source. The recent multiplicity of megafires in Northern California, including the August and Mendocino Complex events in which Lake Pillsbury served as a natural barrier to further progression, also call into particular question the prudence of removing such a critical water resource.

**General Scope Comments**

PG&E's Final Surrender Application and Final Non-Project Use of Project Lands Application (the Final Application) repeatedly uses the phrase "unavoidable adverse effects," which makes it clear PG&E does not duly consider the potential and likely environmental impacts of the proposed decommissioning of the Potter Valley Project, let alone appropriate, feasible mitigations of its effects. It is difficult to see how an EIR can meaningfully be completed unless and until PG&E addresses each "unavoidable adverse effect" referenced its Final Application. For each "unavoidable adverse effect" referenced in a category discussed in PG&E's Final Application, the County asks that the State Water Board analyze potential mitigating measures. The County also requests that Alternative Scenarios discussed separately below and in further detail in the attached SLR Impact Analysis and Adaptation Strategy Evaluation Report (SLR Report), prepared by the County's consultants, be considered by the EIR. Appendices to the SLR Report and additional related materials, which provide further detailed information, are also included for your review and analysis.[2]

PG&E has represented they undertook a good faith process to explore whether public agencies or other parties may have interest in assuming control and operation of Scott and Cape Horn Dams. However, public discourse surrounding those explorations suggested the dams were rife with seismic risks and decommissioning was a matter of urgency; members of your Board will appreciate, such framing affected the probability of a responsible party coming forward. As recently as May 26, 2025, PG&E North Coast Region Vice President, Dave Canny, noted the following:

> *Economics drove the license surrender decision. Later, seismic studies were completed. The hazard risk is one of the key factors that led to the decommissioning plan for expedited removal of Scott Dam in the decommissioning plan and schedule.*[3]

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[1] Available at https://www.lakecountyca.gov/DocumentCenter/View/14816/Pacific-Gas-and-Electric-company-Comments--Potter-Valley-Project-No-77-EIS-1978.

[2] These materials include SLR Existing Conditions Report (June 6, 2025, attached); Farm Bureaus' Letter to Federal Agencies (Apr. 4, 2025), available at https://www.lakecountyca.gov/DocumentCenter/View/14648/Farm-Bureaus-Letter-to-Federal-Agencies-for-Intervention-April-2025.

[3] Available at https://lakeconews.com/news/community/letters/81648-canny-potter-valley-projects-surrender-and-decommissioning.

Yet, in Division of Safety of Dams reports documenting inspections of October 18, 2023, PG&E is noted to have stated, "Based on the instrumentation evaluations included in this [Dam Safety Surveillance Monitoring Report (DSSMR)], and the visual inspections by PG&E, FERC, and DSOD, the [Scott] dam is judged to be performing satisfactorily… [and] the project is suitable for continued safe and reliable operation."[4] The DSOD reviewer affirmed, "Based on the data submitted, I agree with [PG&E's] conclusion [that the Van Arsdale Dam is suitable for continued safe and reliable operation]."[5]

In response to an inquiry from Lake County News (LakeCoNews) of September 23, 2025, Dave Canny emphasized:

> *We have always said the dams are safe, otherwise we would not be operating them. In 2023 PG&E completed a study showing the seismic concerns at Scott Dam were greater than previously understood and required mitigation to meet PG&E's safety standards and those of our dam safety regulators. As mitigation, PG&E kept the maximum reservoir elevation at least 10 feet below the full elevation. Well before the updated analyses, PG&E made the decision to decommission the Potter Valley Project as it is uneconomical. The updated analysis is not the driver of decommissioning, despite what some claim.*[6]

Furthermore, PG&E recently stated that "the interim risk-reduction measures described are sufficient to address the potential risks and vulnerabilities associated with continued operation of Scott Dam. PG&E is no longer considering additional analysis or major remedial projects to reduce long-term risks."[7]

Members of your distinguished Board will appreciate, Canny's comments in May 2025 stated seismic concerns were a "key factor" driving urgency surrounding decommissioning, and the necessity of "expedited removal." If, with "the maximum reservoir elevation at least 10 feet below the full elevation," seismic concerns *do not* demand expedient removal of the dams, as Canny's September 2025 comments suggest, appropriate deliberation should be undertaken to mitigate *all* consequences of Proposed Decommissioning, including those raised here by the County of Lake.

To date, the interests of the County of Lake and its constituents have largely been ignored by PG&E's promotion of the Proposed Decommissioning. Throughout the process, supporters of the Proposed Decommissioning have advocated for the destruction of a small

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[4] Available at https://www.lakecountyca.gov/DocumentCenter/View/14814/97101_Scott_2510_Ins_Report_10182023_Redacted.
[5] Available at https://www.lakecountyca.gov/DocumentCenter/View/14815/97102_Van_Arsdale_2510_Insp_Rep_10182023_Redacted.
[6] Available at https://lakeconews.com/news/82701-lake-county-officials-challenge-pg-es-claims-about-potter-valley-projects-seismic-safety.
[7] PG&E's Potter Valley Hydroelectric Project, FERC Project No.77-318 Application for Non-Capacity License Amendment Response to Additional Information Request (Sept. 26, 2025), page 12 (attached).

but vibrant community in Lake County; as presented by PG&E, decommissioning would create a legacy of long-enduring adverse environmental impacts to the eco-system of this area and, consequently, to much of Lake County. Notably, the Proposed Decommissioning would deprive the Lake County Operational Area of an important source of water that firefighting agencies have determined is critical to addressing increasingly severe wildfires in the area. Please see the attached correspondence dated March 1, 2025 from the Lake Pillsbury Fire Protection District to PG&E's Senior Licensing Project Manager.

Similarly, it was surprising, in light of the potential for environmental disaster should the Proposed Decommissioning proceed absent a robust mitigation strategy, that the State Water Board did not host an EIR scoping meeting in Lake County. However, the County requests and looks forward to more direct engagement with the State Water Board as its work in this matter progresses. This will enable the State Water Board to fully consider the County of Lake's interests and concerns.

**The Proposed Decommissioning Would Result in Considerable Unavoidable Adverse Effects to the County of Lake and its Natural Resources**

For the State Water Board's consideration, the County believes the following environmental factors should be included in the EIR's review of the Proposed Decommissioning's potential impacts. Above all, the County encourages the State Water Board to prominently consider the importance of maintaining availability of water supply for fire suppression purposes, in accordance with the State Water Board's vision statement: "To preserve, enhance, and restore the quality of California's water resources and drinking water for the *protection of the environment*, public health, and all beneficial uses, and to *ensure proper water resource allocation and efficient use*, for the benefit of present and future generations." (*Emphasis* added.)

*Aesthetics*

The EIR must consider and evaluate the various proposed restoration activities PG&E needs to incorporate to improve the aesthetic aspects of the Proposed Decommissioning. PG&E has not detailed specific restoration plans or activities; once these restoration plans are prepared, the plans should be made available for comment, and timeframes for consideration of those plans must be adequate to allow for well-rounded review; 30 days is insufficient.

PG&E should be required to prepare an ecosystem restoration plan that considers management of invasive species, and reintroduction of native and endangered species, along with management and stabilization of the lake sediments. Monitoring should be required to evaluate the progress of revegetation, and any needed adaptation until vegetation is established in the areas affected by the Project.

PG&E has not yet explained how the Scott Dam area recreation facilities would be restored such as the campground, day-use areas, and associated roads and trails. Removing a significant water body will have a significant visual impact; restored facilities should be visually appealing and properly integrated with the natural environment. For example, small ponds and hiking trails could help offset these significant visual impacts and should be included in restoration plans. Please also see the Alternative Scenarios section below.

*Air Quality*

The separate comments of the Lake County Air Quality Management District (AQMD) are attached hereto and incorporated by reference. Issues raised by the AQMD's comments include the following.

Scott Dam and all structures related to the project may contain asbestos and must be thoroughly surveyed prior to proceeding with any demolition or preparation activities. Asbestos in building materials is a significant threat to the community, workers, and the public. Recycling concrete requires thorough testing to ensure no asbestos is within the concrete or structures.

The Proposed Decommissioning location is in close proximity to mapped serpentine areas. Due to the age of the dam and related lack of available documentation, it is unknown whether serpentine was found in or within the project boundaries of the Scott Dam and facilities. Should serpentine be found within the Proposed Decommissioning boundaries, additional precautions are required. A geological review of the project area is recommended due to the potential of serpentine or asbestos containing rock to be in the project area.

Construction emissions are of significant concern and should be evaluated due to the size of this project. Use of CALEEMOD is recommended to determine construction emissions and impacts as well as for determining ongoing impacts.

The sediment behind the dam and throughout the current lakebed will require long-term management plans. The dust bowl that could result if sediment is improperly managed could be significant. Remediation and site management are likely to be required for many years. Access controls to prevent off-road vehicle activities and other ground disturbing activities will be required.

The sediment has potential to be highly concentrated with toxic materials and heavy metals, as these constituents settle out of the water column early while other fine particulates continue to wash further downstream. Thorough testing of the sediment from surface to full depth should be required to determine the potential for toxic materials and heavy metals becoming entrained in dust-generating activities or being washed down stream during dam removal and annual winter runoff.

The impact on groundwater recharge and aquifer storage may be significant. Evaluation of the impacts to groundwater wells, home water supplies, and fire suppression wells is essential for fire safety, household needs, and dust controls in this rural area.

The impact of the lake removal will severely impact fire fighting efforts in the National Forest areas. The flight time to another water body could be significant, potentially resulting in increased fire growth, smoke impacts throughout the region and beyond, and the loss of life due to lack of adequate water storage in close proximity to these rural residential areas.

*Biological Resources*

Lake Pillsbury has its own unique ecosystem – with threatened and endangered species, such as Tule Elk – that attracts visitors to the area. The EIR should evaluate the Proposed Decommissioning's likely impacts on these species and their habitat, and measures must be provided to ensure maximum feasible mitigation of such impacts.

Because alteration of the Eel River's natural flow has disrupted local ecosystems, including fish migration patterns, benefits to fishery resources can be expected, at least to some degree, from the current Proposed Decommissioning. However, the Potter Valley Project can be decommissioned in a way that will provide fish passage while still protecting and retaining availability of water supply for fire suppression and some, if not most of the current recreational and community resources in the Lake Pillsbury area, as discussed later in this letter. Please see the Alternative Scenarios section below.

In terms of fauna, preliminary research has identified 15 total species within 0.5 miles of Lake Pillsbury's perimeter. Among the list of species identified, 3 of 15 are considered endangered according to State criteria: the Bald Eagle, Steelhead, and Humboldt Marten. Additional species known to occur on Lake Pillsbury and Van Arsdale reservoirs include: Northern Spotted Owl, Northern Goshawk, Fisher-West Coast, Pacific Marten, Special-Status Bats, Black-Tailed Deer, Tule Elk, Western Bumble Bees, and Monarch Butterflies.

The predominant plant communities surrounding Lake Pillsbury and Van Arsdale Reservoir are yellow pine forest, mixed evergreen forest, foothill woodland, and chaparral. There are several special-status plant species that are known to occur in the Proposed Decommissioning boundary or adjacent to the Proposed Decommissioning features, including: two populations of three-fingered morning glory; one population located along the tunnel between Cape Horn Dam and Potter Valley Powerhouse; one population located along the southern shoreline of Lake Pillsbury; and one population of Green's narrow-leaved daisy.

The EIR must consider the impacts of the Proposed Decommissioning and alternatives to each of these species, as well as potential mitigations for those impacts.

*Cultural Resources*

Prior to being inundated to make way for Lake Pillsbury and development of the Potter Valley Project, the town of Hullville featured Hunter's Hotel, a blacksmith, a carpenter's shop, school, post office, and a store. The Hullville-Gravelly Valley Cemetery now lies under Lake Pillsbury and it will be impacted by the Proposed Decommissioning, as may the Upper Lake Cemetery at Graveyard Point, where some remains were relocated from Hullville-Gravelly Valley Cemetery.

Local groups such as Lake Pillsbury Alliance must likewise be engaged, to ensure all implications are appropriately considered. Tribal considerations must also receive appropriate attention, as referenced below.

*Geology and Soils*

Impacts on surface and groundwater interactions following dam removal and its effects on groundwater wells as part of the Proposed Decommissioning should be evaluated. Without surface water replenishment from the reservoir, groundwater levels may be impacted and affect existing wells. Appropriate mitigation should be considered and evaluation, including a well monitoring program pre- and post-dam removal, and funding to support well owners to repair or deepen existing wells to maintain equivalent access to groundwater of at least comparable quality as presently available. PG&E has acknowledged this adverse impact and deemed it unavoidable. Nevertheless, mitigation options must be considered and evaluated.

Additional hydrogeological assessments and evaluation of the radius of impact of the groundwater level drawdown should also be conducted. Once the groundwater system is better understood, PG&E should be required to contact residents and landowners in the impacted radius to notify of these potential impacts, and it should request information on existing wells in the affected area. A detailed groundwater monitoring plan should be developed prior to decommissioning of Scott Dam. In addition, all well owners should be notified of the potential impacts.

Suggested mitigation strategies similar to those established for the Lower Klamath Projects should be considered. For example, in that instance, the State Water Board required the following conditions to be met by the Klamath River Renewal Corporation. Note that the distances or number of locations listed below are for information only, and site-specific criteria applicable to this area need to be developed based on further studies:

a) Monitor groundwater levels within a 2.5-mile range of the high-water marks, before, during and after reservoir drawdown;

b) Contact all residents and landowners within 1,000 feet of the reservoirs;

c) Identify potentially affected groundwater wells;

d) Monitor groundwater levels at a minimum of 10 locations within the 1,000 feet radius for at least two months before drawdown activities, and continue monthly monitoring for at least two years; and

e) Mitigate groundwater impacts.

The removal of the abutment structure will require more detailed evaluations including an abutment removal sequence and phasing plan, temporary excavation support systems for worker safety, slope stability analysis for each stage of the proposed removal, permanent slope support systems, and ground movement monitoring and alarm systems. During removals, there is a high risk of triggering landslides. Proper restoration and re-vegetation of these slopes to their original condition may involve complete removal of the abutment.

Upon removal, the existing slopes all around the formally inundated areas will be subject to higher risk of landslides. This risk may be heightened during or soon after water release due to rapid drawdown conditions. The slope conditions of currently submerged areas need to be analyzed and remedial grading should be developed accordingly.

Alternative options should be fully evaluated, as they can provide a well-balanced remedy to many of the above-mentioned geotechnical and geologic concerns.

The County suggests targeted and dam-specific subsurface investigations as a crucial part of this process in the absence of existing documentation to support the post-removal conditions.

Debris management needs to be considered as part of the removal process, including any disposal or reuse of the demolished concrete of the dam, other structures, material found at recreational facilities, and any material (*including solid waste*) discovered during sediment removal. Land disposal of waste in California is regulated under California Code of Regulations Title 27, CEQA and County ordinances. The EIR should set forth necessary permitting processes for disposal of waste and debris.

The risks associated with triggering slides when removing the dam and when cleaning out sediments needs to be evaluated. PG&E's Final Application to Surrender License does not provide details on evaluating these risks, it notes the exposed reservoir bed would be susceptible to erosion and proposes to define specific mitigation measures in management plans that will be prepared in the future.

A comprehensive sediment risk analysis must be completed for the Proposed Decommissioning. The United States Bureau of Reclamation recommends an iterative, risk-based approach for evaluating and managing sediment impacts from dam removal projects.

PG&E acknowledges Lake Pillsbury is designated as mercury-impaired on the State of California's 303(d) list because of elevated mercury concentrations in fish. Prior sediment

studies were only conducted for shallow sediments, therefore additional studies are recommended to evaluate metal concentrations and mobility, including but not limited to mercury, for the deeper sediments impounded.

Additional investigations and planning are needed to verify and protect against a potential release of Naturally Occurring Asbestos (NOA) which is known to occur within the Lake Pillsbury watershed. A NOA management and mitigation plan would need to be developed to mitigate the potential impacts during the dam decommissioning process. The Final Application for Surrender of License cites prior studies on Lake Pilsbury sediments but does not provide specific references. A prior study only tested shallow sediments in the lakebed. Additional studies should be required.

During construction of Scott Dam, heightened river flows during a flood year set a large slide in motion. As noted in the FERC Final Environmental Impact Statement (EIS) for review of PG&E's Potter Valley Project re-licensing application in 1978, an area between Scott Dam and Cape Horn Dam, previously called The Slides, is "an active slope area more than a half-mile wide and more than 1,000 feet high and containing nearly 43 million cubic yards of potential slide material, [and] is located on the north side of the Eel River canyon about 3 miles downstream from Scott Dam. The largest slide in this area has blocked the Eel River within the past few hundred years, and may be expected to do so again." If a copy of the 1978 relicensing Final EIS is not available to the State Water Board for review, please let us know and we will be glad to provide a copy.

Any Scott Dam plan for monitoring slope stability during and after implementation of the Proposed Decommissioning should include measures to address and reduce the potential for reactivating historic landslides in the area, including on the Lake Pillsbury side of the dam, and to protect worker safety. The EIR should require sufficient plan details for an informed evaluation of plan efficacy in regard to historic slope stability issues.

In addition, sufficient details of the Proposed Decommissioning's plan for mitigating water and wind erosion due to declining water surface elevations after dam removal should be required from PG&E. The EIR should consider and evaluate alternative approaches and appropriate mitigations.

The PG&E Final Application for Surrender of License generally acknowledges sediment impacts to water quality, erosion and transport following dam removal. Nevertheless, no clear mitigation alternatives such as sediment stabilization or removal were proposed. The EIR should fully analyze such impacts and propose alternative approaches and appropriate, effective mitigation measures for these adverse impacts. The attached SLR Report provides further details regarding this topic.

*Greenhouse Gases*

The EIR should quantify and compare greenhouse gas emissions under existing and proposed conditions, including long-term carbon sequestration potential. The EIR should also evaluate and propose mitigation measures for greenhouse gas emissions generated by implementation of the Proposed Decommissioning.

*Hydrology and Water Quality*

Prior to 2019, Scott Dam historically provided the highest reliability for water supply in terms of downstream flow, flood risk reduction, groundwater recharge, and drought resilience. This reliability is attributed to the operability and control provided by the needle valve and spillway gates. At its 1922 inception, Lake Pillsbury operated with a maximum storage volume of over 94,900 acre-feet (30 billion gallons), although this capacity has been reduced over time due to changes in operation and sediment accumulation.

The current operational storage conditions (established in 2023) have an average water surface elevation and storage within Lake Pillsbury that are lower than historical operational conditions. The maximum storage under the current operations is projected to be 75,400 acre-feet (24.6 billion gallons), a significant reduction since 1922 operations. This was primarily a result from a reducing the maximum operational water surface elevation, changes in operations of the dam outlet controls, and storage capacity losses from sediment accumulation.

Despite the loss of storage, the Scott Dam system has consistently demonstrated its effectiveness in attenuating flood waters and mitigating drought impacts. Since the beginning of the 21st century, droughts have increased in frequency, intensity, and duration. Trends that are projected to continue as a result of climate change. PG&E's requests for temporary flow variances in the 2013-2016 drought emphasize the increased pressure on regional water resources.

As climate-driven extremes intensify, the need for reliable water supply becomes even more critical. The continued function of Scott Dam is essential for downstream users and the surrounding community, providing both flood protection and drought resilience. Further reductions in the storage capacity from changing operations and ongoing sedimentation will likely diminish the system's ability to buffer against future droughts, resulting in lower available storage and outflows compared to the historic conditions. Flood and drought risk are projected to increase with PG&E's proposed decommissioning.

In the past, PG&E has argued the scope of the Potter Valley Project encompasses both water supply and energy. Now, PG&E frames the Project as a way to provide energy, first and foremost, and as water supply as an afterthought. In response to public calls for greater engagement by the Mendocino County Board of Supervisors surrounding implications of the Proposed Decommissioning, Mendocino County Inland Water and Power

Commission (IWPC) recently submitted a letter to that Board, a copy of which is attached. The third from final paragraph notes the following: "If Mendocino County were to send a letter to FERC opposing PG&E's surrender application, it would jeopardize all of the progress we have made to preserve a water supply for the Russian River basin from the Eel River." An alternative diversion, which would require profound public financial investment is depicted as more "certain" than continued reliance on the diversion facilitated by the Potter Valley Project. Clearly, there is significant fear PG&E is in the driver's seat and regional agencies could be left with no diversion. This is untenable. While the County of Lake and concerned residents of Mendocino County, for example, may have differing views surrounding Scott Dam and alternatives regarding inter-basin water transfer, Lake County is is far from alone in insisting that local community interests and priorities must be sufficiently acknowledged and addressed in response to PG&E's Final Application. It is in everyone's best interest – and the Water Board and State's interest, more broadly – to ensure that the Proposed Decommissioning only proceed if it can be responsibly conducted, which includes ensuring alternative water infrastructure is in place that is reliably sufficient (ideally, prior to decommissioning).

The State Water Board must consider the local beneficial uses of Lake Pillsbury water (see Cal. Const. art. X, § 2), including drinking, recreational, and others, and how loss of this water supply for the various beneficial uses will be mitigated. PG&E notes there are no expected effects on water reliability or cost in the Scott Dam Area because, it states, there are no known surface or authorized water diversions; however, PG&E fails to acknowledge the potential recharge to groundwater recharge contributions from Lake Pillsbury, including potential recharge to the Gravelly Valley Basin. Loss of groundwater recharge will result in reduced water reliability and increased cost for well owners. Water from Lake Pillsbury is also currently used for fire suppression, and removing the lake will affect this resource and water use.

Effects of the Proposed Decommissioning on the water supply and groundwater recharge must be investigated and considered. Water supply insecurity has been a concern during drought conditions in recent years even with Scott Dam in place. It is currently unknown what the anticipated groundwater levels will be in the vicinity of Lake Pillsbury with implementation of the Proposed Decommissioning. Drinking water should be made available to well owners that will be affected by the reduced groundwater recharge resulting from draining the lake. Long-term effects are not expected to be known until several years have passed. The EIR must investigate and consider these impacts and mitigation measures.

Previous hydrological analyses have been conducted over short periods of record and do not fully characterize the system. Further analysis into the operations, storage, and streamflow gage relationship to better understand the flow relationship to operations and assumptions for unimpaired Eel River flows and water supply is also necessary. Further analysis over the entire period of record will also provide a level of confidence in the data and assumptions made for the hydrology and hydraulics. Operations of Scott Dam have changed due to varying regulations and restrictions over the life of the dam. Storage capacity

has likewise changed, due to the accumulation of sediment within Pillsbury Reservoir and permanently opening the spillway gates due to seismic concerns.

A hydraulic model should also be developed to better define the proposed action during varying seasons and recurrence intervals. This would also support quantifying the sediment yield.

Flow timing from upstream tributaries should also be incorporated to understand how seasonal and event-based contributions affect downstream hydraulics and sediment movement. Sediment grain size distribution and flow thresholds for mobility (*i.e., At what flows will sediment move?*) should be analyzed to support channel evolution. Core sampling the full sediment depth within Lake Pillsbury should be analyzed to characterize sediment volume and composition. Tributaries upstream and downstream of Scott Dam are likely ongoing sediment sources that account for and will continue to transport sediment into the Eel River system under all conditions.

Moreover, implementation of the Proposed Decommissioning and releasing the water from Scott Dam without appropriate mitigation to the riverbed and channel could result in erosion of riverbanks and the riverbed that would take many decades to recover from. The Humboldt County Farm Bureau raised concerns in this regard during the October 15, 2025 scoping meeting in Ukiah.

The County urges the State Water Board to require development of a risk and uncertainty analysis for the proposed Scott Dam removal to better understand potential consequences and articulate the level of uncertainty or confidence in the data assumptions. The hydrology informs the hydraulics and the function of the existing and proposed infrastructure and the assumptions under proposed conditions. If hydrologic information is limited or inconclusive, there is a lower level of confidence in the models and assumptions made to fill in gaps within the data.

An additional concern is that the Proposed Decommissioning will harm the ability to meet downstream flow requirements. Under existing conditions, with operational control of the needle valve and spillway, noncompliance has been reported, most often during the dry season. Under Proposed Decommissioning conditions, there would be no operational control or storage. During dry seasons, and especially droughts, the likelihood of noncompliance is higher under proposed conditions for a variety of hydrologic and climate-related factors. PG&E's modeling results concur with this: graphs of flow projections under proposed conditions show flows hovering near and going below 10 cubic feet per second (cfs) during the dry season every year.

PG&E notes expected short-term adverse effects due to suspended sediment and turbidity following dam removal. Based on the level of uncertainty of the models, and lack of grain size analysis data, it appears that the anticipated period of several days up to several months, and additional effects with remobilization for 1 to 3 years may be optimistic. The

State Water Board should require further studies to improve the sediment grain-size distribution dataset and update the sediment transport models with improved data.

PG&E acknowledges there will be impacts to water temperatures during and following the removal of Scott Dam. Given the uncertainty of the existing models, these effects may be underestimated, and additional evaluation with improved data should be required to facilitate the methodologies for temperature monitoring.

*Land Use*

PG&E's Final Application noted that "The Proposed Action may result in unavoidable adverse effects to local fire suppression to properties near Lake Pillsbury due to the replacement of Lake Pillsbury with the Eel River or other sources as a water source, resulting in potentially longer fire response times." The loss of Lake Pillsbury water availability would have significant wildfire suppression-related adverse impacts on infrastructure, property, and human life. Section 3.8 of the SLR Report provides a detailed analysis of key impacts that need to be analyzed in the EIR, including:

- Wildfire management.
- Dry lightning fire risk.
- Water availability related to fire department resources.
- Lake Pillsbury's historic role as a primary water source during fire attack operations.
- Lake Pillsbury's importance to U.S. Forest Service (USFS) and CAL FIRE mutual aid, including rapid response capabilities for air resources, which has been critical to the agencies' success in keeping most fires at 10 acres or less with timely intervention helicopter intervention.
- The Proposed Decommissioning would increase fire behavior in the area due to the exposed lakebed. Previous fires like the August Complex and the Mendocino Complex have already burned an exceptionally sizable portion of the Mendocino National Forest (MNF). Ecosystems will be affected when taking a water source from the area, which means drier fuel moisture content that can increase fire behavior with certain types of fuel models.
- Increased wear on firefighting aviation equipment.
- Strain on fuel infrastructure for firefighting aviation fuel supplies.
- Delay in helicopter availability for other critical missions, such as evacuations or medical response.

- Delays in fire containment.
- Increased costs of fire damage.
- Increased risk to residents.
- Helitack crew safety over longer flight distances.
- Health impacts from smoke due to slower containment.
- Loss of natural resources due to delayed fire suppression response.
- Economic hardship due to destruction of property and infrastructure.

The Proposed Decommissioning will require several road improvements to ensure safe execution during the project and to maintain accessibility for emergency wildfire response, as well as recreation and other purposes afterward. Section 3.8 of the SLR Report also sets forth necessary road improvements that should be incorporated in relation to the EIR's land use impacts assessment and analysis.

The loss of Lake Pillsbury that would result from the Proposed Decommissioning's Scott Dam removal process raises valid concerns about wildfire spread, particularly given the Lake's role as a barrier, preventing small fires from becoming larger events, according to United States Forest Service and Lake Pillsbury Fire Protection District personnel. The lake has frequently acted as a natural firebreak, slowing or stopping the spread of wildfires. Without the lake, vegetation would replace the reservoir basin, potentially increasing fuel loads and creating a continuous pathway for fire spread. Section 3.8 of the SLR Report includes an analysis of technical considerations to address wildfire concerns and the feasibility of retaining or reconfiguring the lake to meet both wildfire mitigation and seismic safety goals, which should be considered and analyzed in the EIR.

The EIR should also address the ownership and management of currently-submerged lands upon implementation of the Proposed Decommissioning, as it appears title to inundated lands may now be vested in the State. (*State of California v. Superior Court* (*Fogerty*) (1981) 29 Cal.3d 240, 248-249.[8]) To the extent PG&E claims continuing ownership of lands submerged for the past century and intends to purport to transfer title to such lands, whether to the MNF or another entity, the County wishes to be fully apprised of such plans, activities and communications. We urge the State Water Board to seek guidance from the State Lands Commission on this matter and to provide relevant details to the public via the EIR and any other appropriate public disclosures. The EIR should fully consider the alternatives for the

[8] See also Civil Code §§ 670, 830; *State of Cal. ex rel. State Lands Com. v. Superior Court* (1995) 11 Cal.4th 50, 63-64; *State of California v. Superior Court (Lyon)* (1981) 29 Cal.3d 210, 222; *Los Angeles v. Aitken* (1935) 10 Cal.App.2d 460, 467.

best and highest uses, and management, of these lands lying within the Mendocino National Forest, upon implementation of the Proposed Decommissioning.

*Noise and Vibration*

Demolition activities associated with the Proposed Decommissioning would generate both significant noise as well as ground borne vibration from the operation of heavy equipment. As with noise, vibration levels would vary depending on the type of equipment used and the distance to receptors. The EIR should analyze the likely impacts and provide appropriate mitigation recommendations.

*Recreation*

Recreation in Lake County has relied on the scenic qualities of Lake Pillsbury for many decades. Lake Pillsbury is the largest waterbody in the Mendocino National Forest and attracts numerous visitors each year who generate revenue for the local economy.

Recreation taking place in and around the lake, includes camping, boating (motorized and non-motorized), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking. Lake fishing includes largemouth bass, Sacramento pikeminnow, bluegill and Rainbow trout. Whitewater rafting has historically occurred downstream of Lake Pillsbury in October and November when PG&E released water from Pillsbury Reservoir to make room for winter rain and snowmelt. There are currently six campgrounds, two with group camping facilities along with three boat ramps around Lake Pillsbury. According to the Forest Service's website "The campgrounds at Lake Pillsbury are operated and maintained by Pacific Gas and Electric," excepting Lake Pillsbury Resort, which is privately owned. Loss of Lake Pillsbury would dramatically impact recreation activities. The annual visitation estimate for the Mendocino National Forest is 254,000 individuals, based on National Visitor Use Monitoring Data Collected FY 2010 through FY2019.[9]

The Proposed Decommissioning also suggests removing several of the existing camping facilities. This would result in a significant recreational impact. To reduce recreation impacts, the State Water Board should consider including recreational restoration in the Restoration Plan PG&E intends to develop. The plan should be developed to offset the loss of recreational facilities, such as swimming opportunities, hiking, picnicking, off-road vehicle paths, camping and fishing. This plan needs to be developed in coordination with both USFS/MNF staff and the County of Lake.

Similarly, the diversion of Eel River water into the Russian River has been responsible to a large measure for extensive resort developments along the Russian River

[9] A visit is defined as "the entry of one person upon a national forest to participate in recreation activities for an unspecified period. A National Forest Visit can be composed of multiple Site Visits." (U.S. Forest Service, National Visitor Use Report (Jan. 28, 2018) for the Mendocino National Forest.)

that would not otherwise be possible. The EIR should consider the scope to which the Proposed Decommissioning would impact recreation in the region. As noted above, PG&E long ago acknowledged that "diversion of Eel River water into the Russian River has been responsible to a large measure for extensive resort developments along the Russian River that would not otherwise be possible."

The recreational features surrounding Lake Pillsbury that will continue to operate with implementation of the Proposed Decommissioning are: Pogie Point Campground, Sunset Point Campground, Oak Flat Campground, Navy Camp Campground, and Trout Creek Campground (Cape Horn Dam Area). The EIR should evaluate the impacts to these facilities and provide additional details on any necessary improvements or changes to the facilities to maintain functionality if Lake Pillsbury is drained.

PG&E's Final Application does not address loss of revenue generated by visitor usage and similar losses that will be experienced in the area. Recreation needs to be quantified as value revenue loss, and PG&E should be required to provide fair, adequate financial restitution to the County of Lake for the permanent loss of recreation anticipated upon implementation of the Proposed Decommissioning. Other locations offering similar lake-based recreational opportunities are a 45-minute to 2-hour drive from Lake Pillsbury, and include Clear Lake, Lake Sonoma, and Lake Mendocino. The EIR should discuss related impacts, including the potential for overcrowding at these other facilities due to closure of recreational facilities at Scott Dam.

Recreation opportunities in the Proposed Decommissioning vicinity are concentrated around Lake Pillsbury, which is surrounded by public land managed by the U.S. Forest Service. Lake Pillsbury provides a variety of reservoir-based recreation opportunities; there are also a variety of developed recreation facilities in the immediate area of Lake Pillsbury (*e.g.*, family campgrounds, group campgrounds, and day-use opportunities). The EIR should consider and explain the impacts of any proposed removal of these facilities and related site restoration.

Hanging over any consideration of alternative recreation is a Conservation Easement, overseen by Mendocino Land Trust, which purportedly covers certain currently inundated and other project area lands. A copy of the Conservation Easement is attached.[10] Participants in a recent State-funded visioning process for the Upper Eel River Basin repeatedly expressed frustration surrounding a lack of clarity from PG&E and Mendocino Land Trust as to what alternative facilities could be developed.

[10] PG&E's related July 28, 2021 advice letter to the California Public Utilities Commission (regarding disposition and encumbrance of certain land), available at https://www.pge.com/tariffs/assets/pdf/adviceletter/ELEC_6269-E.pdf, attaches a copy of the pertinent Land Conservation and Conveyance Plan (LCCP). Map 3 on page 6 of the LCCP (pdf page 21) appears to reflect PG&E's plan for these lands in the period prior to recordation of the attached Conservation Easement.

PG&E has stated in the Final Application that recreation will shift from lake-based to riverine-based and acknowledges the "unavoidable adverse effects" due to the loss of these recreation facilities. However, they have not provided details on how they plan to assist in this transition and the support they will provide the County of Lake in the face of potentially devasting losses to the local recreational economy.

*Population and Housing*

In the areas immediately surrounding Lake Pillsbury, county tax revenues are related to secured and unsecured property taxes, forest service cabins, sales tax and PG&E unitary tax payment related to occupied property. For example, the tax revenues for these properties are approximately $1 million per year.

The County of Lake provides critical services to the public through its collection of real property taxes. The loss of the entire community surrounding Lake Pillsbury will result in a loss to Lake County of valuable property tax revenue, which is extremely significant to the County. Of note, PG&E's Final Application severely miscalculated the amount of property taxes collected in the Lake Pillsbury area.

A preliminary review indicates an aggregate assessed value of the private properties in the Lake Pillsbury area at over forty million dollars ($40,000,000). That value is likely lower than the actual market value and it does not reflect other values associated with Forest Service leases and business activities in the Lake Pillsbury community. These properties generate property taxes annually for schools and local governmental entities. There is further revenue generated by Transient Occupancy Tax and sales tax, which would require further study to analyze. The removal of Scott Dam and destruction of Lake Pillsbury further implicates the taxes assessed by the State against PG&E. Lake County currently receives approximately $1. 7 million from PG&E in annual unitary payments. Those payments will be drastically reduced with the destruction of Lake Pillsbury and surrounding community. Yet, the duty of the local governmental entities to protect and serve remains. The risk of wildfires to this area and all surrounding areas also remains; however, the removal of Scott Dam and the destruction of Lake Pillsbury will deprive the County of Lake of substantial water resources and the public funds necessary to adequately fight the fires.

Additionally, the local economy will likely be impacted by the loss in revenue from the previous tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury. The EIR should assess and analyze these impacts to the County of Lake and provide alternatives and mitigation recommendations.

*Transportation/Traffic*

PG&E has documented existing road infrastructure, transit, and aviation facilities in the vicinity of Scott Dam and Lake Pillsbury area. However, PG&E suggests impact on access road infrastructure is limited to three access roads. The EIR must include assessment and evaluation of all access roads and other infrastructure and their suitability to facilitate the Proposed Decommissioning, including future restoration and use of existing recreational and other facilities. The EIR should also evaluate potential temporary or permanent closures or restrictions on public access roads serving Lake Pillsbury recreation areas and adjacent communities. Additionally, the EIR should analyze the traffic and road network impacts.

The Transportation and Circulation Element of the Lake County General Plan includes goals and policies that may be applicable to the Project. The relevant policies are listed below:

- Policy T-1.8. Level of Service. County maintained roadways should be improved and maintained to provide an adequate peak period Level of Service (LOS) of "C" or better for existing and anticipated traffic volumes if roadway upgrades are feasible, such as roadway widening, addition of lanes via re-striping, and other safety and operational improvements. The County shall allow a limited number of County roadway segments to operate at a level of service of "E" or better where improving the segments to LOS C are deemed infeasible due to cost, negative community and/or environmental impacts, and/or constructability issues. This "E" level of service for certain roadways shall not include any State Highway unless approved by Caltrans.

- Policy T-1.9. Truck Routes. To reduce heavy truck traffic in residential areas and near noise sensitive land uses, the County shall ensure truck routes are designated in a manner such that traffic noise impacts are minimized.

- Policy T-1.10. Construction Methods. The County shall utilize road construction methods that seek to reduce air, water, and noise pollution associated with road and highway development.

Regarding the impacts of dam removal, PG&E will need to prepare inundation maps with respect to releases during dam drainage and removal, and evaluate the potential effects on road infrastructure, including road easements, culverts, bridges and other utilities. Once these effects are sufficiently evaluated, road and other infrastructure improvements deemed necessary to provide for consistent opportunity for ingress/egress by emergency services (including United States Forest Service personnel and fire agencies) and residents of/visitors to the area would need to be conducted prior to removal of Potter Valley Project Dams and Equipment. Additionally, there should be clarification on which entity will be responsible for decommissioning or maintaining project-related access roads that currently connect to the county system; and the EIR should ensure no transfer of long-term maintenance

responsibility for project-related infrastructure to Lake County without adequate funding or appropriate agreements.

There is concern that during high rainfall, there will be localized flooding in areas that were previously unaffected as a result of sediment deposition. These changes can heighten the risk of flooding at downstream structures, such as bridges and road crossings. There is a proposed Sediment/Channel Monitoring and Response Plan to monitor problematic levels and interventions.

Roadwork improvements and modifications will be needed to accommodate the anticipated increased construction traffic and heavy equipment movement on local roads during the dam decommissioning (*e.g.*, Elk Mountain Road and Soda Creek Road). The EIR should identify all haul routes, construction access roads, and staging areas, including those traversing Lake County. The general construction measures need to include speed limits and dust control, and it should also determine appropriate construction traffic road impact fees to mitigate those impacts.

PG&E proposes preparing a Construction Transportation Management Plan and following the General Construction Measures to mitigate transportation network effects. The construction transportation plan needs to include non-construction-related traffic that is expected to be present in the vicinity of Lake Pillsbury. It should also include pre- and post-construction road condition assessments and coordination with Lake County Public Works to ensure repair or mitigation of any damage caused by construction traffic.

PG&E has not provided detailed assessment of road capacity, load-bearing limits, or required upgrades to support increased construction traffic. The EIR should analyze the alternatives for these activities and set forth appropriate alternatives and mitigation measures.

*Tribal Cultural Resources*

The extant Lake Pillsbury Inundation Area and broader geographical region affected by the Potter Valley Project facilities and conveyances hold great historical and cultural significance to more than 20 federally recognized tribes, many of whom have not been robustly consulted by PG&E, to date. Each of these sovereign Tribal Nations must be provided the opportunity through early and substantial consultation to raise their unique concerns.

*Utilities/Public Services*

The EIR should discuss and evaluate the existing power grid in the area and provide mitigation measures for any impact on the power grid due to Scott Dam and Pillsbury Lake removal. The EIR should also consider how distribution capacity at the Powerhouse could be best repurposed; the Proposed Decommissioning should not damage equipment that could be repurposed to distribute power generated by an alternative project. PG&E notes that the project powerhouse in Potter Valley has not generated power since May 2021, and that

decommissioning would permanently eliminate generation capabilities. The Final Application does not explicitly state whether the vicinity of Lake Pillsbury is connected to the electrical grid, and it provides no information concerning impact of the Proposed Decommissioning on electrical utilities. Nor are telecommunications addressed.

The EIR should assess the sufficiency of existing communications infrastructure, whether land-based or wireless/satellite, to ensure public and worker safety and adequacy of emergency response communications during implementation of the Proposed Decommissioning.

*Wildfire*

The residents of the County of Lake have repeatedly faced the horrors of wildfires, and the County has endured more than ten devastating wildfires within the last ten years — many receiving State and Federal Emergency Declarations. As a result, the County has lost greater than 60% of its land mass to wildfire damage. Fire Chiefs and other first responders in Lake County and the broader region can speak to the critical nature of Lake Pillsbury in firefighting efforts, year after year. Reducing water storage is not what California needs, and it is not what Lake County needs at all, especially in the areas surrounding Lake Pillsbury. Lake Pillsbury has historically supported aerial firefighting operations for (and provided a barrier to further progression of) wildfires, including two of the largest events in California's modern history; the Mendocino Complex Fire (2018) and August Complex Fire (2020). The reservoir has reliably supported suppression efforts throughout its operational history, even during drought years; Lake Pillsbury Fire Protection District Chief, Larry Thompson, and others have likewise spoken in public fora to Lake Pillsbury's value in suppressing small wildfires and forestalling the potential for a greater number of significant wildfire events. The USFS and Lake Pillsbury Fire Protection District (the primary responders to fires in the area) have indicated that the transition to land management-based practices would take time, and lack of a nearby water source would result in small fires growing larger, affecting regional resources. While CAL FIRE has in the past offered a statement of no impacts regarding the Proposed Decommissioning, but it is important to note that CAL FIRE is not the primary responder for the area and thus its views may be of less relevance. The views of the USFS and, in particular, the Lake Pillsbury Fire Protection District (see attached letter) should be given due consideration.

In addition to fire suppression, wildfire events produce significant smoke exposure across Lake County, impacting public health and air quality. The reservoir's role in supporting emergency response indirectly contributed to mitigating smoke-related health risks by enabling faster containment of fires.

The proposed removal of Scott Dam and the subsequent loss of Lake Pillsbury present significant challenges for wildfire suppression and vegetation management in Lake County and the surrounding regions. The potential negative impacts include reduced water availability for firefighting, increased turnaround times, limitations on firefighting strategies,

and complex vegetation management issues. Substituting the water used from Lake Pillsbury for fire suppression will have a significant impact on response times, which is particularly concerning because the Lake Pillsbury basin and surrounding forested areas are rated as "Very High" on CAL FIRE's Fire Hazar Severity Zone. The delay in fire suppression efforts would also impact the County's Mutual Aid partners who respond to these fires (*e.g.*, Northshore Fire Protection, Potter Valley Fire Protection District, USFS, and CAL FIRE). These challenges are particularly concerning given the area's history of large-scale wildfires and the increasing fire risk due to climate change. While the dam removal project aims to improve river ecosystem health and fish passage, it is crucial to carefully consider and mitigate the potential increased fire risks and suppression challenges.

Implementing a comprehensive set of mitigation strategies, including enhanced water infrastructure, augmented firefighting resources, proactive vegetation management, and adaptive management approaches, would be essential to maintaining effective fire suppression capabilities and managing vegetation-related fire risks in a post-dam removal landscape. Further research and detailed planning are necessary to fully understand and address the long-term implications of this project on wildfire management in the region. Collaboration between fire management agencies, ecological researchers, and local communities will be crucial in developing effective strategies to navigate these challenges and ensure the continued protection of the area's natural resources and human communities from wildfire threats.

For example, the Lake Pillsbury Fire Protection District suggests that PG&E have a fire crew on-site during any decommissioning construction projects thus providing any needed immediate initial attach on a potential fire. An on-site crew could also provide immediate basic life support medical-aid during any medical emergencies. This fire crew would need common radio and cellular communications with both the Lake Pillsbury Fire Protection District and the Primary Emergency Command Center (Howard Forest ECC). PG&E would need to provide either a mobile cellular tower — or preferably a satellite system — for immediate cellular phone use.

The EIR should evaluate and fully analyze the adverse impacts of the Proposed Decommissioning on wildfire preparedness and response, considering alternatives and recommending mitigation for these impacts.

**Alternative Scenarios**

Despite PG&E's lack of provided alternatives multiple approaches could mitigate above-mentioned issues with the Proposed Decommissioning. Alternatives described below are set forth in greater detail in the attached SLR Report. The County requests each of these

alternatives be included in the EIR's alternatives analysis. The County also looks forward to reviewing the EIR's thorough evaluation and consideration of a "no-project" alternative.

*PG&E's Proposed Removal with Additional Mitigation*

Under this scenario, PG&E proposes the full decommissioning of Scott Dam and the County of Lake proposes additional mitigation efforts implemented to address their concerns and those of the local community. Where PG&E's current Proposed Decommissioning proposes passive sediment removal options, this suggested scenario presents active solutions to mitigate downstream sediment. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the County of Lake, and lessen the environmental impacts when compared to PG&E's current Proposed Decommissioning. This suggested option would eliminate Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and would have higher ecosystem benefits to the community. However, it would include the significant impacts to the community of eliminating Lake Pillsbury and at least some of the current recreational uses of the project area, along with the noted financial impacts to the community and the County noted above.

*Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder*

This scenario is similar to the current operational conditions described, with the addition of a fish pathway. The hydrologic conditions would be similar to the current operational conditions as well. The primary objective of this scenario would be to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure.

To implement this alternative, a seismic retrofit would also likely be required. From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures. The hydrologic and hydraulic evaluation is likely similar to the current condition's scenario, but specific hydrology analysis will need to be performed to accommodate a fish pathway to the existing Scott Dam and sufficient biological evaluation and expert recommendations will be required.

*Lowered Scott Dam with Fish Ladder*

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 feet (PG&E datum) and the installation of a fish passage system, such as a fish ladder structure. The primary goal of this scenario would be

to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir with related likely severe impacts.

Lowering the spillway invert elevation to 1,875 feet would create a reservoir of approximately 40 percent of the current storage volume. Potentially, the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain minimum ecological outflows to the Eel River and allow for managed sediment flushing from Scott Dam as needed. Without the requirement to have a larger water storage as the current maximum operational water surface elevation (WSE) of 1,900 feet, the spillway invert may be further reduced in height by 25 feet below the existing spillway invert, which would result in a new lake footprint. At that reduced maximum operational WSE, the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam abutment area (where previous slides have occurred) by placing the concrete material on both or either of the downstream and upstream sides of the lowered dam. Further evaluation would be required to understand the effects on the adjacent land area and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation, and ecosystem benefits as compared to the full decommissioning.

For this option as well, practices such as planting native vegetation, restoring the channels, creating a restored wetland area for the newly exposed portions of the lakebed would help maintain some of the recreational and economic benefits for the County of Lake, and lessen the environmental impacts. This option would also include some, but not all, of the significant impacts to the community of eliminating a portion of Lake Pillsbury and at least some of the current recreational uses of the project area, likely along with some of the noted financial impacts to the community and the County noted above.

*Reduced Reservoir with Salmon Creek as Main Tributary*

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure. A new 3,100-foot-long earthen dam would be established in the Salmon/Smokehouse and Habematolel Creek watershed areas to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir with contributions. The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction, with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within the Lake Pillsbury footprint can also be reused for the new impoundment construction if suitable. Low-permeability clay/silt sediments can be used in

the core of the dam, and gravelly sediments can be used in the embankment/slopes of the new dam.

The water storage will be based on the Salmon/Smokehouse and Habematolel Creek watershed areas, and it is assumed the maximum capacity (approximately 9,990 acre-feet) would be provided at a maximum operational WSE of 1,885 feet. For this scenario, the approximate surface area would be 580 acres, approximately 30 percent of the current size. The new dam would be constructed at the narrowest location possible that would not impair the flows through the Eel River. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational WSE. An active low-level outlet structure would not be included, though a passive outlet is considered for potential draining. This option will support continued use of some recreation facilities, ecosystem benefits, fire suppression, and the upstream fish pathway along the Eel River, as Scott Dam will be removed as a fish barrier.

*Dam Removal with Off-Channel Pond Restoration*

This scenario presents an enhanced version of proposed restoration efforts mentioned in the first suggested alternative scenario (PG&E's Proposed Removal with Additional Mitigation), but with specific mitigation added to enhance fish passage while maintaining fire suppression and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi- equilibrium[11], off-channel ponds would be constructed.

Two potential pond locations have been identified as shown in Figure A7-24[12] of the attached SLR Report based upon the 1922 historic pre-dam topography and bathymetry. The 1922 topography and bathymetry were georeferenced to the Lake County hillshade to predict how the site will look after restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of the Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table A7-16[13] of the attached SLR Report summarizes the potential footprints and volumes of Ponds A and B assuming a 10-foot normal depth. The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design. The conceptual

[11] Quasi-equilibrium in this case refers to the ability of a river to maintain its dimension, pattern and profile without either aggrading or degrading over time.
[12] SLR Impact Analysis and Adaptation Strategy Evaluation Report, Appendix A, page 65.
[13] SLR Impact Analysis and Adaptation Strategy Evaluation Report, Appendix A, pages 63-64.

design would function similarly to the example provided in Figure A7-25[14] of the attached SLR Report, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Conclusion**

The County of Lake appreciates the opportunity to provide comments relevant to EIR considerations of impacts and mitigations related to PG&E's Potter Valley Hydroelectric Project Surrender and Decommissioning proposal, as well as evaluation of all feasible project alternatives including the no-project alternative. Please contact County of Lake Chief Deputy County Administrative Officer Matthew Rothstein, at Matthew.Rothstein@lakecountyca.gov or (916) 544-8150, if you have any questions, or if you would like to have copies of any of the materials referenced in this letter or the attachments.

Sincerely,

Lloyd C. Guintivano
County Counsel
County of Lake

Attachments:
SLR Impact Analysis and Adaptation Strategy Evaluation Report dated Aug. 21, 2025
SLR Existing Conditions Report dated June 6, 2025
PG&E's Potter Valley Hydroelectric Project, FERC Project No.77-318 Application for Non-Capacity License Amendment Response to Additional Information Request dated Sept. 26, 2025
Lake Pillsbury Fire Protection District letter of March 1, 2025 to PG&E
Lake County Air Quality Management District comments dated Oct. 30, 2025
Mendocino County Inland Water and Power Commission letter of Sept. 16, 2025 to Mendocino County Board of Supervisors
Conservation Easement between PG&E and Mendocino Land Trust dated June 28, 2022

Links:
Division of Safety of Dams inspection report of Van Arsdale Dam (Oct. 31, 2023), available at https://www.lakecountyca.gov/DocumentCenter/View/14815/97102_Van_Arsdale_2510_Insp_Rep_10182023_Redacted

[14] SLR Impact Analysis and Adaptation Strategy Evaluation Report, Appendix A, page 66.

Division of Safety of Dams inspection report of Scott Dam (Nov. 1, 2023), available at https://www.lakecountyca.gov/DocumentCenter/View/14814/97101_Scott_2510_Ins_Report_10182023_Redacted

Farm Bureaus' Letter to Federal Agencies (Apr. 4, 2025), available at https://www.lakecountyca.gov/DocumentCenter/View/14648/Farm-Bureaus-Letter-to-Federal-Agencies-for-Intervention-April-2025

PG&E's comments to FERC (July 24, 1978), available at https://www.lakecountyca.gov/DocumentCenter/View/14816/Pacific-Gas-and-Electric-company-Comments--Potter-Valley-Project-No-77-EIS-1978

PG&E North Coast Region Vice President Dave Canny's remarks regarding seismic concerns of Scott Dam (May 26, 2025), available at https://lakeconews.com/news/community/letters/81648-canny-potter-valley-projects-surrender-and-decommissioning

PG&E North Coast Region Vice President Dave Canny's remarks regarding seismic safety (Sept. 23, 2025), available at https://lakeconews.com/news/82701-lake-county-officials-challenge-pg-es-claims-about-potter-valley-projects-seismic-safety

The County of Lake's comments to FERC (Aug. 25, 2025), available at https://www.lakecountyca.gov/DocumentCenter/View/14812/Protest-and-Comments-of-the-County-of-Lake

cc: Ruth Stoner Muzzin, Esq., Friedman & Springwater LLP

# Impact Analysis and Adaptation Strategy Evaluation Report

## Lake County Resource Assessment – Proposed Scott Dam Decommissioning

### County of Lake

255 N. Forbes Street
Lakeport, CA 95453
Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

August 22, 2025

Revision: 00

Making Sustainability Happen

# Cover Letter

August 22, 2025

County of Lake
255 N. Forbes Street
Lakeport, CA 95453

**RE: Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project, Proposed Scott Dam Decommissioning by Pacific Gas and Electric (PG&E)**

To whom it may concern:

On behalf of the County of Lake, SLR International Corporation (SLR) has prepared this Report as part of our effort to support the Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project. SLR is conducting a comprehensive evaluation aimed at assessing the potential impacts of decommissioning Scott Dam and draining Lake Pillsbury on various resources and stakeholders in the surrounding community. The grant project, funded by the Department of Water Resources (DWR) Agreement Number 4600015984, dated June 12, 2024 with the County of Lake aims to provide a thorough understanding of the existing conditions that would be affected by the removal of the dam and lake. The following tasks define Grant scope of work:

1. Project Administration
2. Existing Condition Evaluation
3. Impact Analysis and Adaptation Strategy Evaluation

This report has been prepared by SLR International Corporation (SLR) to meet the requirements of the DWR agreement. The material and data in this report were prepared under the supervision and direction of the undersigned.

Sincerely,

**SLR International Corporation**

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# Executive Summary

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in Pacific Gas and Electric Company (PG&E)'s Potter Valley Hydroelectric Project (facilities shown in Exhibit 1). Scott Dam is located on the Eel River in Lake County, within the Mendocino National Forest. PG&E is proposing to decommission the Potter Valley Hydroelectric Project (PVP), which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam and other associated facilities. In this regard, PG&E has submitted a Final Surrender Application (PG&E, 2025c) to the Federal Energy Regulatory Commission (FERC). requesting approval of the decommissioning.

The decommissioning process proposed by PG&E includes the removal of Scott Dam and draining Lake Pillsbury. The removal of Scott Dam and loss of Lake Pillsbury is expected to have complex impacts on various aspects of the environment, economy, and community in particularly within the County of Lake. SLR has been contracted by the County of Lake to support them on evaluating these impacts to the county as per a grant provided by the Department of Water Resources of the State of California (DWR). SLR's scope of work is to review existing conditions in the project area, identify the potential impacts of the dam decommissioning and then develop appropriate adaptation strategies to mitigate them to meet county and community needs.

SLR has reviewed the PG&E's Final Application for Surrender of License (PG&E, 2025c) and their suggested mitigation approaches for the PVP and specifically for the Scott Dam and Lake Pillsbury decommissioning to identify the potential impacts and PG&E's suggested mitigation options. SLR then have additionally developed other applicable and feasible mitigation options to achieve project goals desired by both the County of Lake, community and PG&E.

Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Five alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Current Operational Conditions Scenario covers operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Final Application for Surrender of License dated July 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habermatolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.

7. Scenario E describes the removal of Scott Dam and installation of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

Based on SLR's evaluation, PG&E's Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by previous studies, indicating shortcomings in its anticipated outcomes. In contrast, we identified five other alternatives that rank more favorably, and should be further evaluated for implementation before FERC issues a License Surrender Order.

This report summarizes the evaluation of the potential impacts of removing Scott Dam and above scenarios as preliminary adaptation strategies. This report is prepared to meet the deliverable requirements of the DWR grant work plan as well.

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**Appendix D Climate Vulnerability**

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# Acronyms and Abbreviations

| | |
|---|---|
| AACE | Association for the Advancement of Cost Engineering |
| ac-ft | acre-feet |
| BIOS | Biogeographic Information and Observation System |
| CDFW | California Department of Fish and Wildlife's |
| CEED | Center for Environmental Economic Development |
| CESA | California Endangered Species Act |
| CEQA | California Environmental Quality Act |
| cf | cubic feet |
| cfs | cubic feet per second |
| cy | cubic yards |
| CDEC | California Data Exchange Center |
| CNDDB | California Natural Diversity Data Base |
| DSOD | Division of Safety of Dams |
| DWR | Department of Water Resources |
| EIR | Environmental Impact Report |
| EIS | Environmental Impact Statement |
| ESA | Endangered Species Act |
| FERC | Federal Energy Regulatory Commission |
| ft | Foot or feet |
| GRC | General Rate Case |
| HEC-RAS | Hydrologic Engineering Center’s River Analysis System |
| LAT | Large Airtankers |
| NEPA | National Environmental Policy Act |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NERF | New Eel Russian Facility |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NIDIS | National Integrated Drought Information System |
| NMFS | National Marine Fisheries Service |
| NOA | Naturally Occurring Asbestos |
| NOAA | National Oceanic and Atmospheric Administration |
| PAD | Pre-Application Document |
| PG&E | Pacific Gas and Electric Company |

| | |
|---|---|
| PVP | Potter Valley Project |
| PVID | Potter Valley Irrigation District |
| SLR | SLR International Corporation |
| USACE | United States Army Corps of Engineers |
| USDA | United States Department of Agriculture |
| USFS | United States Forest Service |
| USGS | United States Geological Survey |
| VLAT | Very Large Airtankers |
| WY | water year |

# 1.0 Introduction

SLR International Corporation (SLR) has been contracted by the County of Lake (the County) to provide technical assistance needed to better understand the proposed Scott Dam Decommission by the Pacific Gas & Electric Co. (PG&E) as part of the overall decommissioning proposed for the Potter Valley Hydroelectric Project (PVP; *Federal Energy Regulatory Commission [FERC] Proceeding No. 77*). PVP facilities are shown in Exhibit 1. On July 25, 2025, PG&E submitted a Final Application for Surrender of License and Application for NonProject Use of Project Lands for the PVP to the FERC indicating the project is not economically feasible. The Final Application for Surrender of License includes information on the decommissioning process for the Potter Valley Hydroelectric Project, and for the construction of a New Eel Russian Facility, which is identified as non-project use of project lands by the Eel-Russian Project Authority (ERPA). SLR has carefully reviewed the information available to date for this proposed action by PG&E and prepared this report to document several mitigation concepts for discussions between SLR and the County.

Based on our review and project understanding we have listed PG&E's project goals below as applicable to Scott Dam and Lake Pillsbury (located within the County) and the desired goals of the County (including some local community interests) with respect to the proposed actions.

## 1.1 Report Organization

The report is organized into several sections to facilitate understanding and navigation:

**1.0 Introduction:** This section provides an overview of the report's purpose and scope, and an overview of the project goals.

**2.0 Summary of Background Information:** This section provides an overview of the background information for the project.

**3.0 Scott Dam Scenario Description:** This section specifies scenarios for adaptation.

**4.0 Preliminary Comparison of Scenarios:** This section presents a preliminary comparison based on relevant criteria identified in the previous sections.

**5.0 Climate Vulnerability:** This section summarizes climate change effects on Lake County and cumulative effects if Scott Dam is decommissioned.

**6.0 Conclusions:** This section summarizes the key findings and insights derived from the analysis.

**7.0 Remarks:** This section specifies professional scope and remarks in the report.

**8.0 References:** This section provides a list of sources cited in the report.

**Figures:** Figures created by SLR present relevant information.

**Exhibits:** Figures and diagrams created by others are included as exhibits.

**Tables:** Tables summarize information, and are included in text and appended.

**Appendix A:** Presents detailed information on existing information and models relevant to hydraulic aspects.

**Appendix B:** Provides additional information on fire suppression.

**Appendix C:** Provides detailed evaluation on fish passage for each scenario.

**Appendix D:** Presents considerations related to Climate Vulnerability.

**Appendix E**: Presents considerations related to the permitting process.

## 1.2 Decommissioning Project Goals

According to PG&E, its overall project goal is the decommissioning of the PVP as the hydropower generation is no longer economically viable. PG&E therefore expects to obtain FERC's approval for the decommissioning as proposed in the Application for Surrender of License and complete the PVP decommissioning including the Scott Dam and Lake Pillsbury. The County of Lake main goal is to accommodate best and most suitable mitigation options to benefit the county and the community with respect to the Scott Dam decommissioning. More specific discussions of these goals are provided below.

### 1.2.1 PG&E's Project Goals

As stated in their Application for Surrender of License, PG&E's goals at the conclusion of the decommissioning process are:

1. To remove the Project facilities and features including but not limited to Scott Dam and Cape Horn Dam,
2. To remove the Project from FERC jurisdiction, and
3. no longer operate or maintain the Project and Project features in the future.

SLR's general understanding is that the following additional goals, facts and interests played a role in PG&E's proposed decommissioning approach:

- PG&E recognized the PVP as uneconomic for its customers. On January 25, 2019, PG&E rescinded its Notice of Intent and Pre-Application Document for License renewal.
- Operation and Maintenance (O&M) of the PVP is no longer economically feasible and therefore the need for water storage previously created by Scott Dam and Lake Pillsbury for hydropower plant is no longer needed. With FERC's approval of the Application for Surrender of License, PG&E would start the decommissioning process for Scott Dam which would result in the removal of Lake Pillsbury. Once the decommissioning and restoration process is complete, PG&E expects to limit their financial and regulatory responsibilities.
- The initial design capacity of Lake Pillsbury was 76,876-acre feet (ac-ft) at Elevation 1,910 feet (ft)[1]. PG&E reduced this capacity in 2023 due to concerns of dam instability, to a maximum elevation at the spillway at 1,900 ft.
- The CA Division of Dam Safety (DSOD) has agreed with PG&E's seismic evaluation and granted approval to operate at the current spillway crest at Elevation 1,900 ft to reduce risk of failure[2]. In April 2023, DSOD provided comments for the simplified seismic stability analysis and proposed interim risk reduction measures. On July 28, 2025, FERC once again asked for updates to risk reduction measures currently implemented or proposed since this letter was issued.
- Due to the presence of Scott Dam, fish migration along the Eel River to the upper reaches has not been possible. Several interest groups along the Eel River have voiced

[1] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. Elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.

[2] DSOD, 2023. Scott Dam, No. 97-101, Lake County. April 12.

support for the removal of the dam to create an unrestricted fish passage (primarily anadromous salmonids, including chinook salmon, coho salmon and steelhead) upstream of the Scott Dam. A working group evaluated alternatives for the Potter Valley Project, including adding fish passage to the existing Scott Dam structure[3]. The working group parties[4] determined the preferred alternative was to decommission Scott Dam and to add fish passage to Cape Horn Dam[5]. The County of Lake and other stakeholders were not meaningfully considered in these discussions and did not participate in the selection of alternatives.

- FERC has allowed PG&E to formally start the license surrender process to proceed following the FERC procedures. A Final Application for Surrender of License was submitted to FERC on July 25, 2025. As planned, PG&E would begin the detailed planning and design of the decommissioning of the PVP (including Scott Dam) following the final application submittal, and they anticipate one to two years to prepare Management Plans[6].
- PG&E and the Eel-Russian Project Authority (ERPA)[7] have signed an agreement to proceed with the construction of the New Eel-Russian Facility (NERF) to continue Eel River water diversions to the Russian River.

### 1.2.2 County of Lake Project Goals

The presence of Scott Dam and Lake Pillsbury has been a vital resource for local counties and the state of California for over one hundred years. PG&E has not considered the true loss to the community in their planning of the Scott Dam decommissioning, and that is not acceptable to the County of Lake, local communities, and other agencies. The County of Lake would like PG&E and FERC to address their concerns as part of the decommissioning process. If they address the County of Lake's concerns, FERC would include certain mitigation procedures proposed by the County in their final Regulatory Order to PG&E to comply. The County of Lake requested PG&E to accommodate mitigation measures that are important to the County and find ways to integrate those measures into final decommissioning plans and requirements so that lost values can be replaced with new options including compensating for financial losses.

The County of Lake has identified several major areas that would be affected by the removal of Scott Dam and Lake Pillsbury. Those areas include:

1. Water Supply to the nearby users and commercial facilities.
2. Fire Suppression for wildfire management both from aerial attacks and ground attacks (like during the last two major wildfires that happened in the area).
3. Loss of revenue from decrease in land value and the recreation facilities and activities.
4. Recreation in and around the lake (see current facilities in Figure 1).
5. Ecosystem that has been well established over one hundred years around the lake (wildlife, terrestrial, avian, and lake fish).
6. Recharge to groundwater in the vicinity of Lake Pillsbury.

[3] Stillwater Sciences, et. al, 2021. Potter Valley Project Feasibility Study: Capital Improvements. April.

[4] The working group parties include California Trout, Humboldt County, Mendocino County Inland Water and Power Commission, Round Valley Indian Tribes, and Sonoma County Water Agency.

[5] Stillwater Sciences, et. al, 2021. Potter Valley Project Feasibility Study: Alternatives Description and Project Plan. April

[6] Information verbally presented by PG&E in an August 11, 2025 townhall.

[7] A joint powers authority formed by a joint exercise of powers agreement between the County of Sonoma, Sonoma County Water Agency (Sonoma Water), and the Mendocino County Inland Water and Power Commission.

7. Environmental impacts due to the sediment release and debris management and other soil management.
8. Other impacts that may be generated, for example the release of asbestos during deconstruction, impacts on infrastructure, noise and aesthetics.

## 1.3 Other Considerations for Mitigation Options

Based on SLR's review of the PG&E's Final Application for Surrender of License, existing conditions and County provided information, we have identified scenarios that would provide more equitable mitigation options for all the stakeholders.

From a technical perspective, including geological, geotechnical and slope stability around Scott Dam and Lake Pillsbury, there is a need to better address long term site stability. Also, construction concerns associated with the presence of naturally occurring asbestos (NOAs), need to be considered in mitigation. A better plan for road infrastructure improvements is also needed.

From the information provided by PG&E and other parties with respect to the decommissioning of Scott Dam and removal of Lake Pillsbury, PG&E's decision lacks consideration of local community needs and inclusion of other mitigation options. It appears that only certain stakeholders' views were taken into account, and PG&E's decision was not representative of any outreach to the County and local community members residing around Lake Pillsbury. Additional considerations should be given to the County of Lake's goals and that can be accomplished by further evaluation of options as presented in this report. Not only the County and local communities but also entities like CalWild[8] have identified and stated the loss of water storage at Lake Pillsbury is a great concern to all parties, and therefore PG&E needs to invest in additional mitigation measures.

In this report, SLR presents several mitigation option scenarios considering the overall need of both PG&E and the County of Lake. We recommend that these options be evaluated and compared to the Scott Dam removal alternative. The scenarios identified below allow for a comparative analysis and support the selection of a more acceptable mitigation option to address stakeholder needs.

## 1.4 Impact Analysis and Adaptation Strategy Evaluation Scope

SLR has identified various impacts to the County of Lake and the local community based on the Scott Dam decommissioning process proposed by PG&E as presented in their Final Application for Surrender of License published in July 2025. SLR and the County of Lake submitted joint comments on the Final Application for Surrender of License (SLR, 2025a, County of Lake, 2025) and those comments include general comments and specific comments with details to document identified impacts under various technical and resources categories as presented in the application.

SLR's Existing Conditions Report provides a summary assessment of the present conditions in the vicinity of Scott Dam and Lake Pillsbury (SLR, 2025b). Existing conditions were documented in relationship to proposed PG&E decommissioning in the following areas as related to the

[8] CalWild is a California-based public benefit corporation, which goal is to protect and restore California's landscape. As part of the PVP decommissioning process their role is to develop collaborative consensus, their work is funded by grants from the California Coastal Conservancy and the Resources Legacy Fund.

proposed decommissioning: Recreation, ecosystem, wildfire suppression, power, geotechnical, sediment, water supply, groundwater, hydrology, roadways and economic aspects.

Based on the above findings from SLR's reviews to date, we have focused on the following key impact areas to further document impacts and evaluation of those impacts as the basis of identifying preliminary mitigation options to mitigate or eliminate such impacts.

**Recreation**: PG&E's proposed decommissioning plan does not address the need to minimize the recreational impacts or replace the recreational facilities to fully benefit the local economy and community.

**Ecosystem (Wildlife, Botanical and Aquatic resources**): The decommissioning of the Scott Dam and Lake Pillsbury removes many ecosystem resources that have been vital to the current ecosystem elements in the area. Even though the Eel River restoration benefits fish migration upstream along the river, other fish resources in the lake would be lost.

**Wildfire Suppression**: Lake Pilsbury provides a crucial water supply needed for major forest fires in the County of Lake. No consideration is given by PG&E for the replacement of this water supply need in the Final Application for Surrender of License.

**Infrastructure/Water Supply** (including downstream water needs and groundwater use): Lake Pilsbury has been used to maintain the downstream water supply need (through minimum flow releases through needle valve control at Scott Dam) but the reliability of downstream water flow under natural summer conditions cannot meet such downstream requirements. The impact on local groundwater use due to loss of lake recharge would also be lost with the removal of Lake Pillsbury. Proper understanding of the local hydrology/hydraulic are key aspects in evaluating the impacts to water supply needs.

**Restoration** (includes water quality and sediment management): PG&E's current plan considers preparing a sediment and water quality management plan, but no details have been provided. There are various environmental and reuse benefits on sediment reuse that can be incorporated into mitigation options that have not been considered by PG&E.

**Economic and Cost Conditions** (both from project implementation costs as well as revenue lost to the County of Lake and the local community): The current information provided by PG&E is not accurate and incomplete. However, the cost evaluations completed previously by Stillwater Sciences and McMillen Jacobs are summarized where applicable in this report.

**Other Impact Areas** (including geotechnical and roadway infrastructure): Scott Dam is currently operated at 1,900 ft spillway crest elevation to mitigate seismic risk, which has been approved by FERC and DSOD[9]. Roadway improvements or replacements needed for the benefit of local communities are not addressed adequately as part of the decommissioning process.

SLR has considered the above key impact areas and evaluated several mitigation scenarios that would better address the County of Lake goals, local community concerns as well accomplish PG&E goals to achieve more equitable outcome of the proposed decommissioning project. The purpose of this report is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of adaptation strategies required under the Department of Water Resources (DWR) grant. Specifically, this report outlines how potential decommissioning scenarios may affect the Eel River system and

[9] In a virtual townhall meeting on August 11, 2025, PG&E stated with the current operational level there is no immediate safety risk and Scott Dam can continue to operate safely.

identifies potential restoration and adaptation opportunities within the Lake Pillsbury area for different mitigation scenarios.

A range of high-level options were developed and organized into a set of preliminary alternatives. This report presents the initial analyses conducted to assess the feasibility of these alternatives, with a focus on the effects on the Eel River hydrology and hydraulics. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource. Mitigation strategies presented in this evaluation may be used as mitigation measures associated with the proposed PG&E remedy.

This analysis does not represent a final decision or commitment to any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

This report represents an initial step in a broader, ongoing process. In addition to the further work to be done by PG&E after submitting the Application for Surrender of License to FERC in July 2025, additional studies, analyses, and synthesis will be needed to refine the proposed scenarios to make go forward decisions by the County of Lake and other stakeholders. Community and regulatory outreach will also be needed to better accommodate their concerns with respect to selected mitigation options.

SLR's proposed mitigation scenarios are based on our current understanding of the relevant project background information, our evaluation of hydrology and hydraulic conditions, knowledge of local wildfire suppression, recreation, ecosystem specifically understanding of fish passage needs, and the knowledge of local economic conditions. We have provided applicable summaries of these aspects below.

# 2.0 Summary of Background Information

The construction of Scott Dam started in 1920 and was completed in 1922, creating Lake Pillsbury. Snow Mountain Water and Power Company was the original owner. They acquired a Department of Agriculture Power permit in 1920. Then, the Federal Power Commission (the *precursor to FERC*) granted a 50-year license for the Project in 1922. In 1930, PG&E acquired the Project from Snow Mountain Water and Power and assumed the license (PVID, 2024; McMillen Jacobs Associates, 2021).

Scott Dam is a concrete, gravity-type, ogee-shaped structure. It is 130 ft high and 805 ft long and classified as a high hazard dam[10] according to the U.S. Army Corps of Engineer's Institute of Water Resources National Inventory of Dams database. The spillway crest elevation is 1,900 ft[11] built in 1922. Additionally, each of the five radial gates surmounting the spill crest is 32 ft wide by 10 ft high, made of 26 steel slide gates, each 10 ft high and varying width from 7.5 ft to 10.08 ft. The storage releases are 72-inch diameter, riveted-steel outlet pipe passing through the dam at an invert elevation of 1,812 ft. The storage releases are controlled by a 42-inch Lauren-Johnson needle valve.

At initially constructed, Lake Pillsbury had a surface area of approximately 2,275 acres, with an elevation of 1,910 ft and maximum storage capacity of 76,876 ac-ft. Since inception, Lake

[10] High hazard dam classification means that that dam failure could pose serious threat to downstream communities and infrastructure.

[11] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. PG&E also presents elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.

Pillsbury's capacity has reduced based on operational changes, sediment accumulations, and most recently in 2023 due to concerns of dam instability during seismic activity, to a maximum elevation at the spillway at 1,900 ft. Therefore, the reservoir is currently operated for a storage capacity of 66,876 ac-ft. Operational footprints are shown in Figure 2.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The decommissioning process considers removal of Scott Dam and draining Lake Pillsbury. PG&E prepared an Updated Final Application for Surrender of License and Application for Non-Project Use of Project Lands in July 2025 (mentioned as "Final Application for Surrender of License") which has been circulated to agencies and interested parties for comments (PG&E, 2025c). The proposed decommissioning of Scott Dam is also considered by some interested parties as part of a broader initiative to restore the Eel River's ecosystem and improve fish passage.

In addition to power generation, the PVP has been providing water supply for agriculture and recreation in Mendocino and Sonoma Counties. A key aspect of the Potter Valley Project is the Cape Horn Dam, where water diversions to the Russian River basin occur. As part of the license surrender a New Eel Russian Facility (NERF) to maintain water diversions is proposed by PG&E as a non-project use of project lands by the Eel-Russian Project Authority (ERPA). Since these diversions take place downstream of the Lake County's boundary, it is expected that the impact on the County of Lake will be minimal.

SLR's Existing Conditions Report provides a summary assessment of the present conditions in the vicinity of Scott Dam and Lake Pillsbury (SLR, 2025a). Existing conditions were documented in relationship to proposed PG&E decommissioning in the following areas as related to the proposed decommissioning: Recreation, ecosystem, wildfire suppression, power, geotechnical, sediment, water supply, groundwater, hydrology, roadways and economic aspects.

Regulatory flow requirements are specified in the FERC license and the National Marine Fisheries Service's (NMFS's) Reasonable and Prudent Alternative (NMFS, 2002). These minimum flows range from 20 to 60 cfs between June 1 and November 30 and range from 20 to 100 cfs between December 1 and May 31 depending on water year classification.

In the July 2025 Final Application for Surrender of License, PG&E states that they have historically diverted around 39,000 ac-ft from the Eel River to PVID and the East Branch of the Russian River (PG&E, 2025c). ERPA proposes annual diversions from the Eel River of 3,000 – 7,500 ac-ft to PVID and 24,800 – 27,500 ac-ft to the East Branch of the Russian River, (PG&E, 2025c). The combined flows would equal between 55 – 120 cfs diverted during the wet season, and 1 – 6 cfs diverted during the dry season (PG&E, 2025c). ERPA's goal for adjusting diversions seasonally is to support ecological restoration following the dam removal, whereas PG&E's goal with diversions has been to generate hydropower and maintain infrastructure (Sonoma Water, 2024). If PG&E's application results in a transfer of responsibility to ERPA, ERPA will oversee the operations of Cape Horn Dam, Van Arsdale Reservoir, the Van Arsdale diversion and fish screening facility, PVID, tunnels and conveyance systems, and water rights and authorities that currently fall with PG&E. ERPA would coordinate this transfer of rights and responsibilities with PG&E and aim to align this transfer with PG&E's proposed decommissioning schedule. ERPA's next steps, in the case of PG&E's surrender, are to coordinate with PG&E on the transition and SCADA integration, perform geotechnical and hydraulic studies, risk assessments, and life cycle cost analyses for the facilities.

## 2.1 Hydrology and Hydraulic Conditions

To support the hydrologic and hydraulic evaluation of PG&E's proposed Scott Dam removal, SLR reviewed a broad set of technical studies, datasets, and modeling tools. These included PG&E's surrender application, consultant reports, and publicly available data from United States Geological Survey (USGS), National Oceanic and Atmospheric Administration (NOAA), National Integrated Drought Information System (NIDIS), National Centers for Environmental Information (NCEI), and California Data Exchange Center (CDEC). The evaluation was conducted focusing on potential impacts to water supply, flood risk, and seasonal hydrology.

SLR's analysis incorporated reconstructed water balance modeling, flood frequency assessments, and climate condition evaluations. The Stillwater Sciences (2021e) hydraulic model projected a potential increase in 100-year flood water surface elevations by up to 1.2 ft following dam removal. However, the model lacked detailed operational inputs and was not available for independent review.

Climate conditions were evaluated using both precipitation records and impaired gage flow data. While precipitation-based analysis suggested wet seasons from November through March and dry seasons from April through October, gaged outflow data indicated a slightly different seasonal pattern. To maintain consistency with prior studies and operational data, the gage-based wet (December–April) and dry (May–November) seasons were used in scenario evaluations.

SLR also reviewed how Scott Dam responded to historical flood and drought events. During the flood of record in Water Year 1982, the dam attenuated peak flows—limiting downstream impacts—despite outflows reaching 15,400 cfs. The maximum recorded outflow of 45,300 cfs in 1964 did not correspond to a major flood, suggesting operational releases may have been anomalous and due to operational, maintenance , or extreme snow melt conditions. In drought years, such as 1976–1977 and 2012–2016, the dam maintained minimum outflows even as storage dropped below operational thresholds, though temporary flow variances were required. These responses demonstrate the dam's historical role in buffering climate extremes, while also highlighting operational limitations under prolonged stress.

Standardized Precipitation Index (SPI) data from NIDIS indicates increasing frequency and intensity of droughts in recent decades. These trends, along with declining snowpack and greater precipitation variability, reinforce the need for adaptive water management strategies.

SLR documented Lake Pillsbury's current role in supporting aerial wildfire suppression and summarized minimum water storage requirements for future pond design. This information was used to inform conceptual sizing for firefighting support but was not evaluated beyond that scope.

The evaluation considered multiple potential scenarios in response to PG&E's surrender application. Each scenario was assessed under the four defined climate conditions—wet, dry, flood, and drought—to support Lake County's decision-making regarding dam removal and long-term water resource planning. Detailed findings are presented in Appendix A.

## 2.2 Fire Suppression

Lake Pillsbury is currently an optimal resource for current support for fire suppression and operates as an extensive use of water source for wildfire firefighting operations in the Lake County. The SLR Team (SLR and Aanko) has developed a summary of information regarding fire suppression and the use of aerial resources and their needs to support the scenario feasibility analysis.  The information is summarized herein and provided in Appendix B. There

are currently several main types of aerial firefighting support aircraft applicable for combating large-scale wildfires in Lake County, California: rotary-wing Type 1 helicopter aircraft like the Boeing CH-47 Chinook and Sikorsky S-70 Firehawk, multiengine scoopers like the Canadair CL-415 (Super Scooper), Large Airtankers (LATs), and Very Large Airtankers (VLAT's). When determining the suitability of a reservoir or water source for aerial firefighting there are three main considerations: water capacity, accessibility, and operational safety for the aircraft based upon the operational need.

The LATs and VLATs, play a critical role in wildfire suppression by delivering substantial volumes of water or fire retardant—typically 2,000-4,000 gallons for LATs and over 8,000 gallons for VLATs. These aircraft are primarily deployed during large, prolonged wildfire events due to their high capacity and strategic value. However, they do not scoop water from local reservoirs; instead, they rely on established airbases for refilling. Amphibious aircraft like the Super Scooper are an exception, capable of scooping over 1,000 gallons of water from large bodies of water, requiring at least one mile of open water for safe operation as the craft needs 10-12 seconds during a scoop. Lake Pillsbury is currently able to support fixed-wing aerial firefighting operations.

Helicopters can carry 1,000 to over 3,000 gallons of water and are critical for wildfire suppression, especially in the absence of large water bodies. Type 1 helicopters can be utilized for large-scale, sustained operations and effective for initial attack and support ground crews by delivering water directly to fire lines. Helicopters are also more maneuverable, but they are ideal when used near the water source to reduce fuel costs and travel time.

When constructing a new reservoir which can support helicopter operations, specific size and maneuverability criteria should be met. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet (sq. ft) at an average depth of 6 ft. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223-669 sq. ft. These considerations are summarized in Table 1 (below), listed by operational need for aerial firefighting.

**Table 1: General Aerial Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Gallons [ac-ft] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Extensive Use | 100,000+ [0.31] | 6 ft | 2,230 sq. ft. | 47 ft × 47 ft or 30 ft × 75 ft |
| Limited Use (Low Volume) | 10,000 [0.031] | 6 ft | 223 sq. ft. | 15 ft × 15 ft or 10 ft × 23 ft |
| Limited Use (High Volume) | 30,000 [0.092] | 6 ft | 669 sq. ft. | 26 ft × 26 ft or 15 ft × 45 ft |

In the absence of a reliable water supply, forest management practices must prioritize methods that enhance ecosystem resilience and mitigate wildfire risk through non-irrigative means. A primary strategy involves managing stand density via mechanical thinning, which reduces competition for scarce resources and lowers the accumulation of combustible biomass.

Prescribed burning is also a critical tool; when carefully planned and executed under suitable weather conditions, it replicates natural fire regimes, reduces surface fuel loads, and promotes the regeneration of fire-adapted native species.

## 2.3 Recreation

Recreation has relied on the scenic qualities of Lake Pillsbury for many decades. Lake Pillsbury is the largest waterbody in the Mendocino National Forest and attracts numerous visitors each year who generate revenue for the local economy. Figure 1 shows the location of recreational facilities in the vicinity of Lake Pillsbury.

Recreation taking place in and around the lake, includes camping, boating (motorized and non-motorized), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking (USFS, 2018). Lake fishing includes largemouth bass, Sacramento pikeminnow, bluegill and Rainbow trout (Fish Brain, n.d.). Whitewater rafting has historically occurred downstream of Lake Pillsbury in October and November when PG&E released water from Pillsbury Reservoir to make room for winter rain and snowmelt (California Creek). There are currently six campgrounds, two with group camping facilities along with three boat ramps around Lake Pillsbury (summarized in Figure 1). According to the Forest Service's website "The campgrounds at Lake Pillsbury are operated and maintained by Pacific Gas and Electric," excepting Lake Pillsbury Resort, which is privately owned (Lake Pillsbury Resort, 2024).

The annual visitation estimate for the Mendocino National Forest is 254,000 individuals, based on National Visitor Use Monitoring Data Collected FY 2010, FY 2011, FY2012, FY2013, FY2014, FY2015, FY2016, FY2017, FY2018, FY2019 (Visitor Use Report Pacific Southwest Region (R5) USDA Forest Service Region 5) A visit is defined as "A National Forest Visit is defined as the entry of one person upon a national forest to participate in recreation activities for an unspecified period. A National Forest Visit can be composed of multiple Site Visits" (USFS, 2018).

## 2.4 Ecosystem

An ecosystem is defined as a large community of living organisms (plants, animals and microbes) in a particular area. Lake Pillsbury has its own unique ecosystem – with threatened and endangered species, including Tule Elk, and fish – that attracts visitors to the area. SLR has conducted further research on these species to understand the existing ecosystem surrounding Lake Pillsbury as discussed below.

### 2.4.1 Wildlife Resources

The focus of the ecosystem evaluation is on threatened and endangered species. Verifying the presence or absence of these species within the project study area and determining potential impacts will require additional protocol surveys.

The predominant plant communities surrounding Lake Pillsbury and Van Arsdale Reservoir are yellow pine forest, mixed evergreen forest, foothill woodland, and chaparral (FERC, 1978).

SLR conducted a preliminary database review through the California Department of Fish and Wildlife's (CDFW's) BIOS Viewer, part of the CA Fish and Wildlife Wetland – CA Natural Diversity Database (CDFW, 2018). Preliminary research identified 15 total species within 0.5 miles of Lake Pillsbury's perimeter as shown in Exhibit 3A. Among the list of species identified,

3 of 15 are considered endangered according to State criteria. Bald eagle, steelhead, and Humboldt marten are the 3 endangered species located within the 0.5-mile distance radius.

The preliminary database review also located Tule Elk in areas surrounding Lake Pillsbury, as shown in Exhibit 3B. Exhibit 3B demonstrates the likelihood of elk presence within the outlined contours on the map. Contours from the database suggest there is a 50% likelihood that elk would be within the parameters of 0.5 miles, around the southern area of Lake Pillsbury. The second contour demonstrates a 90% likelihood of elk presence within its outlined contours, which exceeds the 0.5-mile distance radius scope for this database review.

PG&E has also identified rare, threatened, and endangered resources in the vicinity of the Project in the Final Application for Surrender of License (PG&E, 2025c). Species known to occur on Lake Pillsbury and Van Arsdale reservoirs include:

- Bald Eagle
- Northern Spotted Owl
- Northern Goshawk
- Fisher-West Coast
- Pacific Marten
- Special—Status bats
- Black-Tailed Deer
- Tule Elk
- Western bumble bees
- Monarch butterflies

### 2.4.2 Botanical Resources

There are four special-status plant species that are known to occur in the FERC Project boundary [see Exhibit 3C, specifically the area of Lake Pillsbury] or are adjacent to Project features, as mentioned in the Final Application for Surrender of License including:

Two populations of three-fingered morning glory (*Calystergia colling ssp. Tridactylosa*). One population located along the tunnel between Cape Horn Dam and Potter Valley Powerhouse; and one population located along the southern shoreline of Lake Pillsbury. One population of Green's narrow-leaved daisy (*Erigeron greeni*).

PG&E's Final Application for Surrender of License has acknowledged and stated various plans to mitigate construction related impacts on botanical resources.

### 2.4.3 Fish and Aquatic Resources

The alteration of the Eel River's natural flow has disrupted local ecosystems and fish migration patterns. The dam has been a barrier to native fish species, such as salmon and steelhead, preventing them from accessing upstream habitats. Disruptions to the system can have significant ecological consequences, affecting the health and biodiversity of the river system on a temporary and long-term basis.

The Eel River downstream of Lake Pillsbury, including the lower river and estuary, provides important aquatic habitat for numerous special-status species. This includes several anadromous salmonids listed as threatened under the Endangered Species Act (ESA), such as Chinook salmon (*Oncorhynchus tshawytscha*), coho salmon (*O. kisutch*), and steelhead (*O. mykiss*), as well as the threatened Pacific eulachon (*Thaleichthys pacificus*) and the

endangered tidewater goby (*Eucyclogobius newberryi*). Species proposed for ESA listing, such as the northwestern pond turtle (*Actinemys marmorata marmorata*) and the longfin smelt (*Spirinchus thaleichthys*), also inhabit this area.

Other notable species include those listed under the California Endangered Species Act (CESA), like the endangered summer steelhead (*O. mykiss irideus*), the candidate species white sturgeon (*Acipenser transmontanus*), and species of special concern such as the green sturgeon (Acipenser medirostris), Pacific lamprey (*Entosphenus tridentatus and Lampetra spp.),* foothill yellow-legged frog (*Rana boylii, Northwest/North Coast clade*), coastal tailed frog (*Ascaphus truei*), southern torrent salamander (*Rhyacotriton variegatus*), California floater (*Anodonta californiensis*), western pearlshell (*Margaritifera falcata*), and the northern red-legged frog (*Rana aurora*).

The river also supports a variety of non-listed native freshwater fish, including sculpin (*Cottus spp.),* stickleback (*Gasterosteus spp.),* and sucker (*Catostomus spp.).* Additionally, occasional runs of pink salmon (*O. gorbuscha*) and chum salmon (*O. keta*) have been documented (Yoshiyama and Moyle 2010).

The construction of Scott Dam created a 130-foot-high barrier that prevents upstream fish migration. Currently, there are no facilities at the dam to allow fish passage. Estimates of the blocked river habitat and potential returning fish numbers vary widely. A 1982 VTN study identified 35.7 miles of primary spawning habitat above the dam, estimating possible returns of 1,250 Chinook salmon and 1,499 steelhead. NMFS (2002) referenced this and suggested that historically, 2,000 to 4,000 fall-run Chinook and winter-run steelhead may have spawned upstream. Other studies estimate 100–150 miles of blocked habitat, while Cooper et al. (2020) projected even greater totals—up to 288 miles for steelhead spawning.

Considerations for fish and aquatic resources, including fish passage alternatives, are further evaluated in Appendix C.

## 2.5 Economic and Cost Conditions

Economic analyses of varied levels of detail and geographic focus have been completed over several decades, documenting the economic resources in the affected region and the projected economic impacts of the PVP. The focus on Lake County and Lake Pillsbury in these analyses is limited and/or analysis needs to be updated with the latest information. This section summarizes the County of Lake specific tax revenue information that can be used in estimating future loss of tax revenue to the County with respect to the decommissioning of Scott Dam and Lake Pillsbury.

In the areas immediately surrounding Lake Pillsbury, county tax revenues are related to secured and unsecured property taxes, forest service cabins, sales tax and PG&E unitary tax payment related to property they occupy. SLR has reviewed the tax revenue information provided by the County of Lake and summarized information as presented below.

The tax revenue for the year 2024 is approximately $872,000, showing a 3.5% increase from the actual 2023 values. These costs are broken down in Table 2 (below).

**Table 2: 2024 Taxes Due**

| Tax Source | Taxes collected |
|---|---|
| Current Secured (2024) | $ 581,976 |
| Current Unsecured (2023) | $ 6,700 |
| Current Unsecured Forest Service (2024) | $ 64,380 |
| Transient Occupancy Tax (TOT) + Sales Tax (2019-2020 estimate) | $ 16,400 |
| PG&E (2023) | $ 202,356 |
| **Total** | **$ 871,812** |

**Source:** Lake County Tax Collector, May 2025

Alternative analysis cost evaluations were completed previously by Stillwater Sciences (2021b) and McMillen Jacobs (2018), and are summarized where applicable in this report.

# 3.0 Scott Dam Scenario Description

PG&E's current proposed action under the Final Application for Surrender of License (PG&E, 2025c) includes the decommissioning and removal of most PVP facilities, including Scott Dam, Cape Horn Dam and the Potter Valley powerhouse. The evaluation below focuses only on the Scott Dam and Lake Pillsbury area.

Replacing Lake Pillsbury with a smaller reservoir can preserve the benefits of recreation, fire suppression, ecosystem maintenance and water supply to the community. Considering the above background and other information discussed, SLR has considered the following aspects in development of mitigation options:

1. Preserving recreational use by maintaining a water source.

2. The water needs for downstream water supply and supporting existing ecosystems in and near the Lake Pillsbury reservoir.

3. The potential for fire suppression using aerial machinery, such as helicopters.

4. Design pathways to increase fish passage to upper reaches on the Eel River.

5. Employ strategies to minimize environmental impacts and ecosystem benefits.

6. Accommodate short- and long-term economic benefits to maintain the current level of revenues for the County of Lake.

SLR has identified several scenarios to accommodate the above objectives as described in this section. For each scenario, a description of the scenario, our analysis where applicable and findings of the scenario evaluation are presented.

## 3.1 Existing Conditions Scenario

The current operational conditions scenario represents the Scott Dam and Lake Pillsbury operational period from 2019 to the present. Based on observed operational data, the maximum

operational water surface elevation for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 ft (PG&E datum) with a correspond water storage volume of 56,322 ac-ft (See **Error! Reference source not found.** in Appendix A). Historically, the maximum operational water surface elevation for Lake Pillsbury was at top elevation of the spillway gates of 1,910 ft. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the water surface elevation at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained. It is not known if there have been operational changes to the needle valve.

### 3.1.1 Existing Conditions Findings

#### 3.1.1.1 Water Supply

The existing conditions provide the highest reliability for water supply in terms of downstream flow, groundwater recharge, and operations due to the operability control provided by the needle valve and spillway gates. Lake Pillsbury has historically operated with an average storage volume of over 45,000 ac-ft (14 billion gallons) of water availability. Even though Lake Pillsbury was designed for hydropower support, floods have historically been attenuated by the reservoir, reducing the flood risk to downstream infrastructure.

Drought trends during the 21st century have been increasing in frequency, intensity, and duration (see also Appendix D). This historical water storage capacity has been reduced over time due to losses from sediment accumulation.

The average water surface elevation and storage in Lake Pillsbury for the current operational storage conditions (established in 2023) are lower than the historical operational conditions due to the reductions of the maximum operational water surface elevation and the passive operation of the spillway. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation.

As currently operated, Lake Pillsbury is expected to continue operating with an average available storage volume over 45,000 ac-ft (14 billion gallons).

Reducing the storage capacity, as sediments continue to build up, will reduce the drought tolerance and it is likely that under drought conditions the storage and outflow will be lower than the historic conditions observed values in the future for this scenario.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.1.1.2 Fire Suppression

The current conditions provide the most dependability for fire suppression support. Lake Pillsbury historically satisfied the operational requirements to extensive fire suppression use (see Appendix B).

#### 3.1.1.3 Ecosystem

Lake Pillsbury historically has provided habitat for various wildlife, lake aquatic and botanical species for many decades. Under the current conditions, the presence of Scott Dam as a fish barrier has been a concern for many fishing advocates for a long period of time.

#### 3.1.1.4 Recreation

Lake Pillsbury has historically allowed recreational use to be maintained. Recreation taking place in and around the lake includes camping, boating (motorized and non-motorized), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking (USFS, 2018).

#### 3.1.1.5 Economic and Cost Conditions

The presence of Lake Pillsbury generates tax revenue and tourism from recreation, benefiting local businesses and the community. The economic conditions described in Section 2.5 are applicable for the existing conditions.

#### 3.1.1.6 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Existing biological communities and habitat conditions would remain unchanged.
- Public recreational areas and activities would continue without disruption.
- Ongoing water supply benefits for local users, including irrigation, fire suppression, flood control, drought resilience, and municipal use, would be preserved.
- Groundwater recharge would continue to occur to support well users.
- The local economy would continue to benefit from the activities associated with the Lake
- Reservoir storage would be maintained, supporting continued water diversion to the Russian River watershed.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.1.1.7 Disadvantages

- Sediment would continue to accumulate in the reservoir, gradually reducing its flow storage capacity.
- Scott Dam would remain a complete barrier to fish passage, preventing access to upstream habitat in the upper watershed.

## 3.2 Scenario A1– PG&E Proposed Removal

PG&E's current proposed action under the Final Application for Surrender of License (PG&E, 2025c) includes the decommissioning and removal of Scott Dam and associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. The pre-dam waterways would be restored, as shown in Figure 3. The recreational facilities would be removed alongside the removal of the dam and would be restored later. A list of the specific Project Facilities/Features that would be removed, left in place, and removed without restoration are outlined in Exhibit 4 (PG&E, 2025a).

The Proposed Action under the Application for Surrender of License includes the following for the Scott Dam and Lake Pillsbury area:

- Decommissioning and removal of Scott Dam and associated facilities and features;
- Removal and restoration of certain Project recreational facilities (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands;
- Restoration of the remnant inundation zone of Lake Pillsbury.

PG&E has chosen a Rapid Dam Removal approach, expected to span three seasons. The timeline may vary depending on flow conditions, potentially extending beyond two consecutive years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards [cy]) downstream of the Eel River. Exhibit 2 illustrates the cross-section of the dam removal plan. The decommissioning plan has been split into three seasons:

1. Initial low-flow season (June–October) to dewater and remove the upper portion of the dam
2. First high-flow season following removal of the upper portion of the dam for sediment flushing (November–May)
3. First low-flow season after sediment flushing to complete final dam removal.

During the low-flow season of the first year, the reservoir drawdown would be initiated when the inflows are within the capacity of the needle valve (generally below 400 cfs). Additionally, Scott Dam Road would be connected to the temporary barge launch downstream of the terminus of the existing plunge pool below the dam by a temporary road for easy access. An adit tunnel would also be built at the base of the spillway, utilizing a temporary plug on the upstream face of the dam. Then, the upper portion of the dam would be removed using dam lowering and notching and the sediments would be dredged near the new tunnel intake. During the high-slow season between the first and second year, the adit plug would be removed, and sediment flushing would begin. Finally, during the low-flow season in the second year, dam removal and sediment flushing would be completed (See Exhibit 2).

Under the Proposed Action, PG&E would develop restoration plans to be implemented in the Scott Dam, Lake Pillsbury and recreation facilities following completion of construction activities. PG&E has not provided detailed restoration plans, and it is expected that final restoration plans would be developed between 2025 and 2027. The duration of construction, currently anticipated to be 2 years (not necessarily consecutive), and it may change based on final engineering design.

The disadvantages associated with this scenario are that there are mentions of various activities associated with removal of recreation facilities, some surveys to document ecosystem aspects, sediment management goals but no specific information provided to address the true impacts. PG&E identified a long list of unavoidable adverse impacts from the decommissioning of Scott Dam.

This scenario corresponds to the complete dam removal option as presented by PG&E (2025c). In the application there are mentions of various activities associated with removal of recreation facilities, some surveys to document ecosystem aspects, sediment management goals but no specific information provided to address the true impacts. PG&E identified a long list of unavoidable adverse impacts from the decommissioning of Scott Dam.

### 3.2.1 Scenario Findings

#### 3.2.1.1 Water Supply

This scenario would eliminate all water storage from Scott Dam, and any benefits of flood control. Water supply to the Russian River watershed would be limited to winter months and during wetter dry seasons where water is not diverted for irrigation. Under baseline existing conditions, the average monthly dry season flows are approximated between 147-222 cfs and the wet season average flows range from 296-375 cfs. In comparison, the wet and dry season average flows would be much higher at 1,237 cfs (wet) and a bit lower at 133 cfs (dry).

This scenario would increase flood risk and likely increase the frequency, intensity, and duration of flood events seen under existing conditions operations. This is attributed to storage and dam controls attenuating the flows and retaining much of the flood waters. This scenario has very low drought tolerance when compared to the baseline scenario where average monthly flows are projected to fluctuate between 17 and 376 cfs during the dry season. Flood and drought risk are difficult to quantify due to the limited unimpaired data analysis assumptions and would require more analysis to provide a higher confidence in the projected response.

The drought conditions for this scenario are difficult to project without more detailed unimpaired flow data. The Eel River, Habematolel Creek, and Rice Fork are classified by the USGS as perennial streams, while the Smokehouse and Salmon Creeks are classified as intermittent. Smokehouse and Salmon Creek are likely not to have flows during a drought condition. The Eel River, Habematolel Creek, and Rice Fork may be more drought tolerant under specific conditions, however, the Eel River is likely to have the most reliable source of flow.

The passive sediment removal will cause flow to fluctuate and become unreliable to predict until the watercourse and sediment are semi stabilized. Semi stabilization should be assumed when there are no major changes to the channel geometry and floodplain limits as well as when vegetation has been re-established and unaffected during normal wet and dry season flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed and recharging would be limited to the watercourse contributions. This could result in a reduction to the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

The changes in hydrology based on the Stillwater HEC-RAS model projected up to a 1.2-foot increase in flood waters during a 100-year event (Stillwater Sciences, 2021e). This was assuming the longitudinal profile of the Eel River would not change from the existing conditions, which is unlikely as the downstream has been sediment starved since the dam's inception and once the dam is removed, will undergo changes to the bed elevation, slope, and roughness. The modeling was also limited to just encompassing the Eel River just downstream of the Dam and utilized hydrology that was not based on precipitation gages within the watershed. The removal would have a potential higher risk for flood damage to downstream infrastructure.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.2.1.2 Fire Suppression

This scenario would eliminate any potential for fire suppression that is currently available through storage at Lake Pillsbury. There are no other alternatives to water storage included as part of this proposed scenario.

#### 3.2.1.3 Ecosystem

The decommissioning would have a high risk for adverse environmental impact during construction activities and during sediment flushing years, particularly without a robust sediment management plan. PG&E proposes passive management of sediment that could take years to decades before the sediment is no longer a concern to water quality and the aquatic and riparian habitats of the Eel River. Stillwater Sciences noted that the removal of the dam will likely result in increased sediment deposition in the Van Arsdale Reservoir in the long-term (Stillwater Sciences, 2021a). In the short-term, brief periods of significant sediment deposition and subsequent flushing by flood events are likely. Long term, there would be a positive environmental impact once the sediment and watercourses have reached a quasi-equilibrium post removal. The removal of the dam would allow for fish passage upstream to the Eel River upstream of dam and therefore provides a biological advantage.

The loss of the reservoir would likely change the ecosystem composition and biodiversity of vegetation. The moisture content of the soil will likely affect the existing and established vegetation buffers and zones where specific plant species thrive. During the transition period after the dam is removed, if no restoration efforts are put in place, a loss in vegetative cover could occur. Without mitigation and restoration, the vegetation can dry out and experience cover loss. This could lead to colonization by either upland or invasive species that are more drought tolerant but also introduce a higher risk for fire. This loss of vegetation and exposure of the reservoir bed can also create a potential risk for erosion. Over time, natural succession may lead to the development of new plant communities that are more adapted to the altered hydrology.

#### 3.2.1.4 Recreation

This scenario would remove recreation associated with activities on Lake Pillsbury such as boating and swimming. The existing piers, docks, and marina would cease to function under current use. Other recreation uses would need to be modified and may be limited in the offerings when compared to the baseline in existing conditions. This scenario could provide opportunities for other types of recreation should an agreement or provisions be made on the existing land trust.

#### 3.2.1.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Early cost estimates indicated Scott Dam rapid removal would likely cost $ 48 (Stillwater Sciences, 2021b).

PG&E's current estimate for the cost of decommissioning the overall PVP project is $532 millions (SFGate, 2025). The 2027 General Rate Case (GRC) application from PG&E suggests costs for the PVP decommissioning project will be recovered over 8 years or two GRC periods. PG&E hydroelectric decommissioning projects, including PVP, were applied a 25 percent contingency to estimates derived from a Class 5 project study. Estimates for the projected cost of $532 million were calculated through internal PG&E cost estimating models and project-specific studies of hydroelectric facilities which follow the Association for the Advancement of Cost Engineering (AACE) guidelines, corresponding to AACE Level 5 (PG&E, 2025b). This estimate is not differentiated for initial vs long-term costs, neither for Scott Dam nor other PVP areas.

#### 3.2.1.5.1 Initial Implementation Costs

This scenario would have a high initial cost for the decommissioning. A risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain. The diversion to the Russian River watershed would also need to be modified to adapt to the change in water supply reliability.

#### 3.2.1.5.2 Long-term Operation and Maintenance Costs

This scenario removes costs associated with the dam operations and maintenance. Long-term, once the sediment within Lake Pillsbury has been released and the watercourses reach a quasi-equilibrium, this scenario would have infrequent maintenance associated with the watercourses upstream of the Van Arsdale reservoir. The NERF, if established, would require sediment management and potential periodic dredging to accommodate for the long-term sediment flushing.

#### 3.2.1.5.4 Other Economic Impacts

The local economy will likely be impacted by the loss in revenue from the previous tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury.

### 3.2.1.6 Complexity

Medium-to high relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.2.1.7 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

### 3.2.1.8 Disadvantages

- Risk of Sacramento Pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and reliable flow diversion capabilities to the Russian River Watershed.

- Requires large-scale removal and disposal of dam structures and materials.
- The removal would have a potential higher risk for flood damage to downstream infrastructure.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment transport downstream of Scott Dam.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of tax revenue associated with lowering of the property values and sales tax. Lowering of property values for nearby residents.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users and lower drought resilience and fire suppression in terms of water supply.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- Less resilient to droughts and floods because storage would no longer be maintained and the outflow would not be operationally controlled.

## 3.3 Scenario A2 – Scenario A1 with Additional Mitigation

Under this scenario, PG&E proposes the full decommissioning of Scott Dam with additional mitigation efforts implemented to address County of Lake and local community concerns. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the region, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and will have higher ecosystem benefits to the community.

As mentioned before, the County of Lake, local recreation communities, fire marshals, other agencies and CalWild have identified the need to find an alternative water source to address the lost benefits currently provided by Lake Pillsbury, for firefighting needs.

The main advantage associated with this scenario is that it addresses the County’s concerns with the full decommissioning approach suggested by PG&E in the Final Application for Surrender of License.

The disadvantage associated with this scenario is that it is not feasible to adequately address all the County’s concerns with the full decommissioning of the dam and removal of Lake Pillsbury.

### 3.3.1 Scenario Findings

Scenario A2 analyzes PG&E's Scott Dam removal and the projected impacts on water supply, fire suppression, the environment, recreation, and associated costs. SLR projects the same outcomes as Scenario A1 for water supply and fire suppression.

#### 3.3.1.1 Ecosystem

Similarly to Scenario A1, the decommissioning would introduce a high risk for adverse environmental impacts during construction activities and during sediment flushing years, but with a more robust sediment management plan including restorative practices, environmental impacts would likely be less harmful for this option. SLR recommends implementing a robust sediment management plan that would designate a timeframe for when all sediment would be removed or flushed downstream and the Eel River would reach its quasi-stable state. In addition to the sediment management plan, SLR recommends performing a wetland, riparian, and channel restoration for Eel River and its tributaries. Revegetation of disturbed areas using native plants and locally sourced plants and seeds, with implementation of adaptive measures such as irrigation, is also recommended. This will help expedite the restoration process so that there are minimal impacts to the ecosystem.

#### 3.3.1.2 Recreation

As in Scenario A1, this scenario would remove recreation associated with activities on Lake Pillsbury such as boating and swimming. The existing piers, docks, and marina would cease to function under current use. Under this option, there would likely be more new recreational opportunities compared to Scenario A1, with the restored Eel River and other major creeks available for kayaking, fishing, and trail access. This scenario could provide opportunities for other types of recreation should an agreement or provisions be made on the existing land trust.

#### 3.3.1.3 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Initial capital costs to fully decommission the dam and provide sediment management were estimated at $65 million, and full dam removal with complete sediment removal and full channer restoration was estimated at $112 million (McMillen Jacob, 2018). Other cost estimates indicate Scott Dam rapid removal will likely cost $ 48 million, additional $ 39 million for sediment management including a pilot channel option and Lake Pillsbury revegetation for $ 75 million (Stillwater Science, 2021b).

##### 3.3.1.3.1 Initial Implementation Costs

This scenario would have a high initial cost for the decommissioning and sediment management, and an associated cost with the sediment management and revegetation plan. A risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain. The diversion to the Russian River watershed would also need to be modified to adapt to the change in water supply reliability.

#### 3.3.1.3.2 Long-term Operation and Maintenance Costs

This scenario does remove costs associated with the dam operations and maintenance. Long-term, once the sediment within Lake Pillsbury has been released and the watercourses reach a quasi-equilibrium, this scenario would have infrequent maintenance associated with the watercourses upstream of the Van Arsdale reservoir. The Van Arsdale reservoir would require sediment management and potential periodic dredging should it remain in place.

#### 3.3.1.3.3 Other Economic Impacts

The local economy will likely be impacted by the loss in revenue from the previous tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury.

The County of Lake will lose revenue from Forest Service cabins and the PG&E unitary tax.

### 3.3.1.4 Complexity

High relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.3.1.5 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Reduced volume of sediment transported downstream.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

### 3.3.1.6 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and flow diversion capabilities to the Russian River Watershed.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.

- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of business and lowering of property values for local residents and reduction of tax revenue for the County.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users and lower drought resilience and fire suppression in terms of water supply.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Less resilient to droughts and floods because storage would no longer be maintained and the outflow would not be operationally controlled.

## 3.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

This scenario is similar to the current operational conditions described in Section 2.1, with the addition of a fish ladder, as previously evaluated by Mead and Hunt (2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, seismic retrofitting would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules, and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures.

To maintain operational conditions would require the dam to be retrofitted and for dam operations and maintenance to be taken over by other agencies or entities. Immediate actions to the dam would require a seismic retrofit which would induce an increase in maintenance costs. This scenario also has the highest long-term costs from maintenance and operations. Permitting considerations would need to be evaluated. This scenario would continue the positive economic impacts on local businesses and tax revenue due to recreational use associated with Scott Dam and Lake Pillsbury.

For the purposes of this initial evaluation the scenario was considered unfeasible, and no detailed evaluation was conducted at this moment. Pending analysis of Scenario C, this scenario may be reconsidered.

### 3.4.1 Scenario Findings

Scenario B maintains the same operational characteristics as the current conditions described in Section 3.1 in terms of hydrology and hydraulic conditions, fire suppression, recreation, ecosystem, and local economic benefits.

The key distinction lies in the seismic retrofitting, addition of a fish ladder and associated infrastructure upgrades. These improvements introduce increased initial implementation costs due to the construction of the fish ladder and necessary seismic retrofitting of the dam. Over the long-term, Scenario B also would entail additional operations and maintenance efforts to ensure the continued functionality of the fish ladder, dam structure, and flow control systems.

#### 3.4.1.1 Cost Conditions

Initial cost estimates valued this scenario to have a capital cost of $26 million for upstream fish passage and $45.5 million for downstream fish passage (McMillen Jacobs, 2018). Other early estimates for adding a modified conventional fish pathway to Scott Dam may add $101 million (Stillwater Sciences, 2021b) and additional maintenance costs for fish ladder operations.

#### 3.4.1.2 Complexity

Medium relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.4.1.3 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Existing biological communities and habitat conditions would remain unchanged.
- Public recreational areas and activities would continue without disruption.
- Potential increase in resident/anadromous fish production.
- Ongoing water supply benefits for local users, including irrigation, fire suppression, flood control, drought resilience, and municipal use, would be preserved.
- Groundwater recharge would continue to occur to support well users.
- Reservoir storage would be maintained, supporting continued water diversion to the Russian River Watershed.
- Would provide upstream fish passage over the dam.
- Construction and maintenance access from existing access road on southern dam abutment.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.4.1.4 Disadvantages

- Sediment would continue to accumulate in the reservoir, gradually reducing its flow storage capacity.
- Uncertain fish passage effectiveness, particularly for downstream migration.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level would need to be maintained within a set range to support operation.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.

- Would require extensive excavation and construction activities on the left abutment of the dam.
- Tunnel through concrete dam face may introduce additional seismic concerns.

## 3.5 Scenario C – Lowered Scott Dam with Fish Ladder

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 ft (PG&E datum) and the installation of a fish passage system such as a fish ladder structure. Scenario C is similar to Scenario B, and it is considered more feasible as the capital improvements costs would be lower for a reduced height dam, and the water pressure reduction would increase stability.

The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 ft would create a reservoir of approximately 40 percent of the current storage volume at water surface elevation 1,900 ft. Potentially the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury water surface elevations and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage as the current maximum operational water surface elevation of 1,900 ft, the spillway invert may be further reduced in height by 25 ft below the existing spillway invert which would result in a new lake footprint (Figure 4). At that reduced maximum operational water surface elevation the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam by placing the concrete material in both or either of the downstream and upstream sides of the lowered dam along either the river or lake embankments. Further evaluation would be required to understand the effects on the adjacent land area, and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation and ecosystem benefits as compared to full decommissioning.

Recreational facilities will need to be relocated to the new lake shore. Groundwater levels will also likely be reduced. Slope stability for the new lake shore needs to be considered, as PG&E currently has a minimum lake level for slope safety.

### 3.5.1 Design and Construction Considerations

Design aspect would need to include additional Geotechnical Feasibility and other material reuse evaluations.

### 3.5.2 Scenario Findings

#### 3.5.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury by 35,870 ac-ft by lowering the maximum operation water surface elevation from 1,900 ft to 1,875 ft. Flood control, downstream water diversion requirements, and operational reliability would be impacted. Flood control and downstream water diversion flow requirements would be affected due to reduced dam operational control and the passive operation of the spillway. An updated low-level outlet structure that would be able to operate at low water surface elevations and have the capacity to maintain minimum ecological flow requirements to Eel River.

This scenario would generally increase flood risk due to the stopping of active operations of the spillway gates which allowed for greater control, attenuation, and retention of flood inflows. This scenario has reduced drought tolerance due to a lower average water surface elevation and storage in Lake Pillsbury than the current operational conditions. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation. It is recommended that existing sediment be mitigated to increase the available water supply.

The Lake Pillsbury reservoir's ability to recharge the groundwater would likely be reduced, due to the reduced reservoir size. Groundwater recharge would continue to occur, but the water table may be lowered as the reservoir level lowers.

Additional evaluation of the slope stability of the existing slope would need to be considered. Currently PG&E has maintained a minimum water surface elevation due to slope safety concerns.

This scenario proposes that seasonal fish passage be provided during two seasons per year. The first season would be during November-December, and the second season would be during April-May to meet fish passage requirements. Lake Pillsbury is expected to be able to provide the required flowrates to properly allow fish passage.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.5.2.2 Fire Suppression

This scenario will be able to provide dependability for providing fire suppression. While Lake Pillsbury water storage and surface area would be reduced when compared to the baseline scenario it is expected to continue to operate as an extensive fire suppression use water source. This results in satisfying the operational requirements for extensive fire suppression use. Expected to allow for full use in support of fire suppression requirements for plane and helicopter methods.

#### 3.5.2.3 Ecosystem

Lake Pillsbury has historically provided habitat for various species which would continue for this scenario. The proposed fish ladder would allow for both fish passage and mitigation of existing sediments. This scenario involves implementing a robust sediment management plan with a timeframe and method for the removal or flushing of sediments in Lake Pillsbury. A low-level outlet structure that would be operational at a lower water surface elevation and allow for managed sediment flushing behind the dam. Sediment management would reduce the impacts to downstream ecosystems when compared to the full removal of Scott Dam.

#### 3.5.2.4 Recreation

This scenario would have an impact on recreational activities on Lake Pillsbury such as boating and swimming. Facilities at the northwestern portion of Lake Pillsbury and Salmon Creek may cease to function under the current use. A portion of other recreational facilities will potentially need to be relocated to the lowered Lake Pillsbury shoreline such as existing piers, docks, and marina. Other recreation uses would need to be modified and may be limited in the offerings when compared to the baseline scenario.

#### 3.5.2.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

Costs for lowering of Scott Dam have not been estimated. Fish passage costs are anticipated to be similar to the current elevation of Scott Dam, with initial cost estimates of a capital cost of $26 million for upstream fish passage and $45.5 million for downstream fish passage (McMillen Jacobs, 2018). Other early estimates for adding a modified conventional fish pathway to Scott Dam may add $101 million (Stillwater Sciences, 2021b) and additional maintenance costs for fish ladder operations.

#### 3.5.2.6.1 Initial Implementation Costs

This scenario would have a high initial cost for the lowering of Scott Dam which would include a new dam spillway, construction of the fish ladder structure, and updating to a low-level outlet structure. The required fish ladder structure for the lowered dam height configuration would be of reduced cost compared to a potential fish ladder structure at the current existing dam height configuration. To continue long-term operations of Scott Dam under this scenario would require the dam operations and maintenance to be taken over by other agencies or entities.

#### 3.5.2.6.2 Long-term Operation and Maintenance Costs

An assessment is recommended to better understand if the existing needle valve structure could be modified or if it would need to be removed to provide adequate scenario requirements. The use of a low-level outlet that has the capability for managed sediment on a periodic basis would potentially reduce the economic impact to downstream structures, facilities and watershed due to large sediment flushing volumes when compared to full removal of Scott Dam.

#### 3.5.2.6.3 Other Economic Impacts

This scenario would provide a slight reduction in benefits to local businesses and tax revenue due to reduced recreational use associated with Scott Dam and Lake Pillsbury.

### 3.5.2.6 Complexity

Medium relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

### 3.5.2.7 Advantages

- Potentially contaminated sediments would remain undisturbed.
- Would provide upstream and downstream fish passage.
- Potential increase in resident/anadromous fish production.
- Restoration of historical ecosystem function and dynamics.
- Minimizes sediment release from Lake Pillsbury reservoir.
- Potential improvement to water quality and temperature due to reduction of reservoir.
- May reduce seismic instability concerns.
- Maintain some benefits of water supply, fire suppression, flood control, drought resilience, and recreation.
- Groundwater recharge would continue to occur, potentially with a lower groundwater table, to support well users.

- Construction and maintenance access from existing access road on southern dam abutment.
- Resilience to droughts and floods because storage would be maintained and outflow would be operationally controlled.

#### 3.5.2.8 Disadvantages

- Potential modifications to the existing recreational activities may need to be maintained or reduced.
- Existing biological communities and habitat conditions will be impacted in the short term from the change in lake footprint and depth.
- Exposed and saturated steep slopes along the adjusted lake perimeter will be susceptible to failure and sloughing.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level has to be maintained within a set range to support operation.
- May not be fully effective during summer periods due to change in water temperature due to the surface draw of the fishway and lower-level release to the river.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.
- Partial Dam removal and material disposal.
- Impacts to downstream water users and reduced flood control and drought response.
- Reducing the storage capacity of Lake Pillsbury would make the area more prone to droughts, and reducing the height of the dam would make the area more prone to floods.

## 3.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure. A new 3,100-foot-long earthen impoundment dam would be established in the Salmon/Smokehouse and Squaw Valley Creeks watershed area to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir (Figure 5). The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within Lake Pillsbury footprint can also be reused for the new impoundment construction. Low permeability clay/silt sediments can be used in the core of the dam and gravelly sediments can be used in the embankment/slopes of the new dam.

The water storage will be based on the Salmon/Smokehouse and Squaw Valley Creeks watershed area, and it is assumed the maximum capacity (approximately 9,990 ac-ft) would be provided at a maximum operational water surface elevation of 1,885 ft. For this scenario, the approximate surface area would be 580 acres, approximately 30 percent of the current size (water surface elevation 1,900 ft). The new impoundment dam would be constructed at the

narrowest location possible that would not impair the flows through the Eel River. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational water surface elevation. An active low-level outlet structure would not be included, though a passive outlet is considered for potential draining. This option will support continued use of recreations facilities, ecosystem benefits, fire suppression and the upstream fish pathway along the Eel River as Scott Dam will be removed as a fish barrier.

### 3.6.1 Design and Construction Aspects

If this scenario is considered, the design would involve:

1. Considerations on geotechnical feasibility.
2. Volumes of materials required for this scenario and cross sections.

A constructability review will be needed to identify challenges, complexity and cost efficiencies.

### 3.6.2 Scenario Findings

#### 3.6.2.1 Water Supply

This scenario would reduce water storage within the area to approximately 30 percent of the current water storage in Lake Pillsbury. Flood control, downstream water diversion flow requirements, and operational reliability would be impacted, as passive management is proposed.

This scenario would generally increase flood risk due to the higher flows from the Upper Eel River watershed. The water supply capacity will not be actively managed.

Groundwater recharge would continue to occur on the north shore, providing some reliability to existing well owners.

The storage feasibility needs to be evaluated, as the alluvial deposits in the north area are not well understood. Geotechnical feasibility needs to be evaluated due to the faults in the area.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.6.2.2 Fire Suppression

This scenario will be able to provide dependability for providing fire suppression. While water storage and surface area would be reduced when compared to the current availability of Lake Pillsbury, the smaller reservoir is expected to continue to operate as an extensive fire suppression use water source. This results in satisfying the operational requirements for extensive fire suppression use. Expected to allow for full use in support of fire suppression requirements for plane and helicopter methods.

#### 3.6.2.3 Ecosystem

Lake Pillsbury has historically provided habitat for various species which would continue for this scenario.

Implementing a robust sediment management plan that designates a timeframe and method for the removal or flushing of would reduce the impacts to downstream ecosystems when compared to the full removal of Scott Dam. The sediment management plan could be similar to what was proposed in Scenario A2.

Due to the smaller watershed area that is being impounded, a fish passage for this watershed is not deemed necessary since the larger Eel River and Rice Fork watersheds would be unimpaired.

#### 3.6.2.4 Recreation

This scenario would have an impact on recreation associated with activities on Lake Pillsbury such as boating and swimming. Facilities in the northwestern portion of Lake Pillsbury and Salmon Creek may experience a reduction in use, unless dredging occurs to accommodate extension of the footprint. A portion of other recreational facilities will potentially need to be relocated to the lowered Lake Pillsbury shoreline such as existing piers, docks, and marina. Other recreation uses would need to be modified and may be limited in the offerings when compared to the current conditions.

#### 3.6.2.5 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

This scenario would include similar initial costs to Scenario A2. Initial capital costs for full decommissioning of the dam with sediment management were estimated at $65 million (McMillen Jacobs, 2018). Additional costs, pending assessment, will be required for the earthen dam construction.

##### 3.6.2.5.1 Initial Implementation Costs

This scenario would have a very high initial cost for removal of Scott Dam and construction of the earthen dam. The reuse of material from Scott Dam, residual sediments within Lake Pillsbury footprint would provide construction materials at lower costs than importing from other sources. The complexity of the engineering design and construction costs need to be further evaluated. The required dam length will need to be approximately 3,100 ft. The quantity of materials to construct the proposed dam are large resulting in high construction costs.

##### 3.6.2.5.2 Long-term Operation and Maintenance Costs

To continue long-term operations of the new earthen dam under this scenario, the long-term costs would be low. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational water surface elevation.

##### 3.6.2.5.3 Other Economic Impacts

This scenario would provide a minor reduction in economic activity, and minor effects on local businesses and tax revenue.

#### 3.6.2.6 Complexity

Highest relative complexity due to design of a new earthen dam, when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.6.2.7 Advantages

- Reduced dam operations and maintenance.
- A portion of the existing biological communities and habitat conditions would remain unchanged or undergo short term changes while the new dam is being constructed.

- Public recreational areas and activities would continue albeit with a small reservoir. Recreation activities may be limited to a shorter duration of the year, dependent upon water supply.
- Eliminates dam-specific operations, maintenance, and long-term safety liabilities of Scott Dam.
- Maintenance of a reduced reservoir for recreation and fire suppression.
- Groundwater recharge would continue to occur to support well users.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat within the Eel River and Rice Fork.
- Restores historic river ecosystem functions and natural hydrologic dynamics within the Eel River and Rice Fork.
- Over time, parts of the Eel River ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### 3.6.2.8 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of controlled water storage and flow diversion capabilities to the Potter Valley Powerhouse.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment plume.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Changes to public recreational opportunities and access with the altered reservoir footprint.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- Requires extensive engineering and construction of new dam.

- Reducing the storage capacity of Lake Pillsbury would make the area more prone to droughts, and reducing the height of the dam would make the area more prone to floods.

## 3.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2 but with specific mitigation added to enhance fish passage while maintaining fire suppression, and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[12], the off-channel ponds would be constructed.

Two potential pond locations have been identified as shown in Figure 6 based on the 1922 pre-dam topography (McBain Associates, 2018). The 1922 topography was georeferenced to the Lake County hillshade to predict how the site will look upon restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table 3 summarizes the potential footprints and volumes of ponds A and B assuming a 10-foot normal depth (Figure 6). The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design.

Figure 7A shows a longitudinal profile along the centerline of the Historic Eel River Upstream from Scott Dam (adapted from Stillwater Sciences' November 2021) with the approximate locations and proposed depths of Ponds A and B.

The design function similarly to the example provided in Figure 8, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table 3: Potential Footprint and Storage Volume of Ponds A and B under Scenario E.**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

The disadvantages associated with this scenario are high constructability costs.

### 3.7.1 Design and Construction Aspects

Considerations on volumes of materials required for sediment removal for this scenario and constructability aspects will be required.

### 3.7.2 Scenario Findings

The predicted scenario outcomes are similar to A2 but with additional benefits and outcomes with the inclusion of the ponds.

#### 3.7.2.1 Water Supply

The water supply for this scenario is of higher benefit than scenarios A1 and A2 but significantly lower than the current conditions. This scenario allows for storage from the proposed ponds.

The benefits of this scenario include the positive environmental impacts, the preservation of capacity for fire suppression and downstream use for municipalities.

Under extreme runoff conditions, the following design considerations need to be evaluated:

- Ponds would only allow for helicopter fire suppression support.
- Ponds may alleviate certain flood flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed and recharging would be limited to the watercourse contributions. This could result in a reduction to the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

This scenario is not likely to drastically change flows during the extreme flood conditions as the ponds will be located within the floodplain. Drought conditions would likely see a reduction from the Scenarios A1 and A2 predictions but are difficult to quantify or predict with the currently available data. The reduction or impacts to the extreme climate conditions would be dependent upon the infiltration rates, design goals, usage (fire suppression), and groundwater tables.

A summary of Hydrology and Hydraulics is presented in Table 4 (appended).

#### 3.7.2.2 Sediment and Channel Restoration

SLR reproduced Stillwater Sciences' longitudinal profiles to show the sediment accumulation from 1921-2016, showing how the dam's presence for 94 years has contributed to the deposition of sediment in Lake Pillsbury's upstream tributaries. Up to 60 ft of sediment has been deposited in Eel River upstream of Scott Dam in some locations (Figure 7A). Up to 10 ft of sediment has been deposited in Salmon Creek upstream of Scott Dam in some locations (Figure 7B). Prior to construction of the ponds, the sediment needs to be managed, channels need to be restored to natural conditions, and the ponds would be constructed on the natural riverbed.

#### 3.7.2.3 Fire Suppression

Depending on the area and depth of the adjacent ponds, any of the three operational scales presented in Table 1 (Section 2.2) could be feasible. For extensive use, there would likely be a need to excavate the ponds to create the depths needed for the high storage capacity. The preliminary pond sizes considered would allow for fire suppression with helicopters.

This option has the added benefit of creating off-channel floodplain habitat and open water sources sufficient in size to be used in fire suppression operations.

#### 3.7.2.4 Ecosystem

The ponds would not function as impoundments and therefore would not impede fish passage in the Eel River. This scenario would eliminate the existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage.

In addition to removal of the existing fish barrier, the proposed off-channel ponds adjacent to the Eel River would create spawning grounds for fish and other aquatic species, which provide additional ecological benefits. However, during the period between dam removal and the system reaching quasi-equilibrium, sediment flux is likely to stall the flourishing of the aquatic ecosystem. Therefore, it is important to develop and follow a robust sediment management plan.

#### 3.7.2.5 Recreation

Restoring natural river channels, along with creating off-channel ponds, can significantly enhance recreational opportunities in comparison to Scenarios A1 and A2. These changes may support expanded fishing, kayaking, birdwatching, and nature-based tourism. Off-channel ponds can provide calm-water areas for recreational activities, while restored channels may improve habitat diversity, attract more outdoor enthusiasts and boost local ecotourism.

#### 3.7.2.6 Economic and Cost Conditions

This subsection examines the economic aspects, focusing on initial implementation costs, long-term costs, and broader impacts on the local economy.

This scenario would include similar initial costs to Scenario A2. Initial capital costs for full decommissioning of the dam and completely remove sediments were estimated at $112 million (McMillen Jacobs, 2018). Additional costs, pending assessment, will be required for construction of the ponds.

##### 3.7.2.6.1 Initial Implementation Costs

This scenario would have a higher initial cost for the decommissioning and sediment management compared to Scenario A2. Associated costs for the sediment management plan, channel restoration and pond creation are the factors making this option more expensive. Similarly to Scenario A2, a risk assessment is recommended to better understand the risks associated with the removal and the potential impacts. This could include downstream infrastructure modification or replacement based upon sediment accumulations in the short and long term as well as changes to the floodplain.

##### 3.7.2.6.2 Long-term Operation and Maintenance Costs

Following initial sediment management and pond construction, this scenario would have infrequent maintenance associated with the watercourses in the footprint of Lake Pillsbury.

The ponds would be designed to be sustainable and resilient so that they would reasonably withstand larger flow events and require little to no maintenance.

##### 3.7.2.6.3 Economic Impacts

Like scenarios A1 and A2, the local economy will likely be impacted by the loss of revenue from tourism and recreation stimulated by Lake Pillsbury. Local businesses could suffer and may not recover. Residents and private ownership could see a reduction in property values and a complete loss in function to facilities that were built specifically for use with Lake Pillsbury. There may be opportunities for minor recreational activities in the vicinity of the newly built ponds that may attract some visitors.

#### 3.7.2.7 Cost/Complexity

High relative complexity when compared to other alternatives, considering permitting, design and construction requirements.

#### 3.7.2.8 Advantages

- Eliminates dam-specific operations, maintenance, and long-term safety liabilities.
- Maintains some benefits of water supply, fire suppression flood control, drought response, and recreation through establishment of standing water in off-channel ponds.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat. This enhancement is increased from scenario A1 by adding ponds to support spawning and shelter for fish and aquatic species.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Reduced volume of sediment transported downstream in the long-term due to active sediment management.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.
- This option provides the highest potential benefit to water supply when compared to the other scenarios where the dam is being removed.
- The wetland ponds would be resilient to flooding, since they act like sponges and absorb flood waters effectively.

#### 3.7.2.9 Disadvantages

- Risk of Sacramento Pikeminnow expanding downstream, potentially threatening native fish populations.
- Elimination of water storage and flow diversion capabilities to the Russian River Watershed.
- Requires large-scale removal and disposal of dam structures and materials.
- Likely presence of contaminated sediments, posing challenges for safe transport and disposal before and after dam removal.
- Short term water quality impacts due to sediment plume.
- Changes to public recreational opportunities and access with the loss of the reservoir.
- Loss of business and lowering of property values for local residents and reduction of tax revenue for the County.

- Need for long-term ecosystem monitoring and potential mitigation measures post-removal. This would be of a lesser degree with active sediment management and restoration efforts than scenario A1.
- Possible negative impacts on downstream water availability for agricultural, fire suppression and municipal users.
- Reduced groundwater recharge, leading to lower groundwater levels and potential for dry well.
- Reduced capacity for downstream flood control and drought response without reservoir regulation.
- The ponds would be vulnerable to drought, since they would be vulnerable to evaporation.

# 4.0 Preliminary Comparison of Scenarios

## 4.1 Criteria for Comparison

McMillen Jacobs (2018) completed a high-level feasibility study of potential alternatives for modifications to the Potter Valley Project. Different criteria were identified and organized in a matrix. SLR has considered similar criteria to prepare a comparison matrix for our evaluation of alternatives and added additional criteria to address the County's specific criteria. These criteria are presented in the sections below.

## 4.2 Rating System

The rating system used by SLR to compare the alternative decommissioning scenarios is adapted from the rating system used in the McMillen Jacobs (2018) evaluation matrix. However, the McMillen Jacobs ranking system was modified to be better suited for the County specific scenarios evaluated. The SLR ranking system is presented below.

| Ranking | Description |
|---|---|
| Very Good | Would be successful or have no impact. |
| Good | Would have a high likelihood of success or minimal impact. |
| Average | Would have a moderate likelihood of success or a significant impact. |
| Poor | Would have a poor likelihood of success or a significant impact. |

### 4.2.1 Modified 2018 McMillen Jacobs' Criteria

Complete descriptions for the criteria below can be found in *Section 4.3.2 Evaluation Matrix* of in McMillen Jacobs (2018). SLR's modified criteria are described below:

**Biological Efficiency (Fish Passage)**

This criterion presents a measure of the ability of the scenario to attract, guide, and pass fish over the dam and/or fishway passage efficiency.

A "poor" for this criterion would indicate that the scenario poorly supports fish passage and. A "very good" for this criterion would indicate that the scenario supports fish passage, both upstream and downstream.

**Constructability Challenges**

This criterion is intended to identify construction challenges. The construction challenges associated with a specific scenario or alternative may lead to elimination of the alternative due to construction challenges or geotechnical stability issues.

Considerations for this may include access availability, such as adequate routes for construction, geotechnical stability issues during construction, availability of utilities to support project construction and other construction considerations.

A “poor” for this criterion would indicate that the scenario faces construction challenges that may require complex solutions to overcome. A “very good” for this criterion would indicate that the scenario can be constructed minimal difficulty, using standard methods and equipment, with few or no geotechnical or logistical constraints.

**Environmental Impact – During Construction**

This criterion is intended to identify environmental impacts during construction that could make the project difficult or costly to construct or difficult to permit. The criterion would be applied considering only the construction phase of the alternative implementation. Consideration relates to riparian habitats, degradation of water quality, disruption to wildlife, and visual or aesthetic impacts.

A rating of “poor” under this criterion indicates that construction activities are likely to cause significant environmental harm, requiring extensive mitigation measures and potentially complicating the permitting process. A rating of “very good” indicates that construction would result in minimal environmental disturbance, with limited or no adverse effects on sensitive resources, and would likely face fewer regulatory hurdles.

**Environmental Impact – Post Construction**

Post-construction environmental considerations relate to the final project configuration and operation, and its impact on the natural resources and environment. This criterion identifies potential long-term impacts to riparian habitats, degradation of water quality, disruption to wildlife, and visual or aesthetic impacts.

A rating of “poor” under this criterion indicates that post-construction activities are likely to cause significant environmental harm, requiring extensive mitigation measures and potentially complicating the permitting process. A rating of “very good” indicates that construction would result in minimal environmental disturbance, with limited or no adverse effects on sensitive resources, and would likely face minimal regulatory hurdles.

**Operational Impact (Long-term operations)**

This criterion captures the potential impacts of long-term operations under the option or alternative operation as well as overall general operation complexity and challenges. It considers water storage and release for downstream users, sediment management, and water quality.

A rating of “poor” indicates that the scenario presents significant operational challenges—such as complex water management, high maintenance demands, or persistent water quality issues—that could be costly or difficult to manage over time. A rating of “very good” indicates that the scenario supports reliable, low-complexity operations with minimal maintenance needs and effective management of sediment and water quality, ensuring long-term sustainability and regulatory compliance.

**Design Approach**

The design approach criterion is intended to determine which scenarios would require relatively simple system designs with proven technologies.

A rating of “poor” indicates that the scenario requires a highly complex or unproven design. A rating of “very good” indicates that the scenario can be implemented using well-established, low-complexity design approaches.

**Initial Implementation Cost**

This criterion considers the anticipated level of capital investment including construction, engineering, planning, regulatory and permitting, and administration that would be associated with the scenario implementation.

A “poor” for this criterion would indicate a high construction/implementation cost. A “very good” for this criterion would indicate low construction/implementation cost.

**Long-term Operation and Maintenance Costs**

This criterion evaluates the long-term operational costs and level of effort associated with each scenario.

A “poor” for this criterion would indicate high operational costs and/or level of effort, and a “very good” for this criterion would indicate low operational costs and/or level of effort.

**Criteria not Considered**

The following criteria, although included in McMillen Jacobs (2018), were not considered in the evaluation of SLRs. These criteria may be revisited and assessed in future phases of this project.

- Cost sub criteria: Certainty in Capital Estimate and Life Span
- Regulatory and Permitting
- Safety Risk

### 4.2.2 County of Lake Specific Criteria

Based on additional considerations not included in the 2018 evaluation by McMillen Jacobs, the following additional criteria is included in this high-level feasibility evaluation.

**Water Supply**

The County is in an ecosystem that historically oscillates between wet and dry years. With changes in the climate, droughts have increasingly become a problem in this region.

Presently, Lake Pillsbury provides residents with access to drinking water via groundwater recharge. This criterion evaluates how each scenario impacts resident accessibility to groundwater as a source of drinking water. This criterion also considers the impact of each scenario on groundwater recharge and how it will affect the residents’ local wells used to access drinking water.

Lake Pillsbury also currently provides the residents of the County with a consistent water storage source. This criterion discusses the impacts of each scenario on the ability to keep a consistent, reliable source of water storage for the residents to use. This criterion offers a comprehensive view of water storage as opposed to the criterion “fire suppression” and “access to drinking water” which have a more specific lens.

A “poor” for this criterion would indicate that the scenario has low potential for access by local residents and other users and also scenario provide no or minimal water storage to manage drought and wet year impacts. A “very good” for this criterion would indicate that the scenario has a higher potential for water user access and provide necessary storage to manage wet and drought tolerances.

**Fire Suppression**

Wildfires are common in Lake County. The Mendocino National Forest and surrounding areas have a long history of significant wildfires. Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts (Friends of the Eel River, 2023).

Many common fire mitigation strategies such as helicopter bucket operations, fixed-wing aircraft water scooping, and ground-based drafting for fire engines and water tenders require a minimum volume of water to function. This criterion evaluates each scenario and its effects on fire suppression strategies.

A “poor” for this criterion would indicate that the scenario poorly supports fire suppression and mitigation efforts. A “very good” for this criterion would indicate that the scenario supports fire suppression and mitigation efforts.

**Recreation**

Lake Pillsbury serves as a recreational facility in and around the lake, including camping, boating, swimming, fishing, picnicking, wind surfing, and bird watching. This criterion evaluates the effects of each scenario on recreational activity, and the complexity of improving/relocating recreational facilities.

A “poor” for this criterion would indicate that the scenario poorly supports current recreational activity and/or future recreational activity. A “very good” for this criterion would indicate that the scenario supports current recreational activity and/or future recreational activity.

**Effects on Local Economy**

Each scenario has broader economic consequences for the local community. Changes in water availability, land use, and recreational opportunities can influence property values, business activity, and municipal tax revenues. This criterion evaluated these economic trade-offs, both for the community and the County of Lake.

A “poor” for this criterion would indicate that the scenario poorly supports homeowners, local businesses and the local economy in the County. A “very good” for this criterion would indicate that the scenario supports property values, local businesses and the local economy in the County.

### 4.2.3 Scenario Comparison Matrix

SLR has developed a Mitigation Scenario comparison table using the above discussed McMillen Jacobs and County of Lake criteria and included in this report (Table 5, appended). Descriptions to support the criteria rating are provided in the scenario outcomes in Section 3 and summarized in the table. The ranking is based on SLR subject matter expertise.

Based on SLR’s evaluation, PG&E’s Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by McMillen Jacobs (2018) as part of a consultation group. In contrast, we identified five other alternatives that rank more favorably and should be further evaluated for implementation.

# 5.0 Climate Vulnerability Considerations

Appendix D includes a description of the California Adaptation Planning Guide, PG&E's identified cumulative impacts from Scott Dam removal, and considerations from County of Lake's Climate Vulnerability Analysis.

Maintaining a safe and reliable water supply is essential for climate resilience in Lake County. The historical operations of Scott Dam and Lake Pillsbury provided strong protection against drought, flooding, wildfire, and extreme temperatures. As climate stressors intensify, the ability to store and manage water becomes increasingly critical for public health, agriculture, infrastructure, and emergency response.

This climate vulnerability analysis shows that scenarios which retain or enhance water storage and operational control offer the highest resilience. In contrast, scenarios involving full dam removal without replacement introduce significant vulnerabilities unless paired with robust mitigation strategies.

# 6.0 Permitting and Conservation Considerations

## 6.1 Permitting

Future permitting aspects are discussed in Appendix E. The main permitting aspects will include the following:

- Federal Permitting Process (FERC Lead), to comply with the National Environmental Policy Act (NEPA). An Environmental Impact Statement (EIS) is anticipated.
- California Permitting Process (State Water Resources Control Board as Lead and DODS) to comply with the California Environmental Quality Act (CEQA). An Environmental Impact Report (EIR) is anticipated.

## 6.2 Conservation Easement

A conservation easement for 5,660 acres of PG&E-owned land, of which approximately 2,234 acres are within the FERC Project boundary, was recorded on June 28, 2022, to permanently protect beneficial public values on lands owned by PG&E (PG&E, 2025a, PG&E, 2022a). The conservation easement is held by Mendocino Land Trust.

PG&E owned lands are already encumbered by easements held by Mendocino Land Trust. The easement permanently restricts development and protects conservation values (habitat, recreation, cultural resources) regardless of PG&E's future use. The conservation easement includes PG&E's right for continued operation and maintenance of hydroelectric facilities and associated water delivery facilities, PG&E's unequivocal ability to comply with any FERC license, FERC license renewal, or other regulatory requirements.

If PG&E ceases power operations, the land could be sold or transferred, but any new owner must comply with the conservation easement. This ensures no future industrial or residential development inconsistent with the agreed conservation purposes. Mendocino Land Trust is to enforce conservation restrictions; they do not own the land but can limit certain uses.

The easement aims to protect, preserve, or enhance the Beneficial Public Values on the property, including:

- Habitat for fish, wildlife, and plants

- Forest resources
- Scenic viewshed
- Outdoor recreation
- Historical and cultural values
- Agricultural values

There may be potential limitations for appropriate restoration activities as proposed in some of the mitigation scenarios discussed in this document. Additional evaluation, beyond the scope of this study, and negotiations with the Mendocino Land Trust for changes in the conservation easement may be required for appropriate restoration.

# 7.0 Conclusions

SLR has completed a preliminary evaluation of the impacts on the Lake County based on the Scott Dam decommissioning activities proposed by PG&E's Application for Surrender of License to FERC. SLR identified impacts on several key areas including water supply, recreation, fire suppression, ecosystem, restoration and economic conditions. Each of these impact areas were evaluated for some details of technical aspects as applicable, such as hydrology and hydraulics, geotechnical consideration and construction. Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Five alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Current Operational Conditions Scenario covers operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Final Application for Surrender of License dated July 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel Creek watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and installation of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

The above scenarios were evaluated for meeting the County of Lake and PG&E goals and a comparison of scenarios provided to help support the County of Lake's decision-making process.

Based on SLR's evaluation, PG&E's Proposed Action (Scenario A1), ranks the lowest among all alternatives considered. This scenario performed poorly across the majority of criteria, even those established by previous studies, indicating shortcomings in its anticipated outcomes. In contrast, we identified five other alternatives that rank more favorably and should be further evaluated for implementation as part of FERC's PVP surrender.

The information provided in this report may also be helpful in the following activities.

- Provide comments to FERC on the Final Application for Surrender of License
- Evaluation of future climate vulnerability aspects associated with the project
- Submit this report to comply with final deliverable per DWR grant requirements

# 8.0 Remarks

The services described in this work product were performed in accordance with generally accepted professional consulting principles and practices. No other representations or warranties, expressed or implied, are made. These services were performed consistent with our agreement with our client. This work product is intended solely for the use and information of our client unless otherwise noted. Any reliance on this work product by a third party is at such party's sole risk.

Opinions and recommendations contained in this work product are based on conditions that existed at the time the services were performed and are intended only for the client, purposes, locations, time frames, and project parameters indicated. The data reported and the findings, observations, and conclusions expressed are limited by the scope of work. We are not responsible for the impacts of any changes in environmental standards, practices, or regulations subsequent to performance of services. We do not warrant the accuracy of information supplied by others, or the use of segregated portions of this work product.

This work product presents professional opinions and findings of a scientific and technical nature. The work product shall not be construed to offer legal opinion or representations as to the requirements of, nor the compliance with, environmental laws rules, regulations, or policies of federal, state or local governmental agencies.

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# Figures

## Impact Analysis and Adaptation Strategy Evaluation Report

Lake County Resource Assessment – Proposed Scott Dam Decommissioning

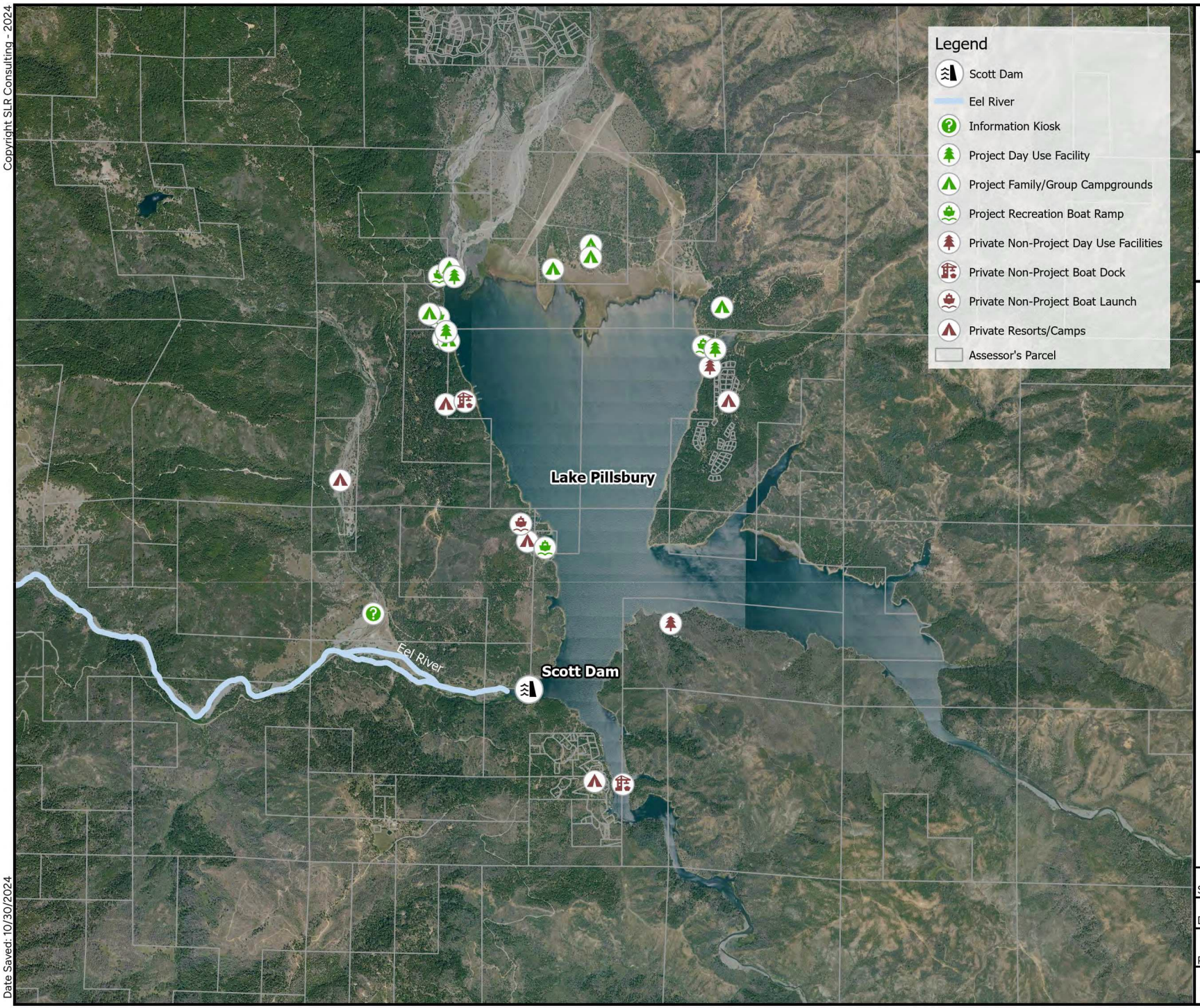
**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

0656

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Date Saved: 10/30/2024

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0 4,000

1 in. = 4,000 ft

SLR

SITE MAP (RECREATION)

Potter Valley Project Decommissioning

County of Lake

SCALE 1:48,000

DATE 10/30/2024

102.021249.00001

PROJ. NO.

**FIG. 1**

0657

Sources:
California Division of Regional Assistance
California Department of Water Resources
PG&E Initial Surrender Application

**Measured Historical Scott Dam Outflow to Eel River (2019-2024)**

**Minimum:** 42 cfs
**Average Annual:** 522 cfs
**Average Dry Season:** 230 cfs
**Average Wet Season:** 954 cfs
**Maximum:** 26,600 cfs

N

**Legend**

Scott Dam Location

**Measured Historical Lake Pillsbury Level**

Minimum - 1871 ft (16,800 ac-ft)

Average - 1895 ft (48,100 ac-ft)

Maximum - 1909 ft (75,400 ac-ft)

**PG&E Operational Lake Pillsbury Levels**

Maximum since 2023 at Spillway Elevation 1900 ft (56,300 ac-ft)

Minimum 1860 ft (10,000 ac-ft)

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 2**

**CURRENT CONDITIONS (WY 2019-2024)**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR

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Layer Credits: Esri, NASA, NGA, USGS, Earthstar Geographics, Bureau of Land Management [2025]

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Date Saved: 5/6/2025

Document Path: C:\Users\apierce\OneDrive - SLR Consulting\Documents\ArcGIS\Projects\AKP_ScottDam\AKP_ScottDam.aprx

Welch Creek
Salmon Creek
Salmon Creek
Smokehouse Creek
Panther Creek
Soda Creek
SALMON CREEK
Habematolel Creek
Squaw Valley Creek
Salt Spring Creek
Salmon Creek
Eel River
Salt Spring Creek
Eel River
Eel River
SCOTT DAM
Benmore Creek
Eel River
Thistle Glade Creek
Rice Fork

**Figure 3:** SCENARIO A1/A2 – SCOTT DAM DECOMISSIONING

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

- Rivers
- Creeks
- Seasonal Creeks

Hillshade
High : 254
Low : 0

1922 Pillsbury Bathymetry
Red: Band_1
Green: Band_2
Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

SLR

0659

N

**Legend**

Scott Dam Location

**Conceptual Operational Lake Pillsbury Level**

Lowered Spillway Elevation 1875 ft (20,000 ac-ft)

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 4**

**SCENARIO C - LOWERED SCOTT DAM WITH FISH LADDER CONCEPTUAL LAKE FOOTPRINT**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR

06-20

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Salmon Creek, Smokehouse Creek, and Habematolel Creek Tributaries = 14% of Scott Dam Watershed

N

Salmon Creek
Smokehouse Creek
Habematolel Creek
Eel River
Salt Spring Creek
Eel River
Eel River
Rice Fork
Eel River

**Legend**

**Conceptual Salmon Creek Dam Layout**

- Crest
- Footprint

**Scenario D Calculated Lake Pilsbury Levels**

- Minimum: 1875 ft (5,000 ac-ft)
- Average: 1885 ft (9,500 ac-ft)
- Maximum: 1900 ft (20,000 ac-ft)

**Watercourse Feeding Lake Pilsburry**

- Eel River
- Tributary

Note: Elevations are based on PG&E Datum, which equals NGVD 29 + 81.7 ft

**FIGURE. 5**

**SCENARIO D - SMALLER RESERVOIR IN THE SALMON CREEK**

SCOTT DAM / LAKE PILLSBURY SCENARIOS

PROPOSED SCOTT DAM DECOMMISSIONING

LAKE COUNTY

CALIFORNIA

0 0.5 1 Miles

1 in = 0.6 Miles

SLR



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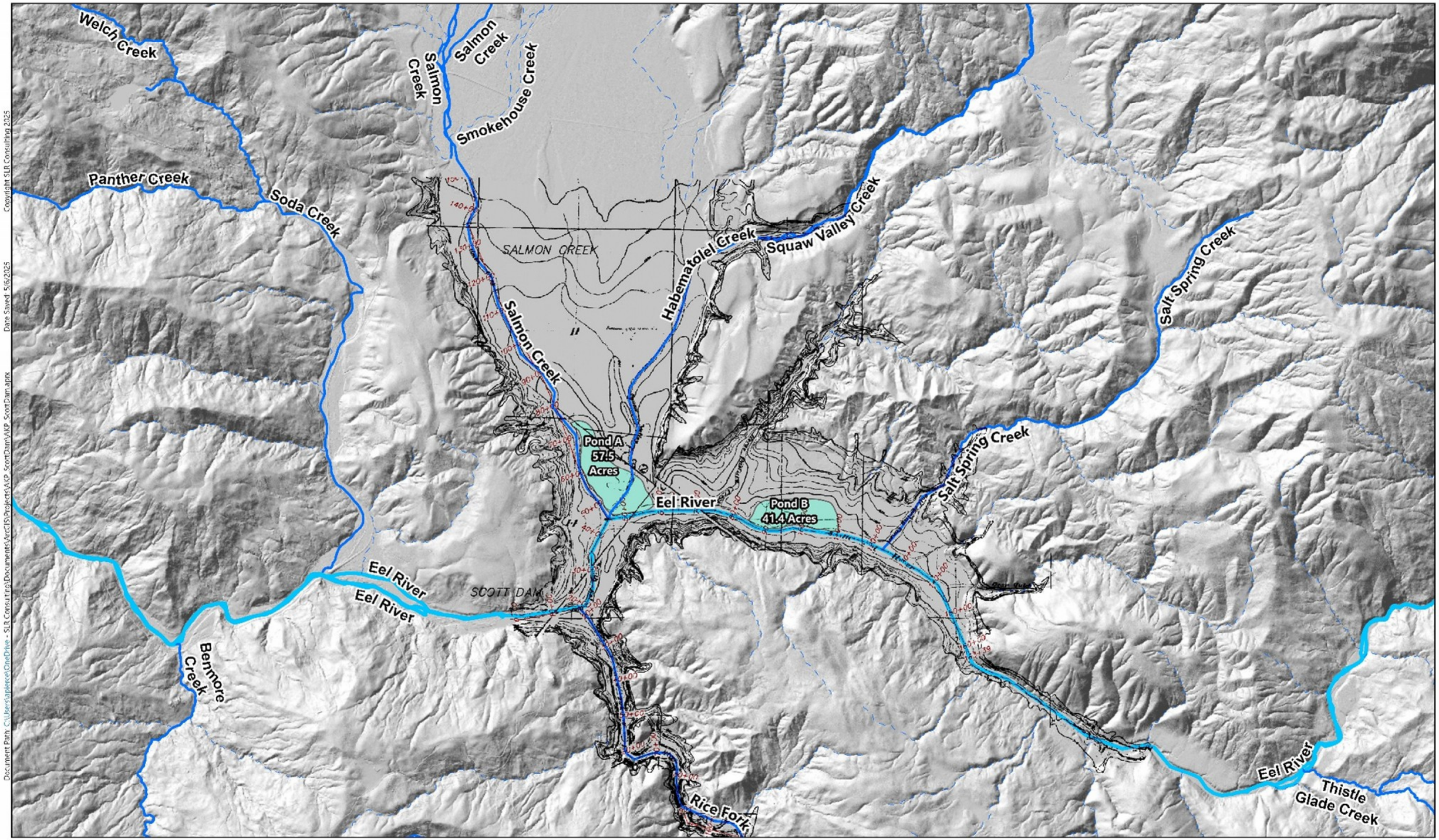


**Figure 6:** SCENARIO E - OFF-CHANNEL PONDS

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

- Rivers
- Creeks
- Seasonal Creeks
- Ponds
- Hillshade
  - High : 254
  - Low : 0
- 1922 Pillsbury Bathymetry
  - Red: Band_1
  - Green: Band_2
  - Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

SLR

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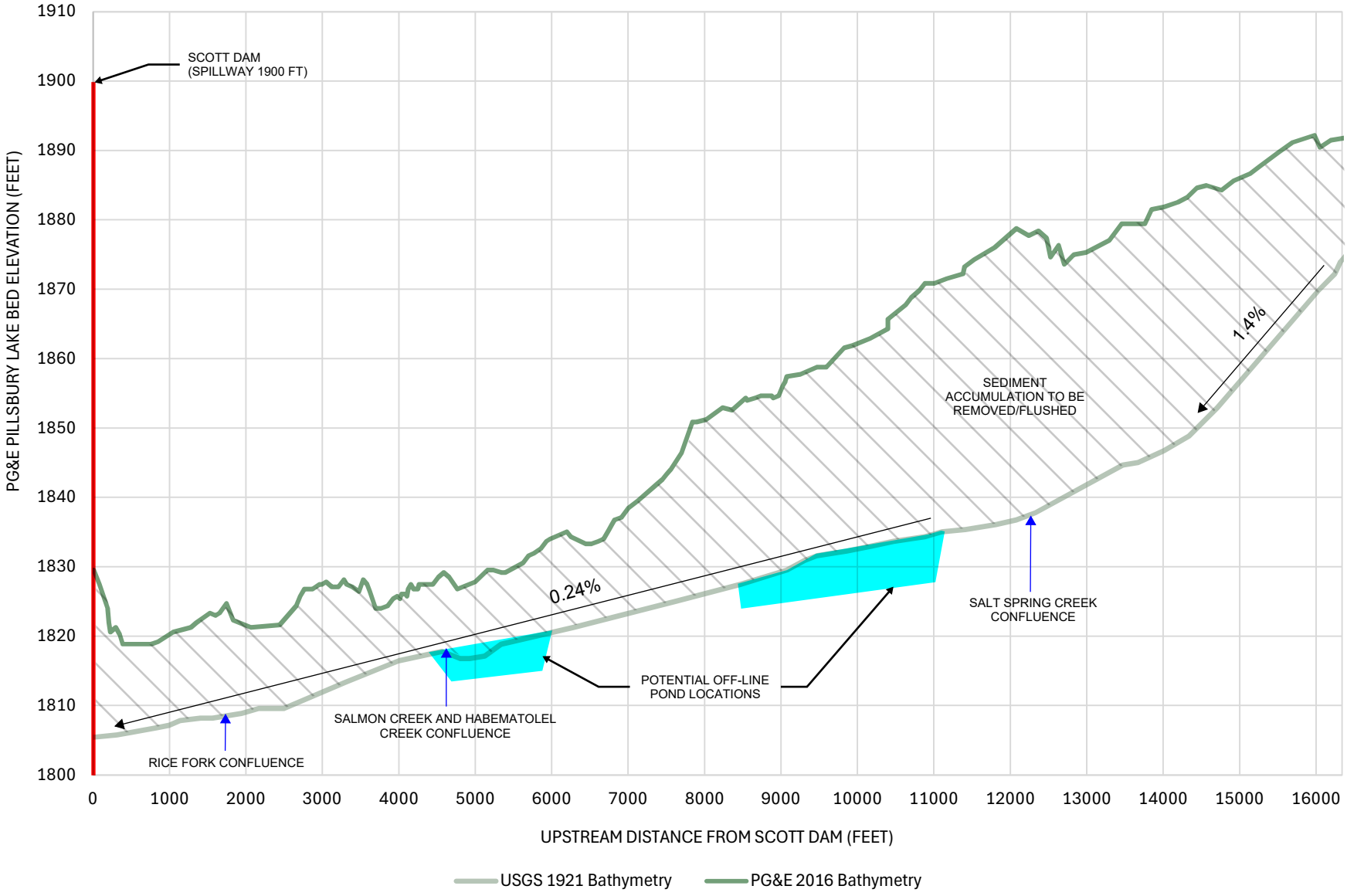
**FIGURE 7A:** LONGITUDINAL PROFILE ALONG THE CENTERLINE OF THE HISTORIC EEL RIVER UPSTREAM FROM SCOTT DAM




SLR

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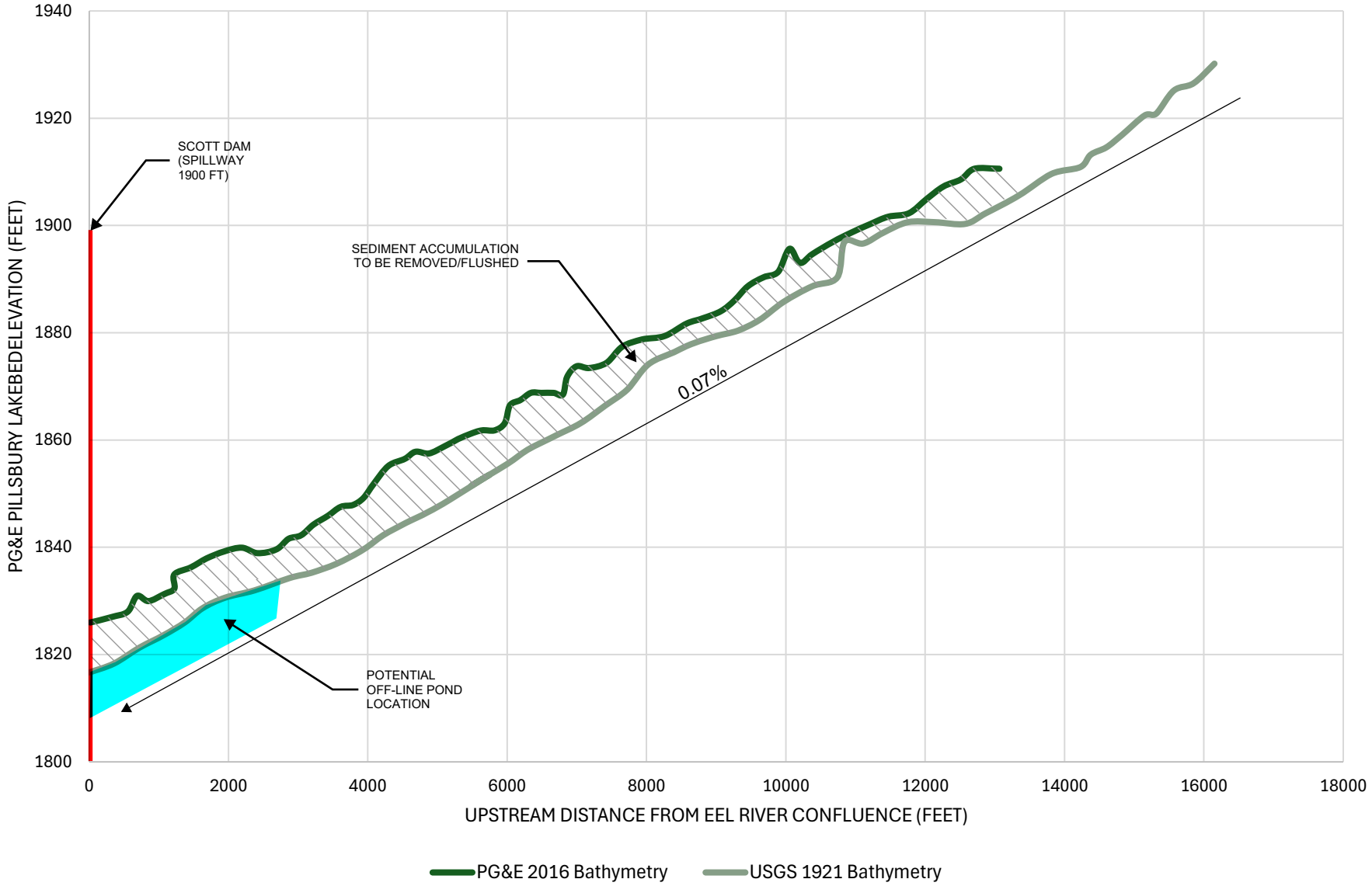
**FIGURE 7B:** LONGITUDINAL PROFILE ALONG THE CENTERLINE OF THE HISTORIC SALMON CREEK UPSTREAM FROM EEL RIVER CONFLUENCE

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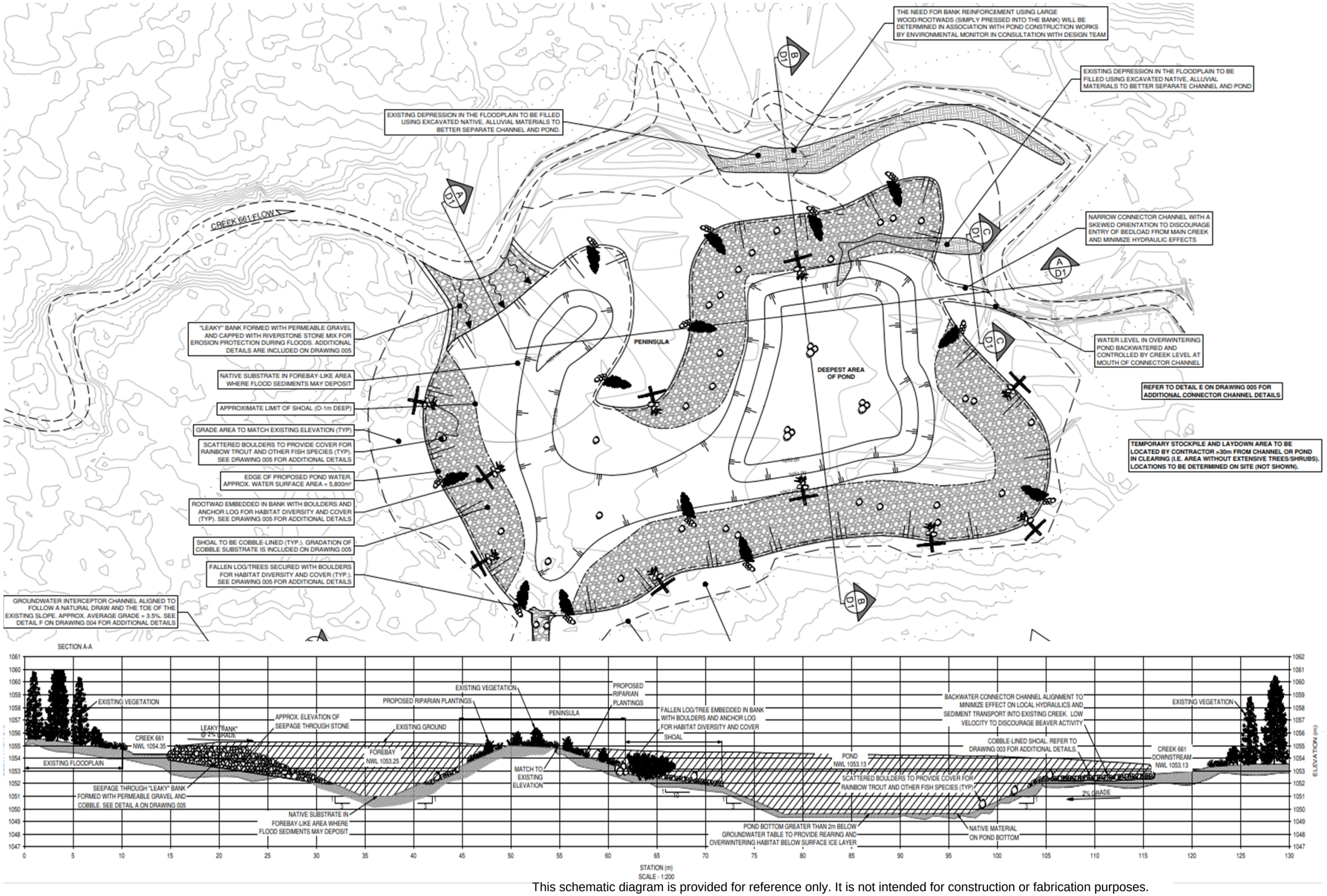


**Figure 8: Example Design of Off-Channel Pond**

This schematic diagram is provided for reference only. It is not intended for construction or fabrication purposes. Scale is not shown. Dimensions, locations, and configurations depicted may be approximate or symbolic. Diagram is based on Palmer/SLR's *Blackwater Gold Fish Habitat Offsetting. Creek 661 Overwintering Pond Detailed Design* dated February 28, 2023.

# Exhibits

## Impact Analysis and Adaptation Strategy Evaluation Report

Lake County Resource Assessment – Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

# Exhibit 1 Potter Valley Hydroelectric Project Site Plan

SLR

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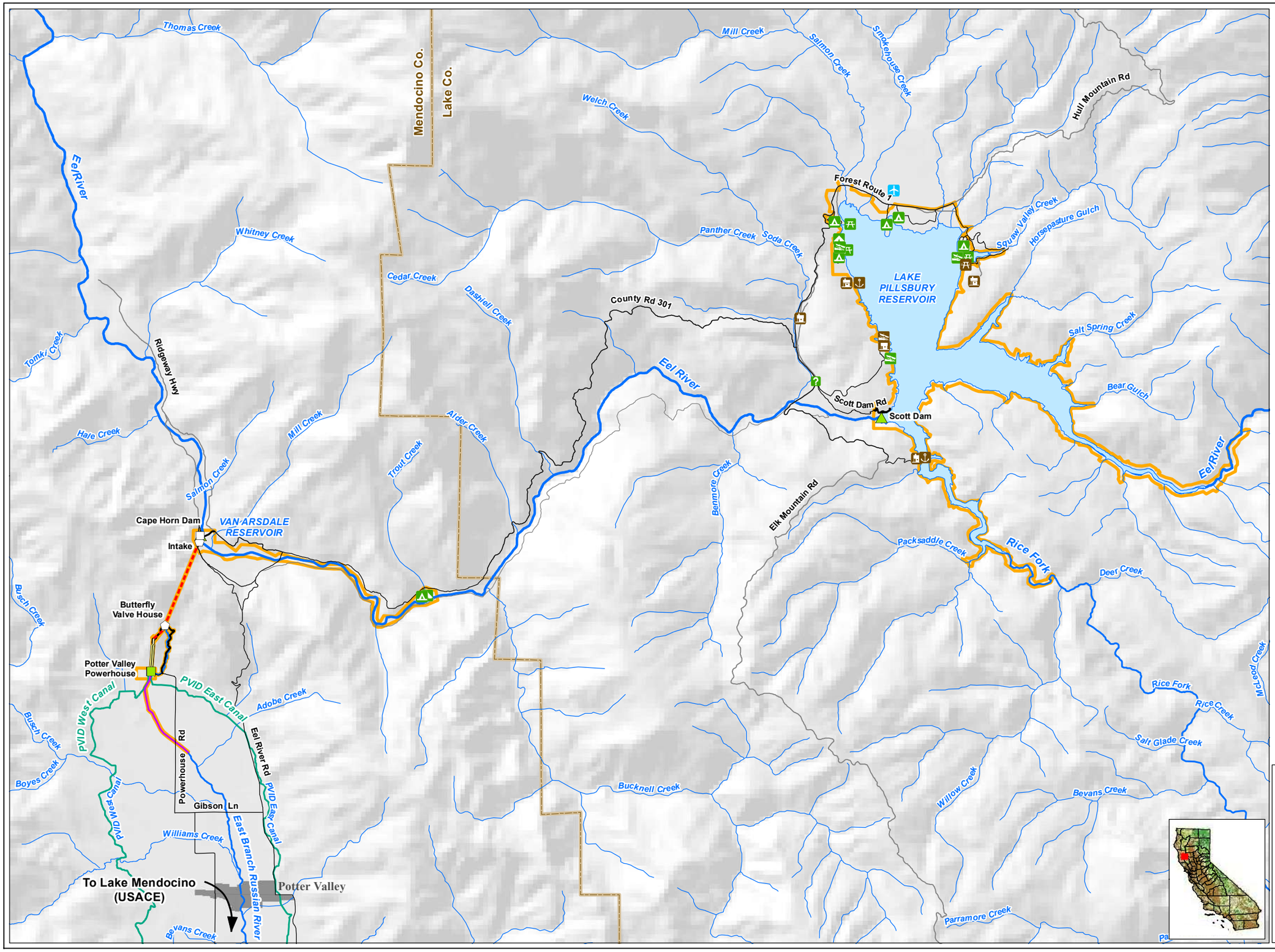
**Exhibit 1**

Potter Valley Hydroelectric Project Site Plan




**PG&E Project Facilities**

- Powerhouse
- Dam
- Water Conveyance Feature
- Water Conduit
- Penstock
- Powerhouse Discharge Canal
- FERC Project Boundary

**Project Recreation Features**

- Boat Ramp
- Family Campground
- Group Campground
- Day Use Facility
- Information Kiosk

**Private Non-Project Recreation Features**

- Boat Launch
- Boat Dock
- Day Use Facility
- Resort, Camp, or Tract

**Other Features**

- Airstrip
- Access Road
- General Access Road
- Other Road
- Canal
- Watercourse
- Waterbody

Pacific Gas and Electric Company®

PG&E Potter Valley Project
FERC Project No. 77

**Map 1-1**

**Project facilities and features.**

0 0.5 1 Miles

Projection: UTM Zone 10 N
Datum: NAD 83

Date: 7/15/2025

C:\Users\kgabel\OneDrive - Stantec\gisnt\185706343\map\LandMaps\Map 1-1_ProjectFacilities&Features_17i11i_10.mxd

Source:

PG&E, 2025. Final Application for Surrender of License VOLUME IA PUBLIC INFORMATION. July.

# Exhibit 2 Scott Dam Removal Cross Sections

SLR

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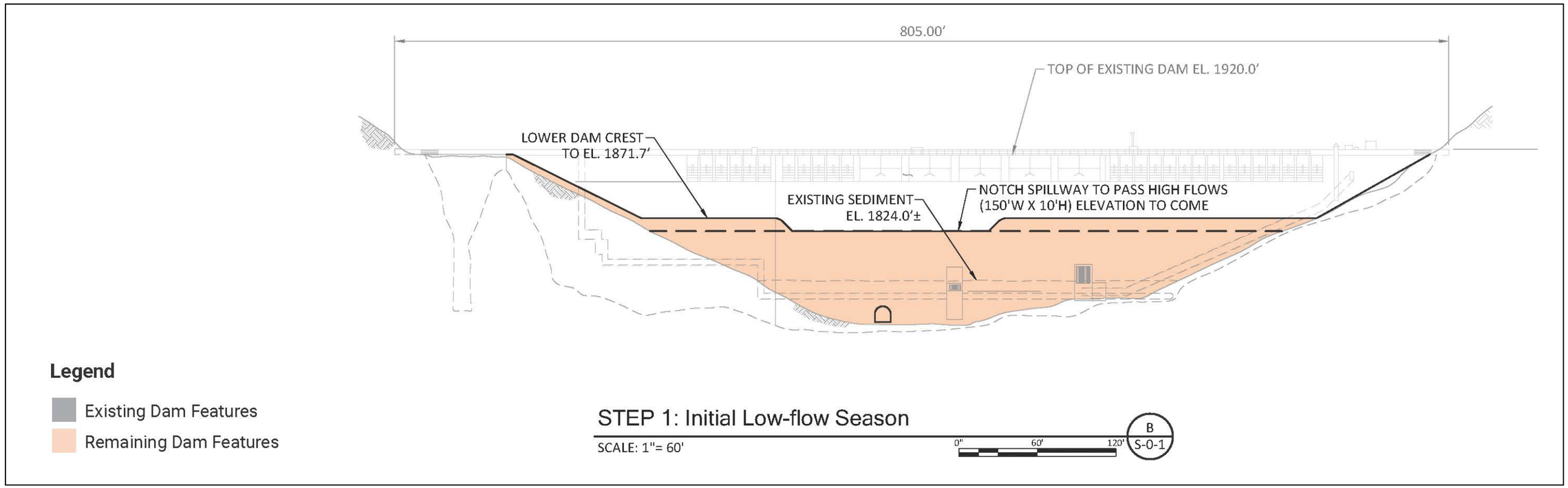


**Figure 2-2. Scott Dam removal—initial dam removal cross section (cross section—initial low-flow season).**


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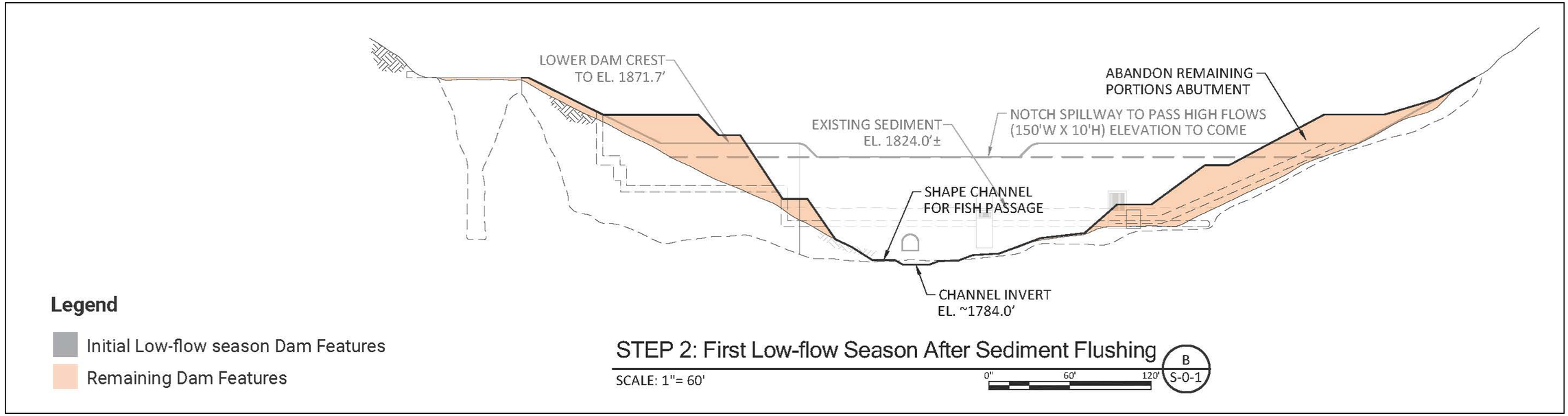


**Figure 2-3. Scott Dam removal—final dam removal cross section (cross section—first low-flow season after sediment flushing).**


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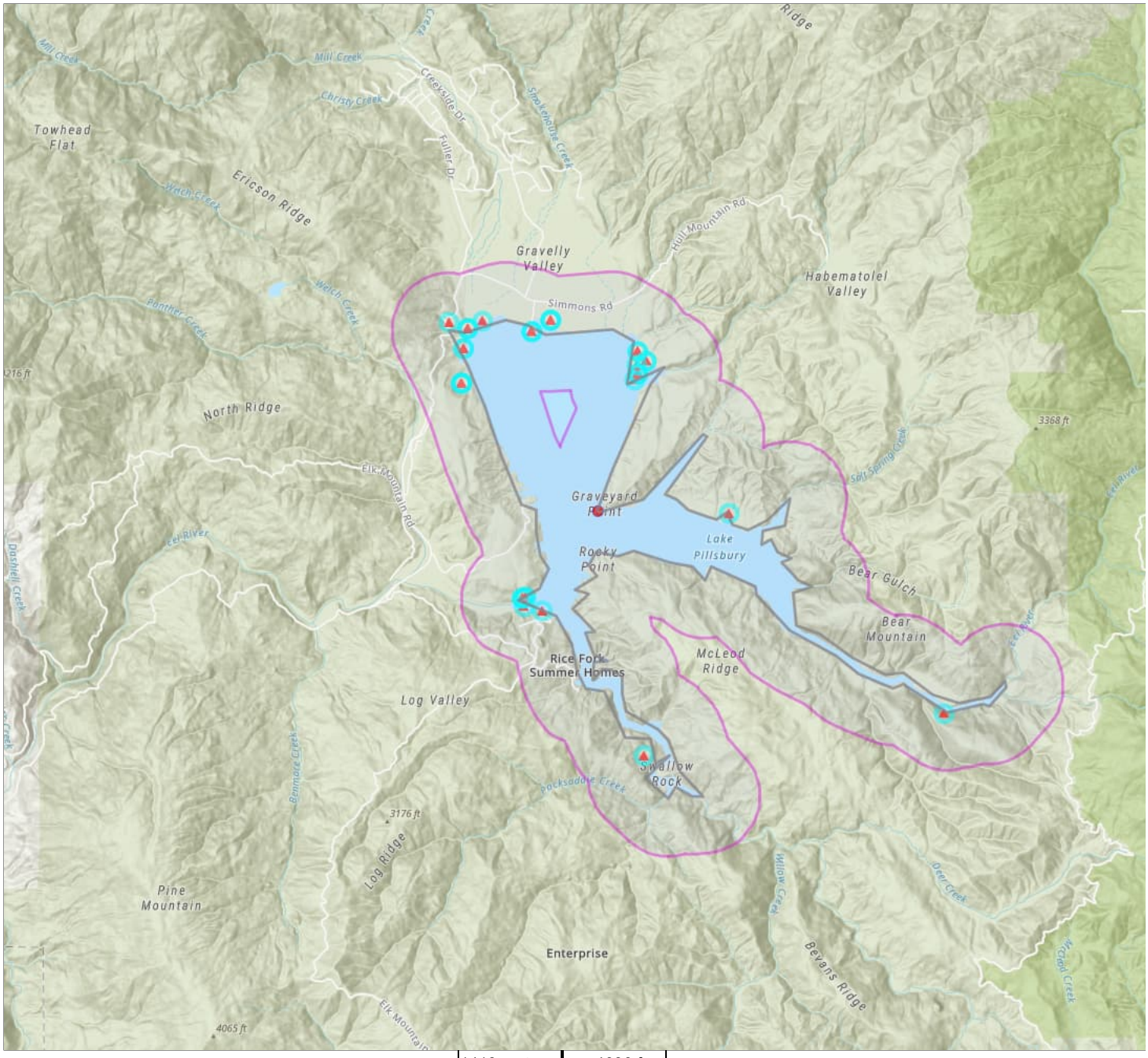
# Exhibit 3A Identified Species Near Lake Pillsbury (0.5 Mile Radius)

SLR

**Exhibit 3A**

Identified Species Near Lake Pillsbury (0.5 Mile Radius)




| 1413 meters | 4636 feet |
|---|---|

Endangered Species identified within 0.5 miles of Lake Pillsbury Permitter

**Map Legend**

**Unprocessed Data from CNDDB Online Field Survey Form [ds1002]**

Unprocessed Data from CNDDB Online Field Survey Form _ ds1002

- Animal
- Plant

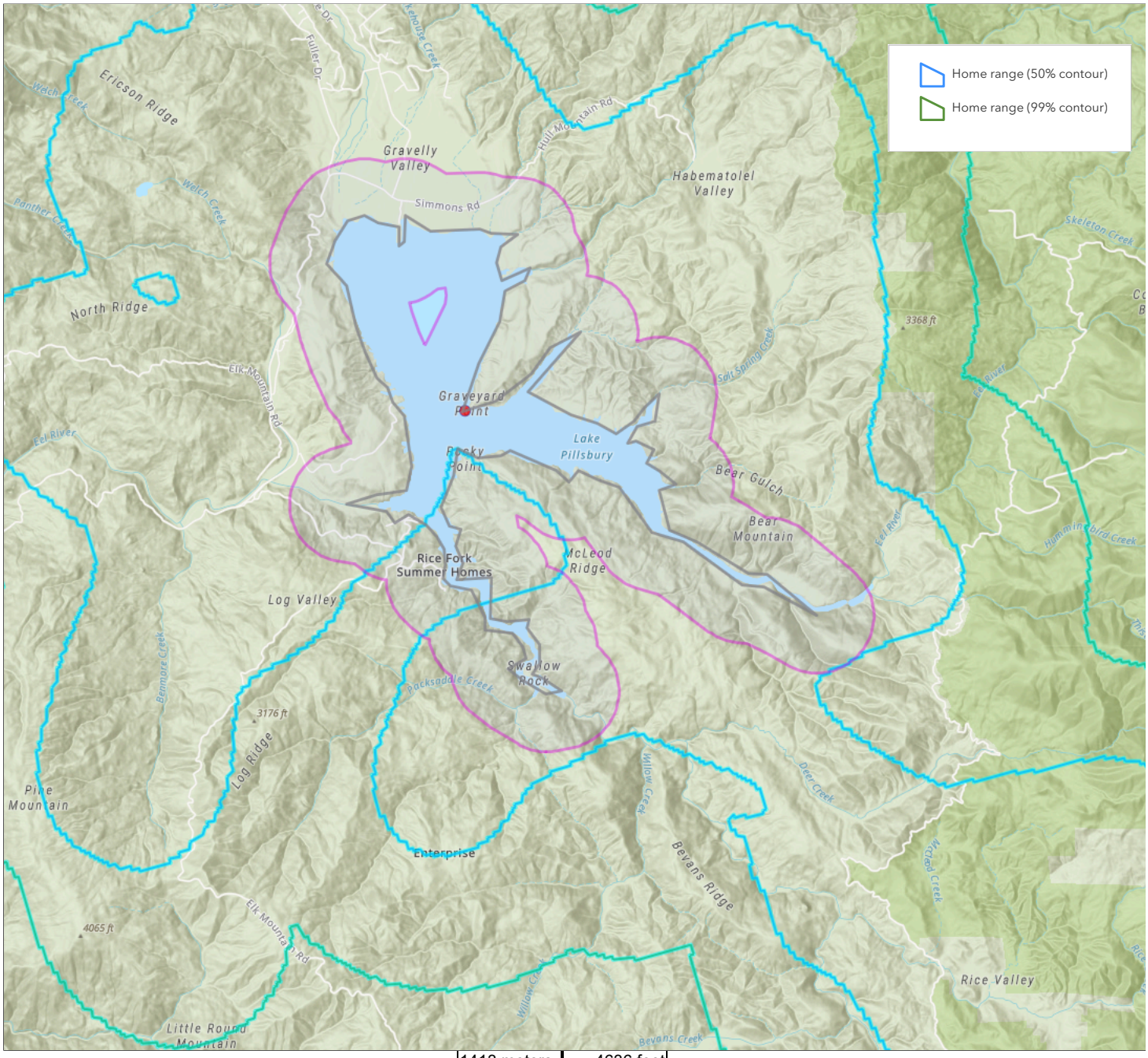
Source:

California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System. https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

# Exhibit 3B Tule Elk Contours

**Exhibit 3B**

Tule Elk Contours




Description: Elks identified nearby Lake Pillsbury that may enter the 0.5 mile radius range.

Source:

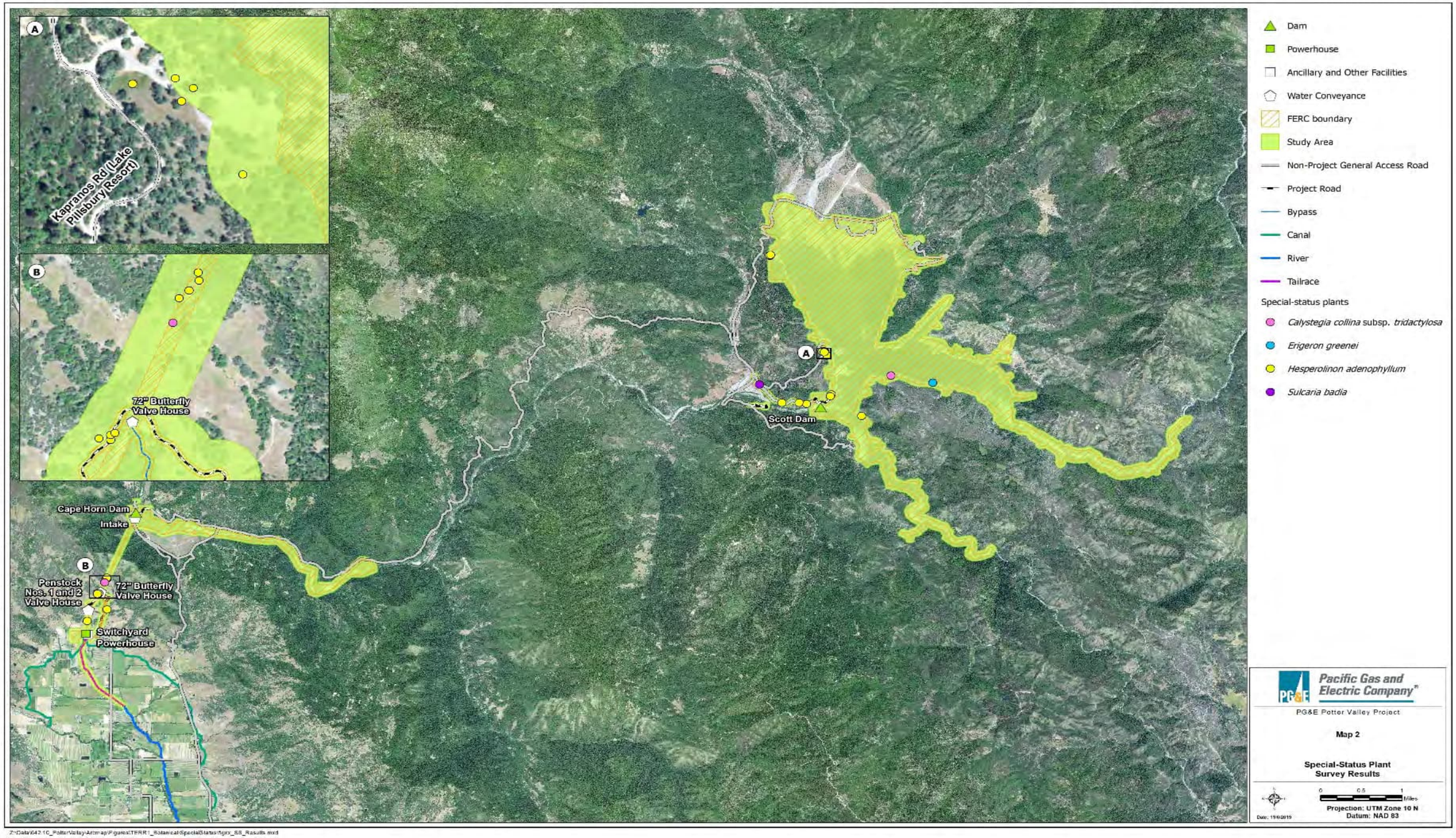
California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System.https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

# Exhibit 3C Special-status Species Plants Documented in the Study Area During the 2018 Botanical Survey

SLR

**Exhibit 3C**

Special-status species plants documented in the Study Area During the 2018 Botanical Surveys






Source:

Cardno and Stillwater Sciences, 2019. TERR 1 – Botanical Resources Data Memorialization, Potter Valley Project (FERC Project No. 77). December 5

# Exhibit 4 Decommissioning of Project Facilities and Features in the Scott Dam Area

SLR

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***Scott Dam Area***

The following provides a description of decommissioning of Scott Dam and associated Project facilities and features and Project recreation facilities. Table 2-8 provides a summary of decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area and the associated existing land ownership. Decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area includes leaving the facility in place (L), removal of the facility with restoration (RR), and removal of the facility with no restoration (X).

**Table 2-8. Decommissioning of Project facilities and features and Project recreation facilities in the Scott Dam Area.**

| Project Facility/Feature or Project Recreation Facility | Land Ownership | Decommissioning |
|---|---|---|
| **Dam and Associated Facility/Features** | | |
| Scott Dam | PG&E | RR |
| Valve Control House | PG&E | X |
| ***Reservoir*** | | |
| Lake Pillsbury (storage reservoir) | PG&E/USFS | RR |
| ***Reservoir Gage*** | | |
| E1—Lk Pillsbury NR Potter Valley CA (11470000) | PG&E | X |
| ***River Gages*** | | |
| E2—Eel R BL Scott Dam NR Potter Valley CA (11470500) | PG&E | L |
| **Leakage Weirs and Piezometers and Associated Trail[1]** | | |
| Scott Dam Leakage Weirs | PG&E | RR |
| Scott Dam Piezometers | PG&E | L (cap) |
| Scott Dam Piezometers and Leakage Weir Access Trail | PG&E | L |
| **Project Communication Line** | | |
| Scott Dam Block Building Communication Line | PG&E | X |
| **Ancillary and Support Facilities** | | |
| Scott Dam Block Building | PG&E | L (remove equipment only) |
| Scott Dam Boat Barrier | PG&E | X |
| **Access Roads[1]** | | |
| Scott Dam Rd (portion) | PG&E | L |
| Upper Scott Dam Access Rd | PG&E | L |


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| Project Facility/Feature or Project Recreation Facility | Land Ownership | Decommissioning |
|---|---|---|
| **Project Recreation Facilities** | | |
| ***Family Campgrounds*** | | |
| Fuller Grove Campground | USFS | RR |
| Navy Campground | USFS | L |
| Oak Flat Campground | USFS | L |
| Pogie Point Campground | USFS | L |
| Sunset Point Campground | USFS | L |
| ***Group Campgrounds*** | | |
| Fuller Grove Group Campground | PG&E | RR |
| **Day-Use Facilities** | | |
| Eel River Visitor Information Kiosk | USFS | RR |
| Fuller Grove Day-Use Area and Boat Launch | PG&E | RR |
| Pillsbury Pines Day-Use Area and Boat Launch | PG&E | RR |
| Pogie Point Day-Use Area | USFS/PG&E | L |

[1] Not listed as a Project road or trail in the license.

Notes: L = leaving the facility in place
RR = full or partial removal of the facility with restoration
X = removal of the facility with no restoration.

*Scott Dam and Associated Project Facility/Feature Removal*

PG&E will implement the Rapid Dam Removal approach to decommission Scott Dam. The description of Rapid Dam Removal is based on the *Scott Dam and Cape Horn Dam Removal Alternatives Technical Memorandum* (McMillen Jacobs Associates 2021), existing site conditions, and technical expertise and is described in more detail below. Scott Dam Area elevations cited herein are based on PG&E's datum, which equals NGVD29 + 81.7 ft.

Rapid Dam Removal entails expedited removal of Scott Dam (approximately 2 years in duration, but the 2 years may not be consecutive depending on site conditions and flows) such that no water is impounded and volitional fish passage and unimpaired flows occur once decommissioning is completed. The structure would no longer be under the jurisdiction of FERC. Rapid Dam Removal would result in the flushing of a large volume of sediment (approximately 12 million cubic yards [cy]) downstream of the remnant reservoir into the Eel River. See Maps 2-8a,b and Figures 2-2 and 2-3 for plan and section views of the dam removal. Provided below is a description of the construction area and rapid removal of Scott Dam and associated Project facilities/features.


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# Tables

## Impact Analysis and Adaptation Strategy Evaluation Report

Lake County Resource Assessment – Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

**Table 4: Summary of Scenario Hydrology and Hydraulics under the Existing and Extreme Climate Conditions**

| Scenario | Existing Climate Conditions | | | | Extreme Climate Conditions | | | |
|---|---|---|---|---|---|---|---|---|
| | Wet Season (December - April) | | Dry Season (May - November) | | Flood | | Drought | |
| | Storage (ac-ft) | Outflow (cfs) | Storage (ac-ft) | Outflow (cfs) | Max Storage (ac-ft) [Date] | Max Outflow (cfs) [Date] | Min Storage (ac-ft) [Date] | Min Outflow (cfs) [Date] |
| Historical Conditions WY 1922-2019 & 2005-2019 | **1922-2019**<br>Min: 10<br>Median: 65,900<br>Average: 59,810<br>Max: 94,900<br><br>**2005-2019**<br>Min: 7,840<br>Median: 55,400<br>Average: 49,078<br>Max: 80,200 | **1922-2019**<br>Min: 2<br>Median: 375<br>Average: 1,033<br>Max: 45,300<br><br>**2005-2019**<br>Min: 37<br>Median: 296<br>Average: 820<br>Max: 26,600 | **1922-2019**<br>Min: 2,100<br>Median: 59,700<br>Average: 58,240<br>Max: 94,900<br><br>**2005-2019**<br>Min: 11,300<br>Median: 45,200<br>Average: 45,911<br>Max: 80,800 | **1922-2019**<br>Min: 0.1<br>Median: 177<br>Average: 222<br>Max: 10,200<br><br>**2005-2019**<br>Min: 27<br>Median: 147<br>Average: 165<br>Max: 2,400 | **1922-2019**<br>Flood of record:<br>76,260 [1/26/1983]<br>Max gage reading:<br>94,900 [4/24/1932]<br><br>**2005-2019**<br>Flood of record:<br>72,470 [12/31/2005]<br>Max gage reading:<br>80,200 [4/29/2005] | **1922-2019**<br>Flood of record:<br>15,400 [1/27/1983]<br>Max gage reading:<br>45,300 [12/22/1964]<br><br>**2005-2019**<br>Flood of record:<br>33,500 [12/31/2005]<br>Max gage reading:<br>33,500 [12/31/2005] | **1922-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>10 [12/9/1931]<br><br>**2005-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>7,840 [2/5/2014] | **1922-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>0.1 [9/8/1924]<br><br>**2005-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>27 [11/5/2015] |
| Current Conditions Historical WY 2019-2024 & Projected | **2019-2024**<br>Min: 16,800<br>Median: 53,200<br>Average: 45,439<br>Max: 67,300<br><br>**Projected**<br>Min: 13,870<br>Average: 48,500<br>Max: 78,910 | **2019-2024**<br>Min: 67<br>Median: 207<br>Average: 768<br>Max: 13,200<br><br>**Projected**<br>Min: 40<br>Average: 1,052<br>Max: 51,230 | **2019-2024**<br>Min: 21,100<br>Median: 44,100<br>Average: 43,789<br>Max: 62,100<br><br>**Projected**<br>Min: 13,910<br>Average: 45,070<br>Max: 67,380 | **2019-2024**<br>Min: 42<br>Median: 115<br>Average: 201<br>Max: 2,150<br><br>**Projected**<br>Min: 40<br>Average: 258<br>Max: 15,110 | **Projected**<br>78,910 | **Projected**<br>51,230 | **Projected**<br>13,870 | Not enough data to adequately predict.<br>Likely <40. |
| Scenario A1/A2 - PGE Proposed Removal | No storage | Min: 791<br>Average: 1,216<br>Max: 1,535 | No storage | Min: 17<br>Average: 134<br>Max: 376 | No storage | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | No storage | Not enough data to adequately predict.<br>Restoration efforts can provide resiliency.<br>Likely <17. |
| Scenario C - Lowered Scott Dam with Fish Ladder | Min: 5,420<br>Average: 18,769<br>Max: 28,460 | Min: 20<br>Average: 1,116<br>Max: 47,470 | Min: 7,340<br>Average: 18,080<br>Max: 31,840 | Min: 20<br>Average: 220<br>Max: 14,923 | 31,840 | 47,470 | 5,420 | Not enough data to adequately predict.<br>Likely <20. |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Min: 9,100<br>Average: 10,110<br>Max: 13,500 | Min: 684<br>Average: 1,051<br>Max: 1,327 | Min: 9,060<br>Average: 9,920<br>Max: 11,860 | Min: 15<br>Average: 43<br>Max: 245 | 13,500 | 51,686 | 9,060 | Not enough data to adequately predict.<br>Restoration efforts can provide resiliency.<br>Likely <15. |
| Scenario E – Dam Removal + Off Channel Pond Restoration | ≥863 | Min: 682<br>Median: 1,067<br>Average: 1,048<br>Max: 1,324 | <863 | Min: 15<br>Median: 43<br>Average: 80<br>Max: 244 | ≥863 | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | <<863 | Not enough data to adequately predict.<br>Restoration design can provide resiliency.<br>Likely <15. |

Notes:
1. All outflow values predicted at the location just downstream of the Scott Dam within the Eel River.
2. All storage values are limited to predicted storage within Lake Pillsbury upstream of Scott Dam except scenario D where values are based within the reduced impoundment footprint.
3. Scenario B hydraulic and hydrologic parameters were not analyzed or reported in this table.

| | Current Conditions | Scenario A1 | Scenario A2 | Scenario B |
|---|---|---|---|---|
| | | PG&E Proposed Removal | Scenario A1 with Additional Mitigation | Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder |
| **Similar Scenarios** | | Proposed action by PG&E in 2025 | Sediment management alternatives (McMillen Jacobs, 2018) | Option A (McMillen Jacobs, 2018) |
| **Biological Efficiency (Fish Passage)** | **Poor** | **Very Good** | **Very Good** | **Average** |
| | No fish passage upstream of Scott Dam | Unimpeded upstream and downstream fish migration through restored Eel River. | Unimpeded upstream and downstream fish migration through restored Eel River. | Upstream passage through fish ladder. Uncertain downstream passage. |
| **Constructability Challenges** | **Not Applicable** | **Average** | **Average** | **Average** |
| | No New Construction | To be addressed in PG&E Management Plan | To be addressed in PG&E Management Plan | PG&E Management Plan should address the fish ladder |
| **Environmental Impact – During Construction** | **Not Applicable** | **Poor** | **Average** | **Average** |
| | No construction under current conditions. | Significant short term sediment release from reservoir | Minor impacts | Minor impacts |
| **Environmental Impact – Post Construction** | **Good** | **Average** | **Good** | **Good** |
| | No major impacts | Long term sediment impacts; Release of non-native species from Lake Pillsbury. | Minor impacts | Minor impacts |
| **Operational Impact** | **Average** | **Very Good** | **Very Good** | **Good** |
| | Same as Current Conditions | To be addressed in PG&E Management Plan | To be addressed in PG&E Management Plan | PG&E Management Plan should address the fish ladder |
| **Design Approach** | **Not Applicable** | **Good** | **Good** | **Average** |
| | No New Design | Additional Designs to be addressed within Management Plan | Additional designs to be addressed within Management Plan | Additional design to be addressed wtihin Management Plan |
| **Initial Implementation Cost** | **Very Good** | **Average** | **Average** | **Average** |
| | No initial implementation cost to maintain current conditions. | Dam removal costs. | Active sediment management will have high short-term costs. | High seismic retrofit cost and fish passage construction cost. |

| | Scenario C | Scenario D | Scenario E |
|---|---|---|---|
| | Lowered Scott Dam with Fish Ladder | Reduced Reservoir within Salmon Creek as Main Tributary | Dam Removal with Off-Channel Pond Restoration |
| **Similar Scenarios** | Option C (McMillen Jacobs, 2018) | | |
| **Biological Efficiency (Fish Passage)** | **Average** | **Good** | **Very Good** |
| | Upstream passage through fish ladder. Downstream passage over lowered spillway. | Unimpeded upstream and downstream fish migration through restored Eel River. | Unimpeded upstream and downstream fish migration through restored Eel River. Additional fish and aquatic habitat provided in off-channel ponds. |
| **Constructability Challenges** | **Average** | **Average** | **Good** |
| | PG&E Management Plan should address the fish ladder | New Design will address constructability | To be addressed in PG&E Management Plan |
| **Environmental Impact – During Construction** | **Average** | **Average** | **Poor** |
| | Significant short term sediment release from reservoir | Significant short term sediment release from reservoir | Significant short term sediment release from reservoir |
| **Environmental Impact – Post Construction** | **Good** | **Good** | **Good** |
| | Long term sediment impacts; Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. | Long term sediment impacts; Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. | Long term sediment impacts partly mitigated by sediment management and revegetation; Release of non-native species from Lake Pillsbury. |
| **Operational Impact** | **Good** | **Very Good** | **Very Good** |
| | PG&E Management Plan should address the fish ladder | New Design will address operational impacts | To be addressed in PG&E Management Plan |
| **Design Approach** | **Average** | **Average** | **Very Good** |
| | Additional design to be addressed within Management Plan | The new design will be included in the Management Plan | The new design will be included in the Management Plan |
| **Initial Implementation Cost** | **Average** | **Average** | **Average** |
| | High seismic retrofit cost and fish passage construction cost. | Active sediment management will have high short-term costs. High construction costs | Active sediment management will have high short-term costs. Medium expected construction costs |

| | Current Conditions | Scenario A1 | Scenario A2 | Scenario B |
|---|---|---|---|---|
| | | PG&E Proposed Removal | Scenario A1 with Additional Mitigation | Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder |
| **Similar Scenarios** | | Proposed action by PG&E in 2025 | Sediment management alternatives (McMillen Jacobs, 2018) | Option A (McMillen Jacobs, 2018) |
| **Long-term Operation and Maintenance Costs** | **Average** | **Very Good** | **Very Good** | **Average** |
| | High maintenance and operational costs. | Minimal maintenance or operational costs. | Minimal maintenance or operational costs. | High maintenance and operational costs. |
| **Water Supply** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Most reliable for flood control, irrigation, and drinking water to Russian River watershed. | No flood control. Limited reliability in providing water for irrigation and drinking water. Dependent upon flows. Limited reliability in the dry season and likely no reliability during a drought. | No flood control. Limited reliability in providing water for irrigation and drinking water. Dependent upon flows. Limited reliability in the dry season and likely no reliability during a drought. | Less reliable than current conditions but still provides a majority of current needs. |
| **Fire Suppresion** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Most reliable option. Provides a high availability of fire suppression support. | No option for fire suppression. | No option for fire suppression. | Less reliable than current conditions, but still provides a high availability of fire suppression support |
| **Recreation** | **Very Good** | **Poor** | **Average** | **Very Good** |
| | Boating, fishing, marina, swimming, camping, hiking. | Loss of many recreational uses including boating and swimming. Loss of related infrastructure such as marina, docks, and piers. | Loss of many recreational uses including boating and swimming. Potential for creation of recreation alternatives as part of mitigation. | Should maintain most baseline recreation with limited use during droughts. |
| **Effects on Local Economy** | **Very Good** | **Poor** | **Poor** | **Very Good** |
| | Supports current local economy. | Loss of recreational uses creates less income from tourism. Loss of jobs. | Loss of recreational uses creates less income from tourism. Loss of jobs. | Maintains economic benefits to current business, landowners and county tax revenue. |

| | Scenario C | Scenario D | Scenario E |
|---|---|---|---|
| | Lowered Scott Dam with Fish Ladder | Reduced Reservoir within Salmon Creek as Main Tributary | Dam Removal with Off-Channel Pond Restoration |
| **Similar Scenarios** | Option C (McMillen Jacobs, 2018) | | |
| **Long-term Operation and Maintenance Costs** | **Average** | **Good** | **Very Good** |
| | High maintenance and operational costs. | Low with proper design. | Minimal with proper design. |
| **Water Supply** | **Very Good** | **Average** | **Poor** |
| | Limited flood control, irrigation, and drinking water reliability. | Limited flood control. Limited reliability for irrigation and drinking water. | Limited flood control, irrigation and drinking water reliability. Mostly dependent on flows. Limited reliability during dry season and no reliability during drought. |
| **Fire Suppresion** | **Very Good** | **Very Good** | **Average** |
| | Less reliability on fire suppression but still likely allowable for extensive use with planes. | Potential for extensive use for fire suppression, including use of planes. | Less reliable than current conditions, a range of fire suppression measures are possible for use of helicopters. |
| **Recreation** | **Good** | **Good** | **Average** |
| | Should maintain most baseline recreation with limited use during droughts. Recreational facilities need to be relocated to new shoreline, or dredging could expand lake footprint. | Provides limited recreation use when compared to baseline based on Salmon and Habematolel Creek watershed water supply. High fluctuations in the WSEs during dry season are probable. | Recreational facilities could be created in the vicinity of ponds or riverbanks. |
| **Effects on Local Economy** | **Very Good** | **Good** | **Average** |
| | Maintains economic benefits to current business, landowners and county tax revenue. Some reductions expected due to reduced reservoir. | Maintains economic benefits to current business, landowners and county tax revenue. Some reductions expected due to reduced reservoir. | Pond recreational could suport tourism and jobs. |

# Appendix A - Hydrologic and Hydraulic Scenario Evaluation

## Analysis of Effects of Proposed Scott Dam Decommissioning

### County of Lake

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SLR Project No.: 102.021249.00001

August 22, 2025

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# Appendices

Appendix A.A Supporting Figures for Hydrologic and Hydraulic Analysis

Appendix A.B PG&E Provided Data

# Acronyms and Abbreviations

| | |
|---|---|
| ac-ft | acre-feet |
| AEP | annual exceedance probability |
| CADSOD | California Division of Dam Safety |
| CDEC | California Data Exchange Center |
| COY | Coyote Lake Mendocino Weather Station |
| cfs | Cubic feet per second |
| DWR | Department of Water Resources |
| ELP | Eel River below Lake Pillsbury Weather Station |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| FFA | flood frequency analysis |
| ft | feet |
| HEC-RAS | Hydrologic Engineering Center's River Analysis System |
| LAT | Large Airtanker |
| LPY | Lake Pillsbury Weather Station |
| MSL | mean seal level |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NIDIS | National Integrated Drought Information System |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NOAA | National Oceanic and Atmospheric Administration |
| NMFS | National Marine Fisheries Service |
| NERF | New Eel Russian Facility |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| SCD | Scott Dam Weather Station |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| SPI | Standardized Precipitation Index |
| USACE | United States Army Corps of Engineers |
| USDM | U.S. Drought Monitor |
| USGS | United States Geologic Survey |
| VLAT | Very Large Airtanker |

| | |
|---|---|
| WSE | water surface elevation |
| WY | water year |

# A1.0 Introduction

SLR International Corporation (SLR) conducted an evaluation of the existing hydrologic and hydraulic conditions of the Eel River and Scott Dam system within Lake County as part of the *County of Lake Impact Analysis and Adaptation Strategy Evaluation Report*. The primary objective was to assess the potential water supply impacts of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam and the removal of Lake Pillsbury. In addition to evaluating PG&E's proposal, SLR developed and analyzed several alternative mitigation scenarios designed to better align with Lake County's objectives. Each scenario, including the historical baseline representing past operational conditions, was assessed under four climate conditions: wet season, dry season, flooding, and drought.

The baseline scenario covers the Historical Operational Conditions during water years[1] (WYs) 1922-2019 and a focused WY period from 2005-2019.

Mitigation Scenarios:

1. Existing Operational Conditions Scenario covers current operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 follows PG&E's proposal per the PG&E Surrender Application dated January 2025, for the complete removal of Scott Dam.
3. Scenario A2 follows PG&E's proposed removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggestion for additional mitigation (including active sediment management).
4. Scenario B assuming Scott Dam to remain as operated currently (i.e. Spillway gates open at elevation 1,900 ft [PG&E datum[2]] and operation of the needle valve to provide water supply downstream) and retrofitted with a fish passage structure.
5. Scenario C assumes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device, and the inclusion of a fish ladder structure.
6. Scenario D assumes Scott Dam is removed and a new earthen dam is placed to restrict flows from only the Salmon and Habematolel Creek watersheds for a smaller reservoir footprint. The spillway is assumed to be at an elevation of 1,885 ft.
7. Scenario E assumes Scott Dam is removed and one or two small off-channel ponds are built along the Eel River mainstem allowing for water storage and natural fish passage.

SLR analyzed available hydrologic and hydraulic data to provide a high-level summary of each scenario's anticipated performance under the four climate conditions. The resulting statistics include approximated ranges for storage availability, outflows, and water surface elevations (WSEs) within Lake Pillsbury, where applicable. These results are intended to help Lake County

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

compare the projected limitations and benefits of each scenario and inform the viability of Scott Dam and Lake Pillsbury alternatives for future study.

## A1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with approximately 2,275 acres of surface area and 31 miles of shoreline at the maximum WSE of 1,910 feet corresponding to the top of the spillway gates (Stillwater, 2021) (PG&E, 2023). The Lake's main flow contributor is the Eel River's upper watershed[3] and is located in Lake County, California. Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam is classified by the U.S. Army Corps of Engineer's Institute of Water Resources National Inventory of Dams database as a high hazard dam. Dams classified as high hazard potential means that dam failure could pose serious threat to downstream communities and infrastructure. The dam primarily provided water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley (in the Russian River watershed).

Scott Dam has also historically provided water to meet the minimum instream flow requirements in the Eel River and East Fork Russian River, and irrigation to the Potter Valley Irrigation District (PVID) (Stillwater, 2021) (PG&E, 2025). The Lake also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID is diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pillsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the Russian River. Scott Dam is currently considered a fish barrier as there are no mechanisms by which fish can safely pass to the upper Eel River watershed.

Scott Dam historically operated with an approximate maximum capacity of 76,876 acre-feet (ac-ft) at the WSE of 1,910 feet and a usable storage capacity was 66,876 ac-ft (Stillwater, 2021). In 2023, due to dam stability concerns, PG&E reduced the maximum WSE to the spillway invert elevation of 1,900 feet with a resulting storage capacity of 55,948 ac-ft. The Lake Pillsbury reservoir is operated to maintain a minimum storage of at least 10,000 ac-ft as the reservoir banks become unstable and the potential for material to slough off and block the dam outlet (needle valve) and introduce sediments downstream (PG&E, 2023).

The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs) according to PG&E (2023) however, the gage observations indicate ranges between 20 and 400 cfs. Water can be released from the dam through two structures: a needle valve[4] located at the base of the dam and the spillway[5]. The flow capacity of the needle valve is approximately 400 cfs but it can operate at varying flows for each WSE. The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[6]) controls.

[3] An area of land drained by a river, river system, or other body of water.

[4] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[5] A structure used to provide controlled release of water downstream of a dam or levee.

[6] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

The spillway consists of multiple sliding and radial gates. Historically, the spillway gates were typically fully opened between November 1 and April 1 and closed for the remainder of the year. Flow through the spillway will only occur when the WSE within Lake Pillsbury exceeds the spillway invert elevation of 1,900 feet. The top of all the spillway gates are at an elevation of 1,910 feet. Slide Gate #13 and one of the five radial gates are SCADA-controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained open.

As part of the license surrender a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E (as non-project use of project land) and the Eel-Russian Project Authority (ERPA). On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023).

## A1.2 Existing Lake Pillsbury and Scott Dam Usage

Scott Dam and Lake Pillsbury play a crucial role in the Potter Valley Project, which diverts water from the Eel River to the Russian River. This diversion supports various essential functions and provides significant benefits to the surrounding communities. Scott Dam and Lake Pillsbury are resources for the region supporting agriculture, providing drinking water, aiding in fire suppression, and offering environmental and recreational benefits. However, there are significant detriments, including environmental impacts and the financial burden of maintaining aging infrastructure. Balancing these benefits and detriments is crucial for ensuring the continued prosperity and safety of the surrounding areas. This analysis focuses on the hydrology and the effects on water supply, while other aspects described below were considered in this evaluation, detailed impact assessment are presented in the Existing Conditions Report prepared by SLR (SLR, 2025).

### A1.2.1 Water Supply

Scott Dam and Lake Pillsbury are integral to the water supply system in the region. According to Lake County (Huettl, 2025), Lake Pillsbury supports local agriculture worth approximately $743 million per year. This water is vital for irrigation, ensuring the productivity and sustainability of local farms. Additionally, the project supplies drinking water to around 600,000 people downstream in inland Mendocino, Sonoma, and Marin counties (Huettl, 2025). This reliable water source is essential for meeting the daily needs of residents and maintaining public health. Additionally, Lake Pillsbury provides groundwater recharge for the area and a consistent water supply for nearby wells. There are 38 recorded groundwater wells in the vicinity of Lake Pillsbury. Further evaluation on the extent of water supply from Lake Pillsbury and its effects on the groundwater and existing wells should be conducted (SLR, 2025).

Regulatory flow requirements are specified in the Federal Energy Regulatory Commission (FERC) license and the National Marine Fisheries Service's (NMFS's) Reasonable and Prudent Alternative (FERC, 2003). These minimum flows range from 20 to 60 cfs between June 1 and November 30 and range from 20 to 100 cfs between December 1 and May 31 depending on water year classification. Historically, PG&E has released substantially higher flows from Scott Dam than the minimum flows due to the required releases for downstream compliance points on the Eel and Russian Rivers and for water deliveries to PVID (PG&E, 2025).

### A1.2.2 Fire Suppression

Lake Pillsbury serves as a critical resource for aerial firefighting operations. The lake provides a reliable water source for helicopters and planes, which are essential for combating wildfires in

the region (Appendix B). The availability of water from Lake Pillsbury allows firefighting aircraft to quickly refill and return to the fire, enhancing the efficiency and effectiveness of fire suppression efforts. This capability is particularly important during the extended fire season, when rapid response and sustained operations are necessary to protect lives and property.

### A1.2.3 Ecosystem and Environmental Impact

Lake Pillsbury supports a diverse range of wildlife, including nesting bald eagles, tule elk, and migratory waterfowl. The lake's ecosystem has developed over nearly a century, creating a habitat for many species. While the lake and surrounding areas currently provide habitats for various wildlife species, this condition has evolved away from natural, pre dam conditions. The alteration of the Eel River's natural flow has disrupted local ecosystems and fish migration patterns. The dam has been a barrier to native fish species, such as salmon and steelhead, preventing them from accessing upstream habitats. Disruptions to the system can have significant ecological consequences, affecting the health and biodiversity of the river system on a temporary and long-term basis.

### A1.2.4 Recreational Impact

The presence of Scott Dam and Lake Pillsbury offers multiple recreational benefits. Lake Pillsbury is a popular destination for recreational activities such as fishing, boating, and camping. These activities attract visitors and support the local economy, providing income for businesses and enhancing the quality of life for residents. There are six Forest Service campgrounds with 153 campsites, which according to the Forest Service’s website "[..] are operated and maintained by Pacific Gas and Electric" (US Forest Service, 1999). There is also a private resort that operates a marina as well as private piers and docks that depend on the reservoir water levels to function throughout the recreational season.

### A1.2.5 Economic Impact

Scott Dam and Lake Pillsbury have a substantial economic impact on the region. Properties with a Lake Pillsbury address paid over $500,000 in taxes due to County of Lake 2024 (SLR, 2025). The availability of water is crucial for sustaining local economies, particularly in agriculture and tourism. The benefits provided by Scott Dam and Lake Pillsbury are essential for the economic well-being of the communities in Mendocino, Sonoma, and Lake counties.

Under the surrender conditions proposed by PG&E, there is no need to maintain Scott Dam and Lake Pillsbury as no hydropower generation is anticipated to occur following decommissioning of the Potter Valley Project. PG&E will be responsible for the decommissioning of the Scott Dam and removal of lake Pillsbury including restoration of the decommissioned areas as approved by FERC after the final surrender application is accepted by FERC. If Scott Dam remained in place for other purposes, the financial responsibility of future operation and maintenance of the dam and lake would be taken on by another party which is yet to be determined. The financial burden of maintaining the dam and associated infrastructure is expected to be high.

## A2.0 Existing Data

Various reports, analyses, and data have been made available for review as part of the Scott Dam Decommissioning Analysis completed by PG&E, PG&E consultants, and other interested or affected parties. Notable information includes hydrology and project operations modeling to project the unimpaired flows post dam removal utilizing a mass or water balance approach, analysis of water supply under various operating conditions and dam alternatives, a hydraulic

model to project the effects of flood waters on the Eel River downstream of the Scott Dam, fish pass ability assessments, and bathymetric data to approximate storage within Lake Pillsbury.

SLR reviewed the existing data and reproduced select data to support the proposed scenarios and climate change assessments. The existing data and assumptions were generally used to maintain continuity between assumptions and the reporting consistency. Raw data and models were not provided for this assessment and limits the highest accuracy of reproduction. Raw data that was provided via public sources were utilized when available and to validate any of the assumptions and analyses. Public records and resources were taken from the United States Geologic Survey (USGS), the National Oceanic and Atmospheric Administration (NOAA), National Integrated Drought Information System (NIDIS), National Centers for Environmental Information (NCEI), and the California Data Exchange Center (CDEC).

## A2.1 Existing Gage and Weather Station Data

PG&E has continuously monitored the storage and outflows from Scott Dam to meet the supply and demands of the system. Other federal and state agencies have recorded climate data such as temperature, precipitation, evaporation, and snowfall from different time periods within the Upper Eel River basin and retrieved from the NOAA NCEI dataset and the California Department of Water Resources CDEC which include the Lake Pillsbury (LPY), Scott Dam (SCD), Eel River below Lake Pillsbury (ELP), and Coyote Lake Mendocino (COY). The PG&E operated stations include USGS 11470500, and USGS 11470000.

A list of the active and inactive gaging stations are noted below:

1. LPY station data records reservoir elevation and reservoir storage reported on a daily basis covering the time period from March 2007 to the present. Data is available from the CDEC (CDWR, 2025c).
2. SCD station data records precipitation reported on a daily basis covering the time period from October 2011 to the present. Data is available from the CDEC (CDWR, 2025d).
3. ELP station data records mean flow reported on a daily basis covering the time period from March 2007 to the present. Data is available from the CDEC (CDWR, 2025b).
4. COY station operated by the United States Army Corps of Engineers (USACE) records regional pan evaporation data on a daily basis covering the time period from January 1994 to the present. Data is available from the CDEC (CDWR, 2025a).
5. USGS gage 11470500 located at Eel River below Lake Pillsbury records the daily average discharge from the Scott Dam in cfs and water temperature in degrees Celsius. The period of record is October 1, 1922, to September 30, 2024. Data is available from the USGS (USGS, 2025a).
6. USGS gage 11470000 located within Lake Pillsbury records the observed daily average storage in ac-ft. The period of record is October 1, 1928, to September 30, 2024. Data is available from the USGS (USGS, 2025b).
7. The NOAA recorded historical data at weather station LAKE PILLSBURY 2, CA US USC00044698, located northwest of the Soda Creek confluence with the Eel River, reports statistics on evaporation, precipitation, temperature, and snowfall/depth based on daily observations. The period of record is January 1, 1966, to December 31, 1968. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
8. The NOAA recorded historical data at weather station LAKE PILLSBURY, CA US USC00044156, located near Lake Pillsbury, reports daily statistics on evaporation,

precipitation, temperature, snowfall/depth, fog, and thunderstorms. The period of record is January 1, 1909, to December 31, 1937. Data can be accessed through NOAA's NCEI (NOAA, 2025a).

9. The NOAA recorded historical weather station HULLVILLE 7 SE, CA US USC00044158, located near Rice Creek above the Rice Fork confluence, reports statistics on evaporation, precipitation, temperature, and snowfall/depth based on daily observations. The period of record is from January 1, 1908, to December 31, 1909. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
10. The NOAA records data at weather station SODA CREEK CALIFORNIA, CA US USR0000CSDC, located along Soda Creek, reports statistics on temperature based on daily observations. The period of record is from January 1, 1994, to December 31, 2024. Data can be accessed through NOAA's NCEI (NOAA, 2025a).
11. The NOAA records data at weather station POTTER VALLEY 8.4 ENE, CA US US1CALK0019, located near Packsaddle Creek, reports statistics on precipitation, snowfall, and snow depth based on daily observations. The period of record is October 24, 2021, to April 21, 2025. Data can be accessed through NOAA's NCEI (NOAA, 2025a).

## A2.2 USGS StreamStats

The USGS StreamStats Application reports watershed statistics and characteristics based on a user-specified delineation point on a specified watercourse within the US (USGS, 2025c). The application treats the delineation point as the most downstream location in the study area and assumes that the watershed is undeveloped (no dam) and computes a regional regression analysis based on the USGS gage data available. StreamStats reports that the Eel River watershed immediately downstream of the Scott Dam has a drainage area of approximately 288.9 square miles. The watershed is assumed to have approximately 42.5% area covered by forest, 3.6% developed, and the remaining coverage consists of a combination of other land coverage such as wetlands, open water, and/or shrubland. The mean annual precipitation is reported to be 53.8 inches. The summary of the StreamStats Peak Flow Statistics Report for the Eel River at Scott Dam is summarized in Table A2-1. The flows are reported based upon their annual exceedance probability (AEP) which is a statistical measure representing the probability of a flood of a certain size occurring or exceeded in any given year. An example would be a 1% AEP flood has a 1 in 100 chance of occurring in any given year and has an average recurrence interval of 100 years and is why this flood often referred to as the 100-year flood.

**Table A2-1: StreamStats Generated Statistical Peak Flow Report for the Eel River Watershed at Scott Dam**

| AEP Flood [Recurrence Interval] (cubic feet per second or cfs) | | | | | | | |
|---|---|---|---|---|---|---|---|
| 50% [2-year] | 20% [5-year] | 10% [10-year] | 4% [25-year] | 2% [50-year] | 1% [100-year] | 0.5% [200-year] | 0.2% [500-year] |
| 15,300 | 26,800 | 34,700 | 44,900 | 52,500 | 60,100 | 67,300 | 76,900 |

The data has limited accuracy when analyzing a watershed that has been at least partially developed or major changes in ground cover. Development within the Eel River watershed above Scott Dam is very low and unlikely to contribute to a high degree of variation. However, the application will also not account for timber harvesting or fires that would impact extreme changes in watershed runoff conditions or how quickly runoff water will route to the watercourse.

It should also be noted that there are limited USGS gages within the area, and those gages are based on impaired flows from Scott Dam, which are used to predict the AEP. There are no other active USGS stream gages or precipitation gages within the watershed above Scott Dam that are used by the StreamStats calculation. StreamStats data should only be used as a reference for flood frequency and more data is required with other known precipitation gages to more accurately project the flood flows and frequency for the watershed above Scott Dam.

## A2.3 Flood Modeling

The Federal Emergency Management Agency (FEMA) has posted several Flood Insurance Studies (FIS) for the County of Lake, with the most recent effective revision completed October 10, 2024, and previous effective FIS reports published in September 2005 and March 1998. The 2024 FIS reports a summary of Pillsbury Lake stage-frequency relationships for the 10-, 50-, 100-, and 500-year floods which are 1,826.3; 1,829.8; 1,831.0; and 1,834.0 stage feet respectively (FEMA, 2024a). The stage-frequency has remained unchanged since the 1998 FIS (FEMA, 1998). The 2005 and 2024 FISs do not detail the hydrology or hydraulics for the Eel River. The 2005 FIS reports approximate studies for the Eel River, Soda Creek, and Smokehouse Creek with a detailed study of the Lake Pillsbury Perimeter (FEMA, 2005). The special flood hazard area (SFHA) inundation boundaries developed as part of the FIS for the Eel River are defined as a zone A, meaning the area inundated by the 1% annual chance floodplains were determined by approximate methods and no base flood elevations (BFEs) or depths are shown per FEMA Flood Insurance Rate Map (FIRM) panels 06033C0140D and 06033C0125D, per the FEMA National Flood Hazard Layer Viewer.

The preliminary July 2024 FEMA FIS for Mendocino County references a more detailed analysis of the Eel River from the Cape Horn Dam downstream approximately 1.8 miles that was completed in 1981. The 1981 study identified the recurrence intervals for the 10%, 2%, 1%, and 0.2% annual chance floods, predicted to be 41,000; 70,000; 82,500; and 112,000 cfs respectively. The recurrence intervals were determined using a USGS Bulletin 17B analysis (FEMA 2024b).

A One-dimensional hydraulic USACE Hydrologic Engineering Center's River Analysis System (HEC-RAS) model was developed and reported by Stillwater Sciences in 2021. Model development included a 49-mile reach of the Eel River from Scott Dam downstream to the Middle Fork Eel River and required gathering and analyzing various data to create the model inputs, boundary conditions, and river geometry. The Stillwater memorandum included a summary of the previous hydrologic assumptions and their own Bulletin 17C analysis for the 1% AEP floods which are summarized in Table A2-2 along with the most recent StreamStats projections (Section A2.2).

**Table A2-2: Summary of Previous 1% AEP Projected Flows by Source and Method**

| Year of Analysis | Source | 1% AEP (100-year Recurrence) (cfs) | Analysis Method |
|---|---|---|---|
| 2012 | USGS StreamStats | 60,200 | Regional Regression |
| 2018 | McMillen Jacobs Associates | 59,200 | Bulletin 17B |
| 2021 | Stillwater Science | 56,417 | Bulletin 17C |
| 2025[1] | USGS StreamStats | 60,100 | Regional Regression |

| Year of Analysis | Source | 1% AEP (100-year Recurrence) (cfs) | Analysis Method |
|---|---|---|---|
| Notes:<br>1. USGS StreamStats 1% AEP reported for 2025 used the same method of analysis for 2012 with a longer period of record to project the peak flood flow | | | |

The previous studies have used either a regional regression or Bulletin 17 methodology for estimating the flood return periods. The regional regression analysis is typically used at sites with few or no stream gages by relating streamflow characteristics to basin and climate variables. The regional regression analysis method recommends excluding or adjusting streamflow data that is heavily regulated with varying operations. The Bulletin 17C superseded 17B in 2018 and is a standard method for determining flood frequency at specific sites using long term streamflow data (USACE, 2025). A Bulletin 17C analysis would ideally use a homogeneous record, where the statistical parameters should not change over time, however regulated flows can be used when there are adjustments made to the impaired flows based on a high confidence in the operations. Alternatively hydrologic models can be used to approximate natural conditions runoff and peak flows, which at the time of this report development were not completed. The operations of the Scott Dam are also not replicated or understood with a high degree of confidence. The previous 1% AEP projections also all utilize the current impaired or controlled streamflow gages which are likely to differ from the actual peak flood flows.

Stillwater Sciences (2021b) developed one-dimensional existing conditions and future conditions hydraulic models for the Eel River downstream of the Scott Dam. The models assumed an in-line weir for the Cape Horn dam where the dam dimensions were to simulate the spillway based on data provided by PG&E and did not include detailed dam operations. The models were used to assess the existing conditions and future flood conditions post Scott Dam removal using the impaired hydrologic FFA results summarized in Table A2-3. The existing conditions model was calibrated using the impaired gaged WSEs. The existing condition 100-year WSEs were within 0.02 to 1.5 feet when compared to the recorded WSEs. When modeling the FEMA flood flows under the existing conditions, the inundation limits were relatively in agreement, with an exception near Todd Road. The existing and future conditions water surface elevation profile is shown in Figure A2-1.

**Table A2-3: Sillwater (2021b) Flood Frequency Analysis (FFA) Flow Report for the Eel River Watershed below Scott Dam**

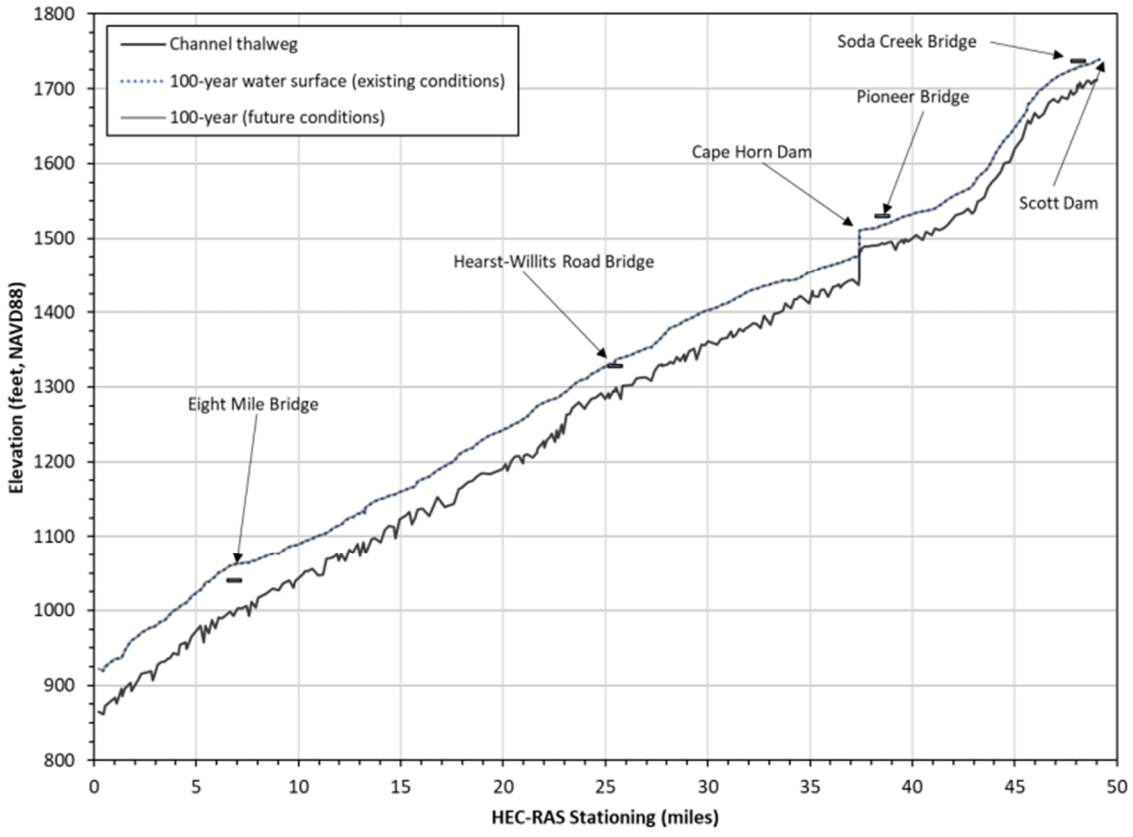
| AEP Flood [Recurrence Interval] (cubic feet per second or cfs) | | | | |
|---|---|---|---|---|
| 50% [2-year] | 10% [10-year] | 4% [25-year] | 2% [50-year] | 1% [100-year] |
| 11,205 | 34,034 | 42,026 | 50,883 | 60,100 |

The future conditions Stillwater hydraulic model (2021b) assumes Cape Horn Dam remains in place and Scott Dam is removed. The model used unimpaired hydrology calculated via StreamStats (Table A2-2, 2012 results) and then uniformly increases the impaired discharges at all downstream flow locations by the difference between impaired and unimpaired flows (roughly 4,000 cfs → 60,200 – 56,417 = 3,783 cfs). The proposed channel geometry was assumed to remain unchanged from existing conditions based on the sediment transport model, projecting the 100-year flows would flush out all sediment released from the removal of Scott Dam (Stillwater, 2021a). The results of the future conditions model projected an increase in water surface elevations for the 100-year event to show a maximum increase in flood waters by up to 1.2-feet (Figure A2-1).

Stillwater concludes that risk is not expected to substantially increase following Scott Dam removal during a 100-year flood event and that additional data is expected to refine these conclusions. Stillwater's results assume that the 100-year event will mainly be categorized by the hydrology and not by coarse sediment bed transport based on a previous Stillwater sediment transport analysis (2021a). However, these conclusions rely on assumptions that lack analytical support. The 2021 Stillwater sediment transport analysis specifically states that "No model was developed to simulate gravel transport dynamics because there is inadequate data to do so." The assumption that the Eel River's longitudinal profile will remain unchanged is speculative and does not account for potential scouring of bridge abutments or infrastructure impacts from coarse sediment and higher flows. The projected 1.2-foot WSE rise, combined with newly accessible sediment and flow magnitudes not seen in over a century, could pose significant risks to downstream infrastructure.

Figure A2-1 shows two crossings that are projected to be overtopped during the 100-year event. Increased river flows and debris movement would likely exacerbate these vulnerabilities. These uncertainties suggest that flood risk may be underestimated and warrant further analysis. Additionally, the Stillwater (2021b) report provides limited visualizations of the increased flood profile and inundation area near Scott Dam. The HEC-RAS model used to generate these results was not made available for review, and further data could not be extrapolated to assess flood risk from the Scott Dam removal. Without access to the model or more detailed outputs, it is difficult to validate the assumptions or evaluate the full extent of potential infrastructure impacts.

**Figure A2-1: Stillwater Sciences (2021a) Existing and Future Conditions 100-year Water Surface Elevation Profiles [HEC-RAS stationing of Eel River from Scott Dam (station 49) downstream to Middle Fork Eel River confluence (station 0)]**



# A3.0 Existing Climate and Hydrology

The climate of the Upper Main Eel watershed is relatively moderate, with occasional extreme temperatures during both the winter and summer. According to the NOAA (2025a), the summer temperatures often exceed 100°F, while winter temperatures usually fall below freezing. Summer temperatures typically range from 70°F to 90°F, while winter average temperatures range from 40°F to 60°F. However, these temperatures vary with elevation and have become more extreme over the past century.

The Soda Creek weather station has been reporting on temperature data since 1992, with observed extremes ranging from -32°F to 114°F (NCEI, 2025). Due to the lack of a consistent long-term dataset covering both precipitation and temperature throughout the lifetime of Scott Dam, only limited inferences can be made. Comparisons rely on overlapping timeframes between station activity and dam operations.

A continuous-record precipitation station operated by the U.S. Weather Bureau (now NOAA) near Lake Pillsbury from 1907 to 1936 recorded mean annual precipitation of 47.62 inches, with monthly totals ranging from 0 to 18.10 inches. During this period, recorded temperatures ranged from 15°F to 107°F. (NOAA, 2025a).

Rainfall in the watershed is relatively high, averaging 50-60 inches per year along the eastern boundary and 70-80 inches per year around Bald Mountain, north of the Salmon and Smokehouse Creek watershed (NOAA, 2025a). In the valleys, rainfall averages 40-50 inches per year, with the lowest amounts in the vicinity of Lake Pillsbury.

Snow occasionally falls at the 2,500-foot level but seldom builds any significant snowpack below 5,000 feet. Higher elevations can receive substantial snowfall, often accumulating several feet during the winter months, often accumulating several feet during winter months. Average snow depths range from 26 to 39 inches (NOAA, 2025b), and spring snowmelt from these accumulations significantly contributes to the Eel River watershed (US Forest Service, 1999).

While the historical and contemporary data presented here offer insights into the climate of the Upper Main Eel watershed, they are limited in their ability to fully reflect long-term climate change trends. The lack of a continuous, multi-variable dataset spanning the full operational history of Scott Dam makes it difficult to draw definitive conclusions about how climate change has influenced local temperature, precipitation, and snowpack.

Observed extremes at the Soda Creek station and historical records from Lake Pillsbury suggest increasing variability, but without consistent overlap or standardized measurements, these datasets cannot be used to quantify climate-driven shifts with high confidence. As such, while the data hint at intensifying seasonal extremes and reduced snowpack at lower elevations—patterns consistent with broader regional climate trends—any assumptions about climate change impacts in this specific watershed must be made cautiously and ideally supplemented with downscaled climate modeling or regional projections such as Cal-Adapt or Integrated Climate Adaptation and Resiliency Program (ICARP) tools; see further details in Appendix D.

## A3.1 Stream Types

Lake Pillsbury has several water courses contributing to water storage availability. These watercourses were historically tributaries that routed directly into the Eel River prior to the

construction of Scott Dam. A majority of the tributaries to Lake Pillsbury are ephemeral[7] or intermittent[8] streams with Eel River, Habematolel Creek (also known as Squaw Valley Creek), and Rice Fork identified as entirely perennial[9] by the USGS's 3D Hydrography Program (USGS, 2024). Other notable named watercourses that have large perennial reaches and short stretches that are intermittent, specifically as they reach Lake Pillsbury, are Smokehouse Creek, Salt Spring Creek, and Salmon Creek.

## A3.2 Climate Conditions

Each scenario was analyzed under four climate conditions: normal wet season, normal dry season, flood, and drought. The full period of storage and outflow gages, along with any precipitation, flood, and drought data was used to project the climate conditions. The precipitation data available is not continuous over the period of analysis (1900- present), and no flow gage data was made or known to be available pre-dam (pre-1922). Assumptions were based on the observed trends over the period of record for which data was available and primarily when the dam was in place.

### A3.2.1 Wet and Dry Season

The approximate wet and dry seasons were predicted for the region using the available data from the NOAA NCEI (Menne, et al, 2012). The annual total precipitation is approximately 44.90 inches and 40.4 inches respectively for the Lake Pillsbury and SCD station, respectively, which is lower than the reported average via StreamStats (Section A2.2). The daily precipitation data from the LAKE PILLSBURY, CA US USC00044156 weather station was used to determine the average monthly precipitation over with a period of record from 1909-1937 as summarized in Figure A3-1. The incremental daily precipitation from the SCD station was used to determine monthly average precipitation from 2011-2025 as shown in Figure A3-2. The average precipitation over the period of record was determined to be approximately 0.12 inches for both stations. Months where the daily precipitation was greater than average precipitation are assumed to be a part of the wet season and include November through March. Months where the daily precipitation was less than the average precipitation includes April through October and are assumed to be a part of the dry season. Figure A3-1 and Figure A3-2 show the average precipitation threshold (average over the period of record), monthly average precipitation, and the assumed wet and dry seasons based on the precipitation records. The Lake Pillsbury and SCD stations are consistent in the assumed wet and dry season.

The approximate wet and dry seasons were also predicted from the Eel River outflow gage below Scott Dam. The average outflow for the period of record (1922-2024) based on daily average discharge[10] reported at USGS gage 11470500 from the Scott Dam was determined to be approximately 553 cfs. The gaged data was used to determine monthly averages over the period of record and compared to the period of record average as shown in Figure A3-3. Months with average daily outflows greater were than the average outflow include December through

[7] This stream flows only briefly after rainfall events. It is usually dry most of the year and only carries water for a short duration following precipitation. Ephemeral streams do not have a continuous source of water and are highly dependent on immediate weather conditions.

[8] This type of stream flows only during certain times of the year, typically when there is enough rainfall or snowmelt to sustain flow. It may have water for weeks or months at a time, but it dries up during periods of low precipitation.

[9] A perennial stream is one that flows continuously throughout the year, having a consistent and reliable source of water, such as groundwater, springs, or sustained rainfall.

[10] Outflow and discharge used interchangeably; the volume of water flowing through a cross-section at a point in time.

April. Months with average daily outflow less than the annual average outflow include May through November (Figure A3-3). Dam operators likely increase the outflow in December, following the start of the wet season in November, and decrease flows in April, following the end of the wet season in March.

Storage[11] within Lake Pillsbury follows a more opposing trend throughout the year, being at its highest during from March through July and the lowest between August through January as observed in Figure A3-5. This is presumably due to PG&E keeping the storage capacity available for irrigation, water supply to Lake Mendocino, increased recreation, and greater fire suppression during the dry season. The wet and dry season periods were generalized, and it should be noted that there is some variability from year to year when the wet and dry season starts and ends.

Using the PG&E controlled outflows and storage provided an initial trend in the wet and dry season assumptions for the watershed. However, as the storage and outflows are controlled, they will dampen the precipitation events, introducing inaccuracy when projecting weather patterns. When comparing the precipitation data to the outflow and storage trends, it was observed that as the outflow and storage are operationally controlled, the flows are not predictive of the seasons themselves. However, to remain consistent with previous studies, the scenario analyses reference the wet and dry season based on the outflow seasonal trends (wet: December through April and dry: May through November), as opposed to precipitation (wet: November through March and dry: April through October). The previous studies and hydrological studies are based on the outflows and follows a similar seasonal trend. This trend is shown in the unimpaired monthly average flow data estimates developed as part of the Results of Initial Water Supply Modeling for PVP and Russian River Alternatives which follow the outflow trends for the seasonality and not the precipitation seasonal trends (Addley et al, 2019) recreated in Figure A3-4. Analytically, unimpaired flows should more closely follow the precipitation trends for seasonal variations.

[11] The volume of water filling a basin (Lake Pillsbury), measured in acre-feet.

**Figure A3-1: Average Monthly Precipitation Calculated from the Lake Pillsbury Weather Station over the Period of Record from 1909-1937**

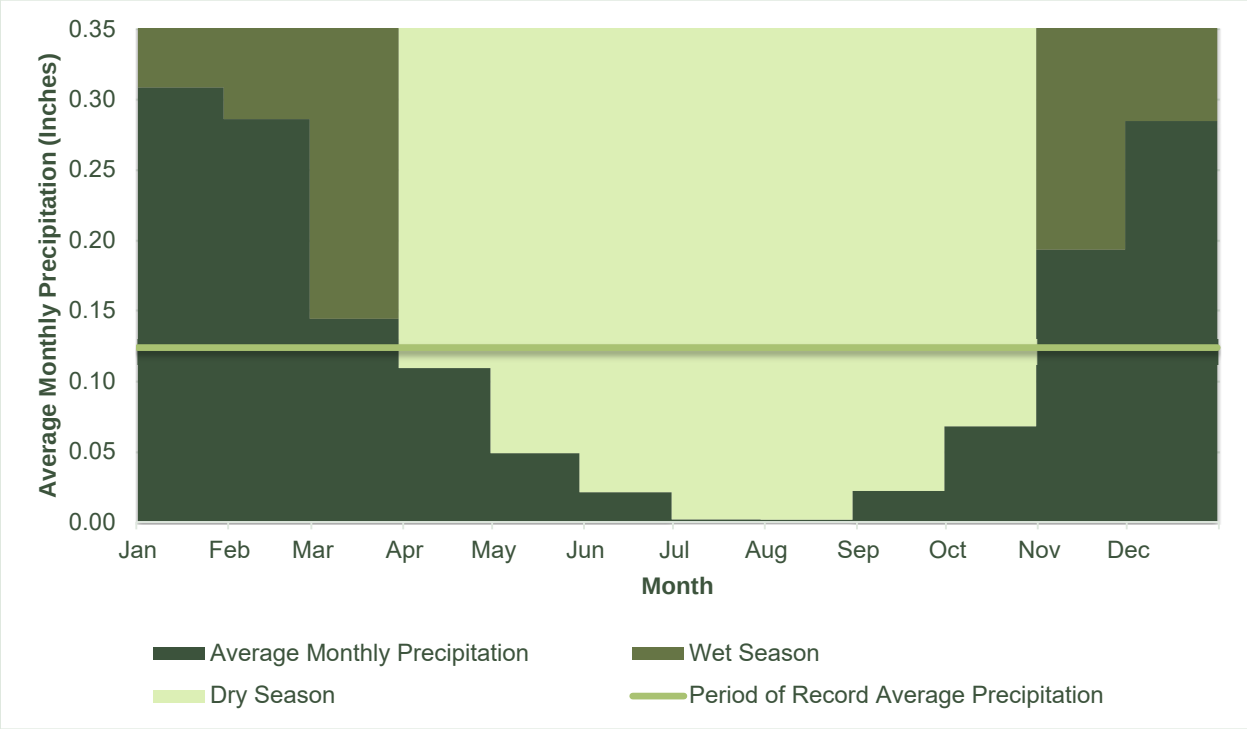



**Figure A3-2: Average Monthly Precipitation Calculated from the SCD Station over the Period of Record from 2011-2025**

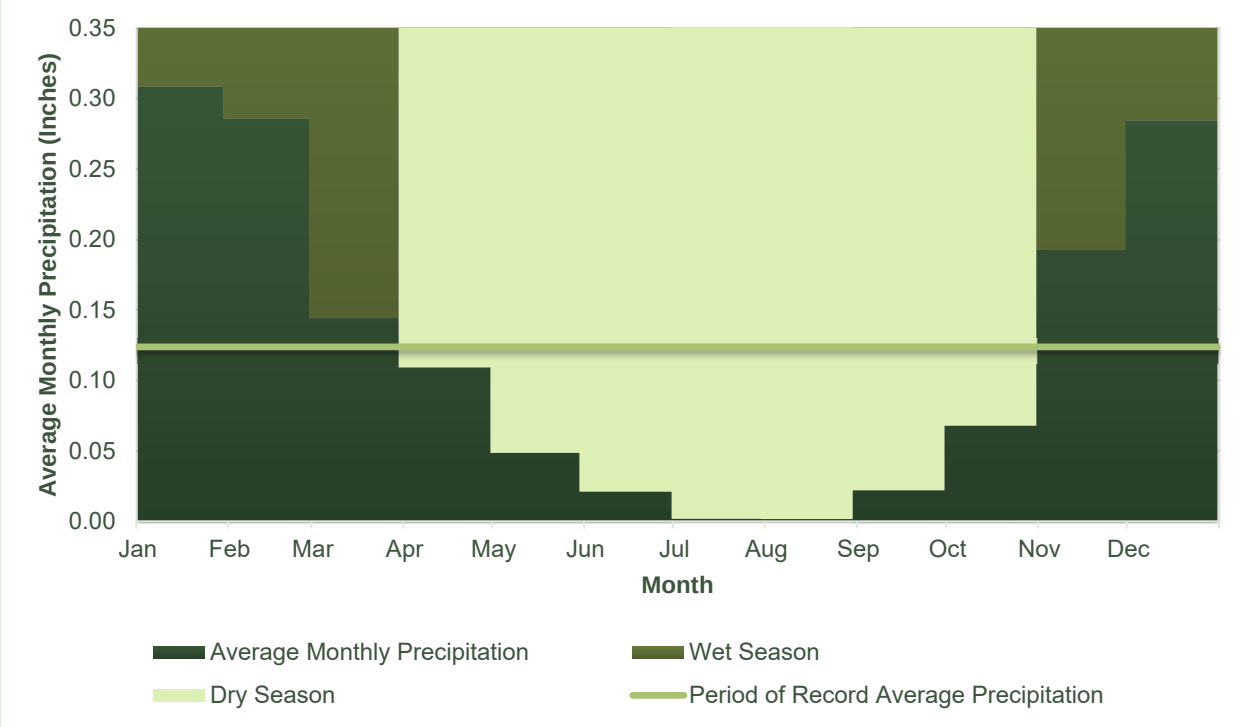


**Figure A3-3: Average Monthly Gage Flows Calculated from the Eel River below Scott Dam Flow Gage over the Period of Record from 1911-Present**

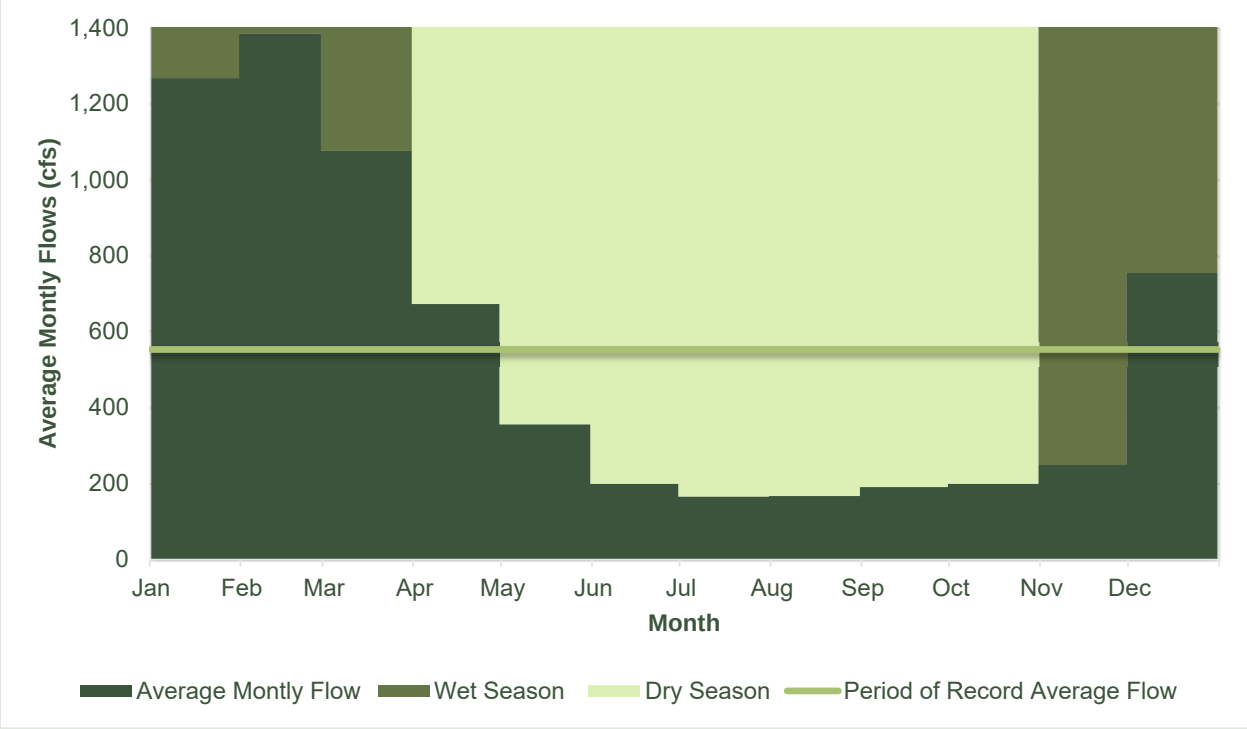



**Figure A3-4: Unimpaired Monthly Average Flows Downstream of Scott Dam (Cardno 2019 & Addley et al. 2019)**

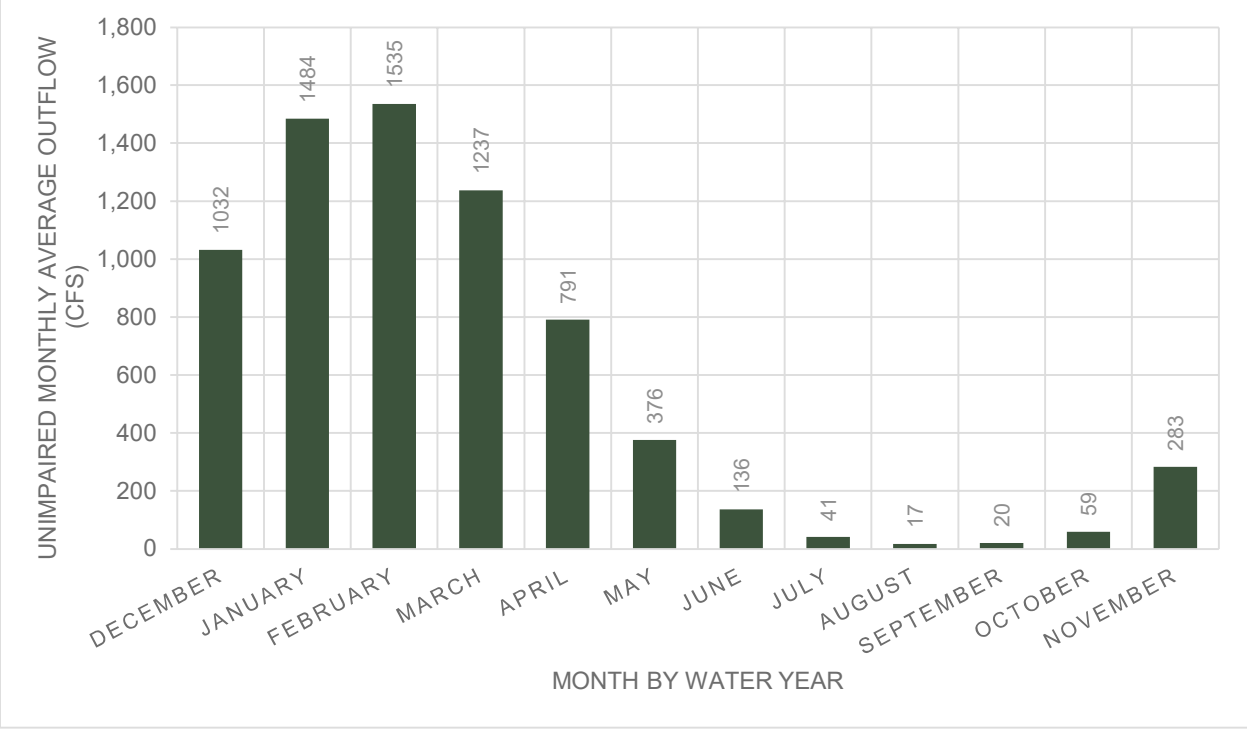
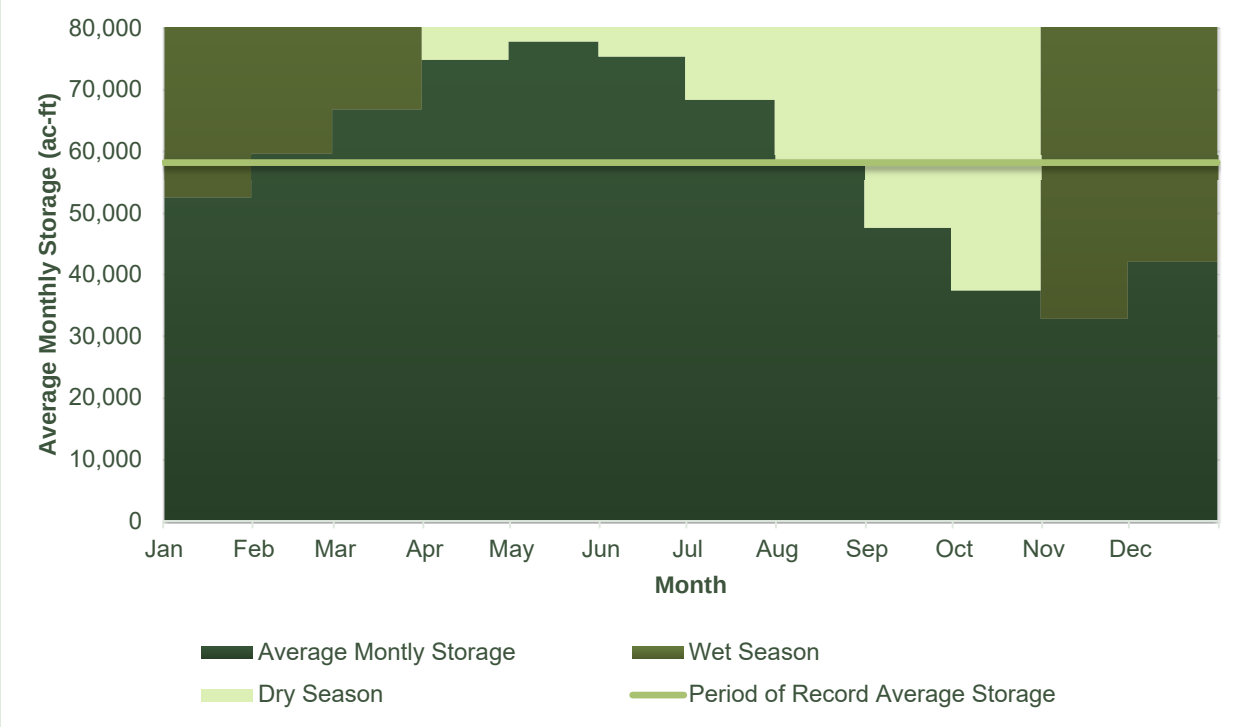


**Figure A3-5: Average Monthly Gage Storage Calculated from the Lake Pillsbury Storage Gage over the Period of Record from 1928-Present**




## A3.2.2 Flood and Drought of Record

The flood and drought of record were determined by using the NIDIS wet and dry conditions monitoring data for California State and County of Lake. NIDIS calculates flood and drought data using a combination of historical climate records, drought indices, hydrologic models, and advanced modeling and mapping techniques. Two key tools used in this analysis are the U.S. Drought Monitor (USDM) and the Standardized Precipitation Index (SPI), which evaluate how much a region's observed precipitation deviates from the climatological average and classify it within a standardized range. The SPI, in particular, quantifies precipitation anomalies by expressing them in terms of standard deviations from the historical mean—negative values indicate below-average precipitation (drought), while positive values suggest above-average precipitation (potential flood conditions). Figure A3-6 shows Lake County's SPI from 1895-Present (NIDIS, 2025) with classifications summarized in Table A3-1. This data is useful for understanding past climate events and trends within Lake County.

**Figure A3-6: Standard Precipitation Index from 1895-Present for Lake County, CA as of April 2025 from Drought.gov**

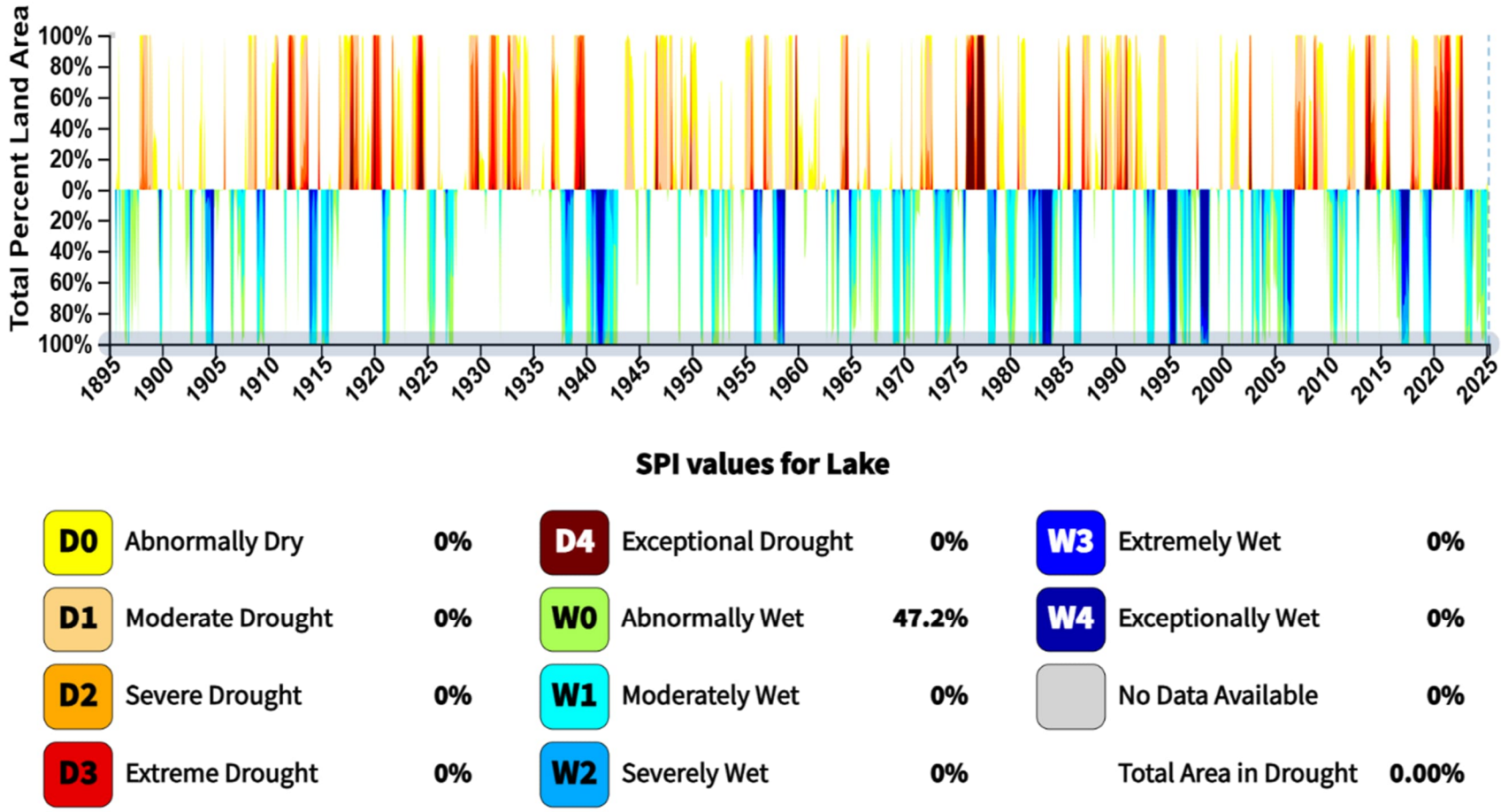


**Table A3-1: NIDIS Wet and Dry Conditions Classifications**

| Conditions | Classification | SPI |
|---|---|---|
| Normal | N/A | -0.4 to +0.4 |
| Abnormally Dry | D0 | -0.7 to -0.5 |
| Moderate Drought | D1 | -1.2 to -0.8 |
| Severe Drought | D2 | -1.5 to -1.3 |
| Extreme Drought | D3 | -1.9 to -1.6 |
| Exceptional Drought | D4 | -2.0 or less |
| Abnormally Wet | W0 | 0.5 to 0.7 |
| Moderate Wet | W1 | 0.8 to 1.2 |
| Severe Wet | W2 | 1.3 to 1.5 |
| Extreme Wet | W3 | 1.6 to 1.9 |
| Exceptional Wet | W4 | 2.0 or greater |

According to historic news archives (Metzler, 1983), the flood of record in Lake County took place during WY 1982 with another large flood event happening in WY 1983. These 1982 and 1983 events are remembered as a substantial flood caused by exceptionally heavy rainfall with flooding that destroyed infrastructure and agricultural land resulting in the loss of 19 lives. The NIDIS SPI classification identifies this period as an exceptionally wet period. A focused SPI can be found in Appendix A.A, Figure A.A9-9 along with Figure A.A 9-2 showing the daily average outflows and storage during the 1982-1983 flood period. The peak outflow from Lake Pillsbury during the flood of record was observed at 15,400 cfs observed on January 27, 1983. The peak flow did not reach the projected StreamStats 50% (2-year) flood level. This is likely due to Scott Dam attenuating the flood flows through Lake Pillsbury. Other prominent floods took place in 1937 (Figure A.A9-8, Appendix A.A) and 1995 in Lake County (Figure A.A 9-3 and Figure A.A9-8, Appendix A.A).

There are several notable droughts within Lake County according to NIDIS over the past century. The driest period in California history took place between 2012-2016, following California state-wide trends (Figure A.A9-10, Appendix A.A). Before the 2012-2016 drought, the most severe drought in Lake County and in California took place from 1976-77 (Figure A.A9-1 and Figure A.A9-11, Appendix A.A), which is attributed to very low precipitation over a long duration. Other notable droughts took place from 1928-1934 (Figure A.A 9-6, Figure A.A 9-7, and Figure A.A9-12; Appendix A.A) which was significant due to its duration, and 2020-22 (Figure A.A 9-4, Figure A.A 9-5, and Figure A.A9-10; Appendix A.A), which was less severe than the 2012-16 drought but alarming for California to have experienced two droughts in quick succession. Several factors made the 2012-2016 drought the most severe, including the duration, temperature extremes exacerbated by climate change, population growth, runoff, and ecological stress. (USGS, 2017). Drought trends over the past two decades have been observed in the NIDIS to show an increase in the frequency, intensity, and duration of drought with lower overall precipitation and snow accumulations.

### A3.2.3 Maximum and Minimum Outflows

Since 1922, the recorded maximum daily average outflow from the PG&E gages at Scott Dam took place on December 22, 1964, reaching 45,300 cfs. The period between December 1964

and January 1965 correlates to between a 4-10% annual chance of exceedance (approximate 10-25 year recurrence interval) flood according to StreamStats as indicated in Figure A3-7. However, the NIDIS historical wet and dry trends do not show any significant precipitation during the 1964-65 wet season (Figure A3-6). The minimum average daily outflow occurred on September 8, 1924, reaching 0.1 cfs as observed in Figure A3-8. The minimum average daily outflow coincidentally occurred during the drought from 1928-34.

**Figure A3-7: Daily average outflow from Lake Pillsbury and inflow from contributing streams, and daily average storage from December 1964 - January 1965 (period of record outflow)**

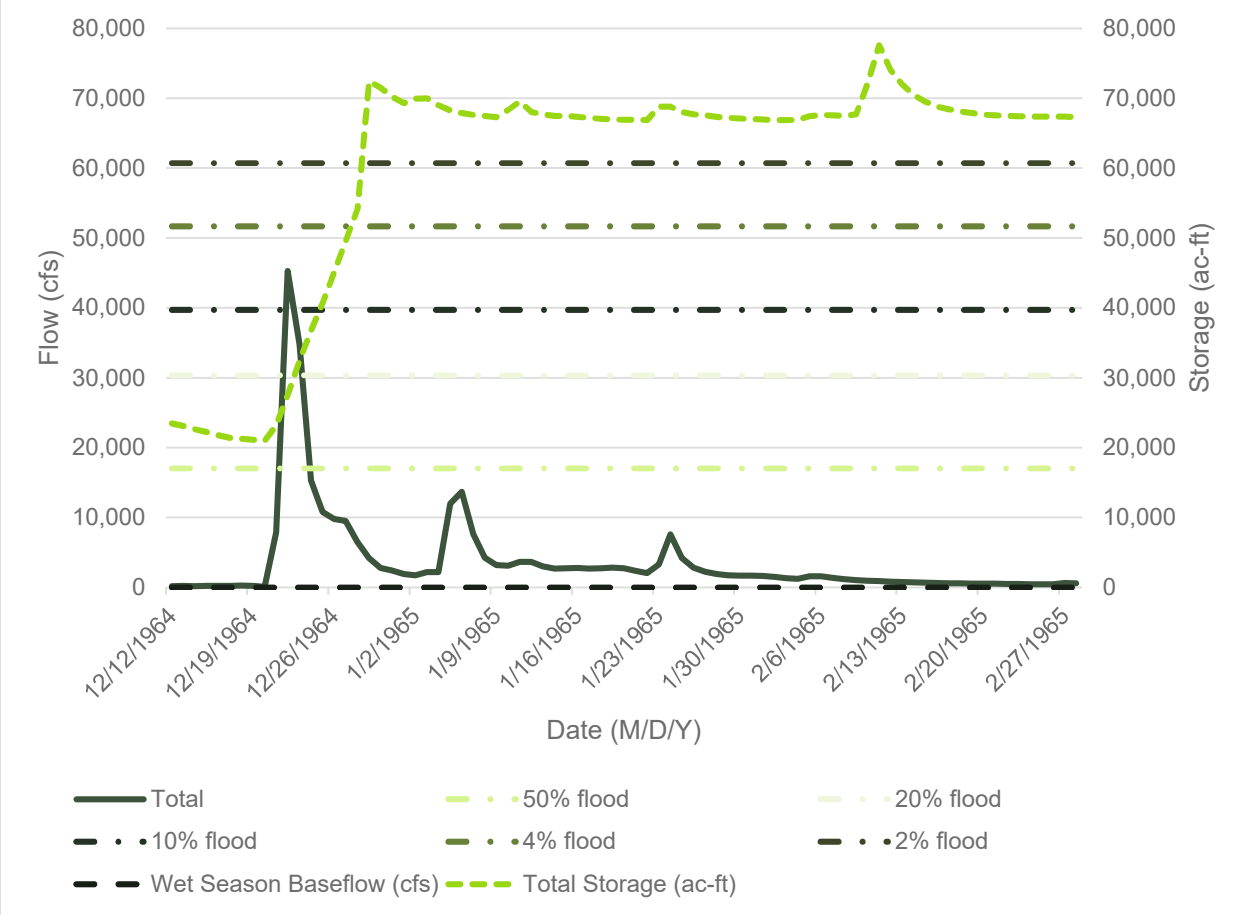
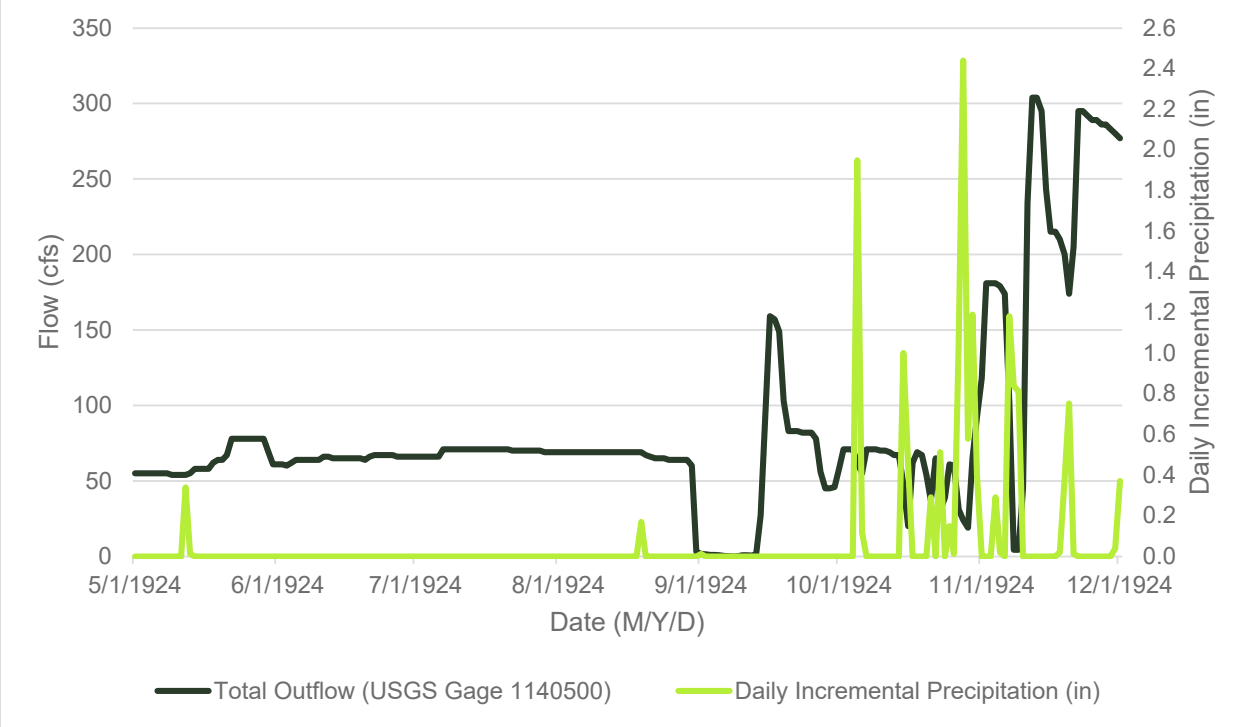


**Figure A3-8: Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 1924 dry season (period of record low outflow)**




SLR was not provided with any operational or maintenance data that has occurred since the inception of Scott Dam. There are notable low storage periods where maintenance activities would have required a conscious operational lowering of the dam WSE. Lower flows could have resulted from maintenance actions, changes in operation such as a drought year when limited to no outflows were required to maintain storage levels, or low inflow water years. The high flows are likely from precipitation events or high volumes of snowmelt.

# A4.0 Hydrologic and Hydraulic Data Analyses

The existing data (A2.0 & A3.0) was used in support of the hydrologic and hydraulic analyses completed for the Scott Dam scenario evaluation. Limited data was provided from previous studies and was recreated with limited precision. The hydrologic data included precipitation data, flood flows, drought conditions, flow contributions by watercourse and water balance recreation. The hydraulic data included stage-storage and rating curves for dam operations.

## A4.1 PG&E Water/Mass Balance Data

PG&E provided water data pertaining to WYs 2005-2024 which included Lake Pillsbury's storage volumes, WSEs measured from the PG&E specified datum, outflow from Lake Pillsbury (gage E-2), outflow from Eel River below Cape Horn Dam (gage E-11), and outflow from diversion to Russian River Basin (gage E-16) (Appendix A.B). PG&E also provided calculated net inflow into Lake Pillsbury values which were based on a mass balance approach per the hydrology and operations study memorandum (Cardno, 2019). The provided Lake Pillsbury net inflow values removed evaporation outflows from the lake water surface. Negative values for net inflow into Lake Pillsbury were reported which were possibly due to discrepancies in individual data points, inconsistencies in the time frames when measurements were recorded, changes to

the operation of the dam, and the nature of utilizing a mass balance approach. The data provided did not include all the required components to precisely reproduce the PG&E water/mass balance values. However, SLR was able to produce a water balance model utilizing available data for Scott Dam and Lake Pillsbury for the purposes of analyses of the potential mitigation scenarios. The water balance model produced by SLR is further discussed in Section A4.5.

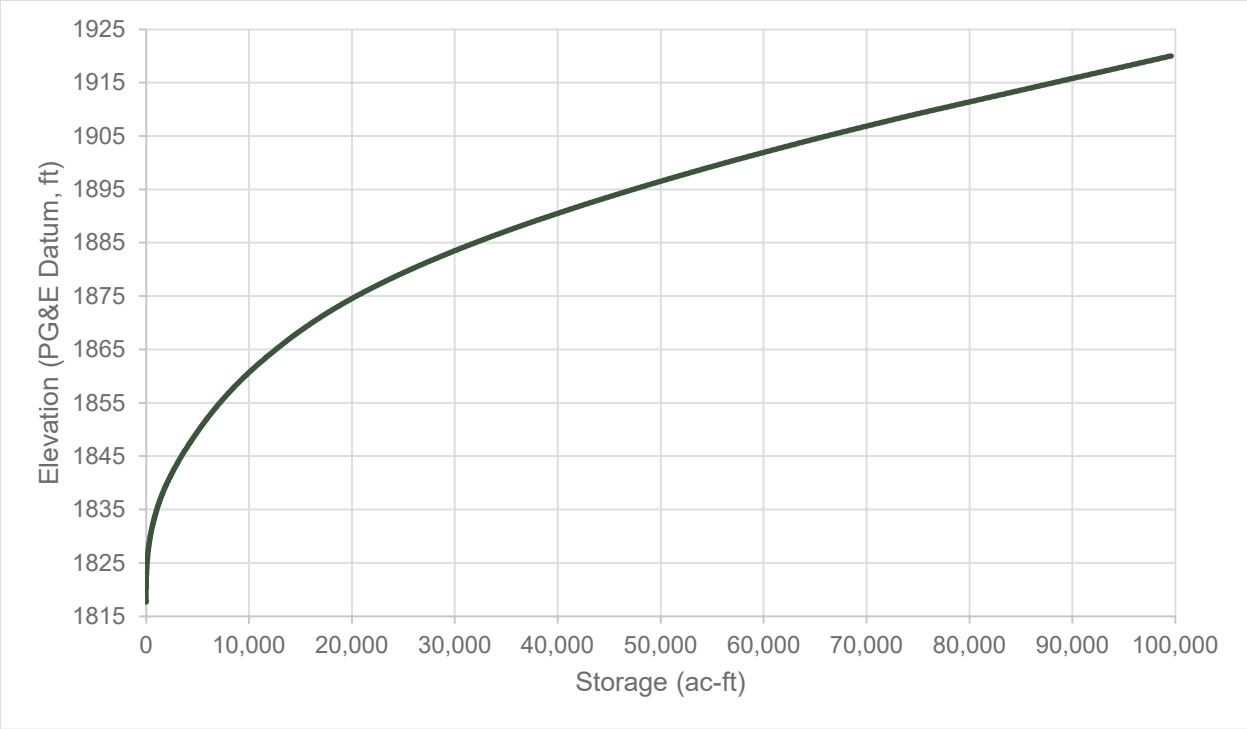
## A4.2 Stage-Storage Curve

Multiple bathymetry[12] surveys have historically been conducted for Lake Pillsbury in 1921, 1959, 1985, 2005, 2015-2016, and 2023 to estimate the water storage capacity impounded by Scott Dam. The original 1921 storage capacity for Lake Pillsbury was 94,400 ac-ft at WSE 1,910 feet using the PG&E datum (Porterfield, 1964). PG&E's bathymetry survey representing the 2015-2016 conditions, estimated the storage capacity for Lake Pillsbury to be 76,876 ac-ft at WSE 1,910 feet (PG&E, 2016). The change in storage capacity is due to sediment accumulation over time.

PG&E's bathymetry survey representing 2015-2016 conditions was used by SLR to generate a stage-storage curve that relates Lake Pillsbury's WSE to its water storage capacity as depicted in Figure A4-1 (PG&E, 2016). The 2015-2016 bathymetry survey was selected due to the availability of lake storage and water surface area values at each WSE on a 0.1 ft interval basis. SLR used this curve to evaluate water storage during each of the relevant scenarios and climate conditions. Since the bathymetry data range covered WSE up to 1,910 feet, SLR extrapolated values from 1,910 feet to 1,920 feet based on a best fit curve to evaluate the storage capacity. This stage-storage curve was used to estimate Lake Pillsbury's storage under each climate and mitigation scenario.

[12] The underwater equivalent to topography, the study of underwater depth in lakes, oceans and rivers.

**Figure A4-1: Recreated Stage-Storage Curve from PG&E 2016 Bathymetry Report**




## A4.3 Rating Curves

Scott Dam has two outflow structures: the needle valve and the spillway. Individual rating curves for the needle valve and the spillway were produced by Cardno (2019) and are presented in Figure A4-2 and Figure A4-3, respectively. Outflows from Lake Pillsbury through the needle valve are a function of the Lake Pillsbury WSE and the needle valve opening. The needle valve outflows can fluctuate over a wide range at each WSE due to the variability of the need valve opening size and flexibility in operations. Figure A4-2 also presents historical combinations of the needle valve opening and Lake Pillsbury WSEs with the needle valve rating curve. A plot of the WSE versus the needle valve outflow range representing minimum and maximum needle valve opening size extents is provided in Figure A4-4.

Figure A4-3 summarizes the variability in the outflows through the Scott Dam spillway. The spillway has five radial gates and 26 steel slide gates, with all but Gate 13 being manually operated. There are difficulties in defining an accurate rating curve as there is a lot of dependence on manual operations and no data to support which gates were operable or open during the period of record used to develop the rating curve. There are also no records of when the needle valve was installed as it is presumed the needle valve was not included in the original dam construction. The figure results show a combination of multiple analyses relating the Lake Pillsbury WSE to spillway outflow based on modeling, previous rating curve developments, and empirical analysis. The revised rating curve as reported by Cardno (2019) was selected as the most sensible for use in the analyses of the potential Scott Dam scenarios and is shown in Figure A4-5.

**Figure A4-2: Needle Valve Rating Curve with Historic Needle Valve Opening Values (Figure AQ 1-17, Cardno, 2019)**

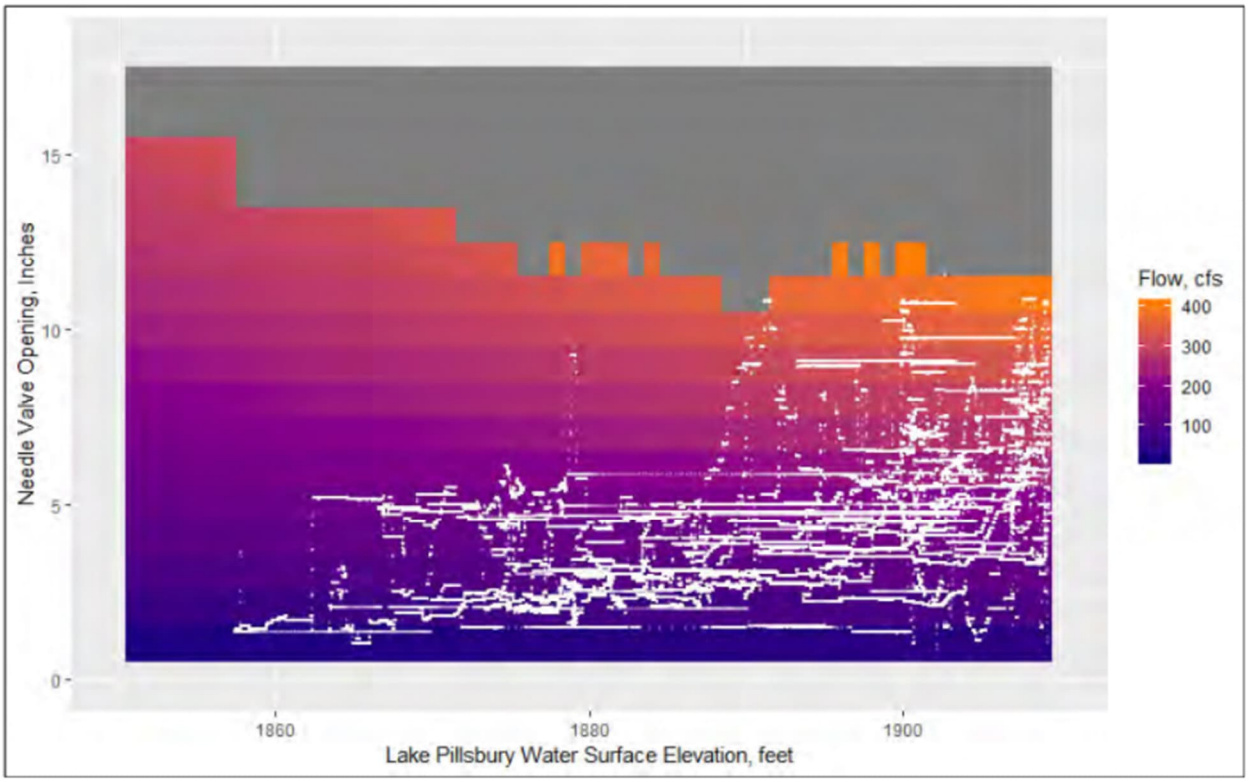


**Figure A4-3: Scott Dam Spillway Rating Curve (Figure AQ 1-18, Cardno, 2019)**

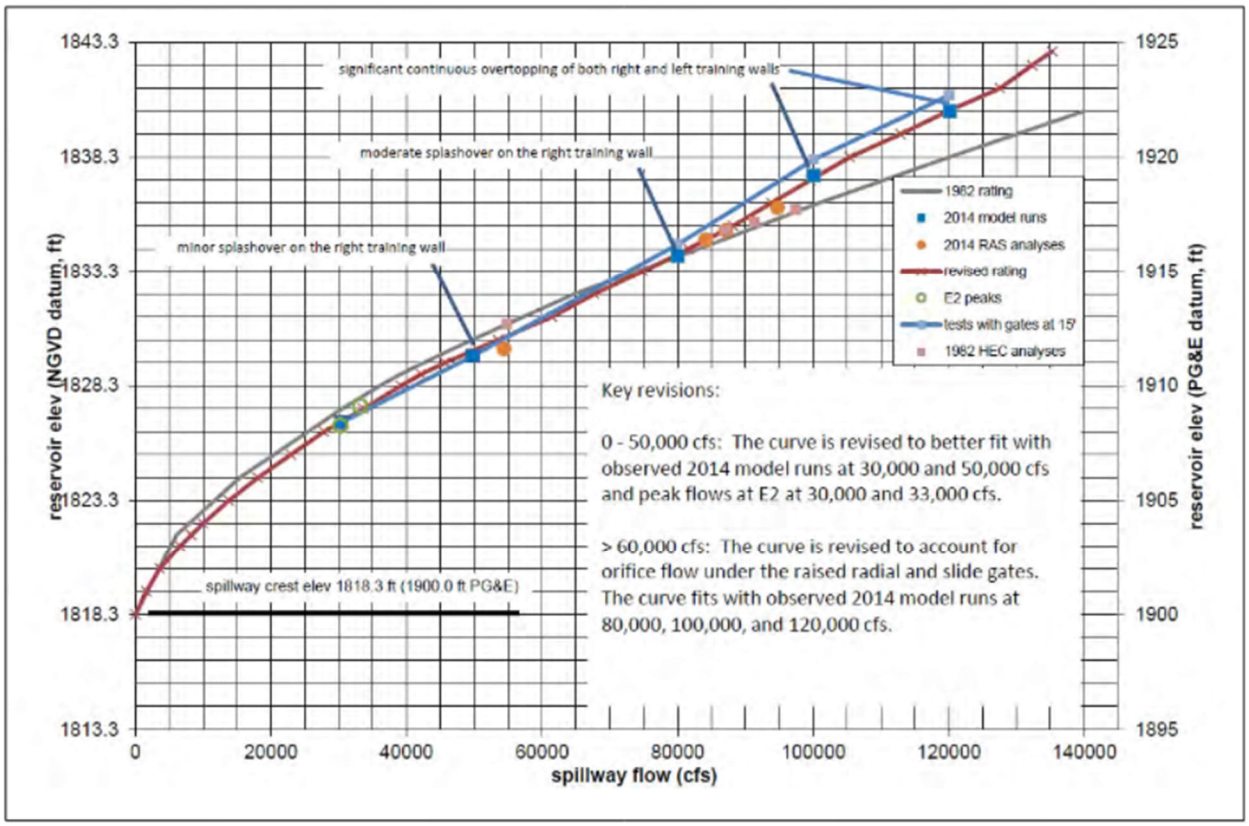


**Figure A4-4: Needle Valve Opening Outflow Range Estimates of the Minimum and Maximum (based on Figure AQ 1-17, Cardno, 2019)**

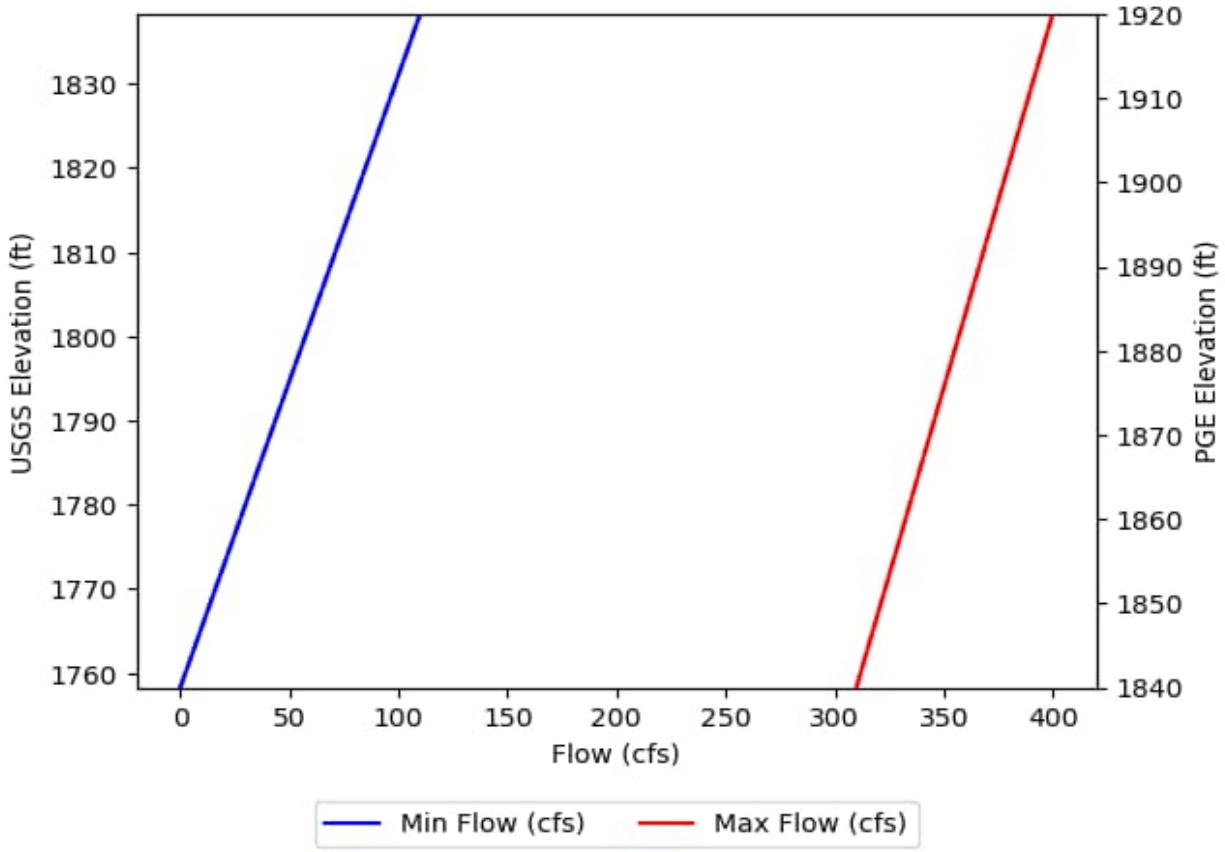
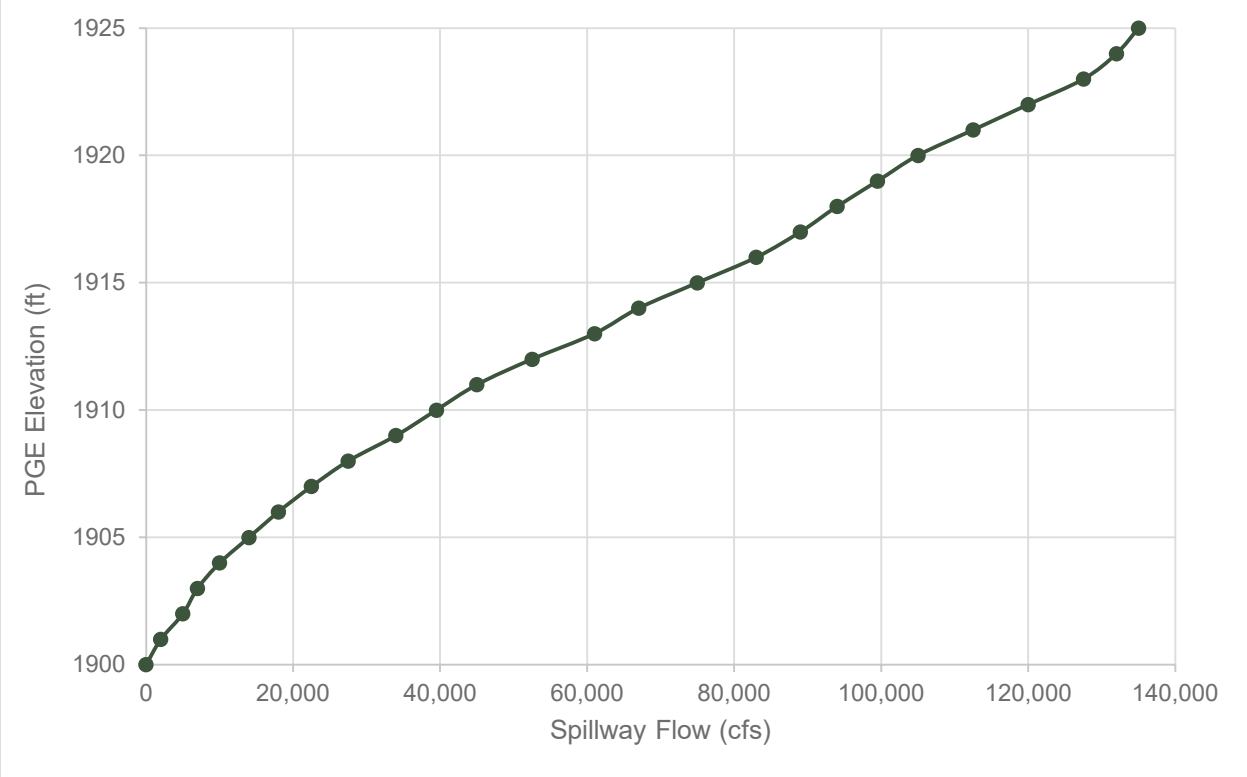


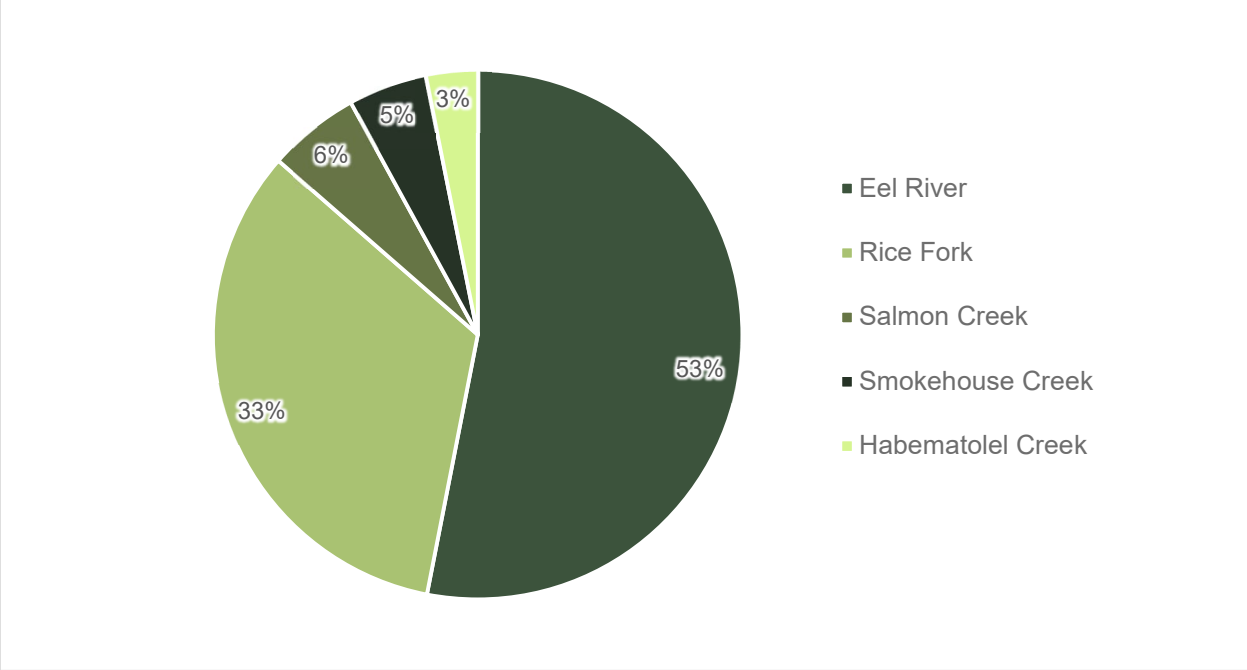
**Figure A4-5: Revised Rating Curve for the Scott Dam Spillway (based on Figure AQ 1-18, Cardno, 2019)**




## A4.4 Flow Contributions to Lake Pillsbury

SLR used StreamStats and USGS's hydrographic dataset to develop a basic understanding of the hydrology of the Eel River and its contributing watercourses under existing and post dam removal conditions. StreamStats reported that the main streams contributing to Lake Pillsbury are the Eel River, Rice Fork, Salmon Creek, Smokehouse Creek, and Habematolel Creek. The Eel River watershed has the highest contribution of flows by watershed area at approximately 53%, with the Rice Fork contributing 33%, and the Salmon, Smokehouse, and Habematolel Creeks combined contributing the other 14%. Figure A4-6 summarizes each major tributary flow contribution based on watershed area. The percentage contributions of each tributary were relatively consistent with the watershed area across all flood events calculated in StreamStats, from the 50% AEP (2-year recurrence) flood to the 0.02% AEP (500-year recurrence) flood. The Eel River ranged from 49-53% contribution, Rice Fork remained consistent at 33%, and the total contribution from the Salmon Creek, Smokehouse Creek, and Habematolel Creek combined ranged from 14-18%. SLR applied the flow contribution percentage based on watershed area to analyses of climate conditions and potential Scott Dam scenarios. These flow contributions are limited to area and flow comparison. Other contributing factors should be analyzed for further efficacy such as precipitation, slope, time of concentration, infiltration, and overland roughness based on the land cover.

**Figure A4-6: Flow Contributions by Watershed Area from Major Tributaries to the Eel River Watershed at Scott Dam**




### A4.4.1 Baseflows

All perennial watercourses have a baseflow[13] that fluctuates during the seasons but always retains some flow. The data available does not allow for any accurate projections in the potential baseflows. Analyzing the Scott Dam impaired Eel River outflow can provide very limited insights into the projected watercourse baseflows due to the storage and controls attenuating flows. The wet and dry season baseflows could roughly be estimated based upon the mass balance inflows provided by PG&E from 2006 to 2024 by removing evaporation and averaging the minimum average inflows, then correlating them to the percentage stream contributions. However, not all the streams are perennial, and the PG&E net inflow data had negative values that skewed the analysis so this method was deemed improbable and another hydrologic analysis using precipitation data would need to be completed for more reasonable results. This analysis did not report or predict baseflow levels and just reports stream typing.

## A4.5 Water Balance Model

A water balance model for Scott Dam and Lake Pillsbury was produced by SLR using Microsoft Excel-based spreadsheets. The water balance model results estimated Lake Pillsbury outflow in the Eel River below Scott Dam, Lake Pillsbury water storage, and Lake Pillsbury WSE based on estimated Lake Pillsbury unimpaired inflow on a daily model calculation timestep. Daily average data was available.

Unimpaired inflow to Lake Pillsbury values were projected for the Scott Dam system using the mass balance approach from Cardno (2019) which estimates Lake Pillsbury inflow by accounting for Scott Dam outflow, changes in Lake Pillsbury water storage, and evaporation

[13] The portion of the streamflow that is sustained between rainfalls, fed by groundwater or snowmelt and represents the normal flow within the watercourse.

losses from the Lake Pillsbury water surface area. Available data from the PG&E operated gages and PG&E mass balance data covering WY 2005 to 2024, USGS downloaded flow gage in the Eel River below Scott Dam and Lake Pillsbury storage gage covering WY 1957-2024, and the regional Coyote Dam (Lake Mendocino) pan evaporation gage covering WY 1994-2024 were used to recreate the mass balance. Since pan evaporation gage data represents a limited period of record, average monthly evaporation rates were calculated to produce a representative year that was repeated in the water balance model. The average annual evaporation is 42.9 inches.

Two sets for unimpaired inflow to Lake Pillsbury values were developed for the water balance model. A minimum daily Lake Pillsbury inflow value of 0 cfs was assumed to avoid negative values. One set utilized the PG&E data representing WY 2005-2024 and the other set utilized the USGS data representing WY 1957-2024. The Lake Pillsbury inflow values set using PG&E data was used for the purpose of calibrating the water balance model. Both data sets were deemed to have negligible value differences for the time periods that overlapped. Hence, the Lake Pillsbury inflow values set using USGS data was used for the evaluation of the potential Scott Dam scenarios since the set covered a longer record.

The stage-storage curve representing the 2015-2016 bathymetry survey discussed in Section A4.2 was used to estimate Lake Pillsbury water storage and WSE values. The rating curve discussed in Section A4.3 for the existing spillway was used to estimate outflow through the spillway when the Lake Pillsbury WSE is above the spillway invert elevation. A simplified rating curve discussed in Section A4.3 for the needle valve was used in the water balance model to estimate outflow through the needle valve. Needle valve outflow values ranged from minimum opening operating flow when the Lake Pillsbury WSE is low to maximum full capacity opening flow when the Lake Pillsbury WSE is high.

The water balance model was adapted based on the specific potential mitigation scenario analyzed. The overviews of the potential scenarios analyses and outcomes are presented in Section A7.0.

# A5.0 Fire Suppression Water Requirements

As discussed in Appendix B, Lake Pillsbury is currently an optimal resource for current support for fire suppression and operates as an extensive use water source for wildfire firefighting operations in the County of Lake. Appendix B information is summarized herein specifically to present the water storage needs with respect to optimal fire suppression within the County of lake.

When constructing a new reservoir which can support helicopter operations, specific size and maneuverability criteria should be met. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet at an average depth of 6 feet. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223-669 square feet. These considerations are summarized in Table  and listed by operational need for aerial firefighting.

**Table A5-1: General Aeiral Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Gallons [acre-feet] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Extensive Use | 100,000+ [0.31] | 6 feet | 2,230 sq. ft. | 47 ft × 47 ft or 30 ft × 75 ft |

| Operational Need | Required Gallons [acre-feet] | Average Water Depth | Minimum Surface Area | Approx. Dimensions |
|---|---|---|---|---|
| Limited Use (Low Volume) | 10,000 [0.031] | 6 feet | 223 sq. ft. | 15 ft × 15 ft or 10 ft × 23 ft |
| Limited Use (High Volume) | 30,000 [0.092] | 6 feet | 669 sq. ft. | 26 ft × 26 ft or 15 ft × 45 ft |

# A6.0 Data Limitations

While the amount of existing data is numerous regarding the proposed decommissioning of Scott Dam, in general the data is not comprehensive and relies on limited and incontiguous data that has been limitedly validated or calibrated. The SLR analyses projected results and assumptions are founded on public records, the limited data provided by PG&E, and hydrologic projections produced by other consultants. Data provided by others (PG&E, Two Basin Partnership, etc.), outside of publicly available data, did not provide the raw data or models used to develop and support the previous findings. The projections made were based upon reproducing the existing data by interpolating information from the various graphs and publicly available data.

The publicly available data, though abundant, was not always continuous and was limited in its viability to produce unimpaired flows or in projecting variability and/or extreme events. The projections and recommendations provided herein are based upon trends and ranges perceived using the data assessments completed by others with minor supplemental information to validate as available.

## A6.1 Station Data

There are various weather stations and gage stations that have recorded data within the Upper Eel River Data as far back as the early 1900's with varying degrees of record and data types reported. The weather station data was not clearly used in any of the previous analyses completed by others, as only the impaired or regulated stream outflow from Scott Dam and the Lake Pillsbury storage records were used for analysis, specifically within Lake County. This weather station data, particularly rainfall records coupled with additional modeling efforts, would further support the unimpaired climate condition projections.

The USGS gages 11470500 and 11470000 are maintained and operated by PG&E and record the average daily discharge from the Scott Dam control structures and the water storage within Lake Pillsbury based on the most recent lake bathymetry, respectively. Using the average daily flows and storage values have several limitations when projecting flow variations and responses to extreme (flood and drought) conditions.

Averaged data limitations can include:

1. Obscuring short-term variations in storage levels and flows, which are important for understanding flood events, ecological functions (fish passage), and/or rapid changes in reservoir levels.
2. Dampening or smoothing out important fluctuations such as evaporation, inflows, and outflows that would show the impact of high or low flow periods and/or changes in the reservoir levels.
3. An inaccurate representation of the actual system dynamics, especially during rapid changes in flow rates or storage levels.

4. Not capturing the full range or complexity of flows or reservoir operation nuances leading to less precise projections and assessments.

## A6.2 Water (Mass) Balance Method

A previous analysis by Cardno (2019) utilized a water or mass balance approach to project the system trends and inflows to Lake Pillsbury. The water balance approach approximated the inflows by accounting for the assumed outflows from the system such as operations and environmental releases and changes in storage. Cardno estimated these operational outflows by developing rating curves for the Scott Dam control structures (needle valve and spillway). The analysis also developed a flow pulse travel time relationship for each major stream reach to estimate flow routing or the travel time for water to move through the system and used statistical methods to estimate data gaps.

Cardno noted several specific concerns with the data interpretation and assessment limitations:

1. The spillway rating curve at Lake Pillsbury gives the spillway capacity at given WSEs with all the gates completely open. However, the reservoir rarely operated with all the gates completely open.
2. which can lead to discrepancies between recorded flows and the rating curve.
3. The needle valve rating curve relates the needle valve opening and Lake Pillsbury water surface elevation to the flow rate (Figure A4-3). There are instances where the calculated needle valve flows do not match the recorded stream gage flows, indicating potential inaccuracies. Additionally, the figure shows that both the needle valve opening, and outflow operate independently of WSE. This could indicate operator error, high variation in operator controls, and/or that a further look into the needle valve's relationship with storage and flow is needed.
4. High flow measurements at the Eel River stream gage just below the Scott Dam generally match the spillway capacity curve, but there are instances where recorded flows exceed the spillway capacity, suggesting the need for revision of the rating curve at high flows.

There are additional limitations with the mass/water balance approach used which include:

1. There are gaps in the data and even when using statistical methods to estimate the missing data, the estimates can introduce uncertainties. Wider and more prevalent gaps coupled with averaged data increases the uncertainty.
2. The travel time estimates and curves are developed using flow pulse data, but these estimates do not always reflect real-world conditions. Especially during extreme events and when there is not data that can be used to calibrate the estimates, such as within the watersheds above Scott Dam.
3. Evaporation rates were estimated using regression analysis, which may not capture all the variability in evaporation processes. Additionally, the reference source Cardno used were only recorded the average monthly evaporation between 1964 and 1970. Additional daily evaporation data is available that could be used to refine the regression and evaporation projections based on temperature and the Lake Pillsbury water surface area.
4. The mass balance flows averaged data using various techniques that can smooth and dampen important variations in flow data.

5. The stream gages are maintained and operated by PG&E and the accuracy of the data can be affected by factors such as operator error and gage bias.
6. There were no projections of the inflow and outflow contributions from groundwater, interception and evapotranspiration, subsurface flows, and snowmelt from Lake Pillsbury.

# A7.0 Scenario Overview

SLR utilized the previous analyses, other referenced reports, and assumptions to evaluate the hydrology and hydraulic conditions related to the baseline scenario and the seven (7) mitigation scenarios and have presented a response under four (4) climate conditions using statistical and analytical methods. These evaluations include projections of each scenario's outcomes for water supply, seasonality, flooding, and drought may affect availability. The baseline scenario reflects to historical conditions from 1922-2019, existing conditions refer to 2019-present, and scenarios A1, A2, B, C, D and E are all possible future pathways that the County of Lake could advocate. These scenarios outline a wide range of options, including partial or full dam removal, inclusion of a fish ladder, changes to the reservoir storage availability, and the creation of off-channel wetlands. These scenarios are based on PG&E's actions and intent as well as additional scenarios to support the County's interests.

## A7.1 Baseline Scenario: Historical Operational Conditions (WY 1922-2019)

The baseline scenario (historical operational conditions) scenario represents the period when Scott Dam and Lake Pillsbury maintained operations per the FERC license at full capacity storage. In 2018, McMillen Jacobs performed the Potter Valley Project Capital Modifications Feasibility Study, where they evaluated several alternatives for Scott Dam. These historical operational conditions scenario was referred to as the "Baseline" scenario in their Feasibility Study (McMillen, 2018).

Figure A7-1 summarized the historical operational conditions scenario and assumes a maximum capacity of 76,876 ac-ft and a usable storage capacity of 66,876 ac-ft as previously observed between 1922-2019 (PG&E, 2023). Over the 97-year period of record, the dam operations have changed along with the release requirements from the dam. Figure A7-2 shows how storage and outflow have changed since its construction in 1922 and indicates operational changes several times over the 100 years. Storage was decreased after each change in operations, reaching its lowest baseline after 2019 when the operational level was reduced by 10 ft. SLR did not observe any substantial changes in outflow correlating to operational changes from 1922-2019. These observed operational changes also correlate to bathymetric surveys completed and the updated reduction in storage capacity due to sediment accumulations as summarize in Table A7-1. Operations were assumed to maintain a consistent storage capacity in between surveys which were completely between 10-30 years since inception. Sediment accumulation typically happens each year with subsequent storage capacity loss. Accumulations and storage loss are not necessarily linear and are dependent upon variables such as sediment grain size, flows, flow velocity, slopes, shear stress, sediment concentration, vegetation and land cover, and anthropogenic activities. Lake Pillsbury on average loses approximately 210 to 230 ac-ft of storage each year.

**Table A7-1: Summary of PG&E Determined Lake Pillsbury Storage Capacities Based on Bathymetric Surveys and/or Changes in Operation**

| Bathymetric Survey Year | Lake Pillsbury Storage Capacity (ac-ft) |
|---|---|
| 1921 | 94,400 |
| 1959 | 86,700 |
| 1985 | 80,643 |
| 2005 | 74,993 |
| 2015 | 77,419 |
| 2016 | 76,876 |
| 2023 | 69,872 |

Notes:
1. Source for 1921 and 1959 from Porterfield (1694) (Porterfield, 1964)
2. Source for 1985 through 2023 from PG&E (2024) (PG&E, 2024)
3. 2015 to 2016 increased due to improved equipment & techniques (PG&E, 2017)

The historical operational conditions analysis primarily focused on the operational period from 2005-2019, however the historical conditions from 1922-2019 are also provided for comparison. Historically, these operational conditions have been able to meet the outflow obligations. However, there have been instances each year between 2013 and 2016 where PG&E has had to request a temporary flow variance due to drought conditions (PG&E, 2023).

**Figure A7-1: Historical Operational Conditions (WY 1922-2019) scenario: measured minimum, average, and maximum Lake Pillsbury WSE levels**

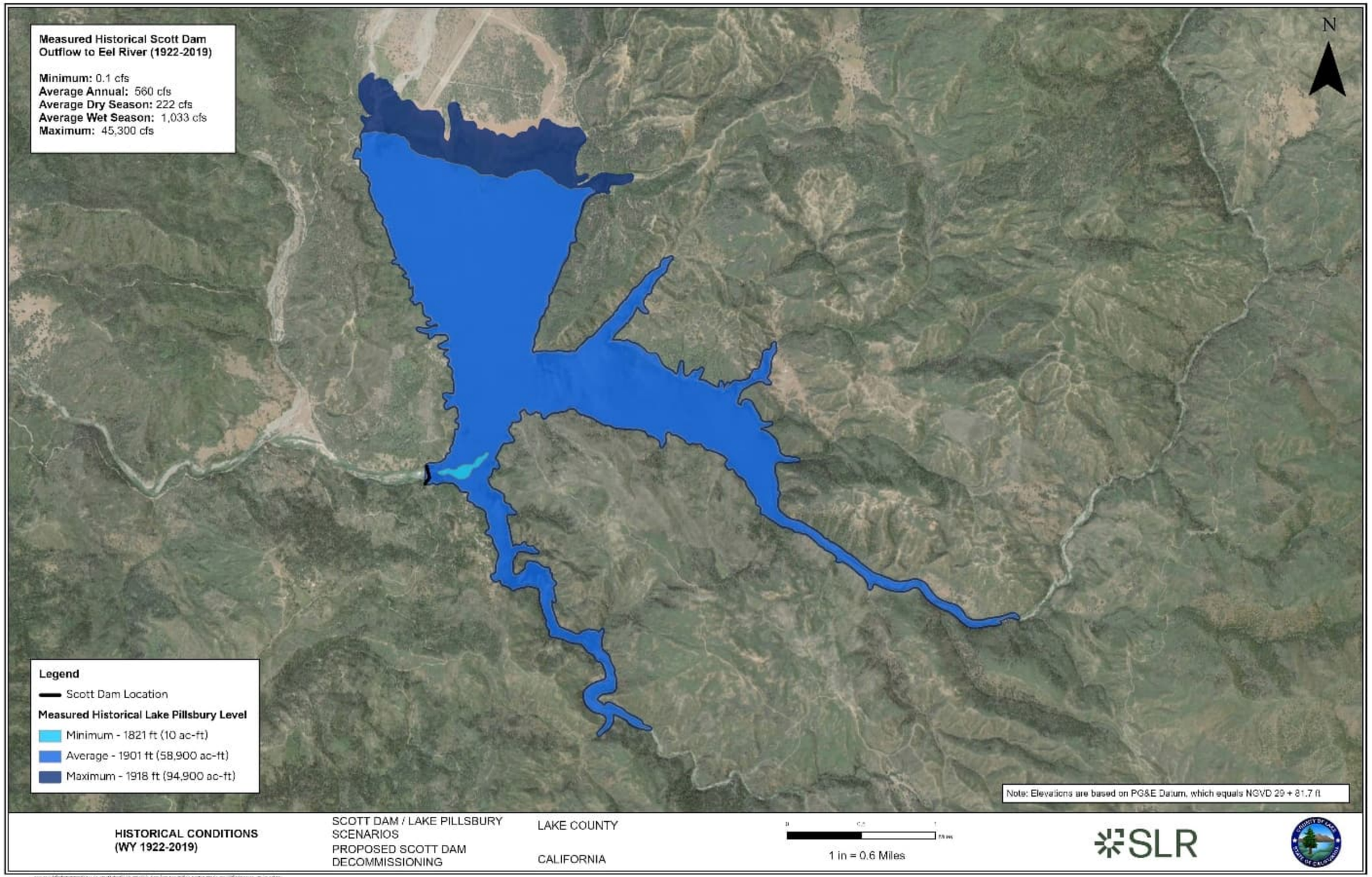
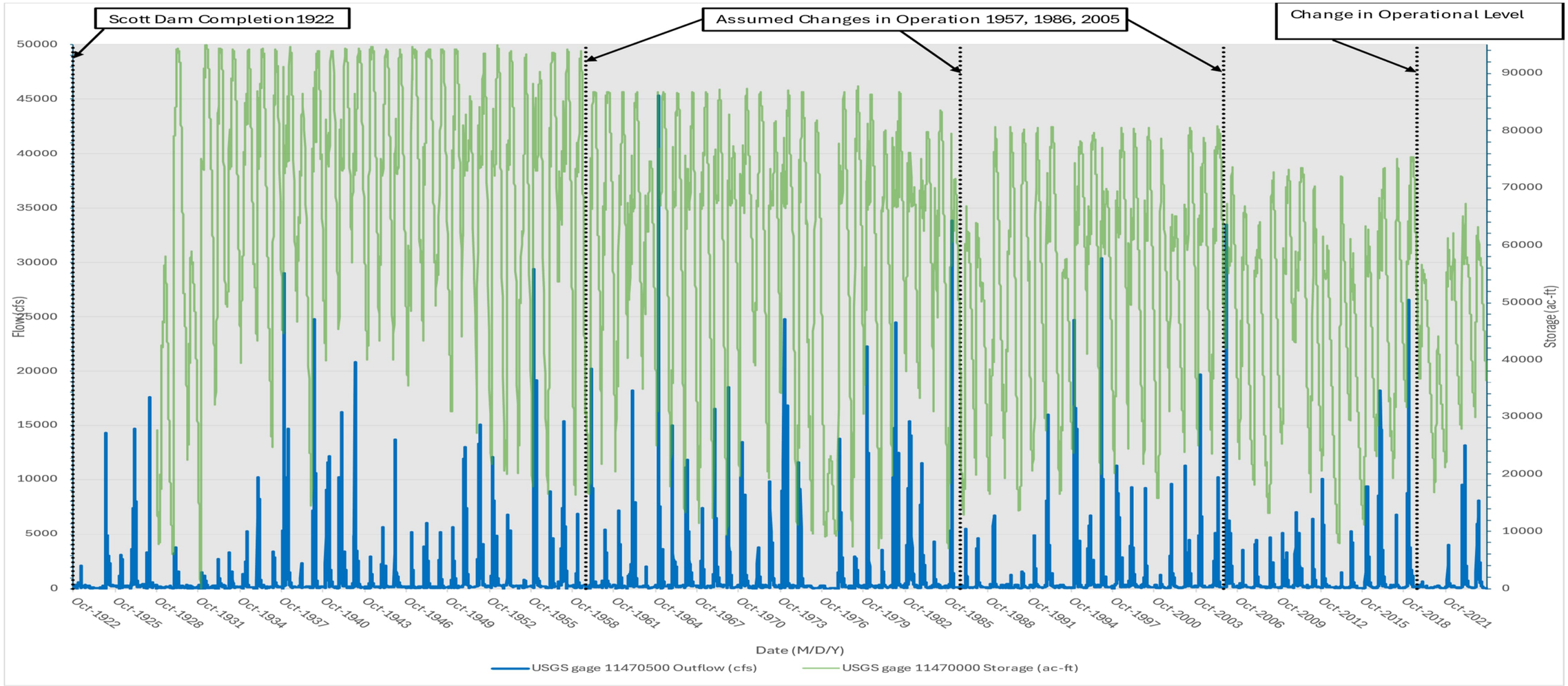


**Figure A7-2: Outflow and storage of Lake Pillsbury from 1922-2024 (dam construction and operational changes are specified)**



### A7.1.1 Hydrologic Analysis

The PG&E-provided data and available gage data were used to statistically analyze the observed outflows and storage for the historical operational historical conditions. Two water year timeframes were analyzed for historical conditions for the wet and dry season climate condition: 1922-2019 and 2005-2019. The 1922-2019 period was used to assess the flood and drought conditions along with the NIDIS.

#### A7.1.1.1 Wet and Dry Season Conditions

The minimum, median, average, and maximum values were provided for the average daily storage and outflow records for the wet and dry season over the WY period of record. Storage and outflow statistics were determined independently of each other, meaning the minimum outflow did not necessarily coincide with the minimum storage. Statistics for the wet and dry seasons are summarized in Table A7-2. These WSEs are provided for reference only as they were determined based upon the 2016 bathymetry and will lose accuracy the farther the storage deviates from the 2016 storage capacity. The USGS Porterfield (1964) report on Lake Pillsbury was developed to study the quantity, distribution, and properties of sediment deposited within Lake Pillsbury and recorded a maximum WSE of 1,829.1 feet Mean Seal Level (MSL/NGVD22) or 1910.8 feet PG&E on May 13, 1925. The maximum recorded by Porterfield was approximately 7.1 feet lower when compared to the 2016 bathymetric storage results. WYs 2005-2019 show a lower overall average in the storage and outflow observed as well as a lower maximum, and a higher minimum. This could be attributed to a longer period of record logging more data from extreme events as well as changes in operating procedures. The records for maintenance were not provided and could also influence the statistical validity.

**Table A7-2: Historical Operational Conditions – Statistics for Storage within Lake Pillsbury and Outflow Downstream of Scott Dam by Season**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| **WY 1922-2019** | | | | | | |
| Minimum | 10 | 1820.9 | 2 | 2,100 | 1840.1 | 0.1 |
| Median | 65,900 | 1904.8 | 375 | 59,700 | 1901.7 | 177 |
| Average | 59,810 | 1901.8 | 1,033 | 58,240 | 1901 | 222 |
| Maximum | 94,900 | 1917.9 | 45,300 | 94,900 | 1917.9 | 10,200 |
| **WY 2005-2019** | | | | | | |
| Minimum | 7,840 | 1856.4 | 37 | 11,300 | 1862.9 | 27 |
| Median | 55,400 | 1909.3 | 296 | 45,200 | 1879.4 | 147 |
| Average | 49,078 | 1896 | 820 | 45,911 | 1894.1 | 165 |
| Maximum | 80,200 | 1911.4 | 26,600 | 80,800 | 1911.7 | 2,400 |

### A7.1.1.2 Flood and Drought Conditions

Table A7-3 summarizes the observed maximum storage and outflow from the Scott Dam and Lake Pillsbury gages in response to extreme flood conditions for the historical operational conditions. As noted in Flood and Drought of Record Section 3.2.2, the flood of record occurred during WY 1982 (Metzler, 1983). The highest average daily outflow and storage observed during the flood of record were on January 26 and 27, 1983 respectively. The observed gage measurements were 76,260 ac-ft for storage and 15,400 cfs for outflow. In comparison, the maximum gage daily average storage measurement recorded within Lake Pillsbury was on April 24, 1932, and happened to be during one of the notable drought seasons within the last century. The highest gage recorded daily average outflow was observed on December 22, 1964, and does not correlate to any specific event.

**Table A7-3: Historical Operational Conditions – Observed Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | | | |
|---|---|---|---|---|---|
| | Date of Record Storage | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Date of Record Outflow | Average Daily Outflow (cfs) |
| Gage | 4/24/1932 | 94,900 | 1917.9 | 12/22/1964 | 45,300 |
| Historic Records | 1/26/1983 | 76,260 | 1909.7 | 1/27/1983 | 15,400 |

The Flood and Drought of Record Section A3.2.2 notes multiple drought occurrences over the past century. The lowest gage recorded average daily storage value of 10 ac-ft occurred on December 9, 1931, which coincides with the 1928-1934 drought. Presumably, this took place prior to the needle valve outflow structure installation at Scott Dam which would have incurred less precision in the outlet and storage controls. There is also a possibility of manual reservoir drawdown for maintenance or operational reasons that were made unavailable to SLR. The lowest gage recorded average daily outflow was 0.1 cfs which occurred on September 8, 1924, which does not correlate to a notable recorded event.

During drought periods, notable storage and outflow rates were observed when the average daily storage was below the recommended needle valve operating range of 10,000 ac-ft which occurred on January 20, 1977, and February 5, 2014. The lowest average daily storage recorded during the 1976-1977 drought was 9,018 ac-ft and the lowest average daily flow was 5 cfs. The lowest average daily storage recorded during the 2012-2016 drought was 7,840 ac-ft and the lowest average daily flow was 27 cfs. Table A7-4 summarizes the observed minimum storage and outflow from the Scott Dam and Lake Pillsbury gages occurring the full record period and during notable drought years.

**Table A7-4: Historical Operational Conditions – Observed Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | | | |
|---|---|---|---|---|---|
| | Date of Record | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Date of Record Outflow | Average Daily Outflow (cfs) |
| Gage | 12/9/1931 | 10 | 1820.9 | 9/8/1924 | 0.1 |
| Historic Records | **1928-1934** | | | | |
| | 12/9/1931 | 10 | 1820.9 | 4/28/1931 | 1.8 |
| | **1976-1977** | | | | |
| | 1/20/1977 | 9,018 | 1858.8 | 2/19/1977 | 5 |
| | **2012-2016** | | | | |
| | 2/5/2014 | 7,840 | 1856.4 | 10/30/2015 | 27 |

### A7.1.2 Baseline Scenario (Historical Operations) Outcomes

There main component being considered when analyzing the baseline historical conditions, is the scenario's historical ability to reliably supply water.

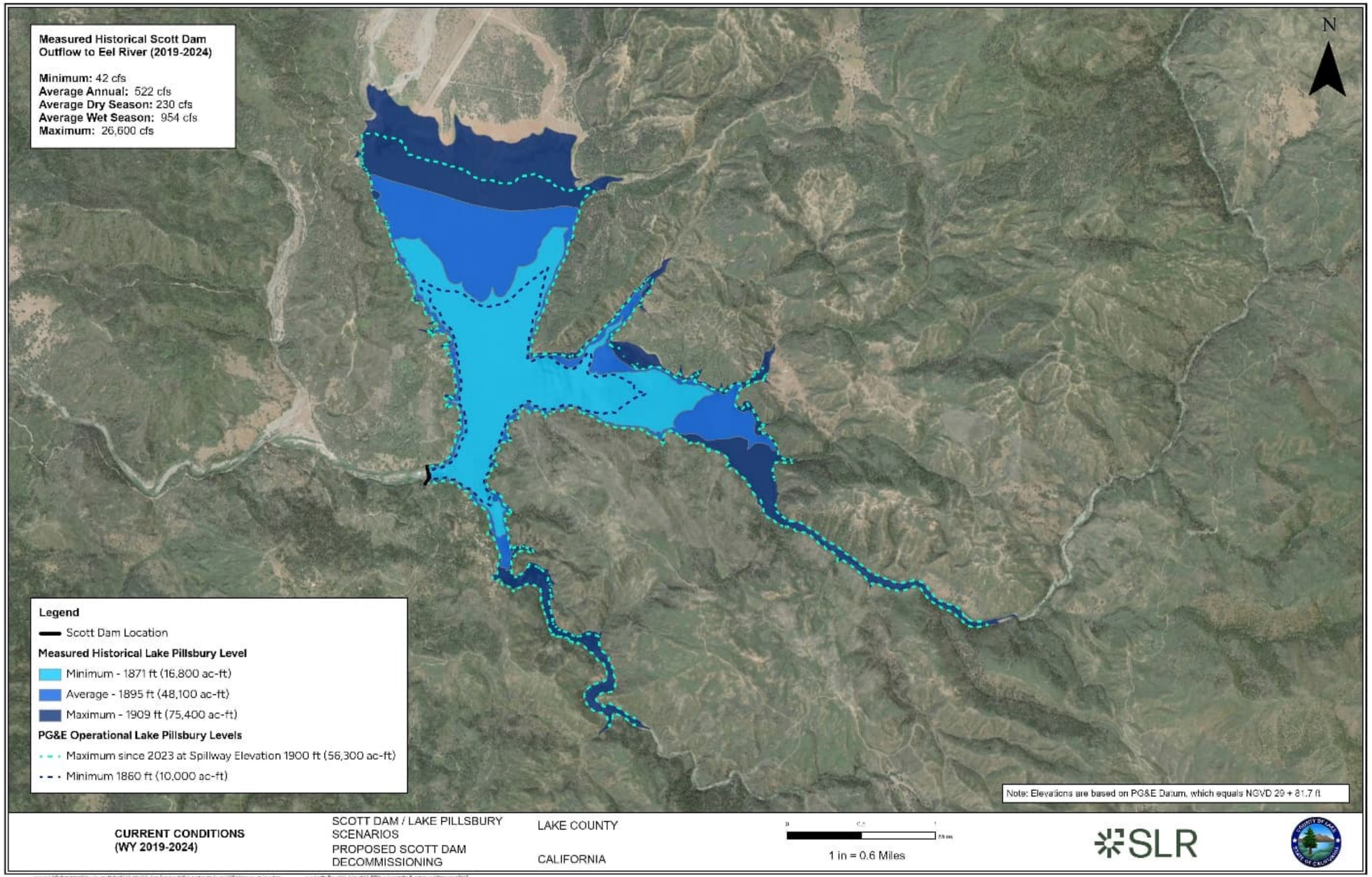
#### A7.1.2.1 Water Supply

This baseline historical scenario provides the most reliability on water supply in terms of flood control, potential groundwater recharge, downstream water diversion flow requirements, and operations due to the operability control provided by the needle valve and spillway gates for the purpose of downstream hydroelectric power generation. Lake Pillsbury has historically operated with an average storage volume of over 45,000 ac-ft (14 million gallons) of water availability. Floods have historically shown to be attenuated by the reservoir, reducing the flood risk to downstream infrastructure. This historical scenario has a long history of providing water supply for downstream uses other than periods of drought where flow variances were required. This scenario did not analyze future climate conditions but noted that the drought trends during the 21st century have been increasing in frequency, intensity, and duration. This historical scenario's water storage capacity has been reduced over time due to losses from sediment accumulation.

## A7.2 Existing Operational Conditions Scenario (WY 2019-Present)

The existing operational conditions scenario represents the Scott Dam and Lake Pillsbury operational period from 2019 to the present. Based on observed operational data, the maximum operational WSE for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 feet with a correspond water storage volume of 56,322 ac-ft as depicted in Figure A7-3. Historically, the maximum operational WSE for Lake Pillsbury was at top elevation of the spillway gates of 1,910 feet. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the WSE at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained. It is not known if there have been operational changes to the needle valve.

**Figure A7-3: Existing Operational Conditions (WY 2019-2024) scenario: measured minimum, average, maximum, and operational Lake Pillsbury WSE levels**



### A7.2.1 Hydrologic and Hydraulic Analysis

The PG&E-provided data and available gage data were used to statistically analyze the observed outflows and storage for the current operation conditions for WYs 2019-2024. An additional analysis was completed utilizing the water balance model to project the storage and outflows under the current operations as the observed values are over a limited timeframe.

#### A7.2.1.1 Wet and Dry Season Conditions

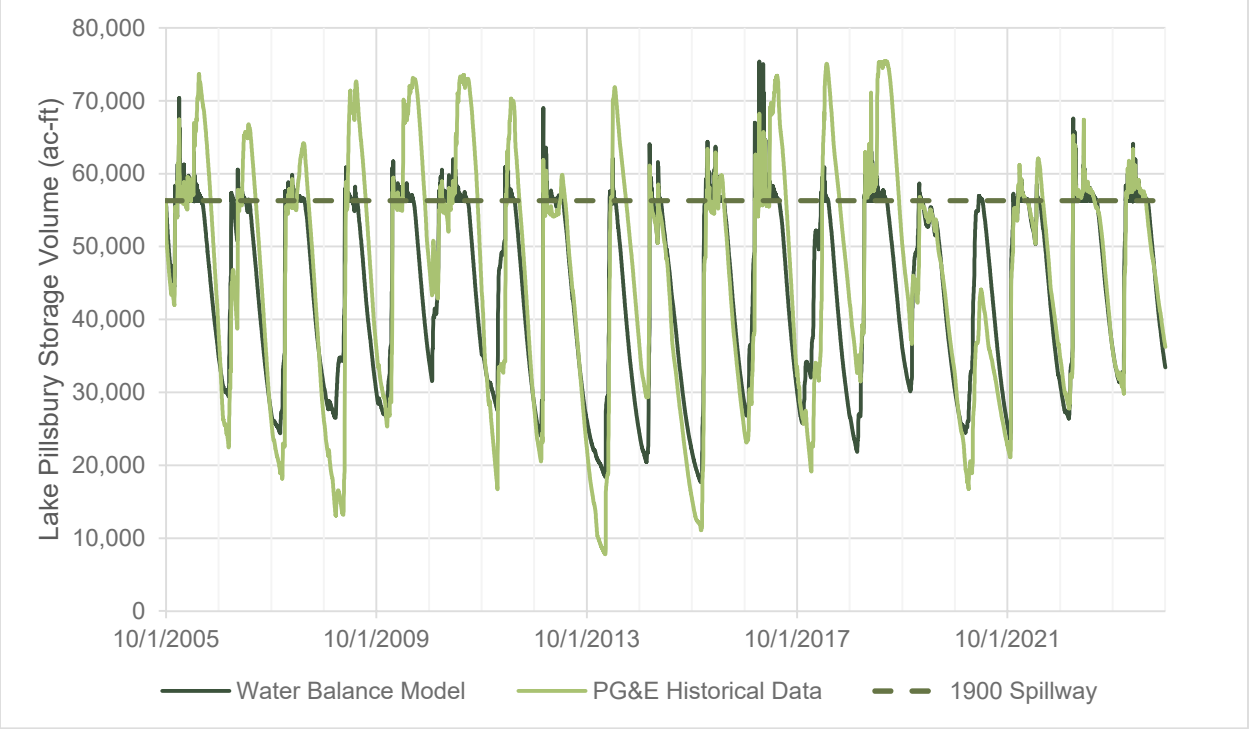
A summary of the observed wet and dry season climate conditions covering the five-year period is presented in Table A7-5. The minimum, median, average, and maximum values were provided for the average daily storage and outflow records for the wet and dry season over the WY period of record. Storage and outflow statistics were determined independently of each other, meaning the minimum outflow did not necessarily coincide with the minimum storage. The WSE that corresponds to the storage value was also provided for reference in the PG&E datum. The observed current operational conditions WY 2019-2024 shows a lower overall average in the storage and outflow when compared to observed historical operational conditions WY 1922-2019. This is attributed to the change in spillway operations resulting from the lowering of the maximum operational WSE for Lake Pillsbury.

**Table A7-5: Existing Operational Conditions – Statistics for Observed Storage within Lake Pillsbury and Observed Outflow Downstream of Scott Dam by Season (WY 2019-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 16,800 | 1870.9 | 67 | 21,100 | 1875.7 | 42 |
| Median | 53,200 | 1898.4 | 207 | 44,100 | 1893.1 | 115 |
| Average | 45,439 | 1893.8 | 768 | 43,789 | 1892.9 | 201 |
| Maximum | 67,300 | 1905.6 | 13,200 | 62,100 | 1903.0 | 2,150 |

Since the observed data record of the current operational conditions for Scott Dam and Lake Pillsbury is over a limited timeframe, the hydrologic analysis of the scenario was also conducted using the water balance model developed by SLR discussed in Section A4.5. The water balance inflow rates to Lake Pillsbury developed from PG&E data (WY 2005-2024) were modeled for current operational conditions for Scott Dam and Lake Pillsbury. The resulting water balance model storage values in Lake Pillsbury are presented in Figure A7-4. The observed historical storage values were included in the figure for comparison.

**Figure A7-4: Lake Pillsbury storage comparison of water balance model results and PG&E historical data**




The water balance inflow rates to Lake Pillsbury developed from the USGS data (WY 1957-2024) were modeled for current operational conditions for Scott Dam and Lake Pillsbury. The resulting water balance model storage and WES values in Lake Pillsbury are presented in Figure A7-5 and Figure A7-6. The resulting water balance model outflow values from Lake Pillsbury to Eel River below Scott Dam are presented in Figure A7-7. The results generally demonstrated a repeating water year cycle. During the dry season, storage within Lake Pillsbury and outflow from Scott Dam are decreasing. The Lake Pillsbury WSE is generally dropping below the dam spillway invert and thus outflow is through the needle valve. During the wet season, storage within Lake Pillsbury and outflow from Scott Dam are increasing. The Lake Pillsbury WSE rises to the dam spillway invert and outflow is through both the needle valve and spillway. Periodic increases in inflow during the wet season momentarily increase storage within Lake Pillsbury as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,900 ft. The long-term overall trend of water storage in Lake Pillsbury is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period is presented in Table A7-6.

**Figure A7-5: Lake Pillsbury storage water balance model results for current conditions**

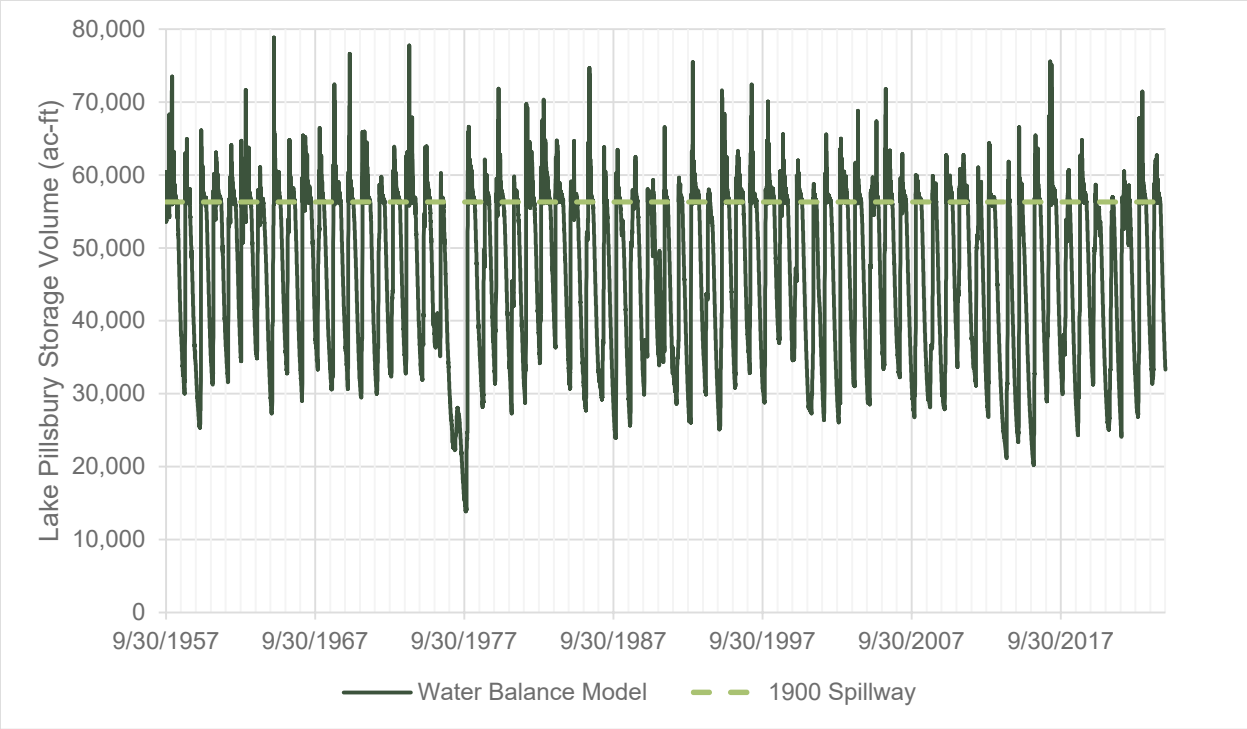



**Figure A7-6: Lake Pillsbury WSE water balance model results for current conditions**

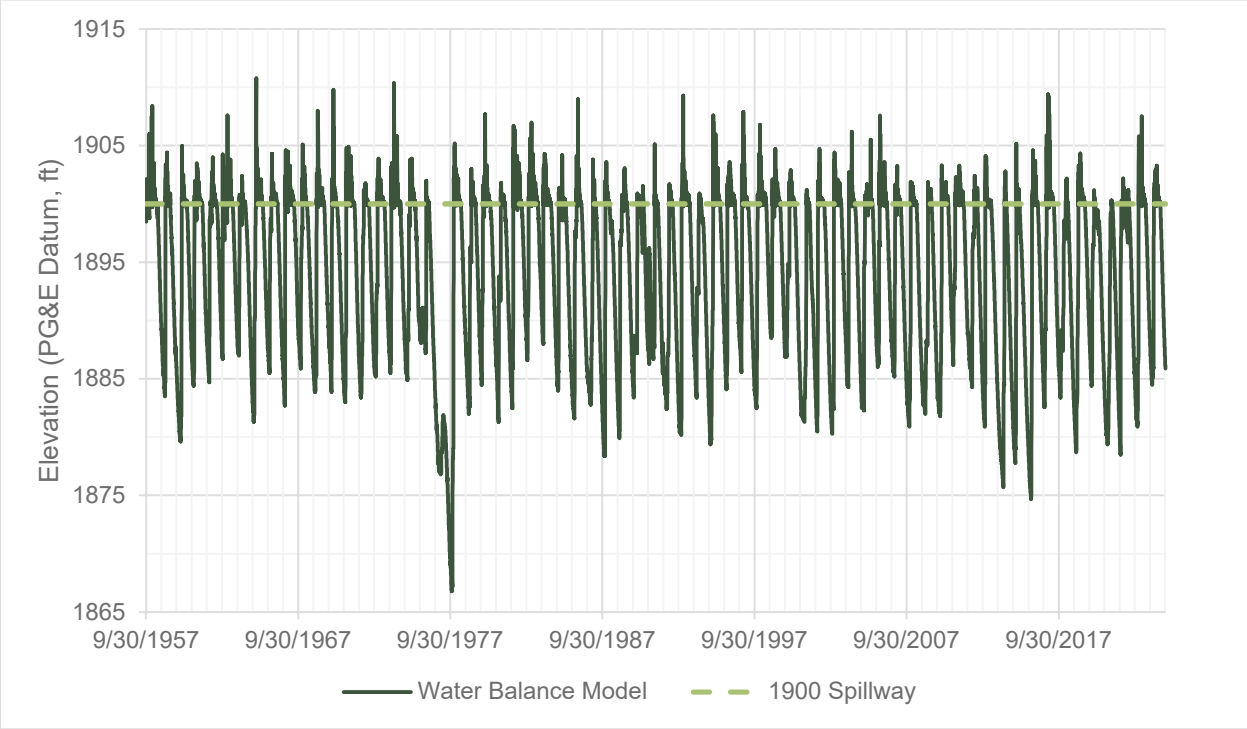


**Figure A7-7: Lake Pillsbury outflow water balance model results for current conditions**

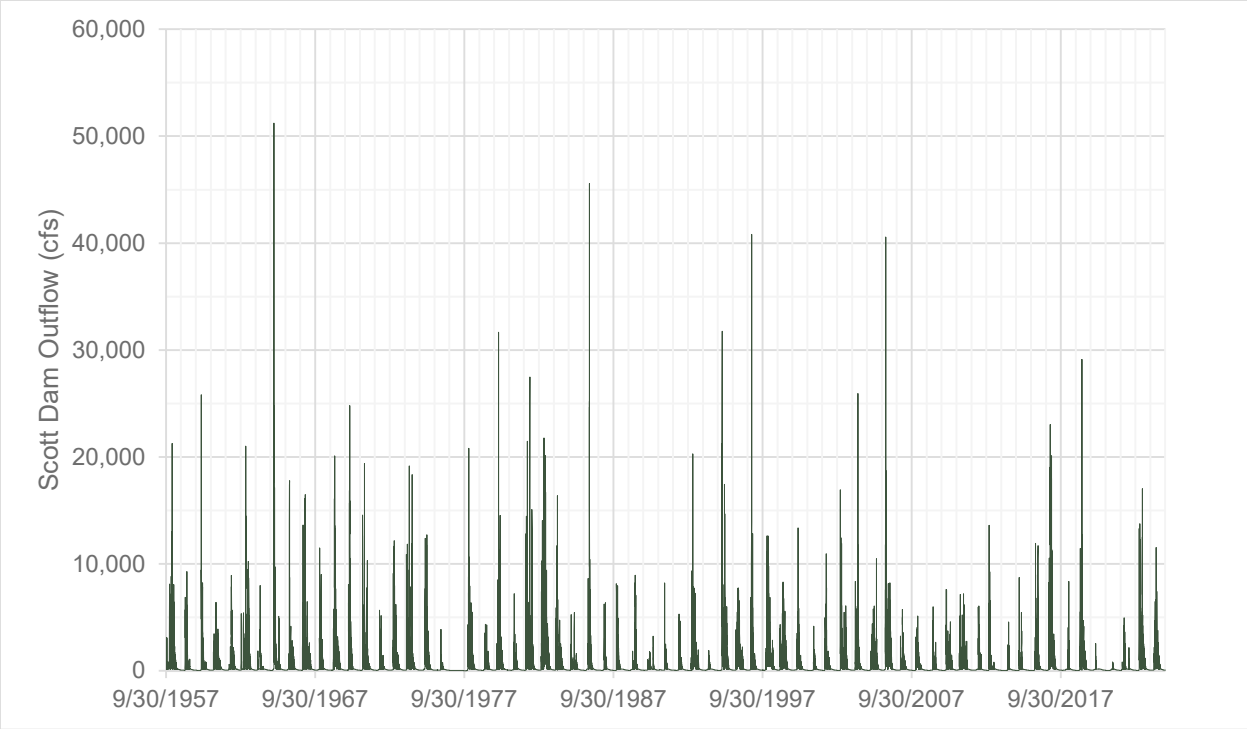



**Table A7-6: Existing Operational Conditions – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam by Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 13,870 | 1866.8 | 40 | 13,910 | 1866.9 | 40 |
| Average | 48,500 | 1895.0 | 1,052 | 45,070 | 1893.2 | 258 |
| Maximum | 78,910 | 1910.8 | 51,230 | 67,380 | 1905.5 | 15,110 |

### A7.2.1.2 Flood and Drought Conditions

This scenario's gage records do not coincide with any notable periods of flooding or drought. The observed minimum and maximum flow and storage can be used to approximate but is limited to the period of record. The minimum and maximum flow and storage values calculated per the water balance can also be used as a reference for projected flood and drought conditions. The gages and water balance flood and drought projections for storage within Lake Pillsbury and outflows from Scott Dam are summarized in Table A7-7 and Table A7-8, respectively. The gage records are lower for flooding and higher for drought conditions. In comparison to the baseline historic operations gaged condition (Table A7-3) the flood projections for storage are lower but the outflow is higher. Whereas the gaged projections under the historic conditions for drought are drastically lower than the current operations even when compared to the historic records per Table A7-4. The drought comparison is misleading when compared to baseline conditions as the circumstances for the gaged values are uncertain and

may have been forced due to operational maintenance. Analytically, reducing the storage capacity will reduce the drought tolerance and it is likely that under drought conditions the storage and outflow will be lower than the historic conditions observed values.

**Table A7-7: Existing Operational Conditions – Observed and Calculated Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Gage | 67,300 | 1905.6 | 13,200 |
| Water Balance | 78,910 | 1910.8 | 51,230 |

**Table A7-8: Existing Operational Conditions – Observed and Calculated Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Gage | 16,800 | 1870.9 | 42 |
| Water Balance | 13,870 | 1866.8 | 40 |

### A7.2.2 Projected Scenario Outcomes

The current operational conditions scenario analyzes the continual operations of Scott Dam at the current maximum operational WSE of 1,900 ft and the projected potential impacts on water supply.

#### A7.2.2.1 Water Supply

Flood control, downstream water diversion flow requirements, and operational reliability are not drastically difference from the baseline historical operational conditions scenario due to similar outflows from Scott Dam to the Eel River. The average WSE and storage in Lake Pillsbury for the current operational storage conditions are lower than the historical operational conditions due to the reductions of the maximum operational WSE and the passive operation of the spillway.

This scenario would generally increase flood risk as active operations of the spillway gates have ceased, which had previously allowed for greater control, attenuation, and retention of flood inflows. This scenario has slightly reduced drought tolerance due to a lower average WSE and storage in Lake Pillsbury than the historical operational conditions. Lake Pillsbury is expected to continue operating with an average storage volume of over 45,000 ac-ft (over 14 billion gallons) of water availability. However, the water supply capacity will continue to be reduced over time due to losses from continued sediment accumulation.

This scenario is not anticipated to drastically affect the potential groundwater recharge or downstream water diversion flows requirements, but could be affected under drought conditions which are difficult to adequately predict or quantify with the data available.

## A7.3 Scenario A1 – PG&E Proposed Removal

Under this scenario, PG&E proposes the full decommissioning of Scott Dam. The proposed approach is divided into three phases:

a) initial dewatering and removal of the upper portion of the dam,

b) initial sediment flushing, and

c) final dam removal.

As proposed by PG&E, the sediments that have accumulated within the impoundment will be managed passively. PG&E's current proposed action under the Final Draft Surrender Application (PG&E, 2025) includes the decommissioning and removal of Scott Dam, draining the Lake Pillsbury and removing associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. PG&E proposed a Rapid Dam Removal approach estimated to take 2 years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards) downstream of the Eel River (PG&E, 2025). PG&E anticipates temporary alteration of the Eel River channel and/or floodplain morphology from sediment deposition after deconstruction of the dam until subsequent high-flow events can resuspend the sediment and transport it farther downstream, until the river channel adapts to its natural condition. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. Other activities proposed by PG&E as part of decommissioning are documented in the Draft Application for Surrender of License (PG&E, 2025).

PG&E may propose additional responses at locations that are identified where natural fluvial processes are not mobilizing sediment to the volumes anticipated. These locations may have continued issues with bank instability and channel incision that may adversely affect habitat and fish passage.

### A7.3.1 Hydrologic and Hydraulic Analysis

The 2019 memorandum by Cardno projected unimpaired flows for the entire Eel River utilizing the mass balance method described in Sections A4.1 and A6.2 (Cardno, 2019). Cardno's data, analysis, and models were not provided for the unimpaired flow projections just downstream of Scott Dam. However, the Scott Dam and Cape Horn Dam Removal Alternatives references and provide a summary graph of the unimpaired monthly average discharge (McMillen, 2021). The graph was presented as part of the Results of Initial Water Supply Modeling for PVP and Russian River Alternatives, which are noted to be estimates from the Cardno unimpaired hydrology calculations (Addley et al, 2019). The unimpaired flow graph was reproduced and is presented in Figure A3-4. These unimpaired flows are used as the basis for the proposed removal climate conditions assessment for consistency and to compare previous report findings.

#### A7.3.1.1 Wet and Dry Season Conditions

The monthly average unimpaired flows from Figure A3-4 were analyzed by season, and statistics were calculated to support the projected outcomes for each season. The wet and dry season statistics are summarized in Table A7-9. The average monthly wet season flows range from 791 cfs in April to 1,535 cfs in February, and the average monthly dry season flows range from 17 cfs in August to 376 cfs in May.

The wet and dry season conditions under the dam removal scenario were roughly estimated for each major watercourse by associating the flow contribution summarized in Section A4.4.

The wet and dry season average monthly flows by watercourse are shown in Figure A7-8 and Figure A7-9, respectively.

**Table A7-9: Dam Removal Scenario A1 Wet and Dry Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 791 | 17 |
| Median | 1,237 | 59 |
| Average | 1,216 | 133 |
| Maximum | 1,535 | 376 |

### A7.3.1.2 Flood and Drought Conditions

The recommended 1% AEP flow just downstream of Scott Dam for this scenario, based on the current analyses, would be the 2025 USGS StreamStats flow of 60,100 cfs until a more in-depth analysis can be completed. A StreamStats assessment was also conducted to project the 1% AEP flows for each major watercourse contributing to flow just downstream of Scott Dam and are summarized in Table A7-10.

**Table A7-10: Projected StreamStats 1% Annual Exceedance Probability Flows for Watercourses Contributing to Flow Just Downstream of Scott Dam**

| Watercourse | Type | 1% AEP Flood [100-year Recurrence] (cfs) |
|---|---|---|
| Upper Eel River1 | Perennial | 34,600 |
| Rice Fork | Perennial | 22,900 |
| Salmon Creek | Intermittent | 5,170 |
| Smokehouse Creek | Intermittent | 4,600 |
| Habematolel Creek | Perennial | 3,110 |
| Eel River Downstream of Scott Dam | Perennial | 60,100 |

Notes:
1. The Upper Eel River flow represents the Eel River contribution just upstream of the Habematolel Creek confluence.

The drought conditions for this scenario are difficult to project without more detailed unimpaired flow data. The Eel River, Habematolel Creek, and Rice Fork are classified by the USGS as perennial streams, while the Smokehouse and Salmon Creeks are classified as intermittent. Smokehouse and Salmon Creek are likely not to have flows during a drought condition. The Eel River, Habematolel Creek, and Rice Fork may be more drought tolerant under specific conditions, however, the Eel River is likely to have the most reliable source of flow.

**Figure A7-8: Unimpaired Wet Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse**

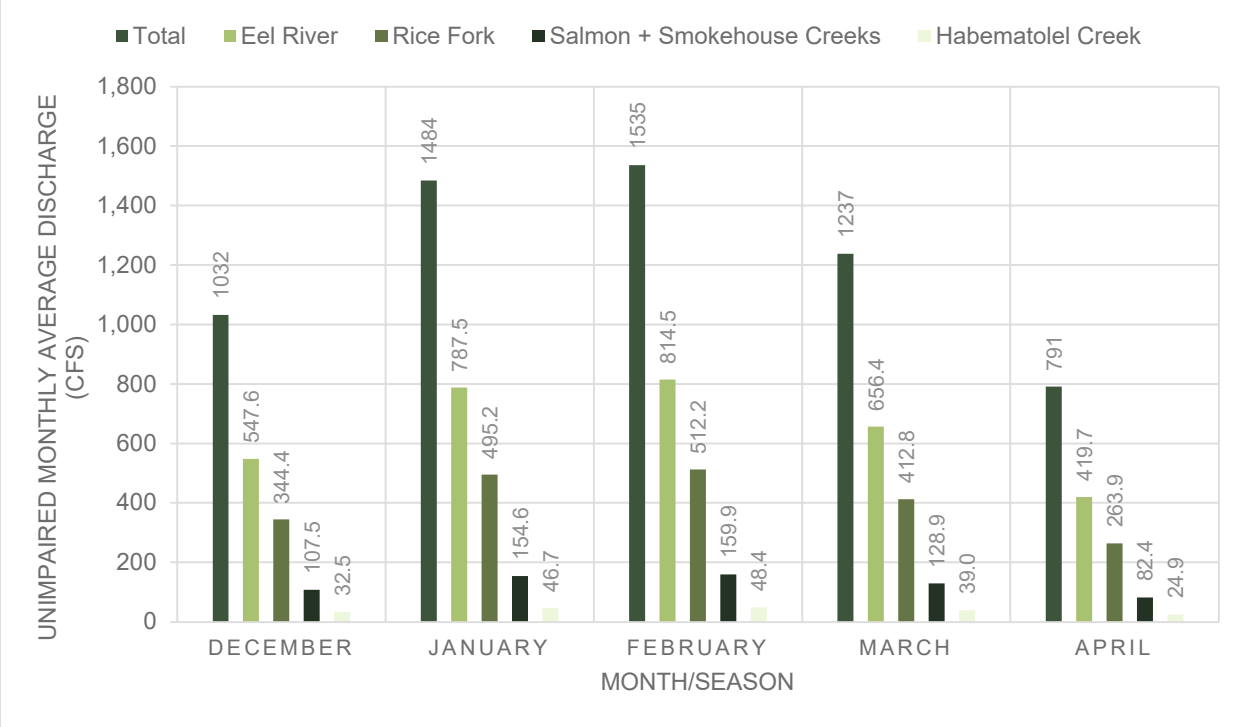
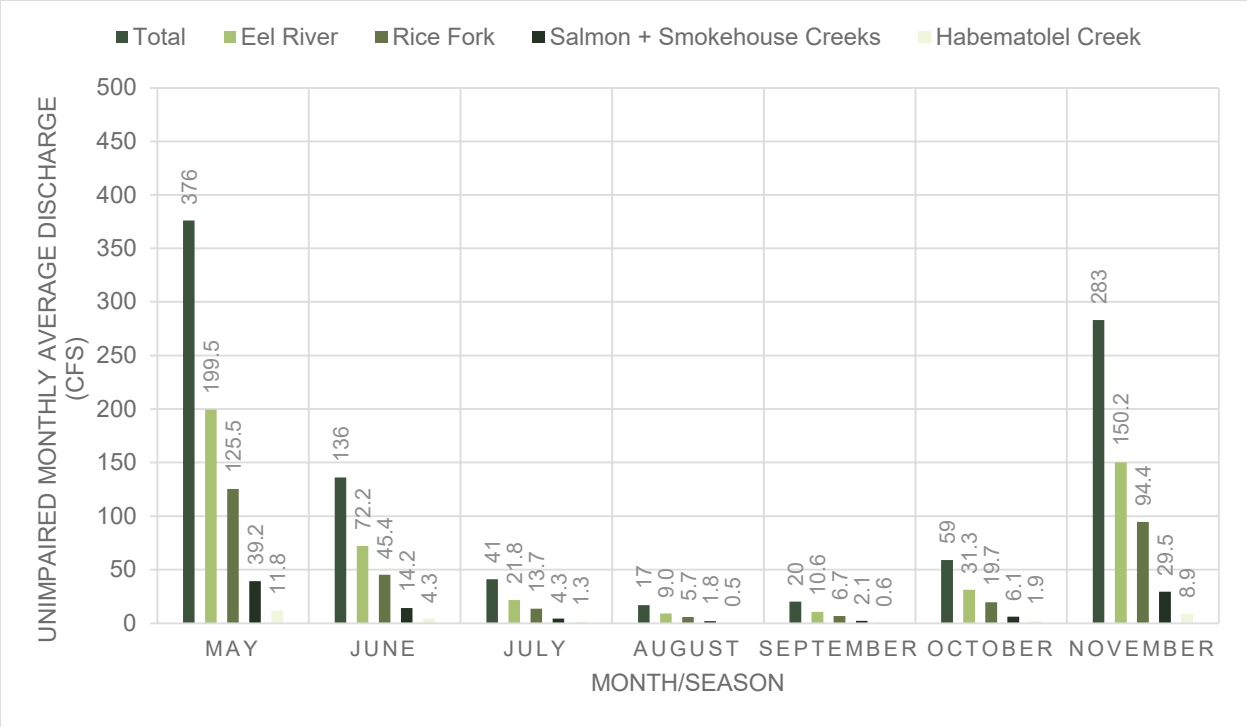



**Figure A7-9: Unimpaired Dry Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse**



### A7.3.2 Projected Scenario Outcomes

Scenario A1 analyzes PG&E's Scott Dam removal and the projected impacts on water supply. A plan view depiction of the Scott Dam and Lake Pillsbury area with projected response to Scenario A1 conditions is presented in Figure A7-10 and Figure A7-11. The Eel River and tributaries are assumed to return to their pre-dam condition as represented in the 1922 topographic and bathymetric mapping compiled by PG&E (2016).

#### A7.3.2.1 Water Supply

This scenario would eliminate all water storage, and any flood control. Water supply to the Russian River watershed would be limited to winter months and during wetter dry seasons where water is not diverted by irrigation. Under baseline existing conditions, the average monthly dry season flows are approximated between 147-222 cfs, and the wet season average flows range from 296-375 cfs. In comparison, the wet and dry season average flows would be much higher at 1,237 cfs (wet) and a bit lower at 133 cfs (dry).

This scenario would increase flood risk and likely increase the frequency, intensity, and duration of flood events seen under existing conditions of operations. This is attributed to the storage and dam controls attenuating the flows and retaining much of the flood waters. This scenario has very low drought tolerance when compared to the baseline scenario, where average monthly flows are projected to fluctuate between 17 and 376 cfs during the dry season. Flood and drought risk are difficult to quantify due to the limited unimpaired data analysis assumptions and would require more analysis to provide higher confidence in the projected response.

The passive sediment removal will cause flow to fluctuate and become unreliable to predict until the watercourses and sediments are semi-stabilized. Semi or quasi stabilization should be assumed when there are no major changes to the channel geometry and floodplain limits, as well as when vegetation has been re-established and unaffected during normal wet and dry season flows.

The Lake Pillsbury reservoir's ability to recharge the groundwater would be removed, and recharging would be limited to the watercourse contributions. This could result in a reduction of the groundwater table, groundwater flow patterns, and localized drying of wells and springs.

**Figure A7-10: Scenarios A1 and A2 PG&E proposed Scott Dam removal: Eel River and tributary watercourses originally feeding Lake Pillsbury**

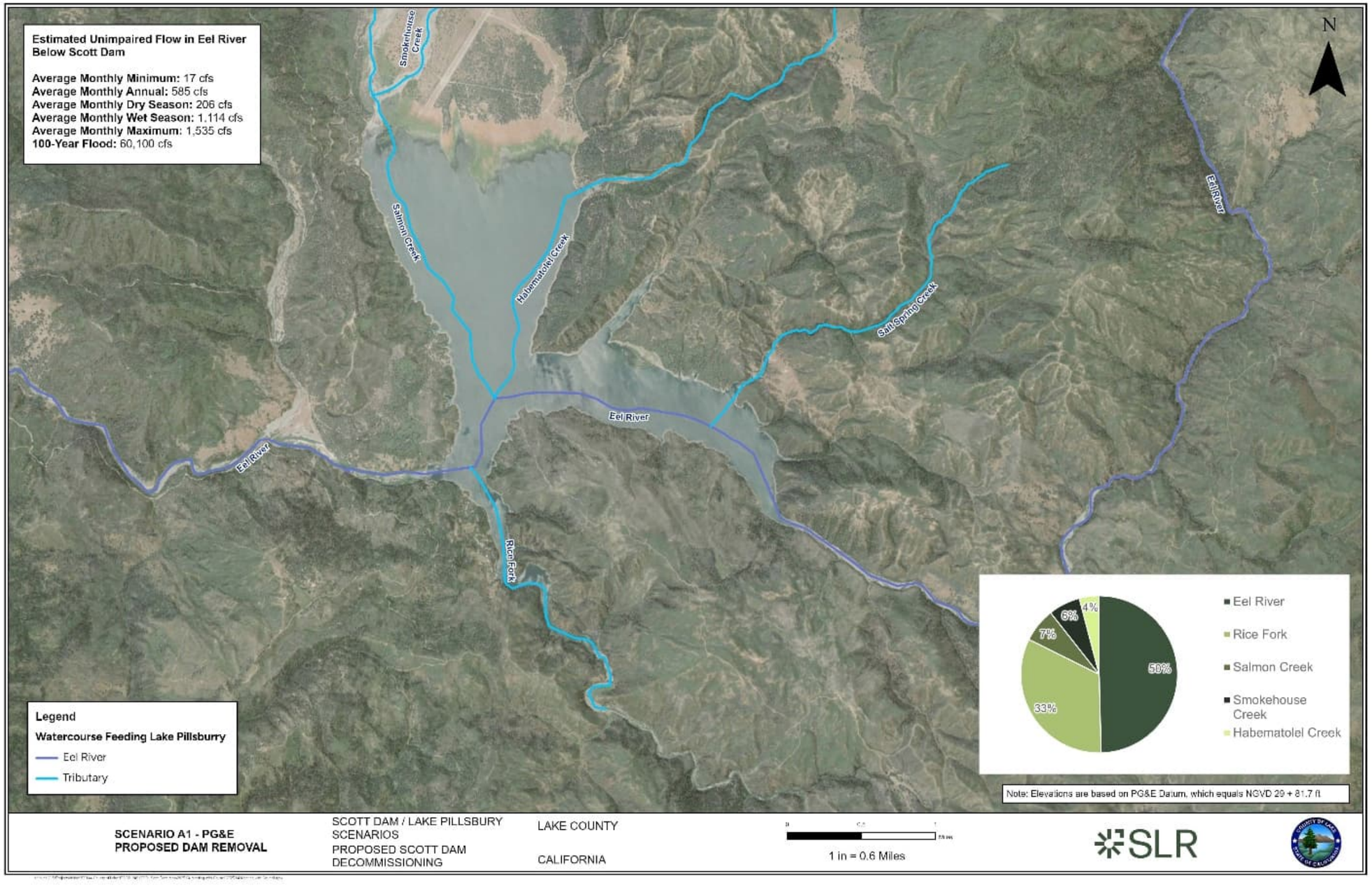
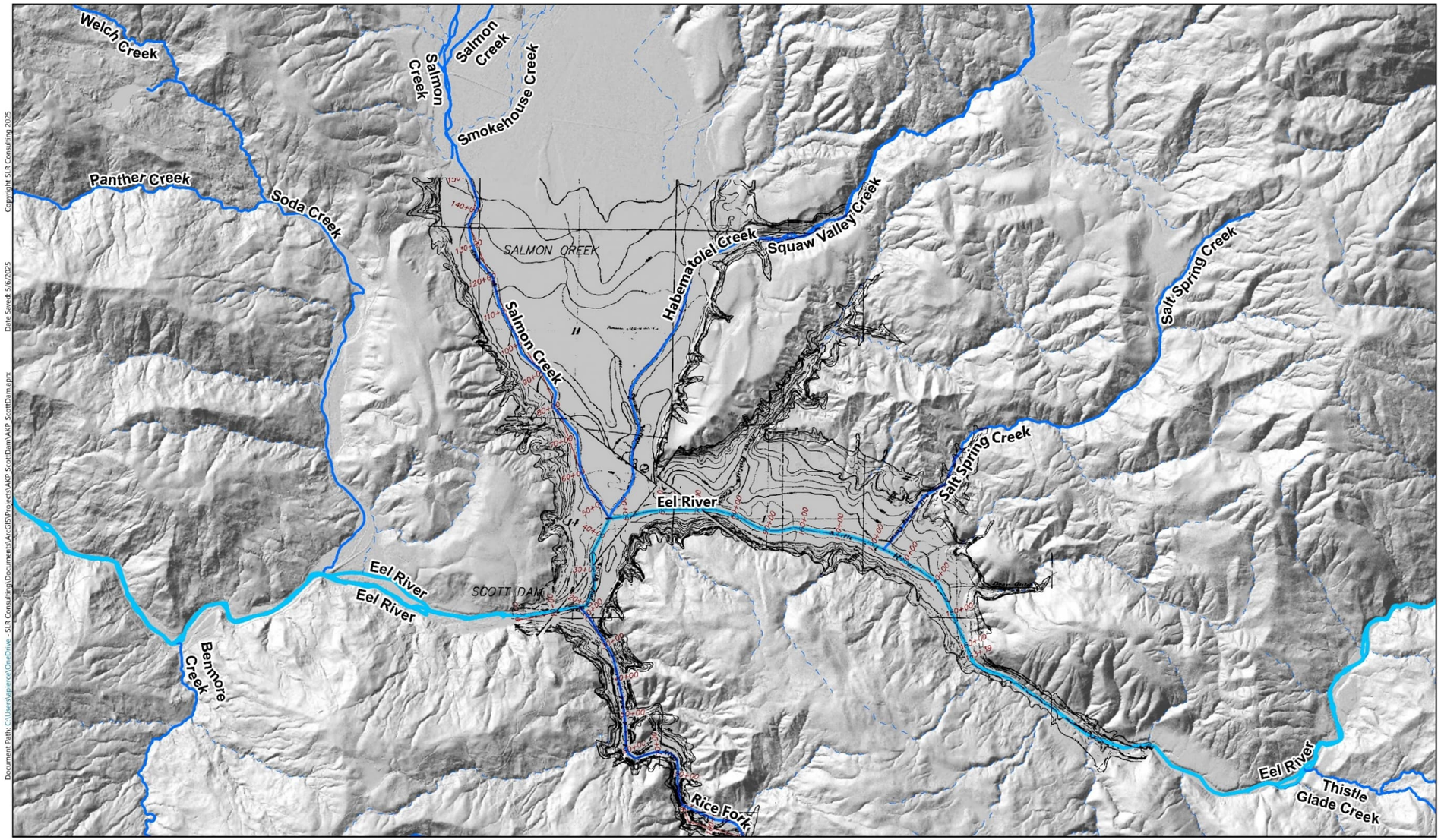


**Figure A7-11: Scenarios A1 and A2 Projected Response Post Dam Removal**




SCENARIO A1/A2 - SCOTT DAM DECOMMISSIONING

SCOTT DAM DECOMMISSIONING ASSESSMENT
COUNTY OF LAKE

**LEGEND**

- Rivers
- Creeks
- Seasonal Creeks

Hillshade
High : 254
Low : 0

1922 Pillsbury Bathymetry
Red: Band_1
Green: Band_2
Blue: Band_3

0 0.5 1
MILES
1 inch = 0 feet
1:30,000

N

SLR
22118 20TH AVE SE
STE. G202
BOTHELL, WA 98021
425.402.8800

## A7.4 Scenario A2 – Scenario A1 with Additional Mitigation

Under this scenario, PG&E proposes the full decommissioning of Scott Dam and the County of Lake proposes additional mitigation efforts implemented to address their concerns and those of the local community. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the County of Lake, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and would have higher ecosystem benefits to the community.

### A7.4.1 Hydrologic and Hydraulic Analysis

The changes in hydrology are expected to be the same as in Scenario A1, see Section A7.3.1. Refer to the unimpaired flows shown in Figure A3-4, Figure A7-8, and Figure A7-9 and the statistics summarized in Table A7-9 and Table A7-10. The only change would be the rate at which the channel and sediment would reach quasi-equilibrium, and flows would normalize. The Scenario A1 Eel River system prediction for reaching a semi-stable state is dependent upon flows and could take years to decades, whereas active restoration and sediment management could expedite this process to within 5 to 10 years post-dam removal.

### A7.4.2 Projected Scenario Outcomes

Scenario A2 projects the same outcomes as Scenario A1 for water supply with the main difference being the rate at which natural or pre-dam conditions flows will be attained. Figure A7-10 and Figure A7-11 will similarly apply when predicting the changes to the Eel River system upstream of Scott Dam upon scenario completion.

## A7.5 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

This scenario is similar to the current operational conditions described in Section A7.2, with the addition of a fish pathway, as previously evaluated by Mead and Hunt (Sullivan, 2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, a seismic retrofit would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures. The hydrologic and hydraulic evaluation is likely similar to the current condition's scenario, but specific hydrology analysis will need to be performed to accommodate a fish pathway to the existing Scott Dam.

## A7.6 Scenario C – Lowered Scott Dam with Fish Ladder

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 feet (PG&E datum) and the installation of a fish passage system,

such as a fish ladder structure. The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 feet would create a reservoir of approximately 40 percent of the current storage volume at WSE 1,900 feet. Potentially, the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury WSEs and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage as the current maximum operational WSE of 1,900 feet, the spillway invert may be further reduced in height by 25 feet below the existing spillway invert, which would result in a new lake footprint shown in Figure A7-16. At that reduced maximum operational WSE, the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam abutment area (where previous slides have occurred) by placing the concrete material on both or either of the downstream and upstream sides of the lowered dam. Further evaluation would be required to understand the effects on the adjacent land area and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation, and ecosystem benefits as compared to the full decommissioning.

### A7.6.1 Hydrologic and Hydraulic Analysis

Hydrologic analysis for the lowered Scott Dam with a fish ladder was conducted using the water balance model developed by SLR. This scenario assumed that the proposed lowered spillway would be constructed such that similar outflow rating curve performance could be able to be achieved compared to the current existing spillway. The lowered Scott Dam spillway would not be actively operated using gates, and instead outflow would be passive when the WSE is above the invert elevation. The assumed maximum operational WSE would be lowered to 1,875 ft, resulting in a storage capacity of 20,453 ac-ft. It is assumed that the lowered Scott Dam would possess an updated low-level outlet structure that would be able to operate at low WSEs. The updated low-level outlet structure is assumed to have similar outflow rating curve performance compared to the existing needle valve structure. The minimum low-level outlet outflow is assumed to be 20 cfs to maintain minimum ecological flow requirements. The new fish ladder structure is assumed to operate during two seasons in the water year. The first season is during November-December, and the second season is during April-May. The fish ladder is assumed to operate continuously during the season at a rate of 55 cfs.

The inflow rates to Lake Pillsbury developed from the USGS data mass balance (WY 1957-2024) were modeled for the operational conditions of this scenario for Scott Dam and Lake Pillsbury to project outcomes for outflow and storage. The resulting water balance model storage and WSE values in Lake Pillsbury are presented in Figure A7-12 and Figure A7-13. The resulting water balance model outflow values from Lake Pillsbury to the Eel River below Scott Dam are presented in Figure A7-14. A comparison of the modeled water balance storage values in Lake Pillsbury for the current operational conditions at spillway elevation 1,900 ft and the lowered Scott Dam with fish passage conditions at the lowered spillway elevation of 1,875 ft are presented in Figure A7-15.

**Figure A7-12: Lake Pillsbury storage water balance model results for lowered Scott Dam with fish ladder**

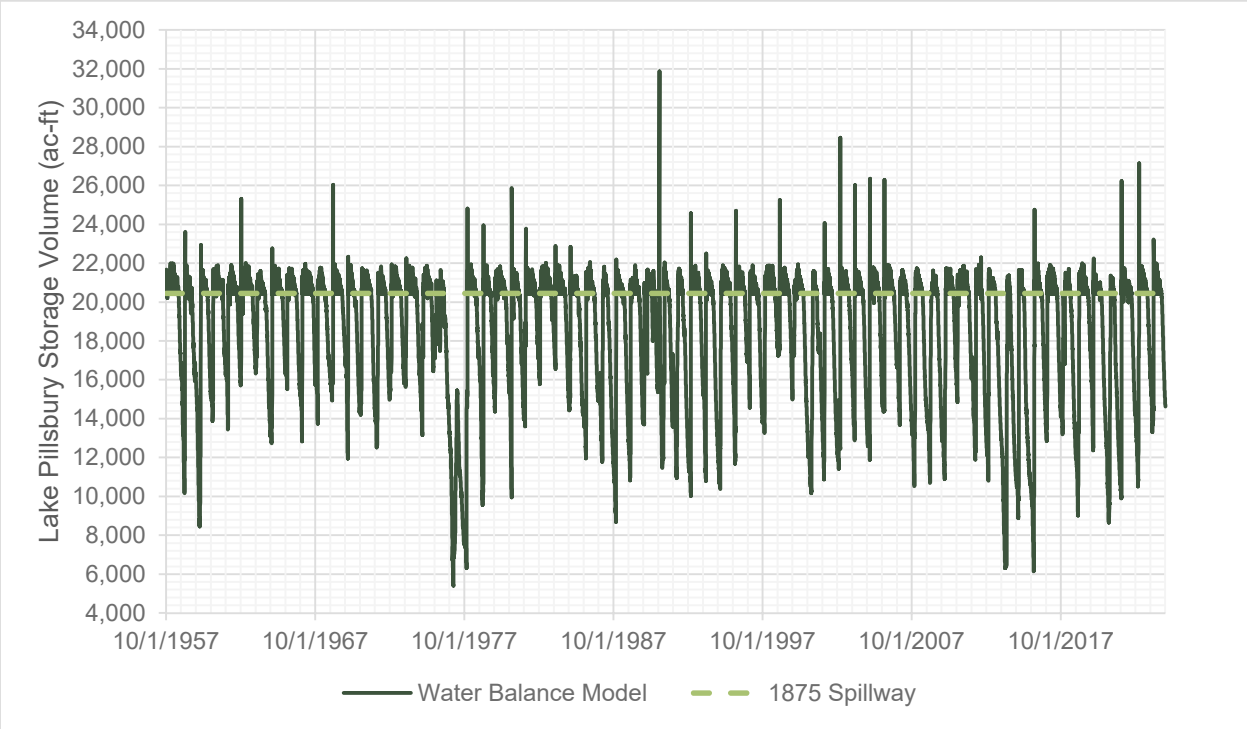



**Figure A7-13: Lake Pillsbury WSE water balance model results for lowered Scott Dam with fish ladder**

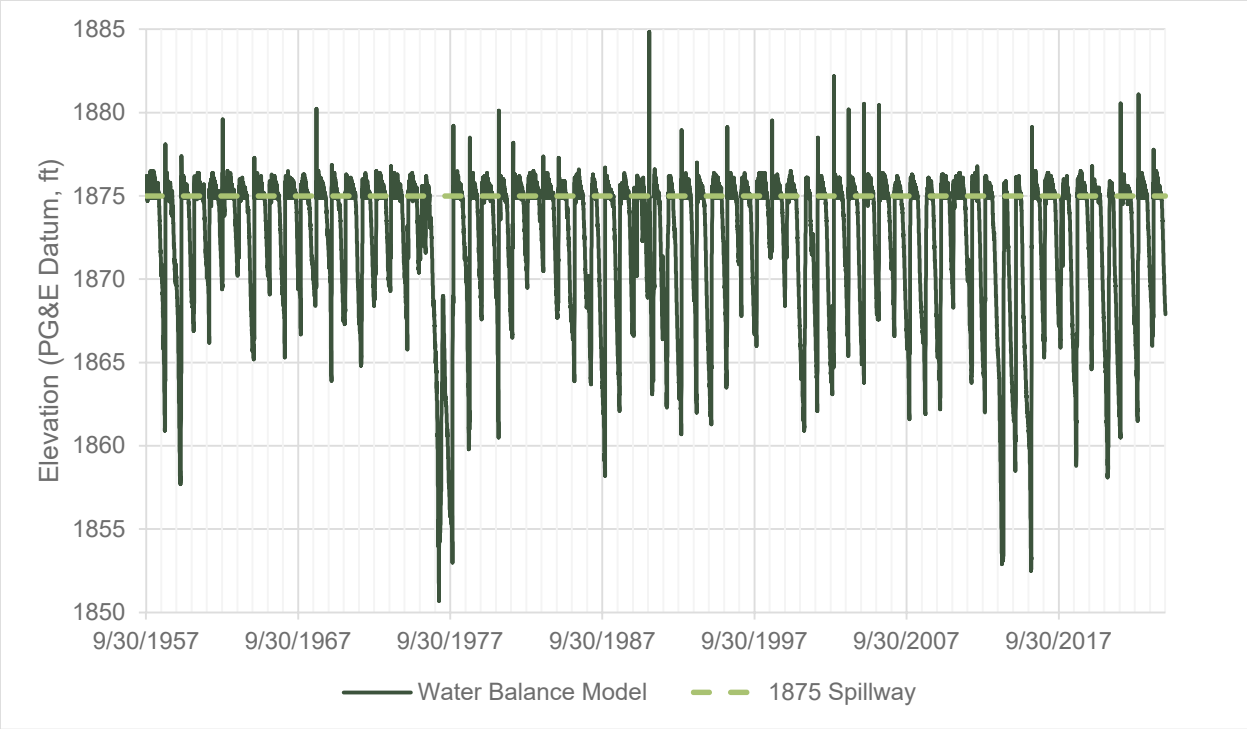


**Figure A7-14: Lake Pillsbury outflow water balance model results for lowered Scott Dam with fish ladder**

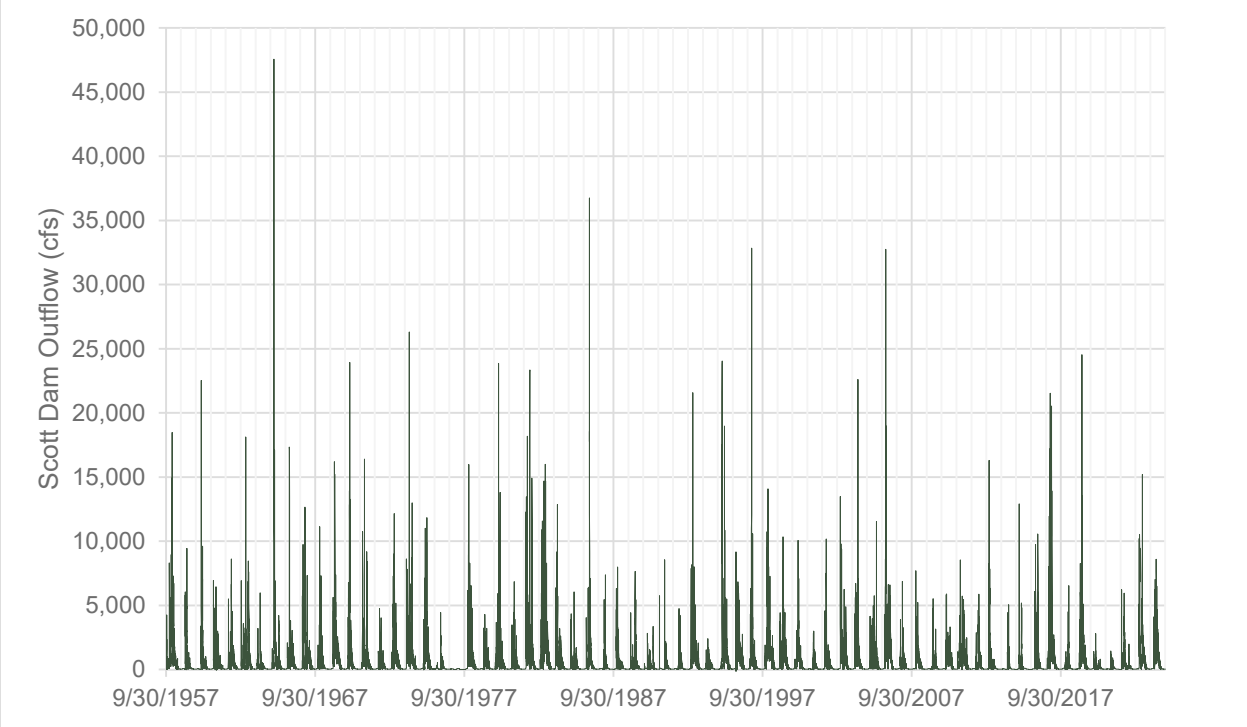
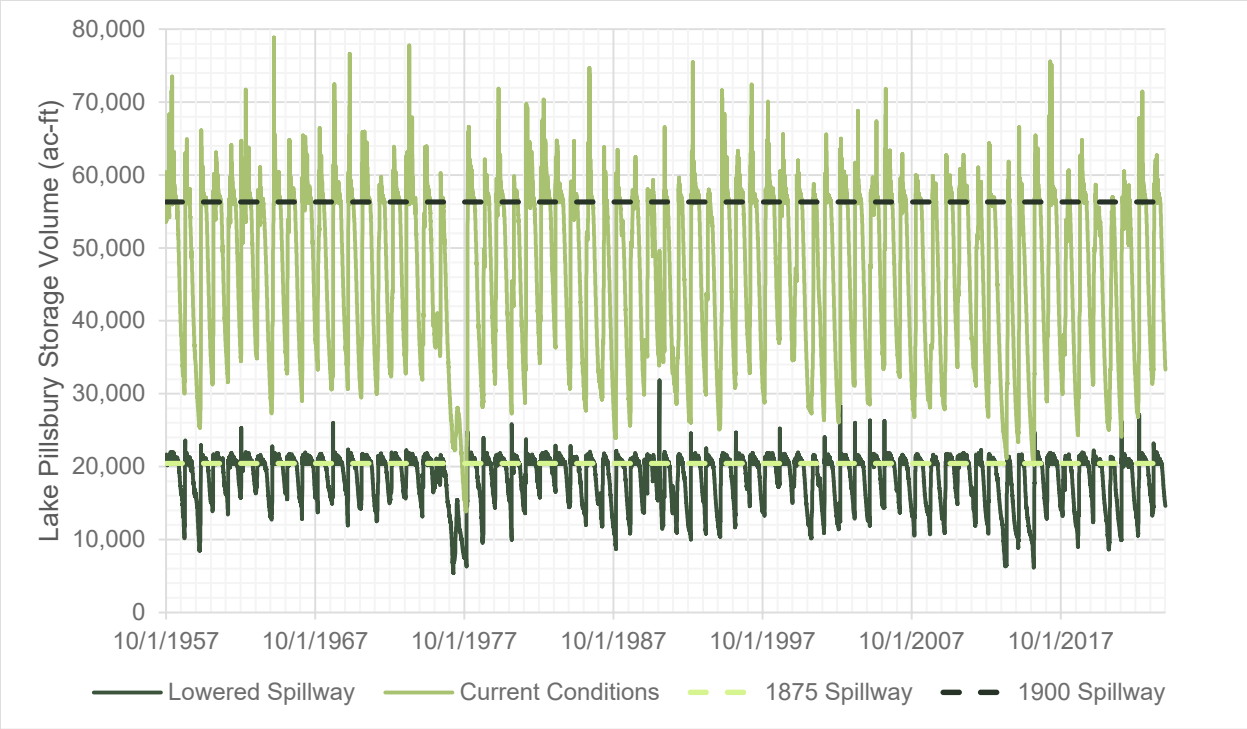



**Figure A7-15: Lake Pillsbury storage comparison of water balance model results for lowered Scott Dam with fish ladder and current conditions**



### A7.6.1.1 Wet and Dry Season Conditions

The mass balance results generally demonstrate a repeating water year cycle. During the dry season, storage within Lake Pillsbury and outflow from Scott Dam are decreasing. The Lake Pillsbury WSE is generally dropping below the lowered dam spillway invert, and thus outflow is through the low-level outlet. During the wet season, storage within Lake Pillsbury and outflow from Scott Dam are increasing. The Lake Pillsbury WSE rises to the lowered dam spillway invert and outflow is through both the low-level outlet and spillway. Periodic larger inflow during the wet season momentarily increase storage within Lake Pillsbury as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,875 ft. The long-term overall trend of water storage in Lake Pillsbury is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period is presented in Table A7-11 and visually shown in Figure A7-17.

**Table A7-11: Lowered Scott Dam with Fish Ladder – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam by Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 5,420 | 1850.7 | 20 | 7,340 | 1855.3 | 20 |
| Average | 18,760 | 1872.7 | 1,116 | 18,080 | 1872.1 | 220 |
| Maximum | 28,460 | 1882.2 | 47,470 | 31,840 | 1884.8 | 14,923 |

### A7.6.1.2 Flood and Drought Conditions

Projecting scenarios C's response to flood and drought conditions can only be estimated based on the water balance model using the gaged data and stage storage curve assumptions. The computed minimum and maximum flow and storage can be used to approximate the response, but further hydrologic and hydraulic analysis would be recommended to validate the assumptions. The water balance summaries for the flood and drought response are summarized in Table A7-12 and Table A7-13 based on the maximum (flood) and minimum (drought) calculated storage and outflows. The lowest storage is predicted to occur during the wet season and the lowest flows are purportedly the same during the wet and dry season. Though outflows are anticipated to be lower than 20 cfs during drought conditions.

**Table A7-12: Lowered Scott Dam with Fish Ladder – Calculated Maximum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Flood Events**

| Source | Flood | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Water Balance | 31,840 | 1884.8 | 47,470 |

**Table A7-13: Lowered Scott Dam with Fish Ladder – Calculated Minimum Storage within Lake Pillsbury and Outflow Downstream of Scott Dam in Response to Extreme Drought**

| Source | Drought | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Water Balance | 5,420 | 1850.7 | 20 |

## A7.6.2 Projected Scenario Outcomes

The lowered Scott Dam with a fish ladder scenario analyzes modified operations of Scott Dam at a maximum operational WSE of 1,875 ft and the projected potential impacts on water supply.

### A7.6.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury by 35,870 ac-ft by lowering the maximum operation WSE from 1,900 ft to 1,875 ft. Flood control, downstream water diversion flow requirements, and operational reliability would be impacted. Flood control and downstream water diversion flow requirements would be affected due to reduced dam operational control and the passive operation of the spillway. An updated low-level outlet structure that would be able to operate at low WSEs and maintain minimum ecological flow requirements to the Eel River.

This scenario would generally increase flood risk due to the stopping of active operations of the spillway gates, which allowed for greater control, attenuation, and retention of flood inflows. This scenario has reduced drought tolerance due to a lower average WSE and storage in Lake Pillsbury than the current operational conditions. The water supply capacity will continue to be reduced over time due to losses from sediment accumulation. It is recommended that existing sediment be mitigated to increase the available water supply.

The Lake Pillsbury reservoir's ability to recharge the groundwater would likely be reduced. Additional evaluation of the slope stability of the existing slope would need to be considered. Currently, PG&E has maintained a minimum WSE due to slope safety concerns.

This scenario proposes that seasonal fish passage be provided during two seasons per year. The first season would be during November-December, and the second season would be during April-May to meet fish passage requirements. Lake Pillsbury is expected to be able to provide the required flow rates to properly allow fish passage.

**Figure A7-16: Scenario C lowered Scott Dam with Fish Ladder: Conceptual Lake Pillsbury Footprint**

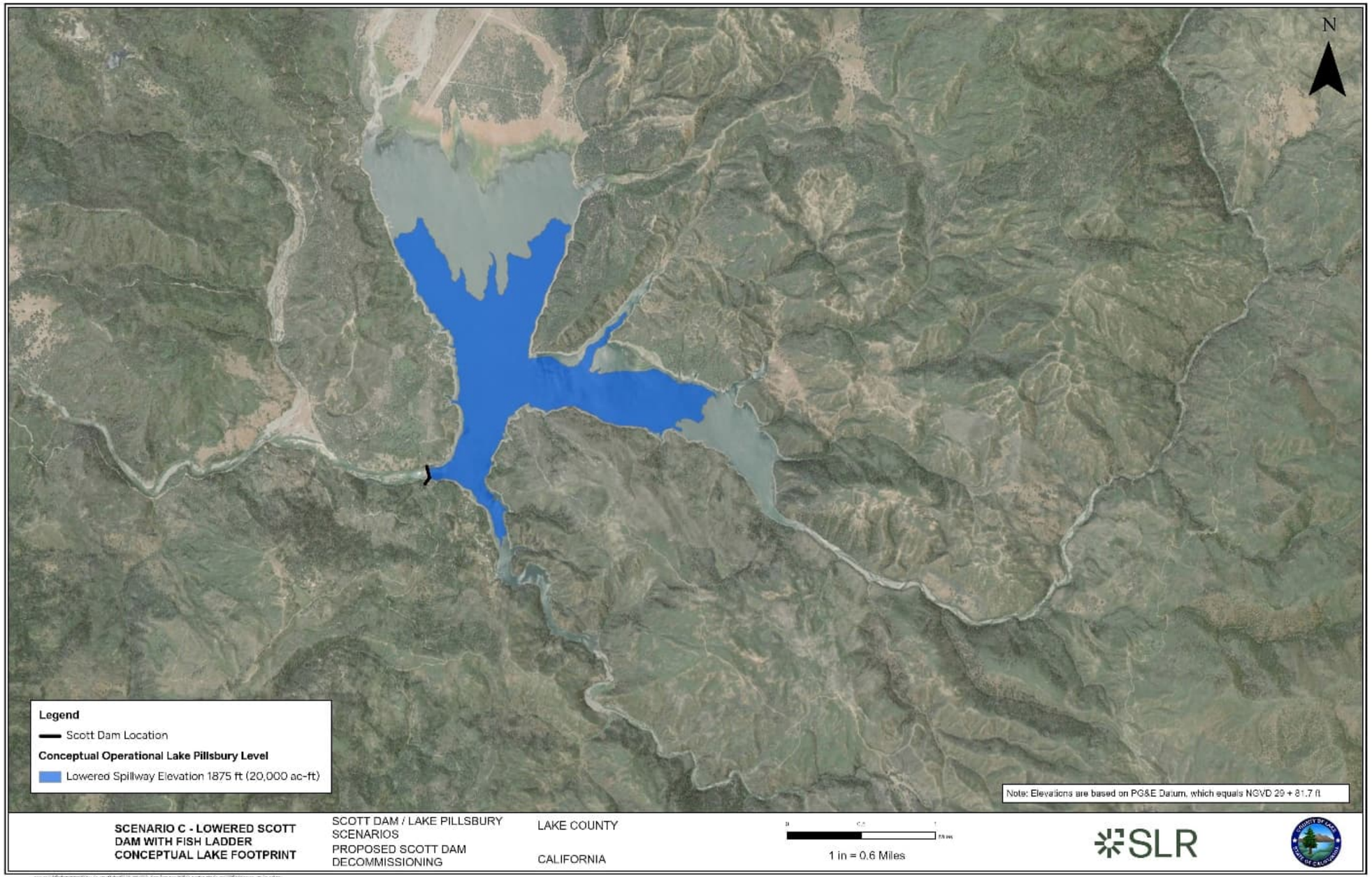
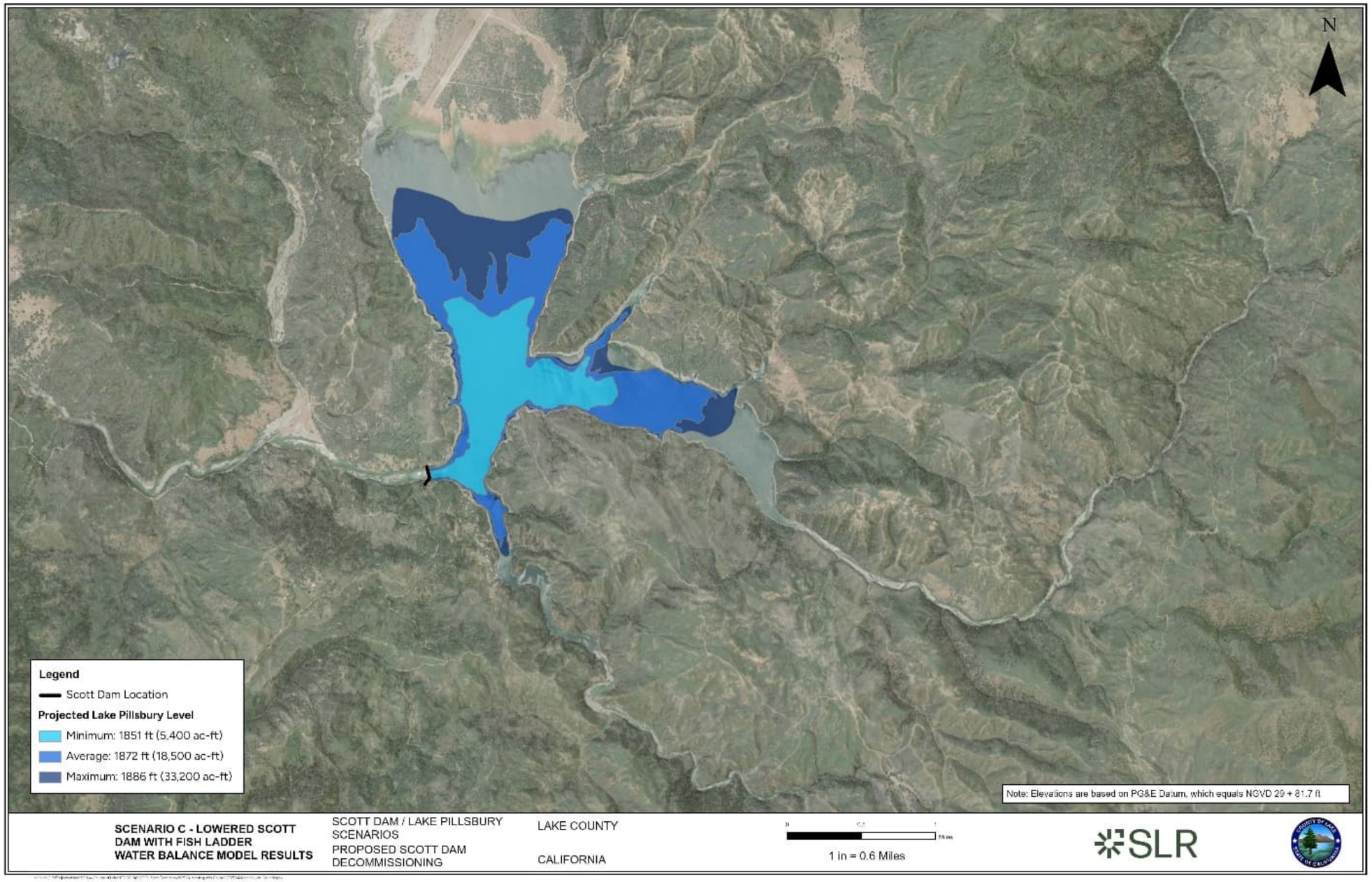


**Figure A7-17: Scenario C lowered Scott Dam with Fish Ladder: Projected Minimum, Average, and Maximum Lake Pillsbury WSE Levels**



## A7.7 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure. A new 3,100-foot long earthen dam would be established in the Salmon/Smokehouse and Habematolel Creek watershed areas to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir with contributions per Figure A7-18. The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction, with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within the Lake Pillsbury footprint can also be reused for the new impoundment construction if suitable. Low-permeability clay/silt sediments can be used in the core of the dam, and gravelly sediments can be used in the embankment/slopes of the new dam.

The water storage will be based on the Salmon/Smokehouse and Habematolel Creek watershed areas, and it is assumed the maximum capacity (approximately 9,990 ac-ft) would be provided at a maximum operational WSE of 1,885 feet as visually depicted in Figure A7-19. For this scenario, the approximate surface area would be 580 acres, approximately 30 percent of the current size (WSE 1,900 ft). The new dam would be constructed at the narrowest location possible that would not impair the flows through the Eel River. The dam would not be actively managed, and outflow would be through a passive spillway structure constructed at the maximum operational WSE. An active low-level outlet structure would not be included, though a passive outlet is considered for potential draining. This option will support continued use of recreation facilities, ecosystem benefits, fire suppression, and the upstream fish pathway along the Eel River, as Scott Dam will be removed as a fish barrier.

**Figure A7-18: Breakdown of percent contributions to the reduced reservoir under Scenario D (reduced reservoir with Salmon Creek as main tributary)**

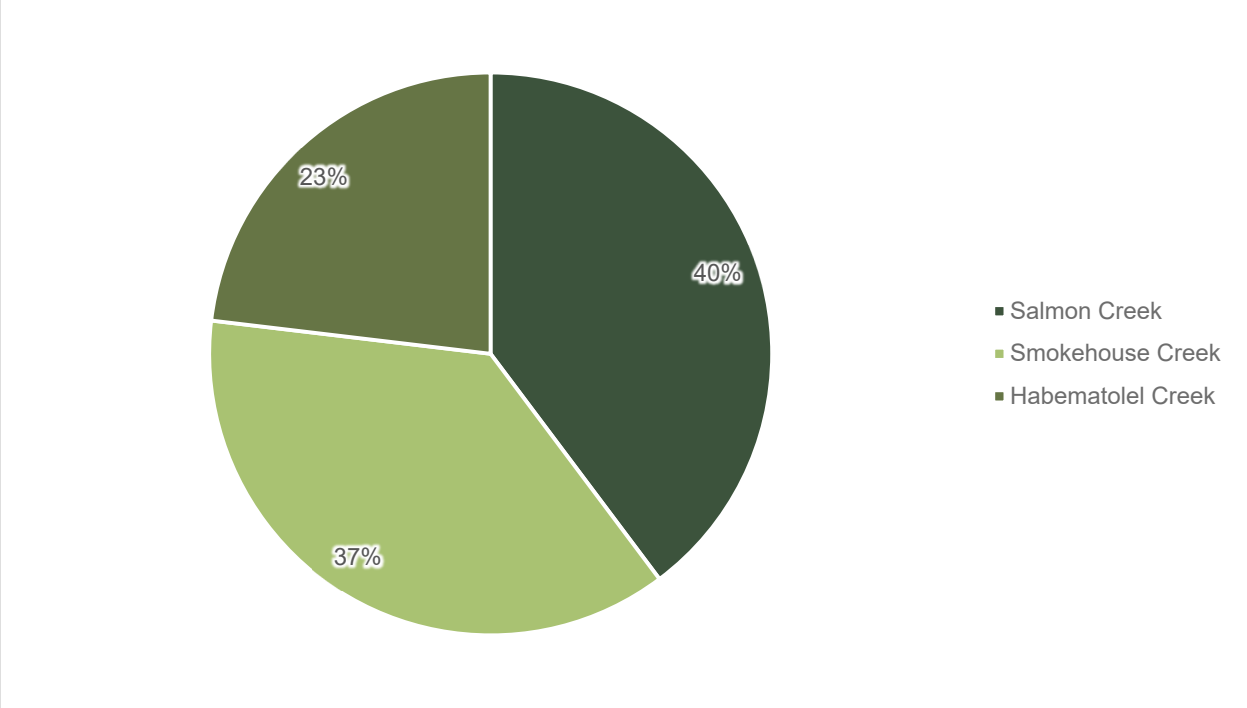
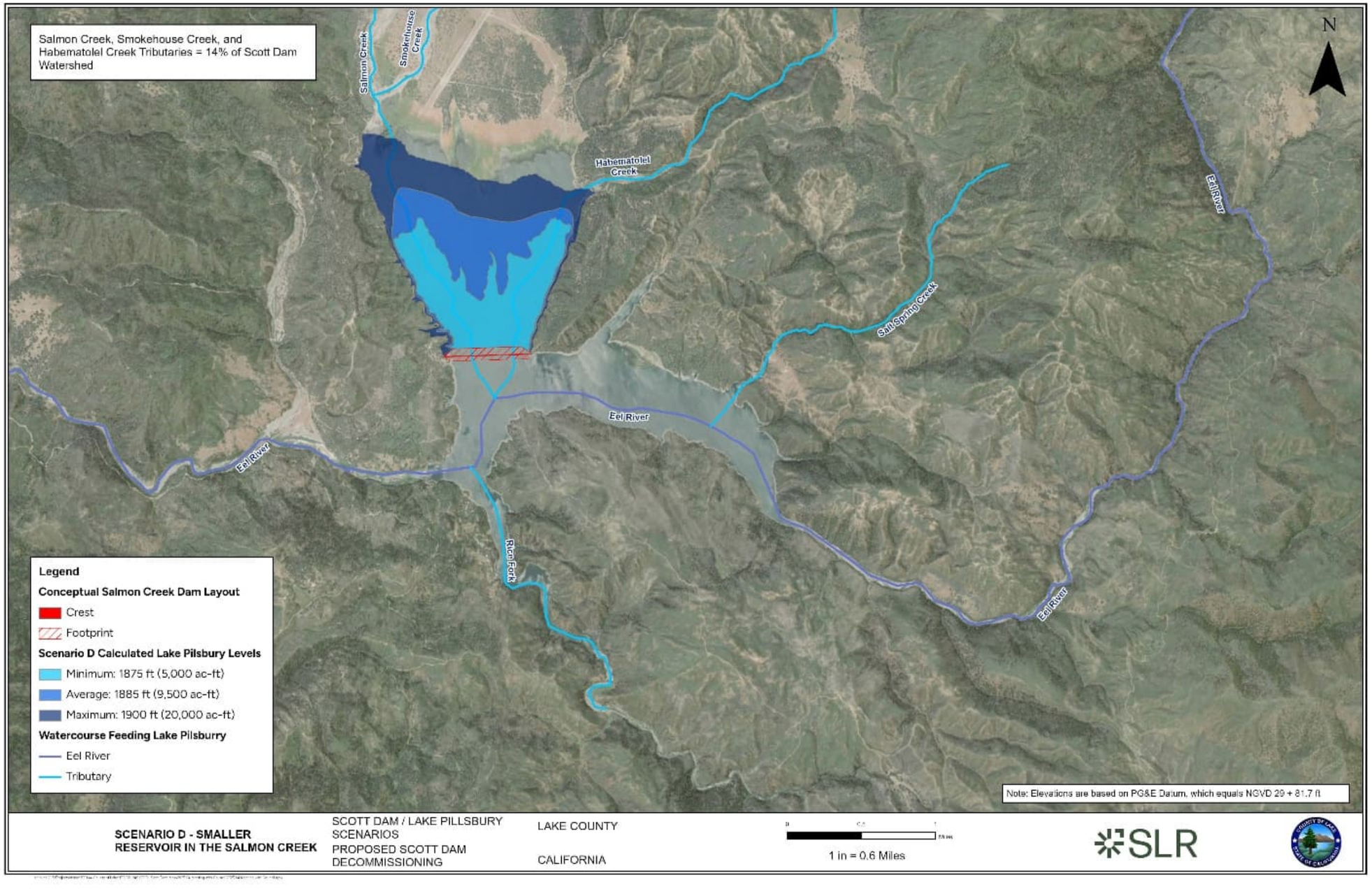


**Figure A7-19: Scenario D smaller reservoir in the Salmon Creek watershed: projected minimum, average, and maximum Lake Pillsbury WSE levels**



### A7.7.1 Hydrologic and Hydraulic Analysis

Hydrologic analysis for this scenario was conducted using the water balance model method originally develop by Cardno and repurposed for the new dam. SLR developed a stage-storage curve for the new dam to project how reducing the reservoir's storage capacity would impact WSE (Figure A7-20). This scenario assumed that only inflow into the reduced reservoir would be scaled from the current existing Lake Pillsbury watershed based on tributary area. Outflow from the reservoir would only occur passively through the spillway when the WSE is above the assumed spillway invert elevation of 1,885 ft. This scenario assumed that the proposed spillway would be constructed along a shorter length than the existing Scott Dam spillway. The analysis scaled the existing Scott Dam spillway rating curve to 25 percent of the outflow performance. No low-level outlet was assumed to be active.

The inflow rates developed from the USGS data (WY 1957-2024) were modeled for this scenario for the reduced reservoir. The resulting water balance model storage and WSE values in the reduced reservoir are presented in Figure A7-21 and Figure A7-22. The resulting water balance model outflow values from the reduced reservoir to the Eel River downstream are presented in Figure A7-23.

**Figure A7-20: Stage-storage capacity curve for the potential reduced reservoir under Scenario D**

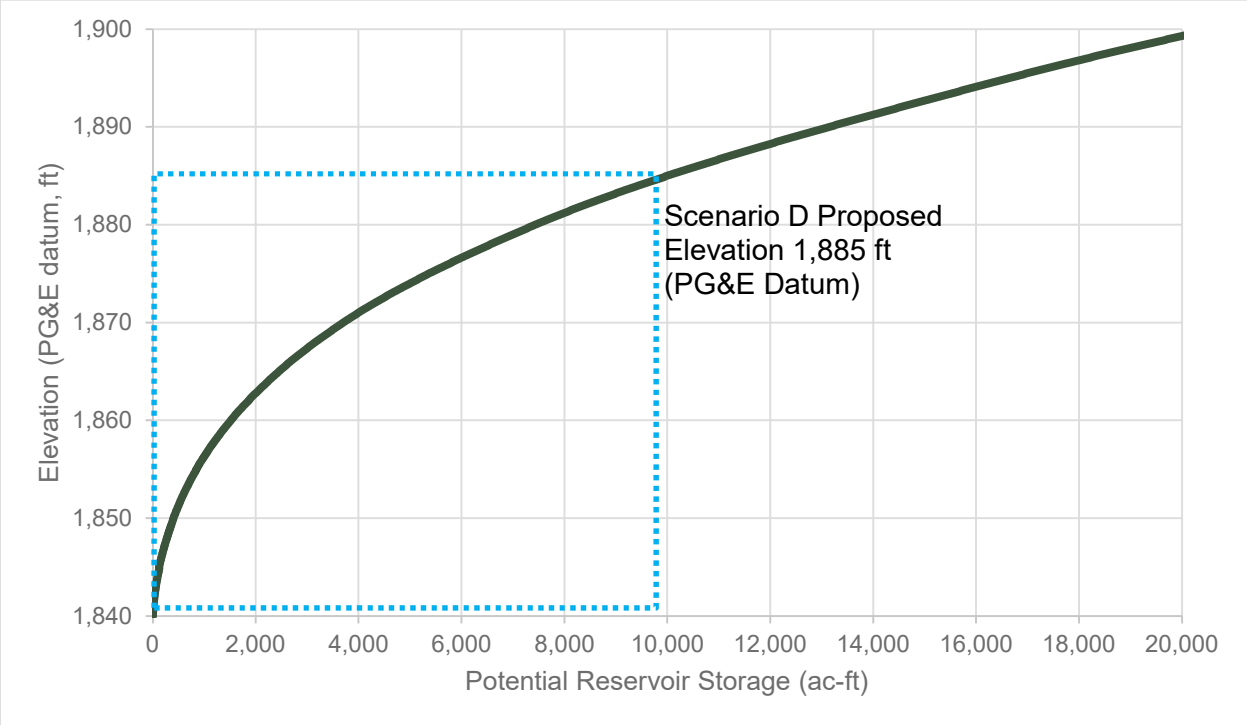


**Figure A7-21: Reduced reservoir with Salmon Creek storage water balance model results**

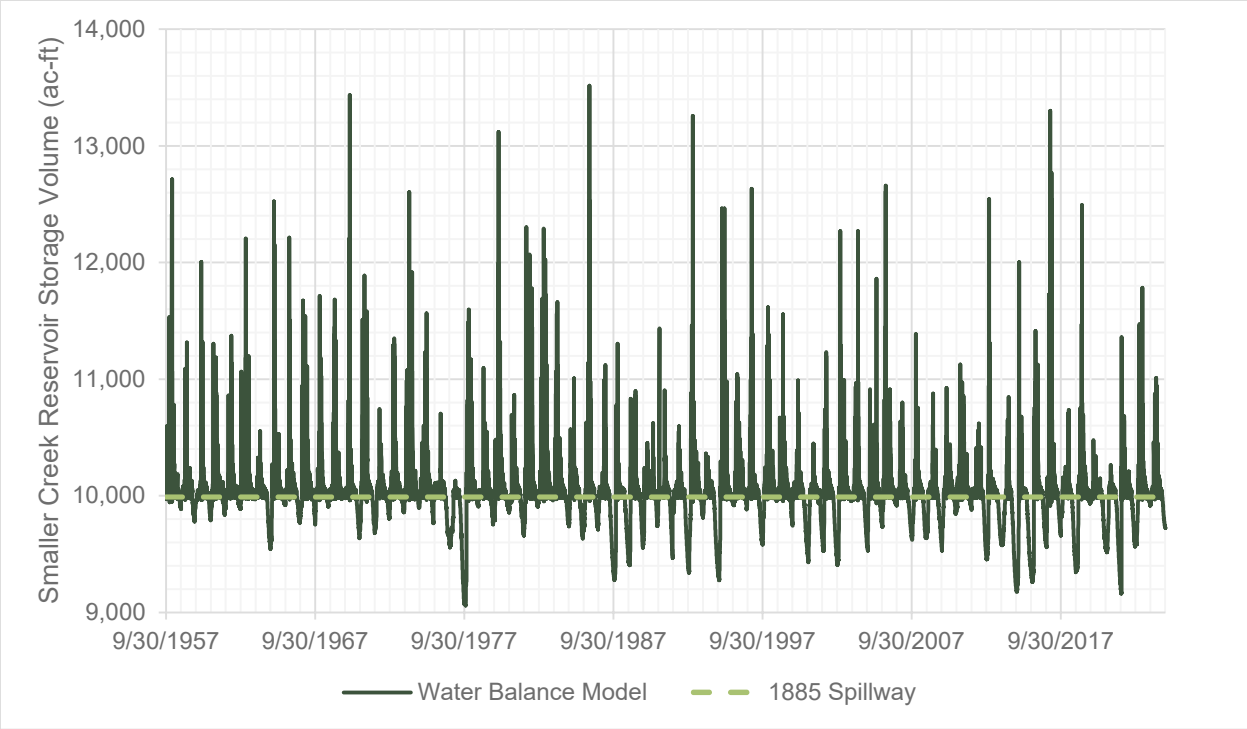



**Figure A7-22: Reduced reservoir within Salmon Creek WSE water balance model results**

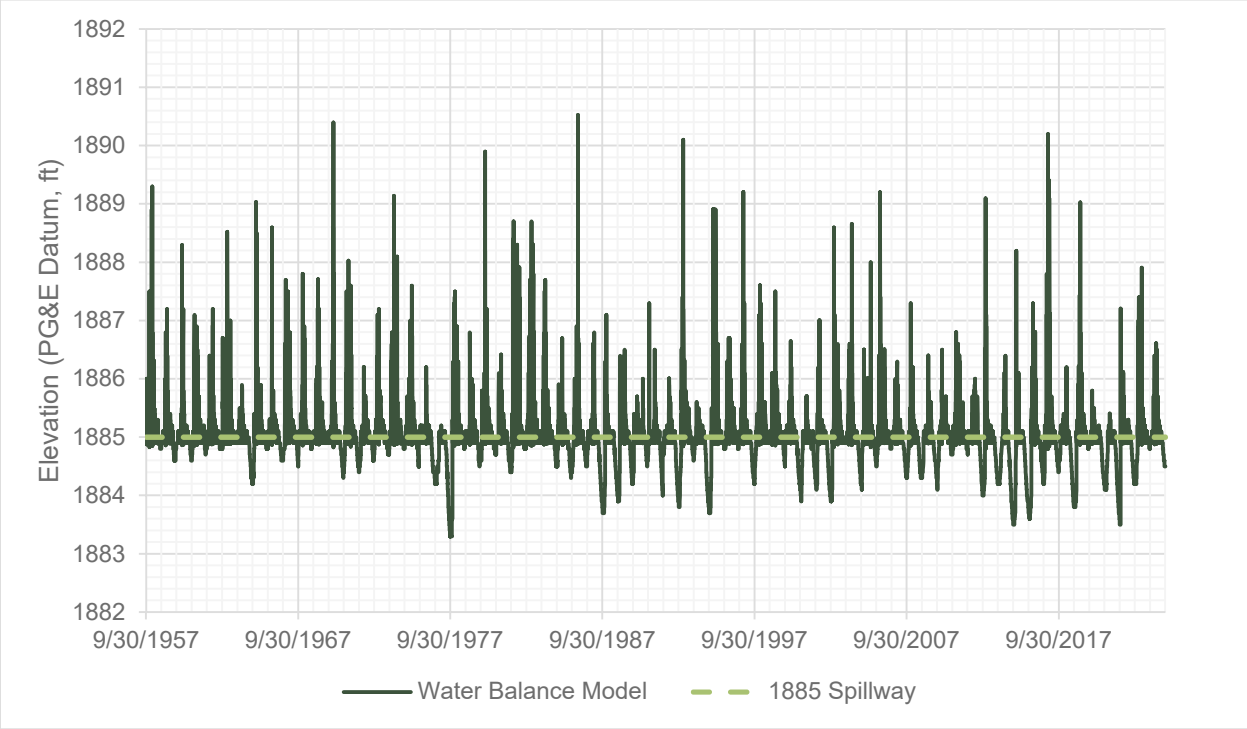
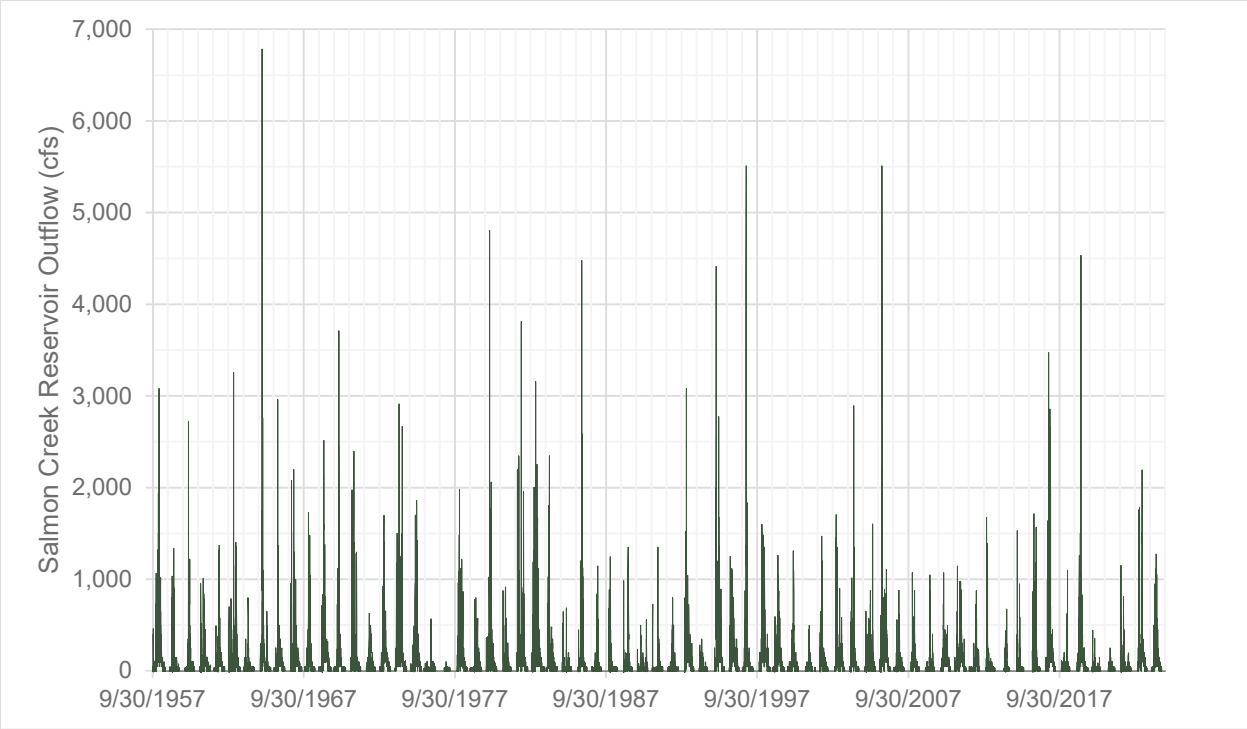


**Figure A7-23: Reduced reservoir within Salmon Creek outflow water balance model results**




### A7.7.1.1 Wet and Dry Season Conditions

The results generally demonstrated a repeating water year cycle. During the dry season, storage within the reduced reservoir will be below the spillway at the end of the dry season. During the wet season, the WSE rises to the dam spillway invert, and outflow occurs through the spillway. Periodic larger inflows during the wet season momentarily increase storage as it passes through the spillway until the WSE stabilizes at the spillway invert elevation of 1,885 ft. There are expected periods of no outflow from the reduced reservoir. The long-term overall trend of water storage in the reduced reservoir is generally constant. A summary of the projected wet and dry season climate conditions covering the modeled period for the reduced reservoir is presented in Table A7-14. The wet and dry season projected statistics for the Eel River immediately downstream of Scott Dam are summarized in Table A7-15 based on unimpaired flows. Table A7-15 does not account for outflows from the new earthen dam spillway.

**Table A7-14: Reduced Reservoir within Salmon Creek – Statistics for Projected Storage within the Reduced Lake Pillsbury Reservoir and Projected Outflow from the New Dam Spillway by Season**

| Statistic | Wet Season (December – April) | | | Dry Season (May – November) | | |
|---|---|---|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 9,100 | 1883.3 | 0 | 9,060 | 1883.3 | 0 |
| Average | 10,110 | 1885.1 | 152 | 9,920 | 1884.8 | 25 |
| Maximum | 13,500 | 1890.5 | 6,750 | 11,860 | 1888.0 | 1,923 |

**Table A7-15: Scenario D Wet and Dry Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 684 | 15 |
| Median | 1,070 | 43 |
| Average | 1,051 | 80 |
| Maximum | 1,327 | 245 |

#### A7.7.1.2 Flood and Drought Conditions

The flood condition within the Eel River downstream of Scott Dam under this scenario could be roughly estimated based on flow contributions and the 1% AEP. The Eel River and Rice Fork contribute approximately 86% of the total unimpaired flows. This would give an approximate 1% AEP flood of 51,686 cfs when assuming zero contribution from the Salmon/Smokehouse and Habematolel Creeks. The storage will likely exceed the spillway WSE and contribute to the flows within the Eel River however the timing could be offset by the dam storage attenuating the flows. Further analysis of the contributions from the new dam spillway during an 1% AEP event and more in-depth hydrologic analysis would be recommended to refine this estimate.

Under a drought condition the storage within the reservoir has the potential to drop precipitously as there is not a consistent flow contribution and of much lower magnitude than the Eel River. The drought conditions for the Eel River downstream of the Scott Dam can only be assumed lower than the minimum projected values per Table A7-15 during the dry season of 15 cfs.

### A7.7.2 Projected Scenario Outcomes

The reduced reservoir scenario analyzes a new dam at a maximum operational WSE of 1,885 ft and the projected potential impacts on water supply.

#### A7.7.2.1 Water Supply

This scenario would reduce water storage within Lake Pillsbury to only include inflow contributions from the Salmon/Smokehouse and Habematolel Creeks watershed areas to maintain a smaller reservoir. The maximum operational WSE at 1,885 ft would produce a storage volume of approximately 9,990 ac-ft which results in an approximate reduction of 80 percent of the current Lake Pillsbury storage volume at WSE 1,900 ft. The new dam is expected to require minimal operations as the outflow would occur passively through the spillway to the Eel River when the WSE is above the spillway invert. A low flow outlet is recommended to remain inactive unless drawdown is required. The drastic changes in the impoundment WSEs are expected to occur as the Habematolel Creek is the only perennial watercourse but has an overall lower contribution of flows when compared to the Salmon/Smokehouse Creeks, which are intermittent. Flood control would be reduced and limited to the smaller reservoir tributary. The storage feasibility needs to be evaluated, as the alluvial deposits in the north area are not well understood. Geotechnical feasibility needs to be evaluated due to the faults in the area.

Flood control and downstream flow diversion requirements would be unreliable and seasonally dependent. The Eel River and Rice Fork tributaries would be restored to unimpaired flow conditions as presented in Figure A7-19 and Table A7-15. Flood control and downstream water

diversion flow requirements would be affected due to the majority of the original Lake Pillsbury watershed being restored to unimpaired flow conditions. Water storage within the new impoundment will be dependent upon the sediment accumulations and further evaluation will be required to determine the accumulation rates from the Salmon/Smokehouse and Habematolel Creeks.

The Scenario D wet and dry seasons will have a reduced storage volume and sustainability when compared to the baseline scenario. There will be no controlled outlet flows but also less inflows. Outflows and inflows will vary depending on the season, flood, and drought conditions and will be affected significantly by evaporation and infiltration. The scenario's reaction to extreme climate conditions will also vary from the baseline. The flood flows will likely see an increase, but the drought flows will likely reduce on average, though a more detailed hydrologic analysis is recommended to validate these assumptions.

## A7.8 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2, but with specific mitigation added to enhance fish passage while maintaining fire suppression and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[14], the off-channel ponds would be constructed.

Two potential pond locations have been identified as shown in Figure A7-24 based upon the 1922 historic pre-dam topography and bathymetry (McBain Associates, 2018). The 1922 topography and bathymetry was georeferenced to the Lake County hillshade to predict how the site will look after restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table A7-16 summarizes the potential footprints and volumes of Ponds A and B assuming a 10-foot normal depth. The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design. The conceptual design would function similarly to the example provided in Figure A7-25, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table A7-16: Potential Pond Footprint and Storage Volume of Ponds A and B under Scenario E**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |

[14] Quasi-equilibrium in this case refers to the ability of a river to maintain its dimension, pattern and profile without either aggrading or degrading over time.

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

**Figure A7-24: Scenario E - Dam Removal with Off-Channel Pond Restoration**

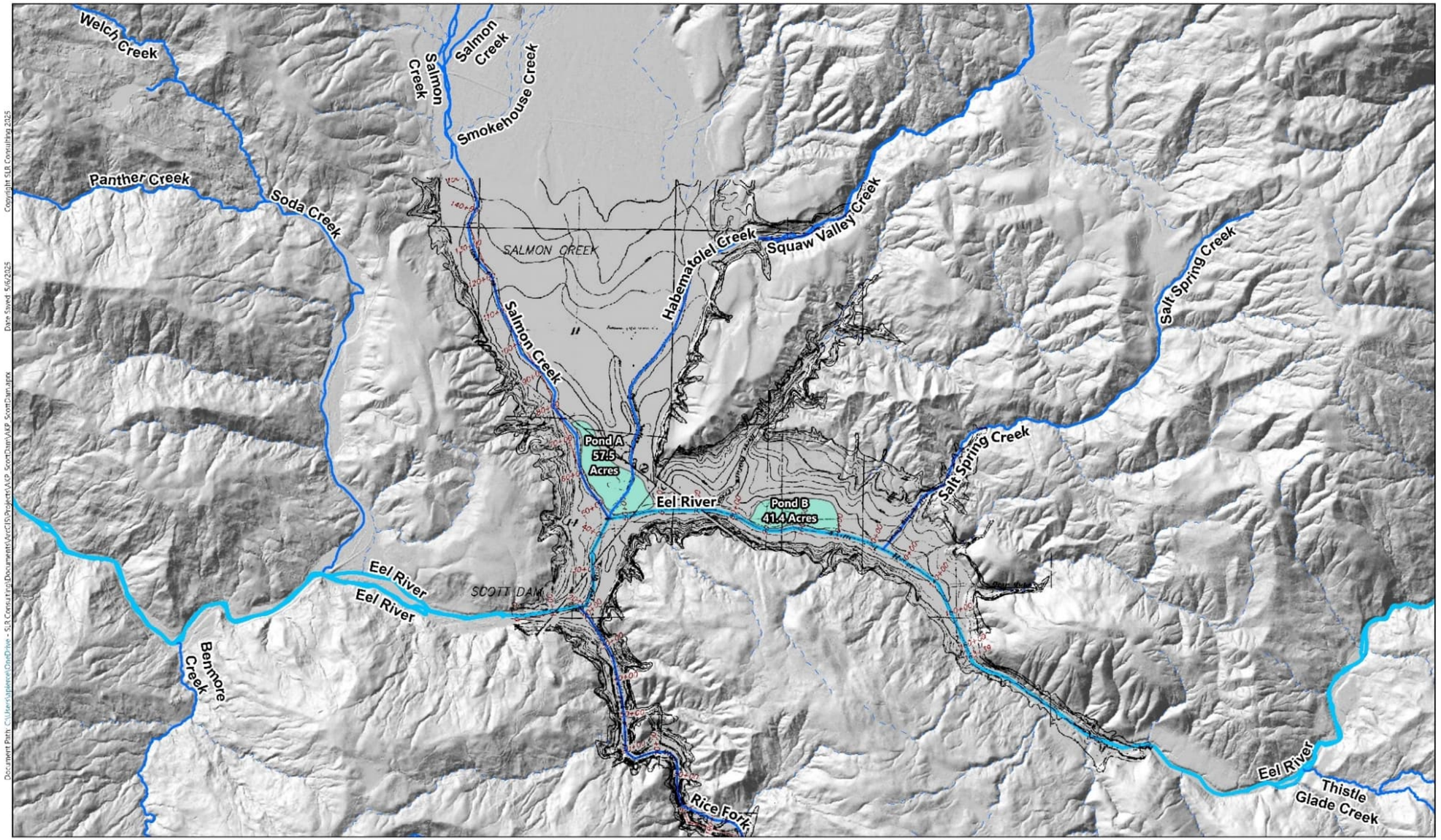
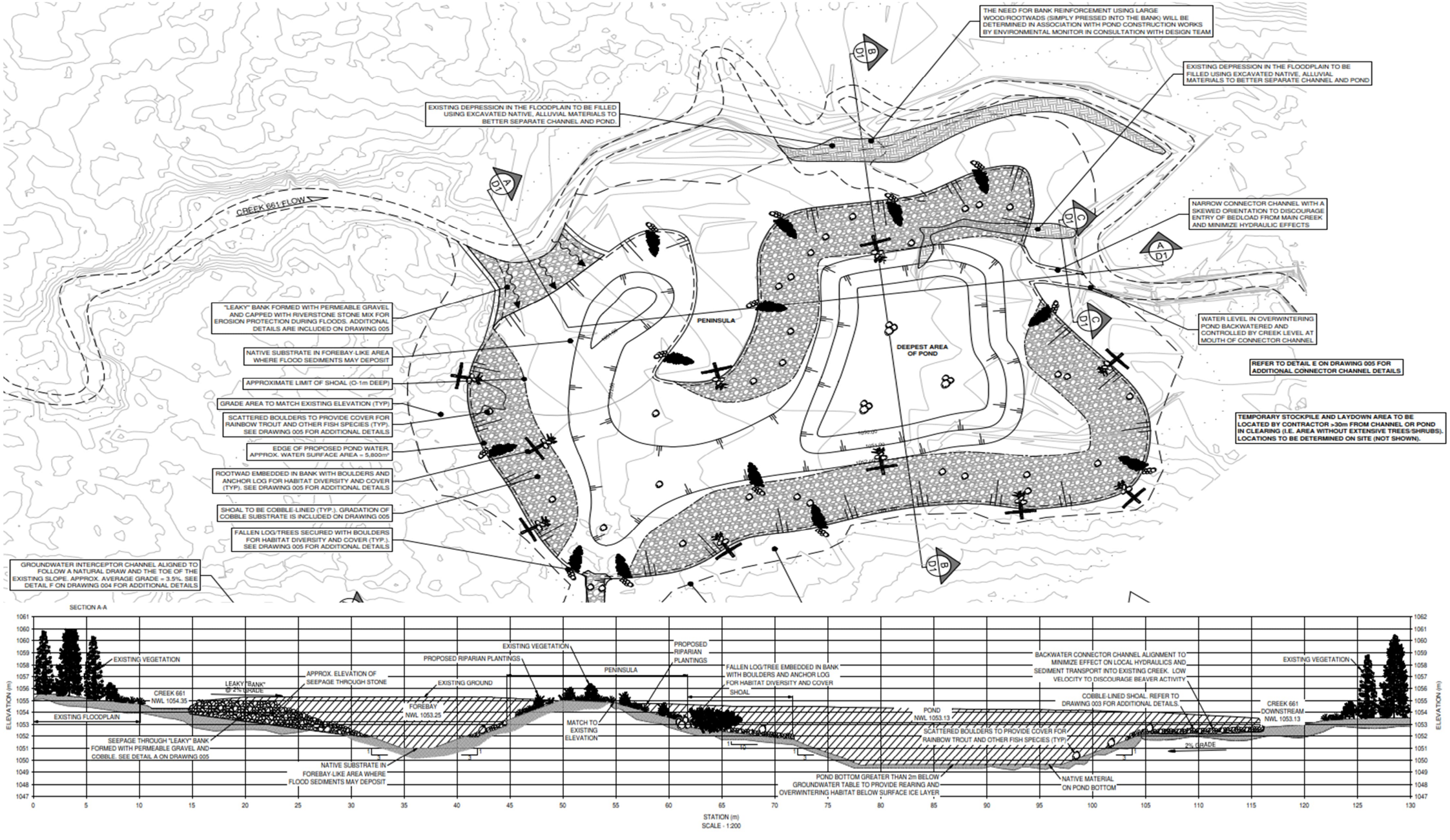


**Figure A7-25: Example Design of Off-Channel Pond**



### A7.8.1 Hydrologic and Hydraulic Analysis

The analysis for this scenario uses the analysis for the dam removal A1/A2 scenarios in terms of the climate conditions. The ponds will allow for water retention and will reduce outflows from the Eel River projected downstream of Scott Dam.

#### A7.8.1.1 Wet and Dry Season Conditions

Under this scenario, it is assumed that both Pond A and B will be constructed. Pond A will be fed by all inflow from the Habematolel Creek and up to 10% of the Salmon and Eel River during normal wet and dry seasons. Pond B assumes a contribution of an additional 10% of flows from the Eel River, meaning both ponds will take up to 10% of the Eel River flows each. Figure A7-26, Figure A7-27, and Table A7-17 summarize the adjusted average monthly predicted unimpaired flows downstream of Scott Dam for the wet and dry season by removing the assumed contributions from both Pond A and B. This scenario will reduce the total flow contribution from the Eel River downstream of Scott Dam by up to 14% when compared to Scenarios A1 and A2 during normal wet and dry seasons. This value vary depending upon the climate conditions and increase or decrease depending on the goals of the design, infiltration rates, groundwater tables, and pond usage.

**Table A7-17: Dam Removal Scenario E Wet and Dry Season Monthly Average Statistical Unimpaired Flows Downstream of Scott Dam**

| Statistic | Wet Season (December through April) | Dry Season (May through November) |
|---|---|---|
| | Average Monthly Flow (cfs) | Average Monthly Flow (cfs) |
| Minimum | 682 | 15 |
| Median | 1,067 | 43 |
| Average | 1,048 | 80 |
| Maximum | 1,324 | 244 |

**Figure A7-26: Unimpaired Dry Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse under Scenario E**

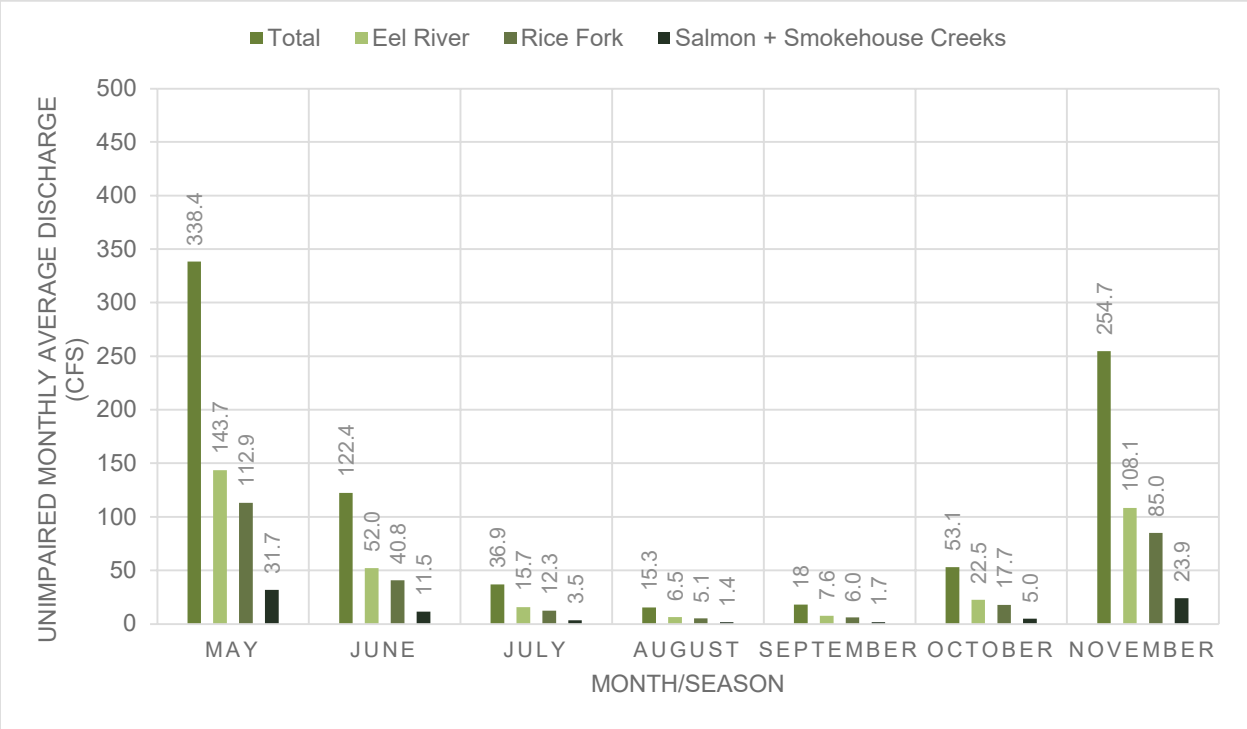
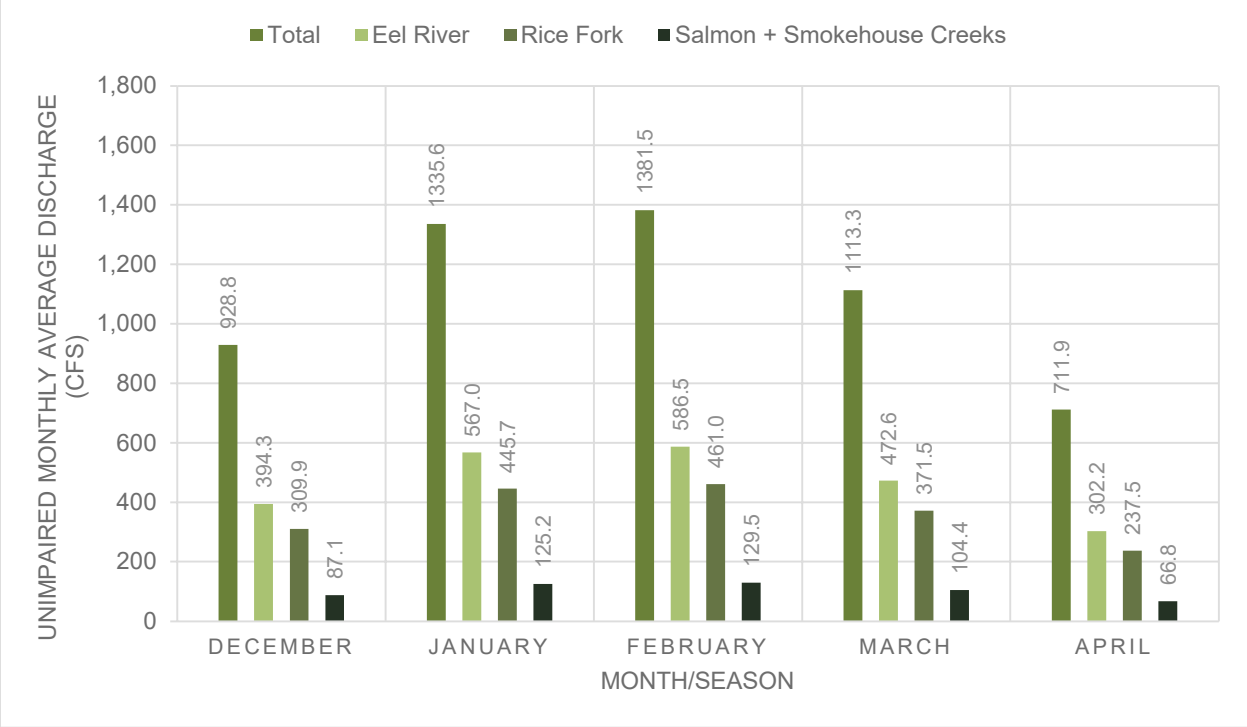



**Figure A7-27: Unimpaired Wet Season Monthly Average Flows Contribution Downstream of Scott Dam by Watercourse under Scenario E**



#### A7.8.1.2 Flood and Drought Conditions

This scenario is not likely to drastically change flows during the extreme flood conditions summarized in Table A7-10 as the ponds will be located within the floodplain. The restoration and the off-channel ponds design is recommended to be resilient against the predicted design flows (1% AEP flows). Drought conditions would likely see a reduction from the Scenarios A1 and A2 predictions but are difficult to quantify or predict with the currently available data. The reduction or impacts to the extreme climate conditions would be dependent upon the infiltration rates, design goals, usage (fire suppression), and groundwater tables. Developing a resilient design to recover from extreme flood and drought conditions would be recommended and is viable from an ecological perspective. Resiliency in providing a consistent water supply can be more difficult to guarantee under this scenario. However, this option provides the highest potential benefit to water supply when compared to the other scenarios where the dam is being removed.

### A7.8.2 Projected Scenario Outcomes

The predicted scenario outcomes are similar to Scenario A2 but with additional benefits and outcomes with the inclusion of the ponds.

#### A7.8.2.1 Water Supply

The water supply downstream for this scenario is at a reduced benefit than Scenarios A1 and A2 due to a percentage of the flows being retained by the ponds. The storage availability is increased from the Scenarios A1 and A2 but significantly lower than the baseline. This scenario may reduce flood risk from storage retention but cannot be quantified with the information available. A flood model of the Eel River upstream and downstream of the Scott Dam location would need to be developed to adequately determine the potential flood risk. This scenario may provide water during drought conditions depending on the design of the pond and the severity of the drought. The ponds can also support groundwater recharge but would still cause a change in the groundwater table and flow patterns when compared to the baseline.

## A8.0 Conclusions

The purpose of this hydrologic and hydraulic analysis completed by SLR is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of mitigation strategies required under the Department of Water Resources (DWR) grant. Specifically, this Appendix outlines how potential decommissioning may affect the Eel River system and identifies potential restoration and alternate mitigation opportunities within the Lake Pillsbury area for different scenarios. All projected scenario response to the climate conditions are presented in Table A8-1.

This Appendix presents the initial analyses conducted to assess the feasibility of these alternatives, with a focus on the effects on the Eel River hydrology and hydraulics. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource.

This analysis does not represent a final decision or commitment for any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

The results present here provides an initial step in a broader, ongoing process. Further studies, analyses, and synthesis will be required, ultimately informing decisions by the County of Lake and other stakeholders.

**Table A8-1: Summary of Scenario Hydrology and Hydraulics under the Existing and Extreme Climate Conditions**

| Scenario | Existing Climate Conditions | | | | Extreme Climate Conditions | | | |
|---|---|---|---|---|---|---|---|---|
| | Wet Season (December - April) | | Dry Season (May - November) | | Flood | | Drought | |
| | Storage (ac-ft) | Outflow (cfs) | Storage (ac-ft) | Outflow (cfs) | Max Storage (ac-ft) [Date] | Max Outflow (cfs) [Date] | Min Storage (ac-ft) [Date] | Min Outflow (cfs) [Date] |
| Historical Conditions WY 1922-2019 & 2005-2019 | **1922-2019**<br>Min: 10<br>Median: 65,900<br>Average: 59,810<br>Max: 94,900<br><br>**2005-2019**<br>Min: 7,840<br>Median: 55,400<br>Average: 49,078<br>Max: 80,200 | **1922-2019**<br>Min: 2<br>Median: 375<br>Average: 1,033<br>Max: 45,300<br><br>**2005-2019**<br>Min: 37<br>Median: 296<br>Average: 820<br>Max: 26,600 | **1922-2019**<br>Min: 2,100<br>Median: 59,700<br>Average: 58,240<br>Max: 94,900<br><br>**2005-2019**<br>Min: 11,300<br>Median: 45,200<br>Average: 45,911<br>Max: 80,800 | **1922-2019**<br>Min: 0.1<br>Median: 177<br>Average: 222<br>Max: 10,200<br><br>**2005-2019**<br>Min: 27<br>Median: 147<br>Average: 165<br>Max: 2,400 | **1922-2019**<br>Flood of record:<br>76,260 [1/26/1983]<br>Max gage reading:<br>94,900 [4/24/1932]<br><br>**2005-2019**<br>Flood of record:<br>72,470 [12/31/2005]<br>Max gage reading:<br>80,200 [4/29/2005] | **1922-2019**<br>Flood of record:<br>15,400 [1/27/1983]<br>Max gage reading:<br>45,300 [12/22/1964]<br><br>**2005-2019**<br>Flood of record:<br>33,500 [12/31/2005]<br>Max gage reading:<br>33,500 [12/31/2005] | **1922-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>10 [12/9/1931]<br><br>**2005-2019**<br>Drought of record:<br>7,840 [2/5/2014]<br>Min gage reading:<br>7,840 [2/5/2014] | **1922-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>0.1 [9/8/1924]<br><br>**2005-2019**<br>Drought of record:<br>27 [11/5/2015]<br>Min gage reading:<br>27 [11/5/2015] |
| Current Conditions WY 2019-2024 & Projected | **2019-2024**<br>Min: 16,800<br>Median: 53,200<br>Average: 45,439<br>Max: 67,300<br><br>**Projected**<br>Min: 13,870<br>Average: 48,500<br>Max: 78,910 | **2019-2024**<br>Min: 67<br>Median: 207<br>Average: 768<br>Max: 13,200<br><br>**Projected**<br>Min: 40<br>Average: 1,052<br>Max: 51,230 | **2019-2024**<br>Min: 21,100<br>Median: 44,100<br>Average: 43,789<br>Max: 62,100<br><br>**Projected**<br>Min: 13,910<br>Average: 45,070<br>Max: 67,380 | **2019-2024**<br>Min: 42<br>Median: 115<br>Average: 201<br>Max: 2,150<br><br>**Projected**<br>Min: 40<br>Average: 258<br>Max: 15,110 | **Projected**<br>78,910 | **Projected**<br>51,230 | **Projected**<br>13,870 | Not enough data to adequately predict. Likely <40. |
| Scenario A1/A2 - PGE Proposed Removal | No storage | Min: 791<br>Average: 1,216<br>Max: 1,535 | No storage | Min: 17<br>Average: 134<br>Max: 376 | No storage | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | No storage | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <17. |
| Scenario C - Lowered Scott Dam with Fish Ladder | Min: 5,420<br>Average: 18,769<br>Max: 28,460 | Min: 20<br>Average: 1,116<br>Max: 47,470 | Min: 7,340<br>Average: 18,080<br>Max: 31,840 | Min: 20<br>Average: 220<br>Max: 14,923 | 31,840 | 47,470 | 5,420 | Not enough data to adequately predict. Likely <20. |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Min: 9,100<br>Average: 10,110<br>Max: 13,500 | Min: 684<br>Average: 1,051<br>Max: 1,327 | Min: 9,060<br>Average: 9,920<br>Max: 11,860 | Min: 15<br>Average: 43<br>Max: 245 | 13,500 | 51,686 | 9,060 | Not enough data to adequately predict. Restoration efforts can provide resiliency. Likely <15. |
| Scenario E – Dam Removal + Off Channel Pond Restoration | ≥863 | Min: 682<br>Median: 1,067<br>Average: 1,048<br>Max: 1,324 | <863 | Min: 15<br>Median: 43<br>Average: 80<br>Max: 244 | ≥863 | StreamStats 1% AEP (100-year recurrence) 60,100 cfs [2025] | <<863 | Not enough data to adequately predict. Restoration design can provide resiliency. Likely <15. |

Notes:
1. All outflow values predicted at the location just downstream of the Scott Dam within the Eel River.
2. All storage values are limited to predicted storage within Lake Pillsbury upstream of Scott Dam except scenario D where values are based within the reduced impoundment footprint.
3. Scenario B hydraulic and hydrologic parameters were not analyzed or reported in this table.

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# Appendix A.A Supporting Figures for Hydrologic and Hydraulic Analysis

## Appendix A - Hydrologic and Hydraulic Scenario Evaluation

Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

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**Figure A.A9-1: Daily average outflow from Lake Pillsbury and daily average storage during the 1976-77 drought**

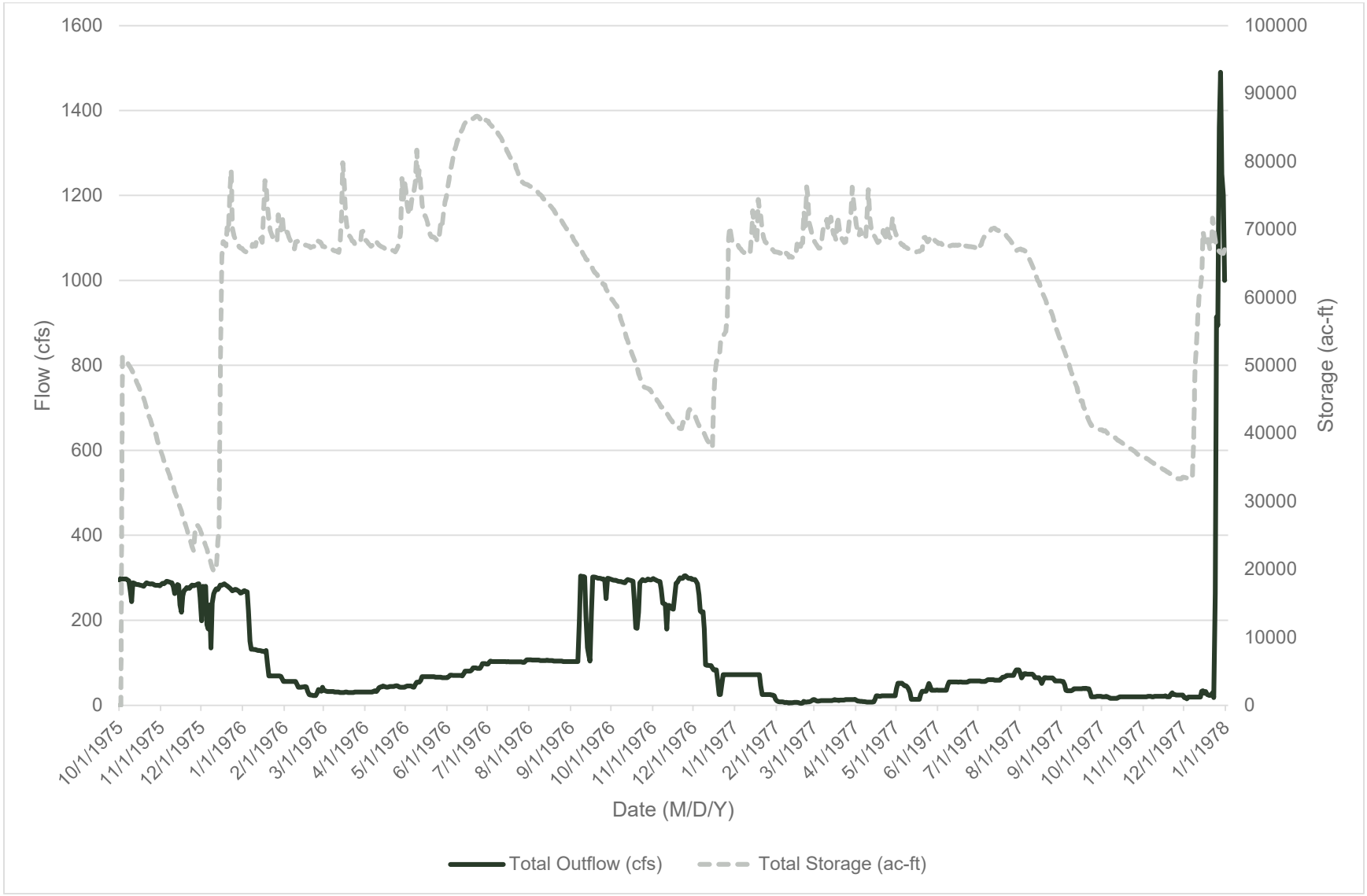


**Figure A.A 9-2: Daily average outflow from Lake Pillsbury and daily average storage during the 1982-83 flood (flood of record)**

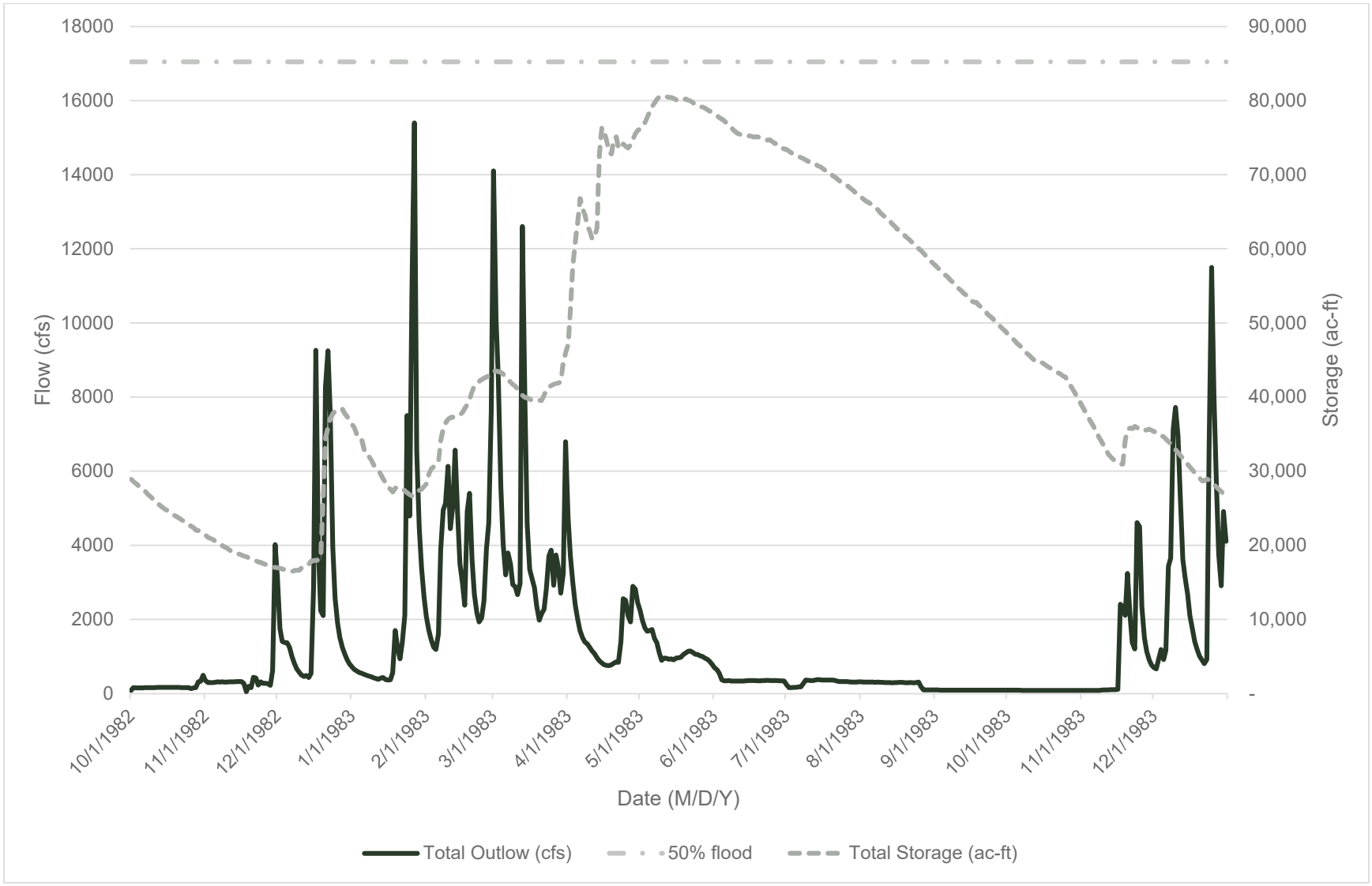


**Figure A.A 9-3: Daily average outflow from Lake Pillsbury and daily average storage during the 1995 flood**

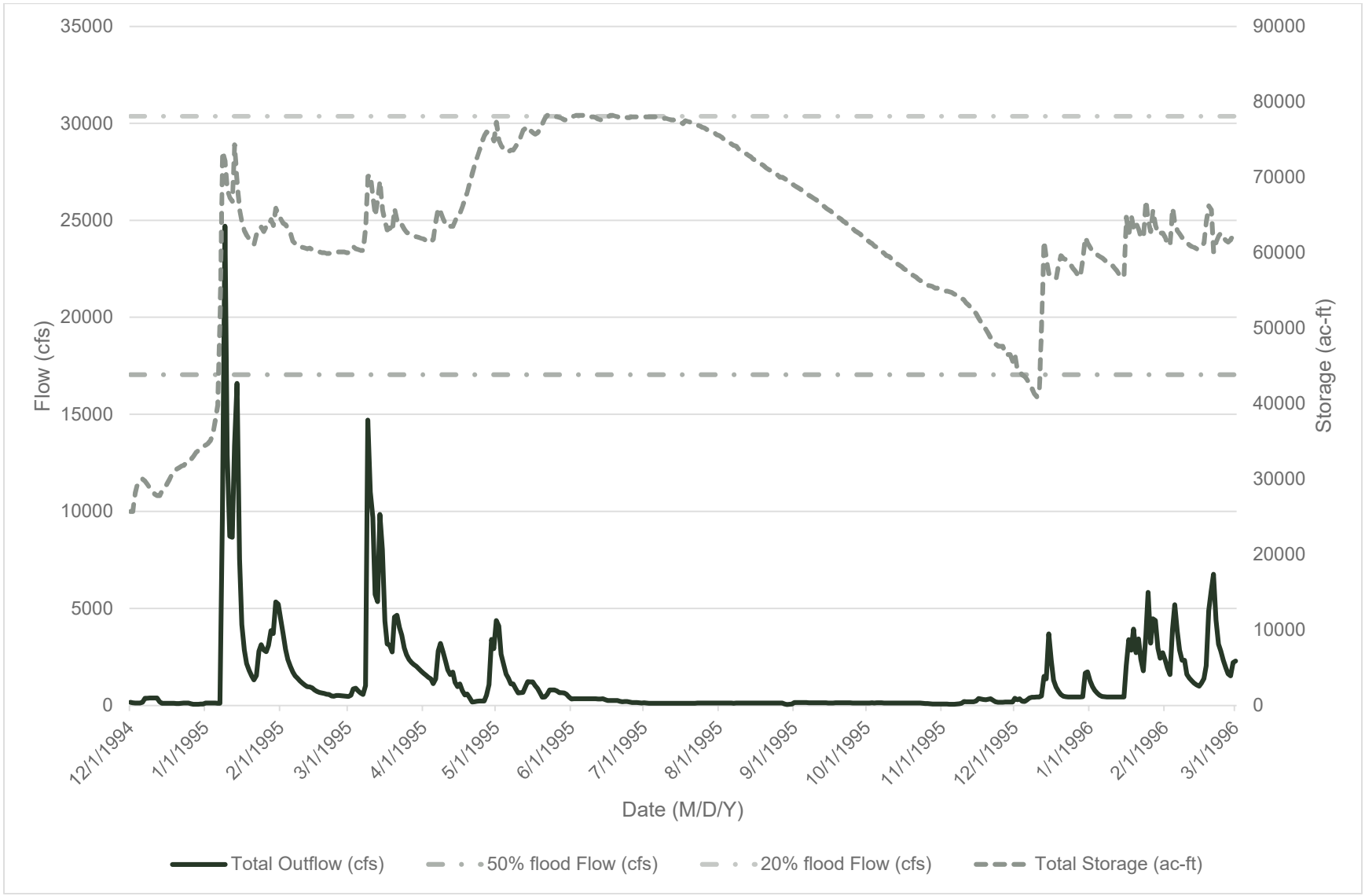


**Figure A.A 9-4: Daily average outflow from Lake Pillsbury and daily average storage during the 2020-22 drought**

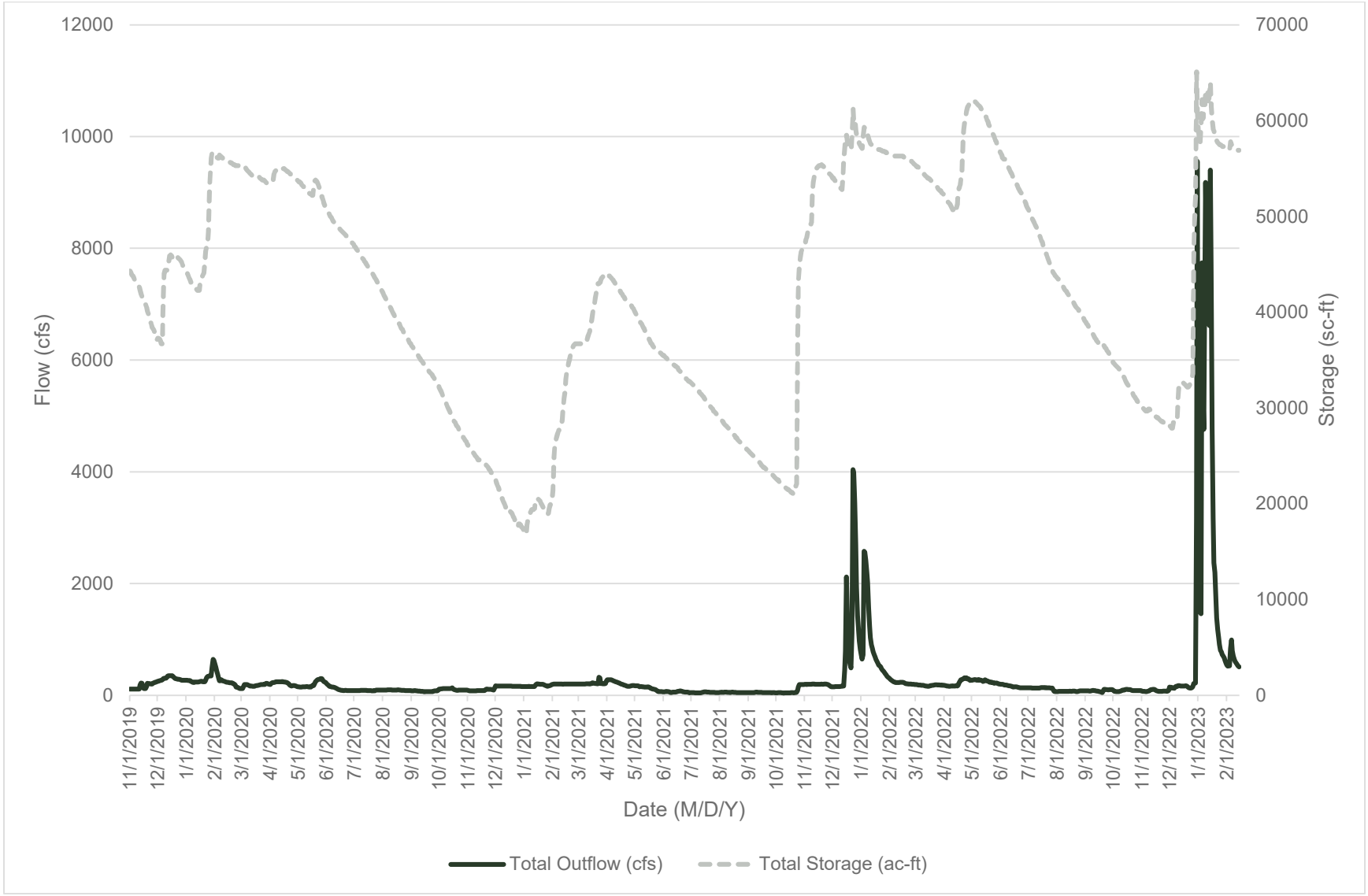


**Figure A.A 9-5:Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 2020-22 drought**

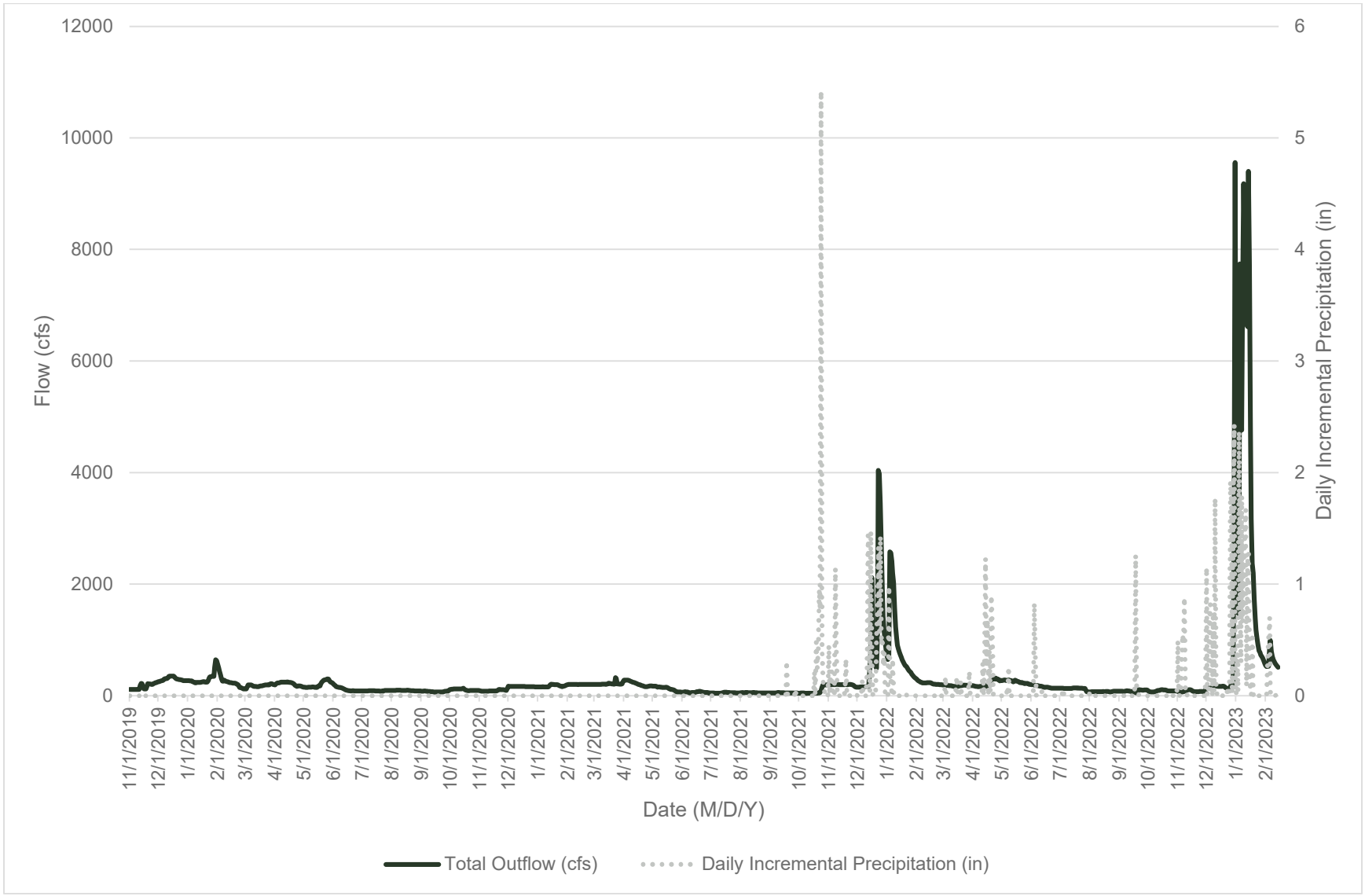


**Figure A.A 9-6: Daily average outflow from Lake Pillsbury and daily incremental precipitation during the 1928-34 drought**

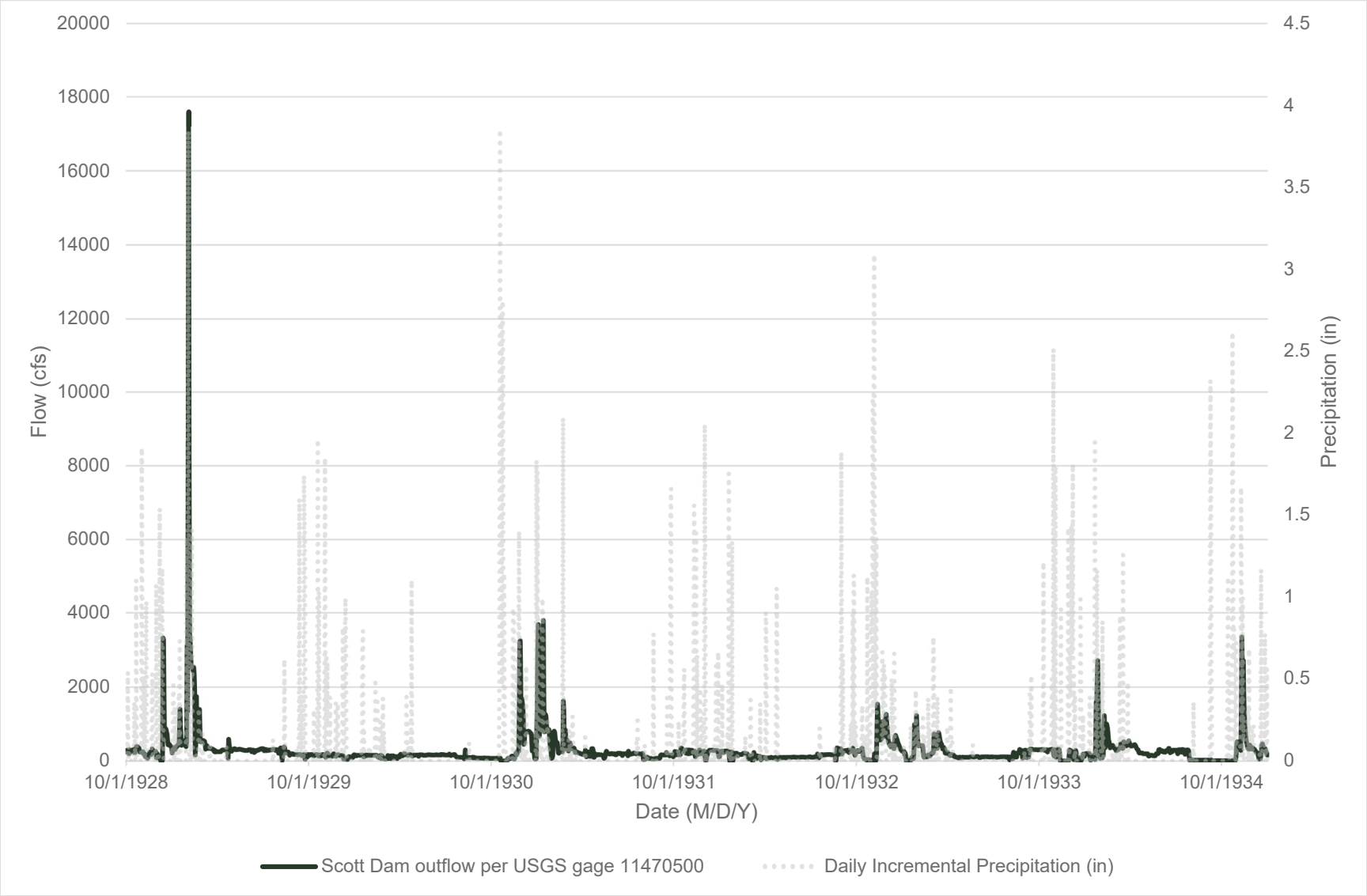


**Figure A.A 9-7: Daily average outflow from Lake Pillsbury and daily average storage during the 1928-34 drought**

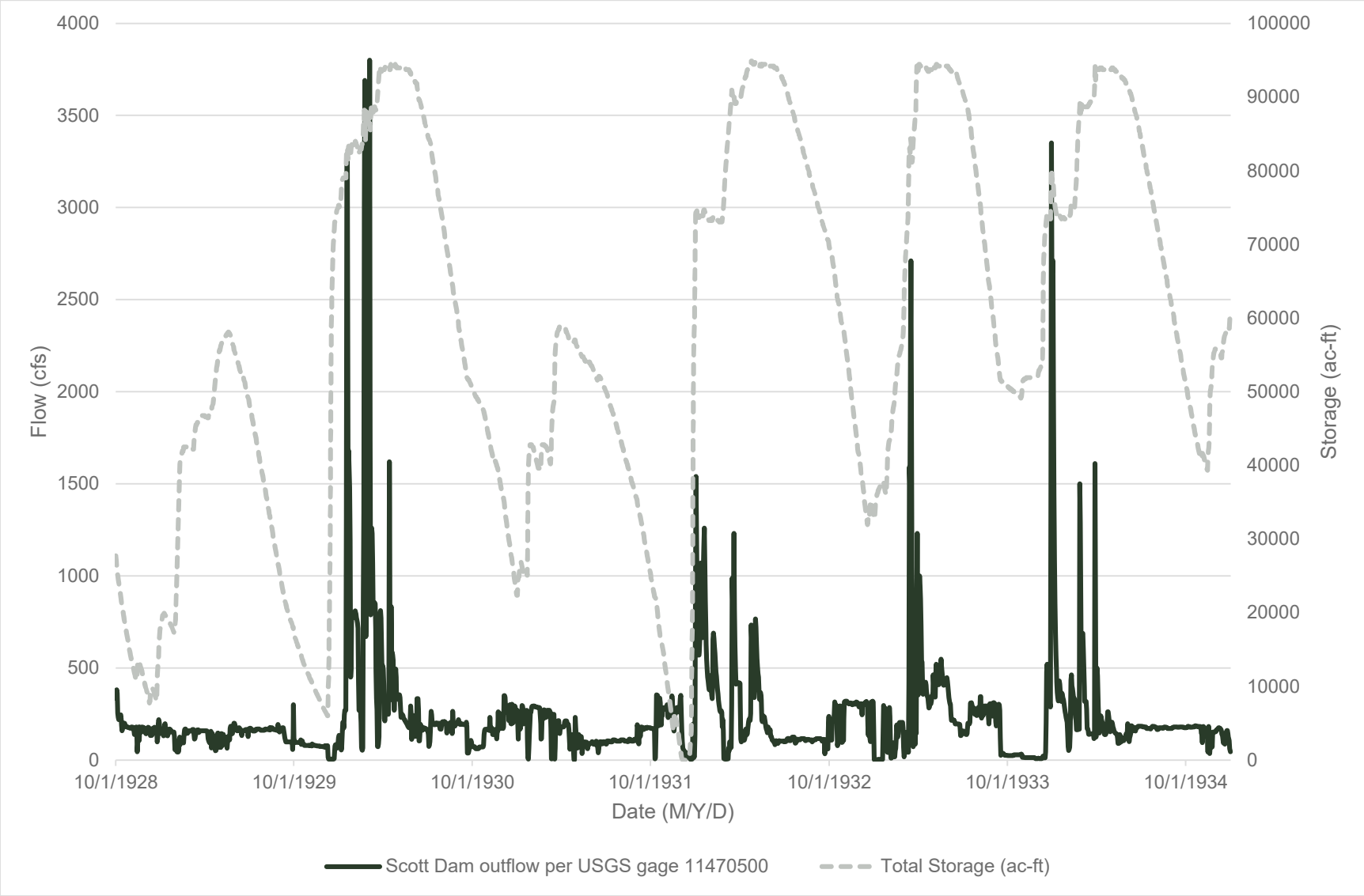


**Figure A.A9-8: Dry and wet conditions in Lake County, CA from 1910-40, including the 1937 flood**

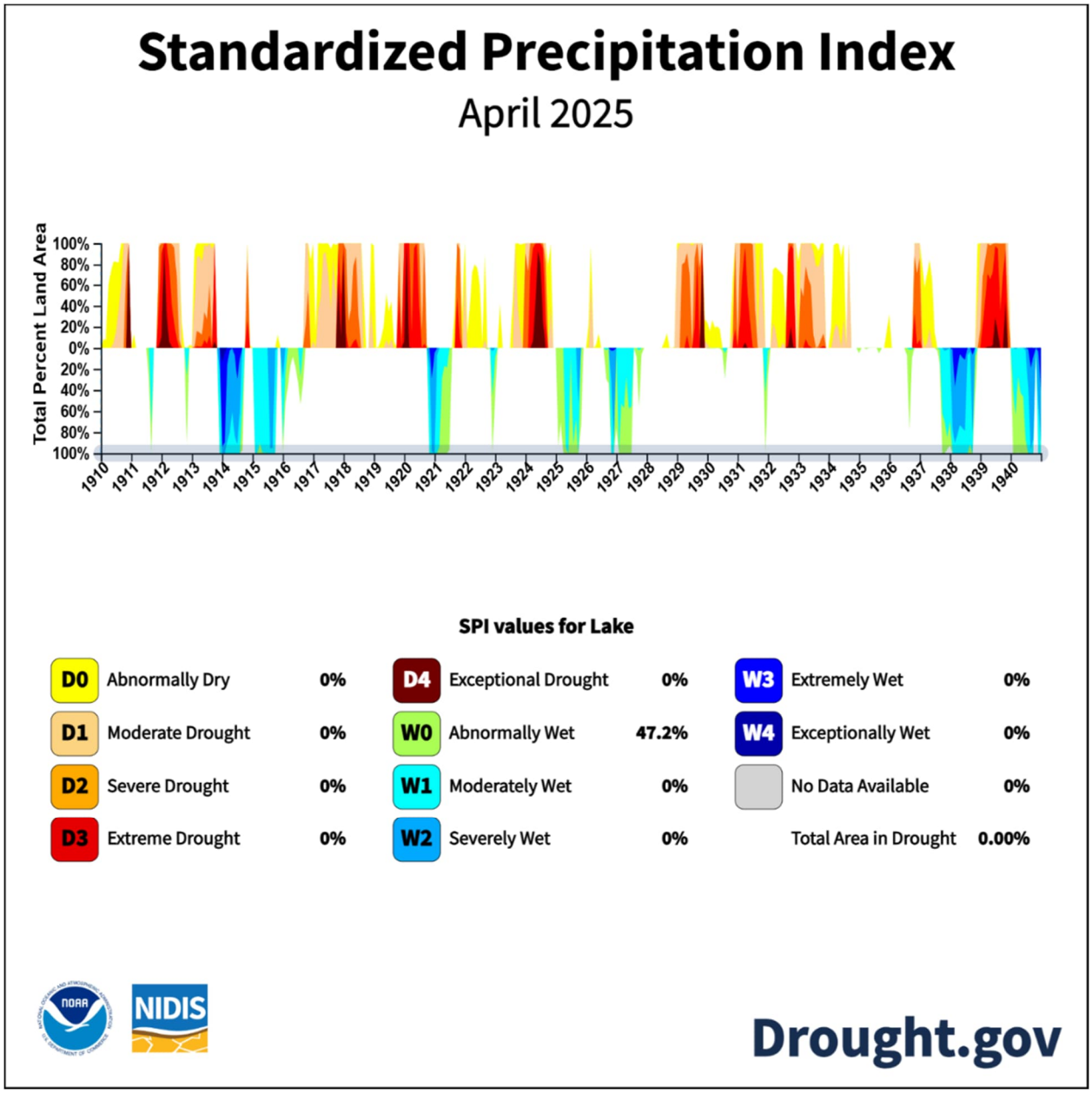


**Figure A.A9-9: Dry and wet conditions in Lake County, CA from 1980-85, including the exceptional 1983 flood**

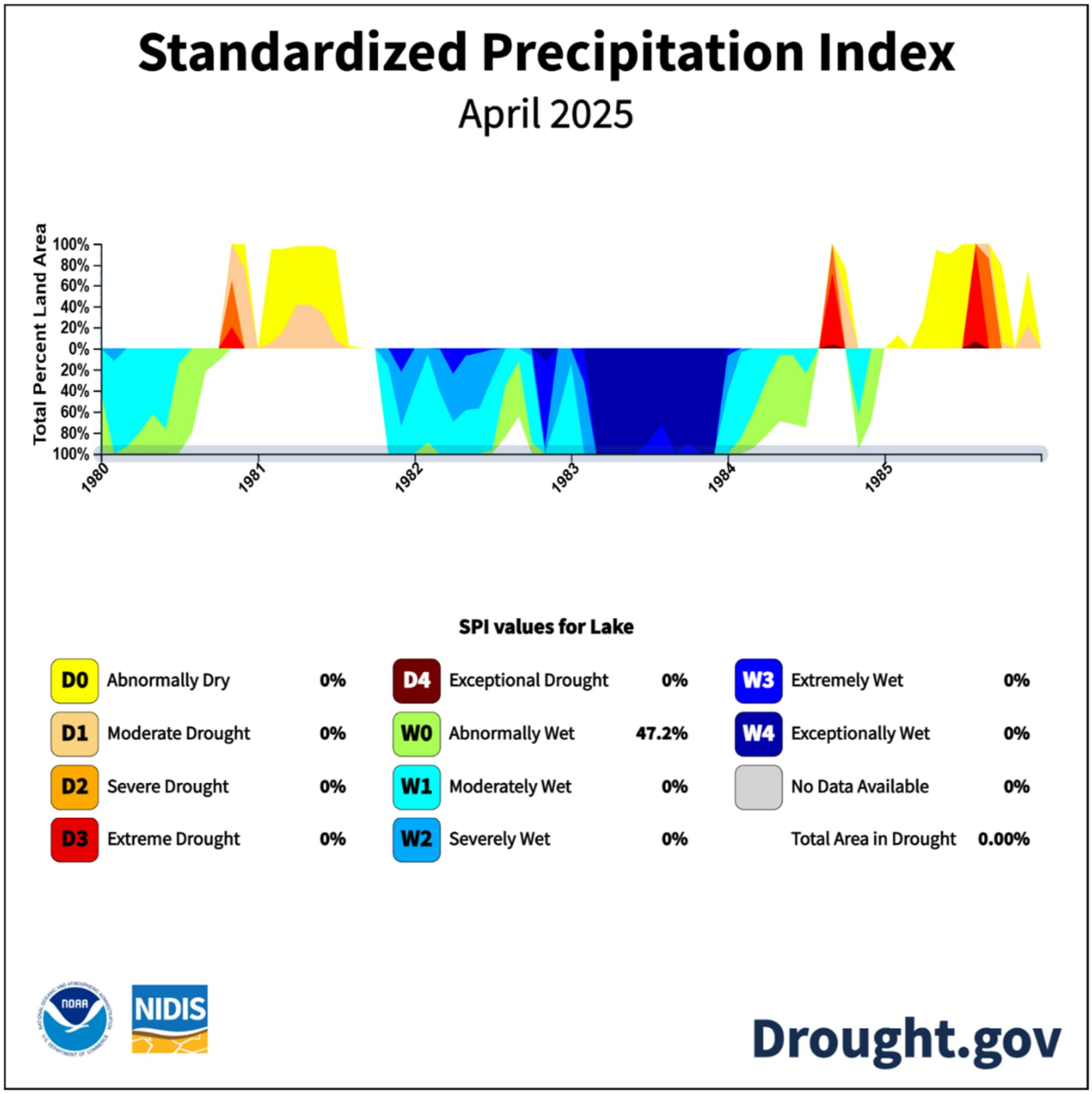


**Figure A.A9-10: Dry and wet conditions for the State of California from 2010-present, including the drought of record from 2012-16 and the recent 2020-23 drought**

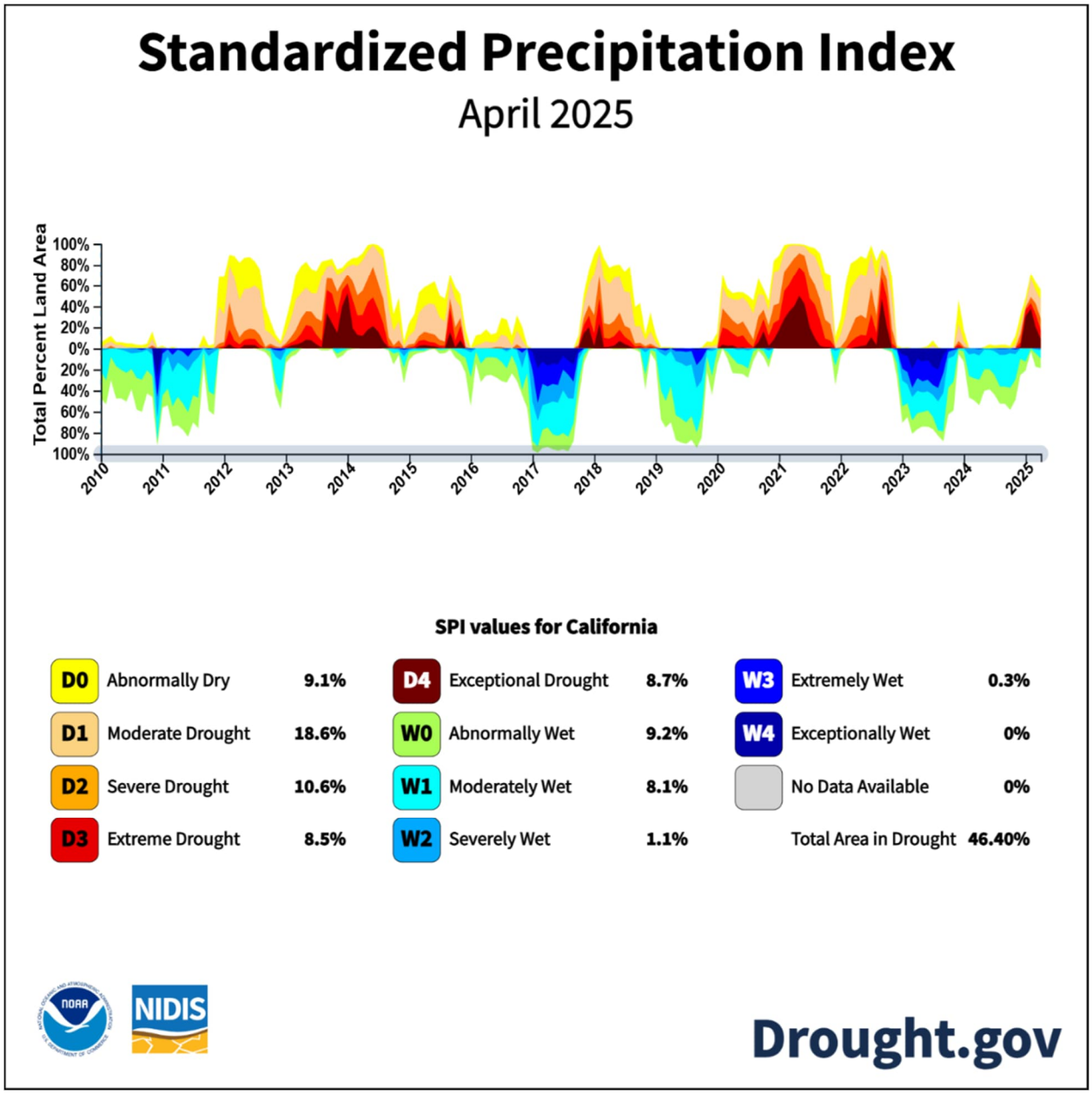


**Figure A.A9-11: Dry and wet conditions in Lake County, CA from 1970-79, including the severe 1976-77 drought**

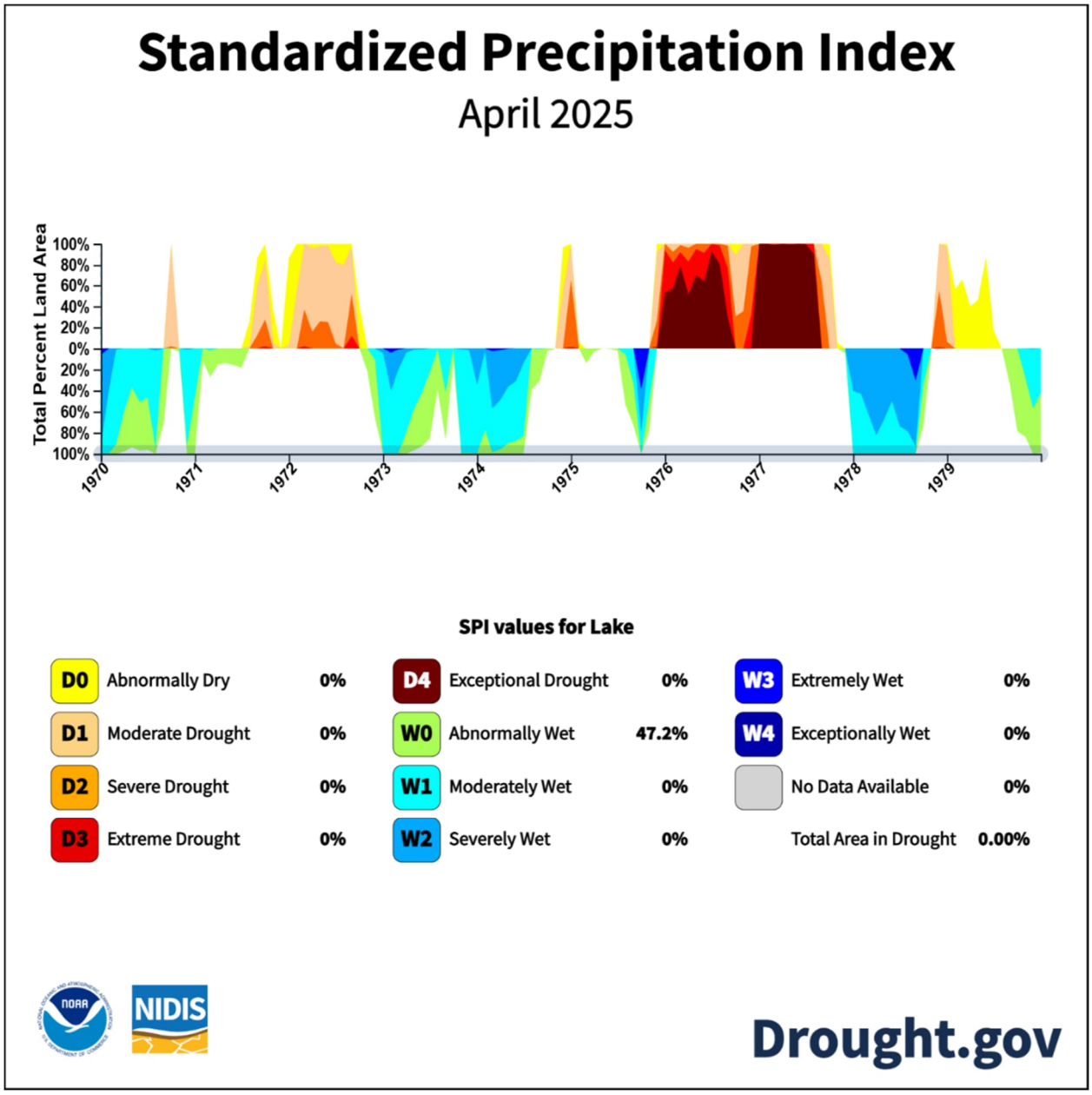


**Figure A.A9-12: Dry and wet conditions in Lake County, CA from 1928-34, including the dry period in 1924 (record low flow)**

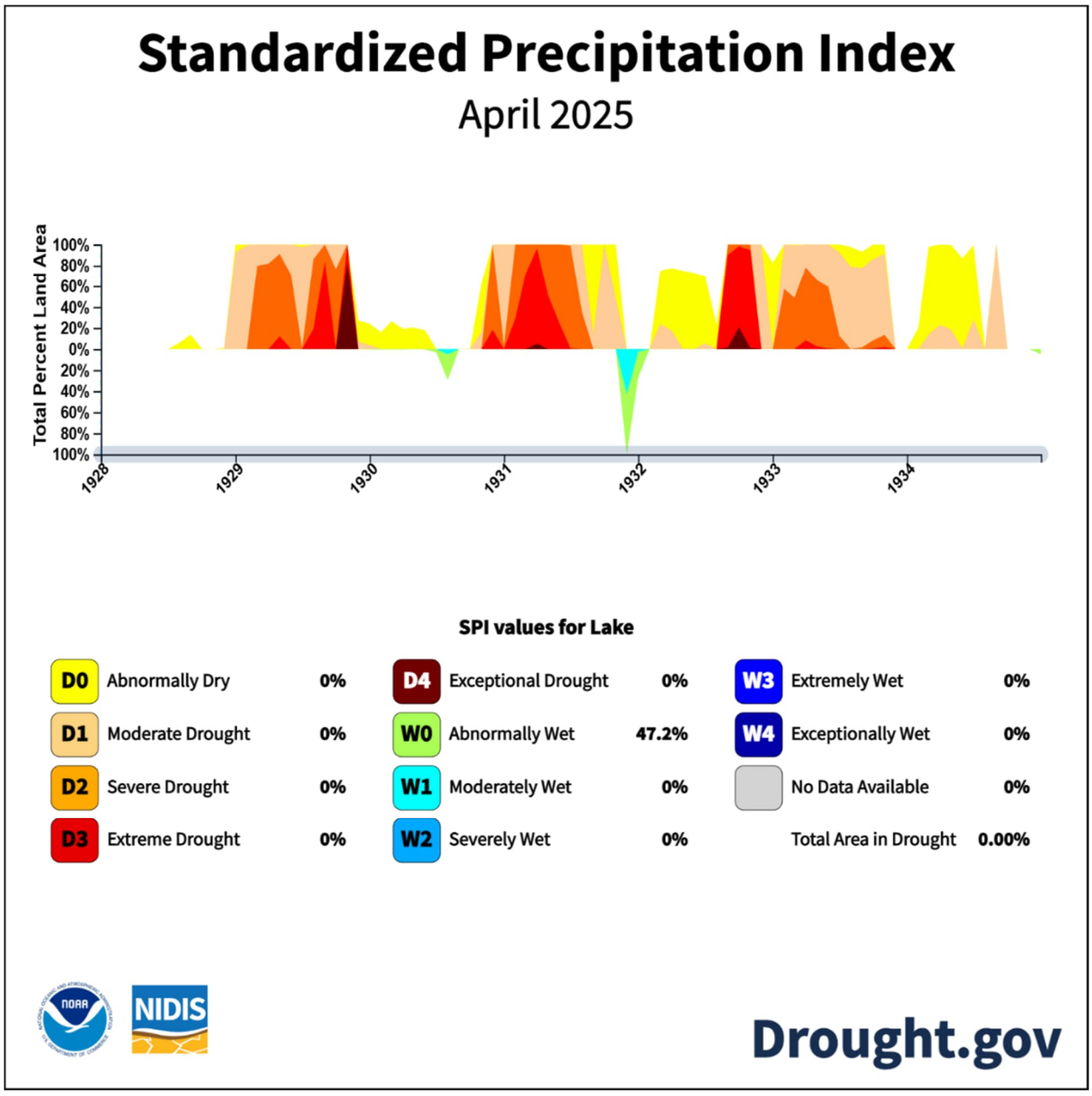
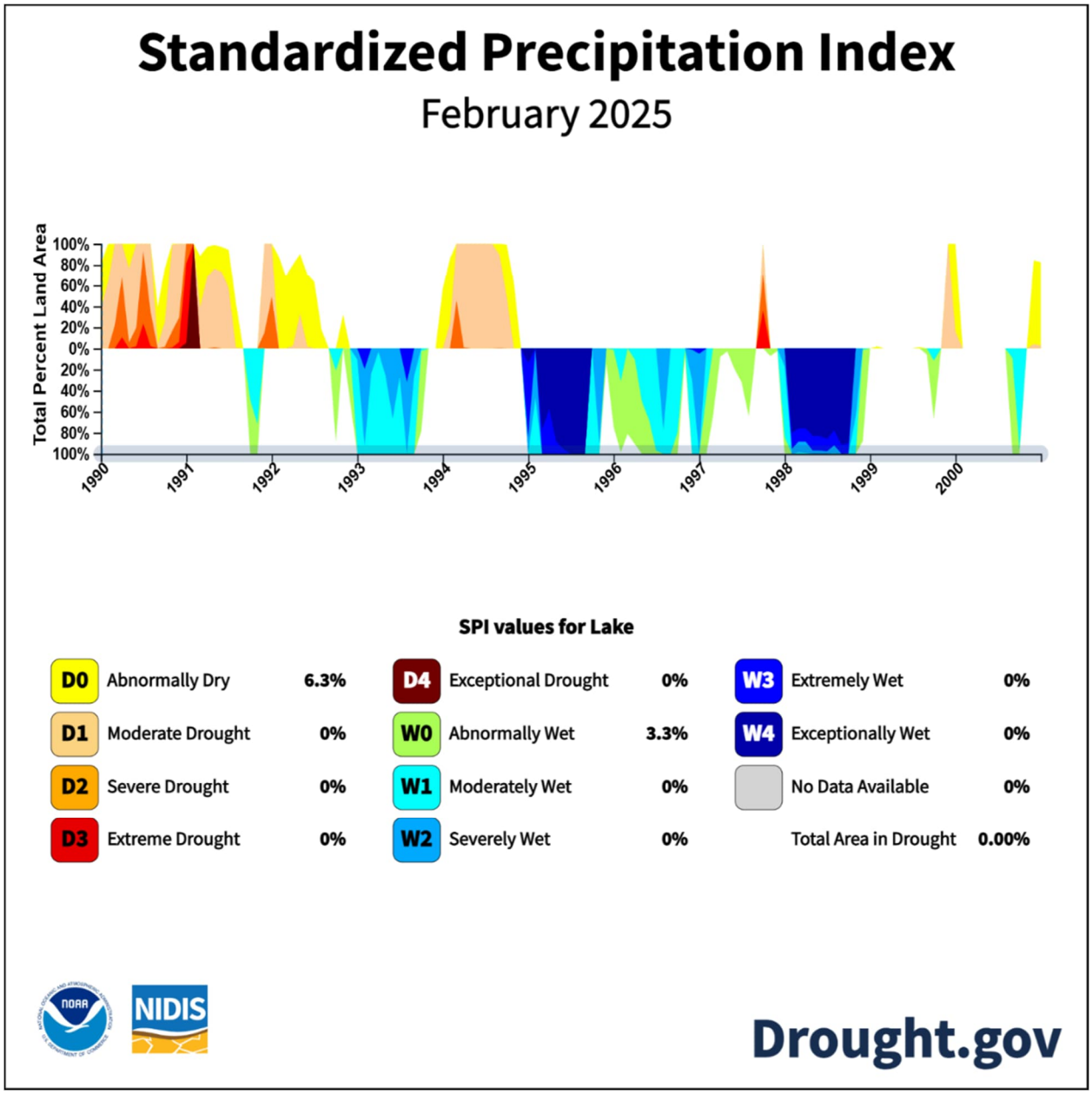


**Figure A.A9-13: Dry and wet conditions in Lake County, CA from 1990-2000 showing the 1995 flood**



# Appendix A.B PG&E Provided Data

## Appendix A - Hydrologic and Hydraulic Scenario Evaluation

Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/1/2005 | 56,469 | 1900.00 | -38 | 136 | 26 | 130 |
| 10/2/2005 | 51,200 | 1897.10 | 0 | 135 | 28 | 129 |
| 10/3/2005 | 51,000 | 1897.00 | 27 | 135 | 28 | 130 |
| 10/4/2005 | 50,700 | 1896.90 | -23 | 135 | 28 | 130 |
| 10/5/2005 | 50,500 | 1896.80 | 27 | 135 | 27 | 129 |
| 10/6/2005 | 50,200 | 1896.60 | -24 | 134 | 28 | 129 |
| 10/7/2005 | 50,000 | 1896.50 | 26 | 134 | 29 | 129 |
| 10/8/2005 | 49,700 | 1896.30 | -24 | 134 | 29 | 129 |
| 10/9/2005 | 49,500 | 1896.20 | 26 | 134 | 30 | 129 |
| 10/10/2005 | 49,200 | 1896.00 | -25 | 133 | 30 | 129 |
| 10/11/2005 | 48,900 | 1895.80 | -25 | 133 | 31 | 129 |
| 10/12/2005 | 48,667 | 1895.70 | 9 | 133 | 32 | 129 |
| 10/13/2005 | 48,433 | 1895.60 | 9 | 133 | 32 | 128 |
| 10/14/2005 | 48,200 | 1895.40 | 9 | 133 | 35 | 130 |
| 10/15/2005 | 47,967 | 1895.30 | 9 | 133 | 36 | 130 |
| 10/16/2005 | 47,733 | 1895.20 | 9 | 133 | 38 | 128 |
| 10/17/2005 | 47,500 | 1895.00 | 8 | 132 | 49 | 131 |
| 10/18/2005 | 47,200 | 1894.90 | -26 | 132 | 61 | 131 |
| 10/19/2005 | 47,000 | 1894.70 | 25 | 132 | 60 | 128 |
| 10/20/2005 | 46,800 | 1894.60 | 25 | 132 | 49 | 104 |
| 10/21/2005 | 46,500 | 1894.40 | -26 | 132 | 50 | 104 |
| 10/22/2005 | 46,300 | 1894.30 | 25 | 132 | 49 | 104 |
| 10/23/2005 | 46,000 | 1894.10 | -26 | 132 | 47 | 107 |
| 10/24/2005 | 45,800 | 1894.00 | 25 | 132 | 45 | 109 |
| 10/25/2005 | 45,600 | 1893.90 | 25 | 132 | 47 | 109 |
| 10/26/2005 | 45,400 | 1893.80 | 26 | 133 | 45 | 117 |
| 10/27/2005 | 45,200 | 1893.70 | 25 | 132 | 44 | 114 |
| 10/28/2005 | 45,100 | 1893.60 | 76 | 133 | 45 | 119 |
| 10/29/2005 | 44,900 | 1893.50 | 25 | 132 | 43 | 114 |
| 10/30/2005 | 44,700 | 1893.40 | 24 | 131 | 45 | 109 |
| 10/31/2005 | 44,500 | 1893.30 | 24 | 131 | 46 | 108 |
| 11/1/2005 | 44,200 | 1893.10 | -27 | 131 | 49 | 104 |
| 11/2/2005 | 44,000 | 1893.00 | 24 | 131 | 50 | 103 |
| 11/3/2005 | 43,900 | 1892.90 | 76 | 133 | 49 | 116 |
| 11/4/2005 | 43,800 | 1892.80 | 76 | 133 | 48 | 127 |
| 11/5/2005 | 43,600 | 1892.70 | 24 | 131 | 47 | 115 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/6/2005 | 43,400 | 1892.60 | 25 | 132 | 50 | 114 |
| 11/7/2005 | 44,300 | 1893.10 | 587 | 140 | 89 | 177 |
| 11/8/2005 | 44,800 | 1893.40 | 380 | 135 | 63 | 186 |
| 11/9/2005 | 44,800 | 1893.40 | 125 | 132 | 69 | 102 |
| 11/10/2005 | 44,700 | 1893.40 | 75 | 132 | 82 | 74 |
| 11/11/2005 | 44,600 | 1893.30 | 75 | 132 | 83 | 68 |
| 11/12/2005 | 44,400 | 1893.20 | 24 | 131 | 83 | 67 |
| 11/13/2005 | 44,200 | 1893.10 | 24 | 131 | 84 | 65 |
| 11/14/2005 | 44,100 | 1893.00 | 74 | 131 | 83 | 65 |
| 11/15/2005 | 43,900 | 1892.90 | 24 | 131 | 83 | 64 |
| 11/16/2005 | 43,700 | 1892.80 | 24 | 131 | 87 | 57 |
| 11/17/2005 | 43,500 | 1892.60 | 24 | 131 | 87 | 58 |
| 11/18/2005 | 43,300 | 1892.50 | 23 | 130 | 85 | 61 |
| 11/19/2005 | 43,100 | 1892.40 | 23 | 130 | 80 | 68 |
| 11/20/2005 | 43,000 | 1892.30 | 73 | 130 | 77 | 72 |
| 11/21/2005 | 42,800 | 1892.20 | 23 | 130 | 75 | 75 |
| 11/22/2005 | 42,600 | 1892.10 | 23 | 130 | 71 | 81 |
| 11/23/2005 | 42,400 | 1892.00 | 23 | 130 | 69 | 84 |
| 11/24/2005 | 42,200 | 1891.80 | 23 | 130 | 65 | 88 |
| 11/25/2005 | 42,200 | 1891.80 | 128 | 135 | 64 | 115 |
| 11/26/2005 | 42,100 | 1891.80 | 73 | 130 | 64 | 107 |
| 11/27/2005 | 42,000 | 1891.70 | 73 | 130 | 67 | 95 |
| 11/28/2005 | 42,900 | 1892.30 | 587 | 140 | 127 | 99 |
| 11/29/2005 | 43,900 | 1892.90 | 636 | 139 | 278 | 107 |
| 11/30/2005 | 44,300 | 1893.10 | 329 | 134 | 103 | 104 |
| 12/1/2005 | 53,500 | 1898.40 | 4,786 | 155 | 1,110 | 88 |
| 12/2/2005 | 55,500 | 1899.50 | 1,355 | 365 | 640 | 74 |
| 12/3/2005 | 55,400 | 1899.50 | 666 | 754 | 721 | 88 |
| 12/4/2005 | 55,100 | 1899.30 | 374 | 553 | 568 | 88 |
| 12/5/2005 | 54,900 | 1899.20 | 245 | 364 | 398 | 95 |
| 12/6/2005 | 54,800 | 1899.10 | 204 | 268 | 273 | 99 |
| 12/7/2005 | 54,700 | 1899.10 | 158 | 219 | 197 | 98 |
| 12/8/2005 | 54,600 | 1899.00 | 138 | 198 | 160 | 99 |
| 12/9/2005 | 54,600 | 1899.00 | 169 | 178 | 121 | 102 |
| 12/10/2005 | 54,500 | 1899.00 | 103 | 162 | 100 | 94 |
| 12/11/2005 | 54,400 | 1898.90 | 92 | 150 | 103 | 75 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/12/2005 | 54,400 | 1898.90 | 136 | 143 | 103 | 64 |
| 12/13/2005 | 54,300 | 1898.90 | 82 | 139 | 107 | 53 |
| 12/14/2005 | 54,200 | 1898.80 | 79 | 136 | 107 | 46 |
| 12/15/2005 | 54,100 | 1898.70 | 78 | 135 | 107 | 41 |
| 12/16/2005 | 54,000 | 1898.70 | 77 | 134 | 107 | 40 |
| 12/17/2005 | 54,000 | 1898.70 | 128 | 135 | 107 | 40 |
| 12/18/2005 | 56,200 | 1899.90 | 1,481 | 391 | 300 | 107 |
| 12/19/2005 | 59,000 | 1901.40 | 5,435 | 4,235 | 4,670 | 71 |
| 12/20/2005 | 58,500 | 1901.10 | 3,484 | 3,932 | 4,260 | 40 |
| 12/21/2005 | 60,300 | 1902.00 | 4,164 | 3,428 | 4,100 | 40 |
| 12/22/2005 | 61,400 | 1902.60 | 8,218 | 8,067 | 8,890 | 15 |
| 12/23/2005 | 58,300 | 1901.00 | 3,466 | 5,294 | 6,570 | 24 |
| 12/24/2005 | 57,000 | 1900.30 | 1,548 | 2,319 | 2,940 | 41 |
| 12/25/2005 | 56,900 | 1900.30 | 1,386 | 1,512 | 1,910 | 41 |
| 12/26/2005 | 58,400 | 1901.00 | 3,869 | 3,277 | 3,820 | 41 |
| 12/27/2005 | 62,300 | 1903.00 | 6,277 | 4,537 | 5,370 | 33 |
| 12/28/2005 | 64,100 | 1903.90 | 17,192 | 17,141 | 18,500 | 0 |
| 12/29/2005 | 59,700 | 1901.70 | 4,966 | 7,562 | 9,450 | 0 |
| 12/30/2005 | 67,300 | 1905.50 | 14,129 | 10,839 | 11,700 | 10 |
| 12/31/2005 | 65,800 | 1904.80 | 29,897 | 32,266 | 39,800 | 0 |
| 1/1/2006 | 63,300 | 1903.50 | 10,474 | 12,352 | 16,100 | 0 |
| 1/2/2006 | 60,300 | 1902.00 | 6,869 | 8,823 | 11,100 | 0 |
| 1/3/2006 | 59,700 | 1901.70 | 4,870 | 5,445 | 7,000 | 22 |
| 1/4/2006 | 59,300 | 1901.50 | 4,348 | 4,790 | 6,020 | 45 |
| 1/5/2006 | 57,900 | 1900.80 | 2,407 | 3,277 | 4,140 | 42 |
| 1/6/2006 | 57,400 | 1900.50 | 1,951 | 2,319 | 2,950 | 41 |
| 1/7/2006 | 57,000 | 1900.30 | 1,523 | 1,815 | 2,250 | 40 |
| 1/8/2006 | 56,700 | 1900.20 | 1,283 | 1,510 | 1,750 | 41 |
| 1/9/2006 | 56,400 | 1900.00 | 989 | 1,200 | 1,390 | 40 |
| 1/10/2006 | 56,300 | 1899.90 | 909 | 1,010 | 1,150 | 51 |
| 1/11/2006 | 56,500 | 1900.00 | 1,231 | 1,190 | 1,350 | 70 |
| 1/12/2006 | 56,300 | 1899.90 | 1,039 | 1,200 | 1,260 | 165 |
| 1/13/2006 | 56,200 | 1899.90 | 1,137 | 1,250 | 1,220 | 265 |
| 1/14/2006 | 56,600 | 1900.10 | 1,437 | 1,300 | 1,280 | 265 |
| 1/15/2006 | 56,300 | 1899.90 | 1,027 | 1,240 | 1,220 | 265 |
| 1/16/2006 | 56,200 | 1899.90 | 1,080 | 1,190 | 1,160 | 265 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/17/2006 | 57,300 | 1900.50 | 1,913 | 1,430 | 1,440 | 265 |
| 1/18/2006 | 58,200 | 1900.90 | 3,711 | 3,428 | 3,910 | 264 |
| 1/19/2006 | 57,400 | 1900.50 | 1,877 | 2,400 | 2,600 | 264 |
| 1/20/2006 | 57,100 | 1900.40 | 1,483 | 1,720 | 1,780 | 264 |
| 1/21/2006 | 56,800 | 1900.20 | 1,758 | 2,010 | 2,120 | 263 |
| 1/22/2006 | 56,500 | 1900.00 | 1,749 | 2,000 | 2,120 | 263 |
| 1/23/2006 | 56,300 | 1899.90 | 1,771 | 1,970 | 2,080 | 264 |
| 1/24/2006 | 56,200 | 1899.90 | 1,821 | 1,970 | 2,090 | 264 |
| 1/25/2006 | 56,100 | 1899.80 | 1,888 | 2,040 | 2,160 | 265 |
| 1/26/2006 | 56,000 | 1899.80 | 1,935 | 2,090 | 2,220 | 265 |
| 1/27/2006 | 55,900 | 1899.70 | 1,935 | 2,090 | 2,240 | 246 |
| 1/28/2006 | 56,400 | 1900.00 | 2,257 | 2,110 | 2,250 | 262 |
| 1/29/2006 | 56,400 | 1900.00 | 2,071 | 2,180 | 2,340 | 262 |
| 1/30/2006 | 57,500 | 1900.60 | 2,683 | 2,240 | 2,410 | 261 |
| 1/31/2006 | 57,000 | 1900.30 | 2,047 | 2,420 | 2,620 | 260 |
| 2/1/2006 | 58,900 | 1901.30 | 3,827 | 3,020 | 3,330 | 260 |
| 2/2/2006 | 58,900 | 1901.30 | 4,437 | 4,670 | 5,940 | 127 |
| 2/3/2006 | 58,200 | 1900.90 | 2,953 | 3,480 | 4,390 | 178 |
| 2/4/2006 | 57,700 | 1900.70 | 2,446 | 2,840 | 3,500 | 265 |
| 2/5/2006 | 57,200 | 1900.40 | 1,971 | 2,340 | 2,960 | 265 |
| 2/6/2006 | 56,800 | 1900.20 | 1,622 | 1,920 | 2,370 | 263 |
| 2/7/2006 | 56,500 | 1900.00 | 1,416 | 1,650 | 2,020 | 263 |
| 2/8/2006 | 56,300 | 1899.90 | 1,296 | 1,470 | 1,930 | 262 |
| 2/9/2006 | 56,100 | 1899.80 | 1,182 | 1,350 | 1,810 | 262 |
| 2/10/2006 | 56,000 | 1899.80 | 1,118 | 1,230 | 1,600 | 262 |
| 2/11/2006 | 55,900 | 1899.70 | 1,014 | 1,120 | 1,410 | 262 |
| 2/12/2006 | 55,700 | 1899.60 | 878 | 1,030 | 1,260 | 262 |
| 2/13/2006 | 55,600 | 1899.60 | 857 | 955 | 1,170 | 262 |
| 2/14/2006 | 55,600 | 1899.60 | 841 | 885 | 1,140 | 262 |
| 2/15/2006 | 55,500 | 1899.50 | 725 | 816 | 1,110 | 262 |
| 2/16/2006 | 55,400 | 1899.50 | 673 | 761 | 1,080 | 262 |
| 2/17/2006 | 55,400 | 1899.50 | 686 | 722 | 1,060 | 263 |
| 2/18/2006 | 55,400 | 1899.50 | 671 | 706 | 1,030 | 263 |
| 2/19/2006 | 55,300 | 1899.40 | 570 | 653 | 1,250 | 66 |
| 2/20/2006 | 55,300 | 1899.40 | 571 | 601 | 1,160 | 96 |
| 2/21/2006 | 55,200 | 1899.30 | 482 | 560 | 1,030 | 163 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/22/2006 | 55,200 | 1899.30 | 494 | 520 | 1,050 | 202 |
| 2/23/2006 | 55,200 | 1899.30 | 463 | 487 | 1,040 | 265 |
| 2/24/2006 | 55,100 | 1899.30 | 386 | 459 | 1,050 | 265 |
| 2/25/2006 | 55,100 | 1899.30 | 411 | 433 | 1,170 | 266 |
| 2/26/2006 | 55,200 | 1899.30 | 455 | 426 | 1,370 | 266 |
| 2/27/2006 | 59,600 | 1901.70 | 2,937 | 756 | 1,840 | 46 |
| 2/28/2006 | 59,200 | 1901.50 | 2,672 | 3,025 | 3,860 | 78 |
| 3/1/2006 | 58,200 | 1900.90 | 1,651 | 2,269 | 2,870 | 162 |
| 3/2/2006 | 57,600 | 1900.60 | 1,374 | 1,765 | 2,120 | 216 |
| 3/3/2006 | 57,300 | 1900.50 | 1,286 | 1,512 | 1,960 | 265 |
| 3/4/2006 | 56,800 | 1900.20 | 1,041 | 1,361 | 1,590 | 265 |
| 3/5/2006 | 59,400 | 1901.60 | 2,748 | 1,512 | 1,930 | 201 |
| 3/6/2006 | 58,900 | 1901.30 | 2,861 | 3,277 | 3,880 | 30 |
| 3/7/2006 | 58,300 | 1901.00 | 2,092 | 2,521 | 3,850 | 108 |
| 3/8/2006 | 57,500 | 1900.60 | 2,470 | 3,025 | 3,910 | 265 |
| 3/9/2006 | 57,200 | 1900.40 | 1,765 | 2,017 | 2,560 | 264 |
| 3/10/2006 | 56,800 | 1900.20 | 1,331 | 1,613 | 2,080 | 264 |
| 3/11/2006 | 56,600 | 1900.10 | 1,336 | 1,512 | 1,710 | 263 |
| 3/12/2006 | 56,400 | 1900.00 | 1,192 | 1,361 | 1,590 | 97 |
| 3/13/2006 | 56,400 | 1900.00 | 1,173 | 1,235 | 1,470 | 0 |
| 3/14/2006 | 56,800 | 1900.20 | 1,567 | 1,437 | 1,790 | 0 |
| 3/15/2006 | 56,600 | 1900.10 | 1,336 | 1,512 | 1,880 | 0 |
| 3/16/2006 | 58,000 | 1900.80 | 2,382 | 1,765 | 2,360 | 0 |
| 3/17/2006 | 57,600 | 1900.60 | 2,241 | 2,571 | 3,560 | 0 |
| 3/18/2006 | 57,000 | 1900.30 | 1,565 | 1,966 | 2,610 | 20 |
| 3/19/2006 | 56,700 | 1900.20 | 1,405 | 1,639 | 1,950 | 167 |
| 3/20/2006 | 56,600 | 1900.10 | 1,386 | 1,512 | 1,750 | 167 |
| 3/21/2006 | 56,400 | 1900.00 | 1,240 | 1,412 | 1,570 | 165 |
| 3/22/2006 | 56,300 | 1899.90 | 1,147 | 1,260 | 1,340 | 165 |
| 3/23/2006 | 56,300 | 1899.90 | 1,102 | 1,160 | 1,220 | 165 |
| 3/24/2006 | 57,300 | 1900.50 | 2,037 | 1,613 | 1,740 | 165 |
| 3/25/2006 | 58,300 | 1901.00 | 2,995 | 2,622 | 3,070 | 165 |
| 3/26/2006 | 57,600 | 1900.60 | 2,090 | 2,571 | 3,130 | 164 |
| 3/27/2006 | 57,300 | 1900.50 | 1,765 | 2,017 | 2,350 | 165 |
| 3/28/2006 | 57,300 | 1900.50 | 1,820 | 1,916 | 2,190 | 164 |
| 3/29/2006 | 57,200 | 1900.40 | 1,770 | 1,916 | 2,180 | 164 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/30/2006 | 57,000 | 1900.30 | 1,623 | 1,815 | 1,960 | 165 |
| 3/31/2006 | 57,800 | 1900.70 | 2,223 | 1,916 | 2,170 | 215 |
| 4/1/2006 | 58,000 | 1900.80 | 2,496 | 2,521 | 2,930 | 266 |
| 4/2/2006 | 58,000 | 1900.80 | 2,395 | 2,521 | 2,920 | 265 |
| 4/3/2006 | 59,400 | 1901.60 | 4,537 | 4,033 | 4,910 | 114 |
| 4/4/2006 | 58,400 | 1901.00 | 3,088 | 3,781 | 4,620 | 28 |
| 4/5/2006 | 57,800 | 1900.70 | 2,380 | 2,823 | 3,450 | 47 |
| 4/6/2006 | 57,400 | 1900.50 | 1,954 | 2,269 | 2,620 | 47 |
| 4/7/2006 | 57,200 | 1900.40 | 1,719 | 1,916 | 2,190 | 47 |
| 4/8/2006 | 57,100 | 1900.40 | 1,674 | 1,815 | 2,030 | 107 |
| 4/9/2006 | 57,100 | 1900.40 | 1,676 | 1,765 | 1,860 | 266 |
| 4/10/2006 | 58,400 | 1901.00 | 2,044 | 1,462 | 1,490 | 265 |
| 4/11/2006 | 60,500 | 1902.10 | 2,160 | 1,160 | 1,160 | 265 |
| 4/12/2006 | 67,000 | 1905.40 | 6,151 | 3,025 | 3,520 | 200 |
| 4/13/2006 | 66,000 | 1904.90 | 3,327 | 4,033 | 4,770 | 144 |
| 4/14/2006 | 64,300 | 1904.00 | 2,256 | 3,277 | 3,800 | 263 |
| 4/15/2006 | 63,900 | 1903.80 | 2,433 | 2,773 | 3,150 | 222 |
| 4/16/2006 | 64,100 | 1903.90 | 2,975 | 3,025 | 3,620 | 100 |
| 4/17/2006 | 63,800 | 1903.80 | 2,244 | 2,521 | 2,780 | 214 |
| 4/18/2006 | 63,900 | 1903.80 | 1,775 | 1,815 | 1,940 | 188 |
| 4/19/2006 | 64,400 | 1904.10 | 1,689 | 1,512 | 1,500 | 215 |
| 4/20/2006 | 65,500 | 1904.60 | 1,704 | 1,210 | 1,060 | 266 |
| 4/21/2006 | 66,400 | 1905.10 | 1,747 | 1,361 | 1,190 | 266 |
| 4/22/2006 | 66,500 | 1905.10 | 1,727 | 1,765 | 1,590 | 266 |
| 4/23/2006 | 66,000 | 1904.90 | 1,520 | 1,865 | 1,720 | 266 |
| 4/24/2006 | 65,800 | 1904.80 | 1,480 | 1,664 | 1,520 | 266 |
| 4/25/2006 | 65,900 | 1904.80 | 1,391 | 1,412 | 1,310 | 265 |
| 4/26/2006 | 66,000 | 1904.90 | 1,368 | 1,386 | 1,330 | 258 |
| 4/27/2006 | 66,300 | 1905.00 | 1,492 | 1,412 | 1,430 | 264 |
| 4/28/2006 | 67,000 | 1905.40 | 1,790 | 1,512 | 1,520 | 263 |
| 4/29/2006 | 67,600 | 1905.60 | 1,835 | 1,613 | 1,670 | 264 |
| 4/30/2006 | 67,600 | 1905.60 | 1,676 | 1,765 | 1,830 | 264 |
| 5/1/2006 | 67,300 | 1905.50 | 1,477 | 1,714 | 1,800 | 264 |
| 5/2/2006 | 67,800 | 1905.70 | 1,689 | 1,512 | 1,600 | 264 |
| 5/3/2006 | 68,800 | 1906.20 | 1,797 | 1,361 | 1,460 | 263 |
| 5/4/2006 | 69,500 | 1906.60 | 1,359 | 1,059 | 1,100 | 262 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/5/2006 | 70,100 | 1906.80 | 1,165 | 907 | 779 | 266 |
| 5/6/2006 | 70,500 | 1907.00 | 1,064 | 907 | 788 | 267 |
| 5/7/2006 | 70,500 | 1907.00 | 934 | 983 | 854 | 266 |
| 5/8/2006 | 70,300 | 1906.90 | 905 | 1,059 | 909 | 266 |
| 5/9/2006 | 70,200 | 1906.90 | 907 | 1,008 | 857 | 249 |
| 5/10/2006 | 70,300 | 1906.90 | 913 | 907 | 722 | 268 |
| 5/11/2006 | 70,800 | 1907.20 | 875 | 655 | 518 | 256 |
| 5/12/2006 | 71,500 | 1907.50 | 785 | 455 | 370 | 192 |
| 5/13/2006 | 72,100 | 1907.80 | 744 | 465 | 386 | 177 |
| 5/14/2006 | 72,700 | 1908.10 | 729 | 449 | 378 | 177 |
| 5/15/2006 | 73,200 | 1908.30 | 705 | 477 | 370 | 177 |
| 5/16/2006 | 73,600 | 1908.50 | 729 | 555 | 453 | 190 |
| 5/17/2006 | 73,700 | 1908.50 | 625 | 605 | 478 | 207 |
| 5/18/2006 | 73,600 | 1908.50 | 596 | 681 | 535 | 220 |
| 5/19/2006 | 73,300 | 1908.30 | 567 | 756 | 571 | 249 |
| 5/20/2006 | 73,000 | 1908.20 | 615 | 807 | 633 | 266 |
| 5/21/2006 | 72,700 | 1908.10 | 567 | 756 | 608 | 241 |
| 5/22/2006 | 72,700 | 1908.10 | 623 | 655 | 488 | 254 |
| 5/23/2006 | 72,600 | 1908.00 | 524 | 605 | 467 | 229 |
| 5/24/2006 | 72,500 | 1908.00 | 462 | 539 | 372 | 242 |
| 5/25/2006 | 72,500 | 1908.00 | 450 | 474 | 375 | 175 |
| 5/26/2006 | 72,400 | 1907.90 | 399 | 473 | 371 | 177 |
| 5/27/2006 | 72,300 | 1907.90 | 397 | 471 | 367 | 177 |
| 5/28/2006 | 72,200 | 1907.80 | 396 | 470 | 364 | 177 |
| 5/29/2006 | 72,000 | 1907.70 | 344 | 468 | 362 | 177 |
| 5/30/2006 | 71,800 | 1907.60 | 342 | 466 | 359 | 177 |
| 5/31/2006 | 71,500 | 1907.50 | 289 | 463 | 358 | 175 |
| 6/1/2006 | 71,400 | 1907.40 | 387 | 460 | 357 | 174 |
| 6/2/2006 | 71,100 | 1907.30 | 282 | 456 | 334 | 191 |
| 6/3/2006 | 71,000 | 1907.30 | 315 | 385 | 285 | 190 |
| 6/4/2006 | 70,900 | 1907.20 | 269 | 336 | 298 | 134 |
| 6/5/2006 | 70,800 | 1907.20 | 216 | 280 | 230 | 133 |
| 6/6/2006 | 70,600 | 1907.10 | 163 | 278 | 222 | 133 |
| 6/7/2006 | 70,300 | 1906.90 | 113 | 278 | 218 | 134 |
| 6/8/2006 | 70,200 | 1906.90 | 213 | 277 | 216 | 134 |
| 6/9/2006 | 70,000 | 1906.80 | 161 | 276 | 214 | 133 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/10/2006 | 69,900 | 1906.70 | 211 | 275 | 216 | 132 |
| 6/11/2006 | 69,800 | 1906.70 | 210 | 274 | 201 | 142 |
| 6/12/2006 | 69,600 | 1906.60 | 159 | 273 | 182 | 155 |
| 6/13/2006 | 69,400 | 1906.50 | 140 | 254 | 167 | 155 |
| 6/14/2006 | 69,300 | 1906.50 | 176 | 238 | 137 | 155 |
| 6/15/2006 | 69,200 | 1906.40 | 141 | 201 | 125 | 148 |
| 6/16/2006 | 69,100 | 1906.40 | 131 | 191 | 99 | 128 |
| 6/17/2006 | 69,000 | 1906.30 | 134 | 194 | 107 | 132 |
| 6/18/2006 | 68,900 | 1906.30 | 131 | 191 | 95 | 134 |
| 6/19/2006 | 68,800 | 1906.20 | 130 | 190 | 92 | 134 |
| 6/20/2006 | 68,700 | 1906.20 | 128 | 188 | 89 | 134 |
| 6/21/2006 | 68,500 | 1906.10 | 76 | 186 | 84 | 134 |
| 6/22/2006 | 68,300 | 1906.00 | 92 | 203 | 92 | 134 |
| 6/23/2006 | 68,100 | 1905.90 | 102 | 213 | 118 | 134 |
| 6/24/2006 | 68,000 | 1905.80 | 148 | 209 | 112 | 134 |
| 6/25/2006 | 67,700 | 1905.70 | 37 | 198 | 97 | 134 |
| 6/26/2006 | 67,500 | 1905.60 | 86 | 197 | 91 | 134 |
| 6/27/2006 | 67,200 | 1905.40 | 33 | 194 | 88 | 134 |
| 6/28/2006 | 67,000 | 1905.40 | 81 | 191 | 85 | 134 |
| 6/29/2006 | 66,800 | 1905.30 | 79 | 189 | 74 | 133 |
| 6/30/2006 | 66,600 | 1905.20 | 86 | 197 | 76 | 133 |
| 7/1/2006 | 66,300 | 1905.00 | 22 | 182 | 52 | 138 |
| 7/2/2006 | 66,100 | 1904.90 | 70 | 180 | 43 | 141 |
| 7/3/2006 | 65,900 | 1904.80 | 70 | 180 | 37 | 142 |
| 7/4/2006 | 65,700 | 1904.70 | 70 | 180 | 36 | 143 |
| 7/5/2006 | 65,300 | 1904.50 | -31 | 180 | 36 | 142 |
| 7/6/2006 | 65,100 | 1904.40 | 70 | 180 | 37 | 142 |
| 7/7/2006 | 64,800 | 1904.30 | 19 | 179 | 39 | 139 |
| 7/8/2006 | 64,600 | 1904.20 | 69 | 179 | 38 | 140 |
| 7/9/2006 | 64,400 | 1904.10 | 69 | 179 | 37 | 141 |
| 7/10/2006 | 64,200 | 1904.00 | 69 | 179 | 35 | 141 |
| 7/11/2006 | 63,900 | 1903.80 | 19 | 179 | 36 | 136 |
| 7/12/2006 | 63,600 | 1903.70 | 19 | 179 | 36 | 137 |
| 7/13/2006 | 63,300 | 1903.50 | 19 | 179 | 34 | 140 |
| 7/14/2006 | 63,100 | 1903.40 | 69 | 179 | 32 | 141 |
| 7/15/2006 | 62,700 | 1903.20 | -32 | 179 | 30 | 140 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/16/2006 | 62,600 | 1903.20 | 120 | 179 | 33 | 139 |
| 7/17/2006 | 62,200 | 1903.00 | -32 | 179 | 37 | 133 |
| 7/18/2006 | 61,900 | 1902.80 | 19 | 179 | 39 | 133 |
| 7/19/2006 | 61,600 | 1902.70 | 20 | 180 | 39 | 133 |
| 7/20/2006 | 61,400 | 1902.60 | 70 | 180 | 40 | 133 |
| 7/21/2006 | 61,100 | 1902.40 | 20 | 180 | 40 | 135 |
| 7/22/2006 | 60,800 | 1902.30 | 20 | 180 | 39 | 133 |
| 7/23/2006 | 60,500 | 1902.10 | 19 | 179 | 35 | 133 |
| 7/24/2006 | 60,200 | 1902.00 | 19 | 179 | 36 | 132 |
| 7/25/2006 | 59,900 | 1901.80 | 19 | 179 | 36 | 129 |
| 7/26/2006 | 59,900 | 1901.80 | 171 | 180 | 35 | 130 |
| 7/27/2006 | 59,600 | 1901.70 | 18 | 178 | 35 | 132 |
| 7/28/2006 | 59,200 | 1901.50 | -33 | 178 | 36 | 132 |
| 7/29/2006 | 58,900 | 1901.30 | 17 | 177 | 38 | 131 |
| 7/30/2006 | 58,600 | 1901.10 | 17 | 177 | 39 | 132 |
| 7/31/2006 | 58,300 | 1901.00 | 17 | 177 | 40 | 133 |
| 8/1/2006 | 58,000 | 1900.80 | 17 | 177 | 41 | 132 |
| 8/2/2006 | 57,700 | 1900.70 | 19 | 179 | 41 | 131 |
| 8/3/2006 | 57,400 | 1900.50 | 21 | 181 | 42 | 132 |
| 8/4/2006 | 57,000 | 1900.30 | -31 | 180 | 44 | 132 |
| 8/5/2006 | 56,700 | 1900.20 | 20 | 180 | 44 | 132 |
| 8/6/2006 | 56,400 | 1900.00 | 20 | 180 | 43 | 131 |
| 8/7/2006 | 56,100 | 1899.80 | 20 | 180 | 43 | 132 |
| 8/8/2006 | 55,800 | 1899.70 | 19 | 179 | 44 | 131 |
| 8/9/2006 | 55,500 | 1899.50 | 19 | 179 | 43 | 132 |
| 8/10/2006 | 55,100 | 1899.30 | -29 | 182 | 43 | 131 |
| 8/11/2006 | 54,800 | 1899.10 | 25 | 185 | 43 | 133 |
| 8/12/2006 | 54,500 | 1899.00 | 25 | 185 | 43 | 134 |
| 8/13/2006 | 54,200 | 1898.80 | 25 | 185 | 42 | 134 |
| 8/14/2006 | 53,900 | 1898.60 | 25 | 185 | 42 | 135 |
| 8/15/2006 | 53,500 | 1898.40 | -27 | 184 | 41 | 135 |
| 8/16/2006 | 53,200 | 1898.30 | 24 | 184 | 40 | 134 |
| 8/17/2006 | 52,900 | 1898.10 | 23 | 183 | 40 | 135 |
| 8/18/2006 | 52,600 | 1897.90 | 23 | 183 | 40 | 134 |
| 8/19/2006 | 52,200 | 1897.70 | -29 | 182 | 40 | 134 |
| 8/20/2006 | 51,900 | 1897.50 | 21 | 181 | 40 | 134 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/21/2006 | 51,600 | 1897.40 | 20 | 180 | 40 | 134 |
| 8/22/2006 | 51,300 | 1897.20 | 20 | 180 | 39 | 134 |
| 8/23/2006 | 51,000 | 1897.00 | 19 | 179 | 39 | 134 |
| 8/24/2006 | 50,600 | 1896.80 | -32 | 179 | 39 | 134 |
| 8/25/2006 | 50,300 | 1896.60 | 18 | 178 | 38 | 134 |
| 8/26/2006 | 50,000 | 1896.50 | 17 | 177 | 40 | 134 |
| 8/27/2006 | 49,700 | 1896.30 | 17 | 177 | 41 | 134 |
| 8/28/2006 | 49,300 | 1896.10 | -34 | 176 | 39 | 134 |
| 8/29/2006 | 49,000 | 1895.90 | 18 | 178 | 39 | 133 |
| 8/30/2006 | 48,700 | 1895.70 | 21 | 181 | 38 | 140 |
| 8/31/2006 | 48,400 | 1895.60 | 21 | 181 | 37 | 136 |
| 9/1/2006 | 48,000 | 1895.30 | -31 | 180 | 39 | 134 |
| 9/2/2006 | 47,700 | 1895.10 | 21 | 181 | 38 | 136 |
| 9/3/2006 | 47,400 | 1895.00 | 21 | 181 | 38 | 135 |
| 9/4/2006 | 47,000 | 1894.70 | -30 | 181 | 40 | 134 |
| 9/5/2006 | 46,700 | 1894.60 | 21 | 181 | 39 | 126 |
| 9/6/2006 | 46,400 | 1894.40 | 21 | 181 | 40 | 129 |
| 9/7/2006 | 46,100 | 1894.20 | 22 | 182 | 41 | 132 |
| 9/8/2006 | 45,700 | 1894.00 | -30 | 181 | 41 | 135 |
| 9/9/2006 | 45,400 | 1893.80 | 25 | 186 | 42 | 135 |
| 9/10/2006 | 45,100 | 1893.60 | 26 | 187 | 41 | 135 |
| 9/11/2006 | 44,800 | 1893.40 | 28 | 189 | 40 | 135 |
| 9/12/2006 | 44,400 | 1893.20 | -21 | 190 | 41 | 136 |
| 9/13/2006 | 44,100 | 1893.00 | 25 | 185 | 41 | 137 |
| 9/14/2006 | 43,700 | 1892.80 | -22 | 189 | 39 | 136 |
| 9/15/2006 | 43,400 | 1892.60 | 28 | 189 | 40 | 137 |
| 9/16/2006 | 43,100 | 1892.40 | 1 | 160 | 43 | 115 |
| 9/17/2006 | 42,900 | 1892.30 | 24 | 131 | 40 | 99 |
| 9/18/2006 | 42,700 | 1892.20 | 23 | 130 | 39 | 98 |
| 9/19/2006 | 42,400 | 1892.00 | -26 | 132 | 39 | 92 |
| 9/20/2006 | 42,100 | 1891.80 | -5 | 154 | 39 | 94 |
| 9/21/2006 | 41,800 | 1891.60 | 22 | 182 | 42 | 115 |
| 9/22/2006 | 41,400 | 1891.30 | -11 | 201 | 41 | 137 |
| 9/23/2006 | 41,000 | 1891.10 | -11 | 200 | 42 | 148 |
| 9/24/2006 | 40,600 | 1890.80 | -12 | 200 | 41 | 168 |
| 9/25/2006 | 40,200 | 1890.60 | -13 | 199 | 41 | 171 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/26/2006 | 39,800 | 1890.30 | -13 | 198 | 40 | 207 |
| 9/27/2006 | 39,300 | 1890.00 | -64 | 198 | 40 | 204 |
| 9/28/2006 | 38,900 | 1889.70 | -15 | 197 | 41 | 202 |
| 9/29/2006 | 38,500 | 1889.50 | -15 | 196 | 42 | 169 |
| 9/30/2006 | 38,100 | 1889.20 | -16 | 196 | 42 | 161 |
| 10/1/2006 | 37,807 | 1889.00 | 38 | 195 | 40 | 154 |
| 10/2/2006 | 37,462 | 1888.80 | 14 | 198 | 40 | 153 |
| 10/3/2006 | 37,125 | 1888.60 | 22 | 202 | 43 | 172 |
| 10/4/2006 | 36,783 | 1888.30 | 19 | 201 | 43 | 198 |
| 10/5/2006 | 36,454 | 1888.10 | 26 | 202 | 43 | 171 |
| 10/6/2006 | 36,117 | 1887.90 | 21 | 201 | 43 | 157 |
| 10/7/2006 | 35,770 | 1887.70 | 17 | 202 | 43 | 159 |
| 10/8/2006 | 35,452 | 1887.40 | 32 | 202 | 43 | 159 |
| 10/9/2006 | 35,092 | 1887.20 | 16 | 208 | 36 | 158 |
| 10/10/2006 | 34,717 | 1886.90 | 15 | 214 | 29 | 163 |
| 10/11/2006 | 34,318 | 1886.60 | 8 | 221 | 34 | 185 |
| 10/12/2006 | 33,883 | 1886.30 | -4 | 227 | 42 | 202 |
| 10/13/2006 | 33,478 | 1886.00 | 12 | 228 | 40 | 202 |
| 10/14/2006 | 33,044 | 1885.70 | -2 | 229 | 38 | 201 |
| 10/15/2006 | 32,619 | 1885.40 | 4 | 230 | 37 | 201 |
| 10/16/2006 | 32,251 | 1885.10 | 33 | 230 | 37 | 203 |
| 10/17/2006 | 31,775 | 1884.80 | -20 | 231 | 39 | 198 |
| 10/18/2006 | 31,361 | 1884.50 | 12 | 232 | 36 | 201 |
| 10/19/2006 | 30,960 | 1884.20 | 19 | 233 | 37 | 198 |
| 10/20/2006 | 30,512 | 1883.80 | -3 | 234 | 39 | 206 |
| 10/21/2006 | 30,109 | 1883.50 | 20 | 235 | 42 | 204 |
| 10/22/2006 | 29,679 | 1883.20 | 7 | 236 | 45 | 206 |
| 10/23/2006 | 29,269 | 1882.90 | 18 | 237 | 43 | 207 |
| 10/24/2006 | 28,813 | 1882.50 | -4 | 238 | 46 | 201 |
| 10/25/2006 | 28,426 | 1882.20 | 32 | 239 | 47 | 205 |
| 10/26/2006 | 28,005 | 1881.90 | 15 | 239 | 51 | 197 |
| 10/27/2006 | 27,707 | 1881.60 | -7 | 151 | 71 | 148 |
| 10/28/2006 | 27,380 | 1881.30 | 16 | 190 | 44 | 151 |
| 10/29/2006 | 27,069 | 1881.10 | 23 | 189 | 45 | 150 |
| 10/30/2006 | 26,747 | 1880.80 | 17 | 189 | 45 | 151 |
| 10/31/2006 | 26,400 | 1880.50 | -46 | 136 | 44 | 125 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/1/2006 | 26,308 | 1880.40 | 73 | 126 | 45 | 46 |
| 11/2/2006 | 26,331 | 1880.50 | 95 | 88 | 43 | 44 |
| 11/3/2006 | 26,263 | 1880.40 | 40 | 78 | 41 | 43 |
| 11/4/2006 | 26,158 | 1880.30 | 31 | 88 | 47 | 43 |
| 11/5/2006 | 26,054 | 1880.20 | 27 | 84 | 52 | 45 |
| 11/6/2006 | 25,958 | 1880.10 | 30 | 82 | 44 | 44 |
| 11/7/2006 | 25,867 | 1880.10 | 32 | 82 | 41 | 46 |
| 11/8/2006 | 25,749 | 1880.00 | 18 | 82 | 41 | 45 |
| 11/9/2006 | 25,647 | 1879.90 | 26 | 82 | 41 | 45 |
| 11/10/2006 | 25,556 | 1879.80 | 30 | 80 | 40 | 45 |
| 11/11/2006 | 25,532 | 1879.80 | 66 | 82 | 41 | 45 |
| 11/12/2006 | 25,521 | 1879.80 | 73 | 83 | 41 | 45 |
| 11/13/2006 | 25,649 | 1879.90 | 144 | 84 | 48 | 45 |
| 11/14/2006 | 25,729 | 1879.90 | 119 | 83 | 76 | 45 |
| 11/15/2006 | 25,728 | 1879.90 | 84 | 89 | 96 | 45 |
| 11/16/2006 | 25,668 | 1879.90 | 69 | 104 | 116 | 45 |
| 11/17/2006 | 25,557 | 1879.80 | 54 | 116 | 125 | 45 |
| 11/18/2006 | 25,374 | 1879.60 | 45 | 144 | 166 | 44 |
| 11/19/2006 | 25,246 | 1879.50 | 84 | 156 | 140 | 45 |
| 11/20/2006 | 24,964 | 1879.30 | 6 | 156 | 141 | 45 |
| 11/21/2006 | 24,798 | 1879.10 | 50 | 141 | 137 | 45 |
| 11/22/2006 | 24,645 | 1879.00 | 43 | 126 | 103 | 43 |
| 11/23/2006 | 24,511 | 1878.90 | 51 | 125 | 110 | 45 |
| 11/24/2006 | 24,400 | 1878.80 | 53 | 115 | 97 | 46 |
| 11/25/2006 | 24,289 | 1878.70 | 45 | 106 | 85 | 46 |
| 11/26/2006 | 24,298 | 1878.70 | 104 | 105 | 103 | 45 |
| 11/27/2006 | 24,357 | 1878.70 | 110 | 85 | 96 | 45 |
| 11/28/2006 | 24,331 | 1878.70 | 68 | 85 | 49 | 45 |
| 11/29/2006 | 24,246 | 1878.60 | 57 | 105 | 79 | 45 |
| 11/30/2006 | 24,105 | 1878.50 | 42 | 119 | 102 | 45 |
| 12/1/2006 | 23,920 | 1878.30 | 50 | 151 | 124 | 45 |
| 12/2/2006 | 23,769 | 1878.20 | 41 | 123 | 143 | 17 |
| 12/3/2006 | 23,611 | 1878.10 | 42 | 128 | 153 | 1 |
| 12/4/2006 | 23,423 | 1877.90 | 28 | 129 | 153 | 1 |
| 12/5/2006 | 23,251 | 1877.70 | 36 | 129 | 152 | 0 |
| 12/6/2006 | 23,103 | 1877.60 | 49 | 130 | 153 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/7/2006 | 22,905 | 1877.40 | 26 | 132 | 149 | 1 |
| 12/8/2006 | 22,613 | 1877.10 | 44 | 202 | 178 | 29 |
| 12/9/2006 | 22,480 | 1877.00 | 142 | 220 | 166 | 72 |
| 12/10/2006 | 22,841 | 1877.30 | 392 | 221 | 241 | 82 |
| 12/11/2006 | 23,405 | 1877.90 | 500 | 227 | 219 | 93 |
| 12/12/2006 | 25,712 | 1879.90 | 1,383 | 232 | 397 | 148 |
| 12/13/2006 | 26,561 | 1880.70 | 651 | 235 | 225 | 200 |
| 12/14/2006 | 27,485 | 1881.40 | 697 | 243 | 186 | 262 |
| 12/15/2006 | 28,435 | 1882.20 | 766 | 302 | 219 | 262 |
| 12/16/2006 | 29,409 | 1883.00 | 834 | 361 | 172 | 226 |
| 12/17/2006 | 30,407 | 1883.80 | 812 | 325 | 236 | 114 |
| 12/18/2006 | 31,069 | 1884.30 | 603 | 283 | 247 | 58 |
| 12/19/2006 | 30,867 | 1884.10 | 162 | 277 | 184 | 125 |
| 12/20/2006 | 30,558 | 1883.90 | 156 | 328 | 191 | 156 |
| 12/21/2006 | 32,293 | 1885.20 | 1,234 | 378 | 350 | 236 |
| 12/22/2006 | 33,069 | 1885.70 | 750 | 378 | 250 | 264 |
| 12/23/2006 | 33,158 | 1885.80 | 404 | 378 | 199 | 235 |
| 12/24/2006 | 33,077 | 1885.80 | 318 | 378 | 182 | 228 |
| 12/25/2006 | 32,896 | 1885.60 | 268 | 378 | 180 | 220 |
| 12/26/2006 | 38,473 | 1889.50 | 3,219 | 429 | 703 | 209 |
| 12/27/2006 | 43,058 | 1892.40 | 2,696 | 405 | 626 | 246 |
| 12/28/2006 | 44,223 | 1893.10 | 950 | 382 | 309 | 170 |
| 12/29/2006 | 44,745 | 1893.40 | 604 | 358 | 236 | 130 |
| 12/30/2006 | 45,002 | 1893.60 | 448 | 335 | 186 | 136 |
| 12/31/2006 | 45,066 | 1893.60 | 328 | 311 | 168 | 146 |
| 1/1/2007 | 45,028 | 1893.60 | 218 | 288 | 144 | 152 |
| 1/2/2007 | 44,969 | 1893.50 | 191 | 265 | 126 | 152 |
| 1/3/2007 | 45,086 | 1893.60 | 278 | 263 | 148 | 153 |
| 1/4/2007 | 46,005 | 1894.20 | 682 | 262 | 302 | 158 |
| 1/5/2007 | 46,433 | 1894.40 | 433 | 260 | 212 | 160 |
| 1/6/2007 | 46,678 | 1894.60 | 366 | 296 | 205 | 164 |
| 1/7/2007 | 46,817 | 1894.60 | 325 | 314 | 202 | 163 |
| 1/8/2007 | 46,850 | 1894.70 | 270 | 312 | 216 | 155 |
| 1/9/2007 | 46,840 | 1894.60 | 247 | 310 | 211 | 156 |
| 1/10/2007 | 46,764 | 1894.60 | 212 | 308 | 206 | 161 |
| 1/11/2007 | 46,698 | 1894.60 | 216 | 306 | 185 | 160 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/12/2007 | 46,493 | 1894.40 | 145 | 304 | 183 | 160 |
| 1/13/2007 | 46,148 | 1894.20 | 72 | 302 | 185 | 159 |
| 1/14/2007 | 45,802 | 1894.00 | 71 | 300 | 181 | 159 |
| 1/15/2007 | 45,534 | 1893.90 | 108 | 297 | 185 | 159 |
| 1/16/2007 | 45,251 | 1893.70 | 99 | 295 | 184 | 159 |
| 1/17/2007 | 44,953 | 1893.50 | 90 | 293 | 186 | 158 |
| 1/18/2007 | 44,652 | 1893.30 | 87 | 291 | 188 | 158 |
| 1/19/2007 | 44,316 | 1893.10 | 68 | 289 | 182 | 161 |
| 1/20/2007 | 43,993 | 1892.90 | 73 | 287 | 178 | 161 |
| 1/21/2007 | 43,667 | 1892.70 | 71 | 285 | 172 | 161 |
| 1/22/2007 | 43,360 | 1892.60 | 79 | 283 | 164 | 157 |
| 1/23/2007 | 43,026 | 1892.40 | 64 | 281 | 159 | 160 |
| 1/24/2007 | 42,714 | 1892.20 | 73 | 279 | 160 | 157 |
| 1/25/2007 | 42,381 | 1892.00 | 61 | 277 | 160 | 156 |
| 1/26/2007 | 42,038 | 1891.70 | 55 | 275 | 157 | 155 |
| 1/27/2007 | 41,721 | 1891.50 | 66 | 273 | 150 | 154 |
| 1/28/2007 | 41,444 | 1891.40 | 85 | 271 | 129 | 159 |
| 1/29/2007 | 41,157 | 1891.20 | 79 | 269 | 115 | 155 |
| 1/30/2007 | 40,874 | 1891.00 | 79 | 267 | 115 | 159 |
| 1/31/2007 | 40,601 | 1890.80 | 83 | 264 | 106 | 160 |
| 2/1/2007 | 40,306 | 1890.70 | 70 | 262 | 102 | 159 |
| 2/2/2007 | 40,014 | 1890.50 | 70 | 260 | 103 | 158 |
| 2/3/2007 | 39,726 | 1890.30 | 71 | 258 | 102 | 159 |
| 2/4/2007 | 39,434 | 1890.10 | 67 | 256 | 102 | 160 |
| 2/5/2007 | 39,113 | 1889.90 | 51 | 254 | 102 | 160 |
| 2/6/2007 | 38,824 | 1889.70 | 66 | 252 | 102 | 160 |
| 2/7/2007 | 38,761 | 1889.70 | 158 | 220 | 102 | 116 |
| 2/8/2007 | 40,023 | 1890.50 | 790 | 169 | 105 | 49 |
| 2/9/2007 | 45,251 | 1893.70 | 2,770 | 142 | 112 | 49 |
| 2/10/2007 | 56,791 | 1900.20 | 5,994 | 202 | 1,008 | 49 |
| 2/11/2007 | 57,811 | 1900.70 | 2,314 | 2,521 | 4,033 | 23 |
| 2/12/2007 | 57,003 | 1900.30 | 1,476 | 2,640 | 3,025 | 61 |
| 2/13/2007 | 56,648 | 1900.10 | 1,179 | 1,890 | 2,269 | 119 |
| 2/14/2007 | 56,173 | 1899.90 | 776 | 1,400 | 1,512 | 154 |
| 2/15/2007 | 55,796 | 1899.70 | 551 | 1,008 | 1,109 | 152 |
| 2/16/2007 | 55,438 | 1899.50 | 490 | 907 | 856 | 153 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/17/2007 | 55,270 | 1899.40 | 480 | 756 | 635 | 157 |
| 2/18/2007 | 55,082 | 1899.30 | 382 | 630 | 508 | 159 |
| 2/19/2007 | 54,989 | 1899.20 | 377 | 555 | 424 | 158 |
| 2/20/2007 | 54,877 | 1899.20 | 332 | 504 | 361 | 158 |
| 2/21/2007 | 54,989 | 1899.20 | 445 | 504 | 359 | 157 |
| 2/22/2007 | 56,363 | 1900.00 | 1,610 | 1,260 | 1,210 | 153 |
| 2/23/2007 | 55,947 | 1899.70 | 806 | 1,400 | 1,400 | 150 |
| 2/24/2007 | 56,648 | 1900.10 | 1,264 | 1,250 | 1,330 | 119 |
| 2/25/2007 | 57,761 | 1900.70 | 2,626 | 2,900 | 3,580 | 45 |
| 2/26/2007 | 57,126 | 1900.40 | 1,437 | 2,460 | 3,050 | 45 |
| 2/27/2007 | 56,648 | 1900.10 | 1,138 | 1,920 | 2,380 | 56 |
| 2/28/2007 | 56,192 | 1899.90 | 869 | 1,520 | 1,720 | 107 |
| 3/1/2007 | 55,890 | 1899.70 | 793 | 1,300 | 1,280 | 154 |
| 3/2/2007 | 55,758 | 1899.60 | 781 | 1,160 | 1,060 | 152 |
| 3/3/2007 | 55,928 | 1899.70 | 940 | 1,170 | 1,060 | 155 |
| 3/4/2007 | 56,022 | 1899.80 | 986 | 1,290 | 1,210 | 154 |
| 3/5/2007 | 56,022 | 1899.80 | 959 | 1,320 | 1,250 | 154 |
| 3/6/2007 | 56,003 | 1899.80 | 943 | 1,310 | 1,230 | 154 |
| 3/7/2007 | 55,965 | 1899.80 | 912 | 1,280 | 1,220 | 118 |
| 3/8/2007 | 55,871 | 1899.70 | 856 | 1,240 | 1,160 | 151 |
| 3/9/2007 | 55,796 | 1899.70 | 809 | 1,160 | 1,109 | 82 |
| 3/10/2007 | 55,645 | 1899.60 | 701 | 1,060 | 1,059 | 42 |
| 3/11/2007 | 55,683 | 1899.60 | 782 | 1,040 | 1,008 | 42 |
| 3/12/2007 | 56,477 | 1900.00 | 993 | 797 | 857 | 42 |
| 3/13/2007 | 57,703 | 1900.70 | 973 | 457 | 504 | 42 |
| 3/14/2007 | 58,674 | 1901.20 | 864 | 484 | 479 | 43 |
| 3/15/2007 | 59,400 | 1901.60 | 722 | 458 | 454 | 43 |
| 3/16/2007 | 60,153 | 1902.00 | 651 | 337 | 345 | 42 |
| 3/17/2007 | 60,952 | 1902.40 | 599 | 230 | 242 | 42 |
| 3/18/2007 | 61,656 | 1902.70 | 556 | 237 | 240 | 43 |
| 3/19/2007 | 62,223 | 1903.00 | 491 | 242 | 232 | 42 |
| 3/20/2007 | 62,781 | 1903.30 | 491 | 249 | 233 | 40 |
| 3/21/2007 | 63,266 | 1903.50 | 449 | 241 | 225 | 44 |
| 3/22/2007 | 63,754 | 1903.80 | 436 | 221 | 204 | 42 |
| 3/23/2007 | 64,243 | 1904.00 | 433 | 216 | 192 | 42 |
| 3/24/2007 | 64,735 | 1904.20 | 437 | 220 | 192 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/25/2007 | 65,230 | 1904.50 | 440 | 222 | 192 | 43 |
| 3/26/2007 | 65,214 | 1904.50 | 185 | 226 | 212 | 43 |
| 3/27/2007 | 65,263 | 1904.50 | 222 | 232 | 210 | 43 |
| 3/28/2007 | 65,296 | 1904.50 | 214 | 231 | 172 | 75 |
| 3/29/2007 | 65,231 | 1904.50 | 164 | 231 | 157 | 87 |
| 3/30/2007 | 65,304 | 1904.50 | 233 | 230 | 166 | 71 |
| 3/31/2007 | 65,427 | 1904.60 | 258 | 229 | 178 | 50 |
| 4/1/2007 | 65,531 | 1904.60 | 248 | 229 | 173 | 42 |
| 4/2/2007 | 65,714 | 1904.70 | 287 | 228 | 192 | 42 |
| 4/3/2007 | 65,739 | 1904.70 | 207 | 227 | 195 | 41 |
| 4/4/2007 | 65,755 | 1904.70 | 202 | 227 | 199 | 43 |
| 4/5/2007 | 65,722 | 1904.70 | 177 | 226 | 198 | 43 |
| 4/6/2007 | 65,689 | 1904.70 | 176 | 225 | 198 | 43 |
| 4/7/2007 | 65,649 | 1904.70 | 172 | 225 | 193 | 42 |
| 4/8/2007 | 65,649 | 1904.70 | 192 | 224 | 179 | 42 |
| 4/9/2007 | 65,714 | 1904.70 | 224 | 223 | 178 | 42 |
| 4/10/2007 | 65,567 | 1904.70 | 117 | 222 | 195 | 42 |
| 4/11/2007 | 65,608 | 1904.70 | 211 | 222 | 201 | 74 |
| 4/12/2007 | 65,403 | 1904.60 | 87 | 221 | 208 | 74 |
| 4/13/2007 | 65,313 | 1904.50 | 144 | 220 | 171 | 66 |
| 4/14/2007 | 65,255 | 1904.50 | 160 | 220 | 169 | 69 |
| 4/15/2007 | 65,190 | 1904.50 | 156 | 219 | 171 | 58 |
| 4/16/2007 | 65,108 | 1904.40 | 147 | 218 | 168 | 53 |
| 4/17/2007 | 65,084 | 1904.40 | 176 | 218 | 163 | 53 |
| 4/18/2007 | 64,978 | 1904.40 | 134 | 217 | 158 | 53 |
| 4/19/2007 | 64,937 | 1904.30 | 166 | 216 | 158 | 52 |
| 4/20/2007 | 64,896 | 1904.30 | 166 | 216 | 157 | 53 |
| 4/21/2007 | 65,313 | 1904.50 | 396 | 215 | 183 | 53 |
| 4/22/2007 | 66,306 | 1905.00 | 671 | 193 | 309 | 60 |
| 4/23/2007 | 66,553 | 1905.10 | 311 | 216 | 159 | 87 |
| 4/24/2007 | 66,727 | 1905.20 | 299 | 252 | 180 | 88 |
| 4/25/2007 | 66,768 | 1905.20 | 232 | 251 | 198 | 88 |
| 4/26/2007 | 66,793 | 1905.30 | 223 | 250 | 193 | 88 |
| 4/27/2007 | 66,727 | 1905.20 | 177 | 249 | 189 | 88 |
| 4/28/2007 | 66,686 | 1905.20 | 188 | 248 | 185 | 87 |
| 4/29/2007 | 66,603 | 1905.20 | 167 | 247 | 183 | 87 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/30/2007 | 66,471 | 1905.10 | 141 | 247 | 180 | 88 |
| 5/1/2007 | 66,405 | 1905.10 | 174 | 246 | 193 | 88 |
| 5/2/2007 | 66,306 | 1905.00 | 157 | 245 | 213 | 88 |
| 5/3/2007 | 66,281 | 1905.00 | 193 | 244 | 206 | 88 |
| 5/4/2007 | 66,281 | 1905.00 | 205 | 243 | 221 | 88 |
| 5/5/2007 | 66,215 | 1905.00 | 171 | 242 | 203 | 88 |
| 5/6/2007 | 66,092 | 1904.90 | 142 | 241 | 190 | 88 |
| 5/7/2007 | 65,968 | 1904.90 | 141 | 240 | 190 | 85 |
| 5/8/2007 | 65,821 | 1904.80 | 129 | 239 | 181 | 88 |
| 5/9/2007 | 65,632 | 1904.70 | 107 | 238 | 176 | 89 |
| 5/10/2007 | 65,419 | 1904.60 | 94 | 237 | 174 | 88 |
| 5/11/2007 | 65,108 | 1904.40 | 44 | 236 | 224 | 99 |
| 5/12/2007 | 64,880 | 1904.30 | 85 | 235 | 172 | 89 |
| 5/13/2007 | 64,693 | 1904.20 | 105 | 234 | 159 | 90 |
| 5/14/2007 | 64,482 | 1904.10 | 92 | 234 | 158 | 89 |
| 5/15/2007 | 64,255 | 1904.00 | 84 | 233 | 140 | 109 |
| 5/16/2007 | 63,988 | 1903.90 | 63 | 232 | 128 | 131 |
| 5/17/2007 | 63,715 | 1903.70 | 59 | 231 | 127 | 131 |
| 5/18/2007 | 63,449 | 1903.60 | 62 | 230 | 125 | 130 |
| 5/19/2007 | 63,201 | 1903.50 | 70 | 229 | 116 | 130 |
| 5/20/2007 | 62,912 | 1903.30 | 49 | 228 | 102 | 130 |
| 5/21/2007 | 62,689 | 1903.20 | 82 | 227 | 109 | 125 |
| 5/22/2007 | 62,466 | 1903.10 | 69 | 209 | 117 | 131 |
| 5/23/2007 | 62,220 | 1903.00 | 58 | 209 | 97 | 127 |
| 5/24/2007 | 61,982 | 1902.90 | 67 | 217 | 86 | 117 |
| 5/25/2007 | 61,738 | 1902.80 | 64 | 217 | 85 | 118 |
| 5/26/2007 | 61,478 | 1902.60 | 50 | 208 | 83 | 117 |
| 5/27/2007 | 61,234 | 1902.50 | 51 | 198 | 74 | 118 |
| 5/28/2007 | 61,029 | 1902.40 | 64 | 188 | 70 | 117 |
| 5/29/2007 | 60,747 | 1902.30 | 30 | 195 | 69 | 117 |
| 5/30/2007 | 60,490 | 1902.10 | 49 | 205 | 65 | 126 |
| 5/31/2007 | 60,233 | 1902.00 | 46 | 200 | 62 | 131 |
| 6/1/2007 | 59,977 | 1901.90 | 41 | 192 | 58 | 130 |
| 6/2/2007 | 59,714 | 1901.70 | 36 | 190 | 58 | 129 |
| 6/3/2007 | 59,459 | 1901.60 | 40 | 190 | 58 | 129 |
| 6/4/2007 | 59,205 | 1901.50 | 40 | 190 | 60 | 128 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/5/2007 | 58,929 | 1901.30 | 25 | 184 | 57 | 129 |
| 6/6/2007 | 58,616 | 1901.20 | 2 | 178 | 53 | 128 |
| 6/7/2007 | 58,380 | 1901.00 | 41 | 178 | 48 | 129 |
| 6/8/2007 | 58,129 | 1900.90 | 33 | 178 | 47 | 130 |
| 6/9/2007 | 57,879 | 1900.80 | 34 | 178 | 47 | 129 |
| 6/10/2007 | 57,622 | 1900.60 | 30 | 178 | 46 | 129 |
| 6/11/2007 | 57,374 | 1900.50 | 34 | 177 | 46 | 130 |
| 6/12/2007 | 57,104 | 1900.40 | 23 | 177 | 45 | 130 |
| 6/13/2007 | 56,858 | 1900.20 | 32 | 173 | 42 | 130 |
| 6/14/2007 | 56,612 | 1900.10 | 27 | 166 | 35 | 130 |
| 6/15/2007 | 56,359 | 1900.00 | 23 | 164 | 34 | 130 |
| 6/16/2007 | 56,100 | 1899.80 | 20 | 164 | 33 | 129 |
| 6/17/2007 | 55,850 | 1899.70 | 24 | 164 | 32 | 130 |
| 6/18/2007 | 55,570 | 1899.50 | 7 | 161 | 31 | 130 |
| 6/19/2007 | 55,306 | 1899.40 | 12 | 156 | 29 | 130 |
| 6/20/2007 | 55,028 | 1899.30 | 4 | 156 | 29 | 127 |
| 6/21/2007 | 54,744 | 1899.10 | 1 | 156 | 29 | 127 |
| 6/22/2007 | 54,483 | 1899.00 | 13 | 156 | 28 | 129 |
| 6/23/2007 | 54,230 | 1898.80 | 17 | 156 | 26 | 129 |
| 6/24/2007 | 53,949 | 1898.70 | 3 | 156 | 25 | 128 |
| 6/25/2007 | 53,711 | 1898.50 | 24 | 156 | 25 | 127 |
| 6/26/2007 | 53,395 | 1898.40 | -15 | 156 | 25 | 130 |
| 6/27/2007 | 53,145 | 1898.20 | 18 | 156 | 24 | 130 |
| 6/28/2007 | 52,867 | 1898.10 | 4 | 156 | 23 | 130 |
| 6/29/2007 | 52,597 | 1897.90 | 8 | 156 | 23 | 129 |
| 6/30/2007 | 52,307 | 1897.80 | -2 | 156 | 23 | 129 |
| 7/1/2007 | 52,018 | 1897.60 | -1 | 156 | 24 | 130 |
| 7/2/2007 | 51,743 | 1897.40 | 6 | 156 | 23 | 130 |
| 7/3/2007 | 51,462 | 1897.30 | 2 | 155 | 22 | 131 |
| 7/4/2007 | 51,189 | 1897.10 | 6 | 155 | 22 | 127 |
| 7/5/2007 | 50,923 | 1897.00 | 10 | 155 | 22 | 130 |
| 7/6/2007 | 50,639 | 1896.80 | 1 | 155 | 22 | 131 |
| 7/7/2007 | 50,334 | 1896.70 | -10 | 155 | 22 | 131 |
| 7/8/2007 | 50,051 | 1896.50 | 1 | 155 | 22 | 130 |
| 7/9/2007 | 49,754 | 1896.30 | -6 | 155 | 22 | 130 |
| 7/10/2007 | 49,473 | 1896.20 | 2 | 155 | 22 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/11/2007 | 49,192 | 1896.00 | 2 | 155 | 22 | 130 |
| 7/12/2007 | 48,893 | 1895.80 | -7 | 155 | 22 | 129 |
| 7/13/2007 | 48,601 | 1895.70 | -3 | 155 | 22 | 130 |
| 7/14/2007 | 48,303 | 1895.50 | -7 | 155 | 22 | 130 |
| 7/15/2007 | 48,013 | 1895.30 | -3 | 155 | 22 | 130 |
| 7/16/2007 | 47,717 | 1895.20 | -6 | 155 | 22 | 130 |
| 7/17/2007 | 47,409 | 1895.00 | -12 | 155 | 22 | 130 |
| 7/18/2007 | 47,156 | 1894.80 | 16 | 155 | 22 | 130 |
| 7/19/2007 | 46,883 | 1894.70 | 6 | 155 | 22 | 130 |
| 7/20/2007 | 46,592 | 1894.50 | -3 | 155 | 22 | 130 |
| 7/21/2007 | 46,288 | 1894.30 | -10 | 154 | 22 | 129 |
| 7/22/2007 | 46,005 | 1894.20 | 1 | 154 | 22 | 129 |
| 7/23/2007 | 45,749 | 1894.00 | 14 | 154 | 22 | 130 |
| 7/24/2007 | 45,417 | 1893.80 | -24 | 154 | 22 | 129 |
| 7/25/2007 | 45,138 | 1893.60 | 3 | 154 | 22 | 129 |
| 7/26/2007 | 44,833 | 1893.50 | -11 | 154 | 22 | 129 |
| 7/27/2007 | 44,535 | 1893.30 | -7 | 154 | 22 | 129 |
| 7/28/2007 | 44,227 | 1893.10 | -12 | 154 | 22 | 129 |
| 7/29/2007 | 43,932 | 1892.90 | -6 | 154 | 22 | 130 |
| 7/30/2007 | 43,620 | 1892.70 | -14 | 154 | 22 | 130 |
| 7/31/2007 | 43,328 | 1892.50 | -4 | 154 | 23 | 130 |
| 8/1/2007 | 43,017 | 1892.40 | -14 | 154 | 23 | 129 |
| 8/2/2007 | 42,714 | 1892.20 | -10 | 154 | 22 | 129 |
| 8/3/2007 | 42,387 | 1892.00 | -22 | 154 | 23 | 129 |
| 8/4/2007 | 42,088 | 1891.80 | -8 | 154 | 23 | 130 |
| 8/5/2007 | 41,771 | 1891.60 | -17 | 154 | 22 | 131 |
| 8/6/2007 | 41,466 | 1891.40 | -11 | 154 | 23 | 130 |
| 8/7/2007 | 41,163 | 1891.20 | -10 | 153 | 22 | 130 |
| 8/8/2007 | 40,849 | 1891.00 | -16 | 153 | 23 | 131 |
| 8/9/2007 | 40,555 | 1890.80 | -6 | 153 | 23 | 131 |
| 8/10/2007 | 40,239 | 1890.60 | -17 | 153 | 23 | 130 |
| 8/11/2007 | 39,929 | 1890.40 | -14 | 153 | 23 | 130 |
| 8/12/2007 | 39,620 | 1890.20 | -13 | 153 | 23 | 130 |
| 8/13/2007 | 39,326 | 1890.00 | -6 | 153 | 22 | 130 |
| 8/14/2007 | 39,008 | 1889.80 | -18 | 153 | 22 | 129 |
| 8/15/2007 | 38,717 | 1889.60 | -4 | 153 | 22 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/16/2007 | 38,414 | 1889.40 | -10 | 153 | 23 | 129 |
| 8/17/2007 | 38,106 | 1889.20 | -13 | 153 | 23 | 128 |
| 8/18/2007 | 37,807 | 1889.00 | -8 | 153 | 23 | 130 |
| 8/19/2007 | 37,508 | 1888.80 | -8 | 153 | 23 | 130 |
| 8/20/2007 | 37,224 | 1888.60 | -1 | 153 | 22 | 130 |
| 8/21/2007 | 36,927 | 1888.40 | -8 | 153 | 21 | 129 |
| 8/22/2007 | 36,633 | 1888.20 | -6 | 153 | 22 | 130 |
| 8/23/2007 | 36,340 | 1888.00 | -6 | 153 | 21 | 130 |
| 8/24/2007 | 36,031 | 1887.80 | -14 | 153 | 21 | 130 |
| 8/25/2007 | 35,724 | 1887.60 | -13 | 152 | 22 | 129 |
| 8/26/2007 | 35,424 | 1887.40 | -9 | 152 | 22 | 130 |
| 8/27/2007 | 35,120 | 1887.20 | -11 | 152 | 22 | 129 |
| 8/28/2007 | 34,835 | 1887.00 | -2 | 152 | 21 | 129 |
| 8/29/2007 | 34,517 | 1886.80 | -18 | 152 | 21 | 129 |
| 8/30/2007 | 34,224 | 1886.60 | -6 | 152 | 22 | 129 |
| 8/31/2007 | 33,937 | 1886.40 | -3 | 152 | 22 | 131 |
| 9/1/2007 | 33,614 | 1886.10 | -21 | 152 | 22 | 131 |
| 9/2/2007 | 33,326 | 1885.90 | -4 | 152 | 22 | 131 |
| 9/3/2007 | 33,006 | 1885.70 | -20 | 152 | 22 | 131 |
| 9/4/2007 | 32,689 | 1885.50 | -18 | 152 | 22 | 131 |
| 9/5/2007 | 32,373 | 1885.20 | -18 | 152 | 22 | 130 |
| 9/6/2007 | 32,086 | 1885.00 | -3 | 152 | 21 | 130 |
| 9/7/2007 | 31,790 | 1884.80 | -8 | 152 | 23 | 131 |
| 9/8/2007 | 31,497 | 1884.60 | -6 | 152 | 23 | 131 |
| 9/9/2007 | 31,188 | 1884.40 | -14 | 152 | 23 | 130 |
| 9/10/2007 | 30,893 | 1884.10 | -7 | 152 | 21 | 131 |
| 9/11/2007 | 30,604 | 1883.90 | -4 | 151 | 23 | 130 |
| 9/12/2007 | 30,302 | 1883.70 | -11 | 151 | 22 | 130 |
| 9/13/2007 | 30,017 | 1883.50 | -2 | 151 | 22 | 130 |
| 9/14/2007 | 29,734 | 1883.20 | -3 | 151 | 22 | 130 |
| 9/15/2007 | 29,438 | 1883.00 | -3 | 151 | 23 | 131 |
| 9/16/2007 | 29,194 | 1882.80 | -3 | 125 | 23 | 112 |
| 9/17/2007 | 28,981 | 1882.60 | -3 | 114 | 23 | 91 |
| 9/18/2007 | 28,764 | 1882.50 | -3 | 110 | 22 | 89 |
| 9/19/2007 | 28,553 | 1882.30 | -3 | 111 | 22 | 89 |
| 9/20/2007 | 28,353 | 1882.10 | -3 | 103 | 22 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/21/2007 | 28,155 | 1882.00 | -3 | 111 | 23 | 89 |
| 9/22/2007 | 27,957 | 1881.80 | -3 | 110 | 22 | 89 |
| 9/23/2007 | 27,746 | 1881.60 | -3 | 110 | 23 | 89 |
| 9/24/2007 | 27,550 | 1881.50 | -3 | 110 | 22 | 87 |
| 9/25/2007 | 27,346 | 1881.30 | -3 | 110 | 22 | 88 |
| 9/26/2007 | 27,139 | 1881.10 | -3 | 112 | 21 | 87 |
| 9/27/2007 | 26,938 | 1881.00 | -3 | 112 | 23 | 88 |
| 9/28/2007 | 26,723 | 1880.80 | -3 | 112 | 24 | 87 |
| 9/29/2007 | 26,520 | 1880.60 | -3 | 113 | 23 | 88 |
| 9/30/2007 | 26,318 | 1880.50 | -3 | 113 | 23 | 88 |
| 10/1/2007 | 26,100 | 1880.30 | -3 | 113 | 22 | 86 |
| 10/2/2007 | 25,900 | 1880.10 | -3 | 113 | 22 | 87 |
| 10/3/2007 | 25,700 | 1879.90 | -3 | 115 | 22 | 87 |
| 10/4/2007 | 25,500 | 1879.80 | -3 | 115 | 23 | 88 |
| 10/5/2007 | 25,300 | 1879.60 | -3 | 117 | 24 | 89 |
| 10/6/2007 | 25,100 | 1879.40 | -3 | 112 | 23 | 89 |
| 10/7/2007 | 24,900 | 1879.20 | -3 | 110 | 24 | 88 |
| 10/8/2007 | 24,725 | 1879.10 | 27 | 114 | 23 | 88 |
| 10/9/2007 | 24,550 | 1878.90 | 28 | 115 | 26 | 87 |
| 10/10/2007 | 24,375 | 1878.80 | 28 | 116 | 26 | 88 |
| 10/11/2007 | 24,200 | 1878.60 | 28 | 115 | 28 | 87 |
| 10/12/2007 | 24,033 | 1878.40 | 34 | 118 | 26 | 88 |
| 10/13/2007 | 23,867 | 1878.30 | 34 | 118 | 25 | 89 |
| 10/14/2007 | 23,700 | 1878.10 | 30 | 113 | 26 | 89 |
| 10/15/2007 | 23,600 | 1878.00 | 62 | 110 | 27 | 88 |
| 10/16/2007 | 23,500 | 1877.90 | 38 | 76 | 27 | 59 |
| 10/17/2007 | 23,400 | 1877.90 | 37 | 75 | 27 | 46 |
| 10/18/2007 | 23,400 | 1877.90 | 92 | 81 | 27 | 45 |
| 10/19/2007 | 23,400 | 1877.90 | 91 | 79 | 69 | 43 |
| 10/20/2007 | 23,500 | 1877.90 | 139 | 76 | 54 | 44 |
| 10/21/2007 | 23,400 | 1877.90 | 43 | 83 | 41 | 44 |
| 10/22/2007 | 23,300 | 1877.80 | 52 | 96 | 61 | 44 |
| 10/23/2007 | 23,167 | 1877.60 | 39 | 101 | 54 | 44 |
| 10/24/2007 | 23,033 | 1877.50 | 39 | 101 | 58 | 44 |
| 10/25/2007 | 22,900 | 1877.40 | 39 | 102 | 58 | 44 |
| 10/26/2007 | 22,767 | 1877.30 | 41 | 104 | 59 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/27/2007 | 22,633 | 1877.10 | 37 | 99 | 53 | 46 |
| 10/28/2007 | 22,500 | 1877.00 | 29 | 87 | 44 | 46 |
| 10/29/2007 | 22,400 | 1876.90 | 41 | 80 | 33 | 44 |
| 10/30/2007 | 22,300 | 1876.80 | 40 | 79 | 30 | 44 |
| 10/31/2007 | 22,200 | 1876.70 | 40 | 79 | 30 | 44 |
| 11/1/2007 | 22,100 | 1876.60 | 40 | 79 | 30 | 44 |
| 11/2/2007 | 22,000 | 1876.50 | 40 | 79 | 30 | 44 |
| 11/3/2007 | 21,900 | 1876.40 | 40 | 79 | 29 | 44 |
| 11/4/2007 | 21,800 | 1876.30 | 40 | 79 | 29 | 44 |
| 11/5/2007 | 21,700 | 1876.20 | 40 | 79 | 29 | 44 |
| 11/6/2007 | 21,600 | 1876.10 | 40 | 79 | 29 | 29 |
| 11/7/2007 | 21,500 | 1876.00 | 40 | 79 | 28 | 45 |
| 11/8/2007 | 21,400 | 1875.90 | 40 | 79 | 28 | 44 |
| 11/9/2007 | 21,300 | 1875.80 | 40 | 79 | 28 | 44 |
| 11/10/2007 | 21,300 | 1875.80 | 76 | 80 | 28 | 46 |
| 11/11/2007 | 21,225 | 1875.70 | 38 | 80 | 29 | 47 |
| 11/12/2007 | 21,150 | 1875.70 | 37 | 79 | 29 | 38 |
| 11/13/2007 | 21,075 | 1875.60 | 37 | 79 | 29 | 32 |
| 11/14/2007 | 21,000 | 1875.50 | 37 | 79 | 30 | 48 |
| 11/15/2007 | 20,900 | 1875.40 | 27 | 81 | 29 | 45 |
| 11/16/2007 | 20,820 | 1875.30 | 38 | 82 | 30 | 45 |
| 11/17/2007 | 20,740 | 1875.20 | 39 | 83 | 30 | 45 |
| 11/18/2007 | 20,660 | 1875.20 | 39 | 83 | 31 | 44 |
| 11/19/2007 | 20,580 | 1875.10 | 39 | 83 | 31 | 44 |
| 11/20/2007 | 20,500 | 1875.00 | 39 | 83 | 30 | 45 |
| 11/21/2007 | 20,400 | 1874.90 | 28 | 83 | 29 | 45 |
| 11/22/2007 | 20,300 | 1874.80 | 28 | 83 | 29 | 46 |
| 11/23/2007 | 20,200 | 1874.70 | 28 | 83 | 29 | 46 |
| 11/24/2007 | 20,100 | 1874.60 | 28 | 83 | 29 | 46 |
| 11/25/2007 | 20,000 | 1874.50 | 28 | 83 | 29 | 46 |
| 11/26/2007 | 19,900 | 1874.40 | 28 | 83 | 29 | 46 |
| 11/27/2007 | 19,800 | 1874.30 | 28 | 83 | 29 | 46 |
| 11/28/2007 | 19,600 | 1874.00 | 56 | 165 | 29 | 45 |
| 11/29/2007 | 19,500 | 1873.90 | 27 | 82 | 29 | 43 |
| 11/30/2007 | 19,400 | 1873.80 | 27 | 81 | 29 | 43 |
| 12/1/2007 | 19,200 | 1873.60 | 12 | 119 | 29 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/2/2007 | 19,000 | 1873.40 | 50 | 159 | 74 | 43 |
| 12/3/2007 | 18,900 | 1873.30 | 108 | 167 | 103 | 43 |
| 12/4/2007 | 20,000 | 1874.50 | 729 | 184 | 280 | 44 |
| 12/5/2007 | 20,000 | 1874.50 | 166 | 175 | 135 | 44 |
| 12/6/2007 | 20,000 | 1874.50 | 200 | 211 | 148 | 44 |
| 12/7/2007 | 20,300 | 1874.80 | 364 | 224 | 206 | 44 |
| 12/8/2007 | 20,200 | 1874.70 | 156 | 217 | 156 | 43 |
| 12/9/2007 | 20,100 | 1874.60 | 154 | 215 | 146 | 44 |
| 12/10/2007 | 19,840 | 1874.30 | 76 | 218 | 145 | 43 |
| 12/11/2007 | 19,580 | 1874.00 | 65 | 206 | 135 | 43 |
| 12/12/2007 | 19,320 | 1873.70 | 74 | 216 | 141 | 41 |
| 12/13/2007 | 19,060 | 1873.40 | 87 | 230 | 149 | 43 |
| 12/14/2007 | 18,800 | 1873.20 | 65 | 206 | 133 | 43 |
| 12/15/2007 | 18,600 | 1872.90 | 83 | 193 | 126 | 43 |
| 12/16/2007 | 18,400 | 1872.70 | 76 | 186 | 123 | 44 |
| 12/17/2007 | 18,200 | 1872.50 | 72 | 182 | 125 | 45 |
| 12/18/2007 | 19,400 | 1873.80 | 749 | 152 | 314 | 45 |
| 12/19/2007 | 20,200 | 1874.70 | 571 | 176 | 212 | 44 |
| 12/20/2007 | 22,700 | 1877.20 | 1,453 | 203 | 603 | 43 |
| 12/21/2007 | 23,000 | 1877.50 | 355 | 215 | 230 | 44 |
| 12/22/2007 | 23,000 | 1877.50 | 203 | 214 | 178 | 43 |
| 12/23/2007 | 22,900 | 1877.40 | 157 | 218 | 163 | 43 |
| 12/24/2007 | 22,900 | 1877.40 | 215 | 226 | 162 | 42 |
| 12/25/2007 | 22,900 | 1877.40 | 210 | 221 | 156 | 43 |
| 12/26/2007 | 22,700 | 1877.20 | 105 | 217 | 151 | 43 |
| 12/27/2007 | 22,600 | 1877.10 | 157 | 218 | 150 | 43 |
| 12/28/2007 | 22,500 | 1877.00 | 158 | 219 | 151 | 43 |
| 12/29/2007 | 22,500 | 1877.00 | 196 | 206 | 161 | 44 |
| 12/30/2007 | 22,700 | 1877.20 | 278 | 187 | 170 | 44 |
| 12/31/2007 | 22,800 | 1877.30 | 228 | 187 | 148 | 45 |
| 1/1/2008 | 22,700 | 1877.20 | 132 | 192 | 146 | 45 |
| 1/2/2008 | 22,600 | 1877.10 | 132 | 192 | 143 | 43 |
| 1/3/2008 | 23,500 | 1877.90 | 639 | 195 | 213 | 45 |
| 1/4/2008 | 39,300 | 1890.00 | 8,204 | 251 | 1,940 | 44 |
| 1/5/2008 | 44,100 | 1893.00 | 2,649 | 241 | 922 | 44 |
| 1/6/2008 | 46,200 | 1894.30 | 1,285 | 238 | 548 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/7/2008 | 47,300 | 1894.90 | 839 | 299 | 322 | 129 |
| 1/8/2008 | 49,400 | 1896.10 | 1,385 | 343 | 448 | 180 |
| 1/9/2008 | 51,100 | 1897.10 | 1,174 | 334 | 454 | 183 |
| 1/10/2008 | 53,400 | 1898.40 | 1,476 | 333 | 482 | 184 |
| 1/11/2008 | 54,900 | 1899.20 | 1,214 | 482 | 428 | 186 |
| 1/12/2008 | 55,600 | 1899.60 | 1,350 | 1,050 | 786 | 186 |
| 1/13/2008 | 55,600 | 1899.60 | 1,178 | 1,240 | 1,070 | 186 |
| 1/14/2008 | 55,500 | 1899.50 | 1,052 | 1,160 | 957 | 185 |
| 1/15/2008 | 55,300 | 1899.40 | 887 | 1,040 | 778 | 184 |
| 1/16/2008 | 55,100 | 1899.30 | 797 | 945 | 634 | 184 |
| 1/17/2008 | 55,000 | 1899.20 | 736 | 828 | 510 | 185 |
| 1/18/2008 | 54,900 | 1899.20 | 641 | 728 | 413 | 186 |
| 1/19/2008 | 54,800 | 1899.10 | 570 | 653 | 347 | 186 |
| 1/20/2008 | 54,800 | 1899.10 | 570 | 600 | 306 | 186 |
| 1/21/2008 | 54,700 | 1899.10 | 471 | 549 | 266 | 186 |
| 1/22/2008 | 54,700 | 1899.10 | 486 | 512 | 235 | 186 |
| 1/23/2008 | 54,600 | 1899.00 | 392 | 466 | 209 | 176 |
| 1/24/2008 | 54,500 | 1899.00 | 369 | 442 | 196 | 170 |
| 1/25/2008 | 55,300 | 1899.40 | 857 | 478 | 305 | 171 |
| 1/26/2008 | 57,000 | 1900.30 | 2,862 | 2,110 | 2,700 | 170 |
| 1/27/2008 | 57,200 | 1900.40 | 2,371 | 2,390 | 2,680 | 169 |
| 1/28/2008 | 56,600 | 1900.10 | 2,101 | 2,530 | 2,820 | 169 |
| 1/29/2008 | 56,000 | 1899.80 | 1,341 | 1,730 | 1,840 | 169 |
| 1/30/2008 | 55,600 | 1899.60 | 1,071 | 1,340 | 1,310 | 116 |
| 1/31/2008 | 57,800 | 1900.70 | 2,667 | 1,640 | 2,370 | 119 |
| 2/1/2008 | 56,700 | 1900.20 | 2,172 | 2,870 | 3,490 | 170 |
| 2/2/2008 | 57,200 | 1900.40 | 1,981 | 1,820 | 2,290 | 169 |
| 2/3/2008 | 57,900 | 1900.80 | 4,248 | 4,100 | 5,070 | 99 |
| 2/4/2008 | 56,700 | 1900.20 | 2,017 | 2,760 | 3,310 | 74 |
| 2/5/2008 | 56,100 | 1899.80 | 1,322 | 1,710 | 2,000 | 90 |
| 2/6/2008 | 55,800 | 1899.70 | 1,160 | 1,380 | 1,390 | 172 |
| 2/7/2008 | 55,700 | 1899.60 | 1,118 | 1,230 | 1,110 | 171 |
| 2/8/2008 | 55,600 | 1899.60 | 1,061 | 1,170 | 988 | 172 |
| 2/9/2008 | 55,600 | 1899.60 | 1,083 | 1,140 | 921 | 173 |
| 2/10/2008 | 55,600 | 1899.60 | 1,102 | 1,160 | 947 | 173 |
| 2/11/2008 | 55,700 | 1899.60 | 1,190 | 1,200 | 1,050 | 110 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/12/2008 | 55,800 | 1899.70 | 1,219 | 1,230 | 1,100 | 91 |
| 2/13/2008 | 55,900 | 1899.70 | 1,304 | 1,320 | 1,200 | 91 |
| 2/14/2008 | 55,700 | 1899.60 | 1,106 | 1,270 | 1,110 | 91 |
| 2/15/2008 | 55,500 | 1899.50 | 973 | 1,130 | 917 | 91 |
| 2/16/2008 | 55,400 | 1899.50 | 928 | 1,030 | 781 | 91 |
| 2/17/2008 | 55,400 | 1899.50 | 959 | 1,010 | 738 | 91 |
| 2/18/2008 | 55,400 | 1899.50 | 969 | 1,020 | 739 | 91 |
| 2/19/2008 | 55,400 | 1899.50 | 969 | 1,020 | 733 | 91 |
| 2/20/2008 | 55,300 | 1899.40 | 900 | 1,000 | 708 | 90 |
| 2/21/2008 | 55,300 | 1899.40 | 918 | 966 | 667 | 90 |
| 2/22/2008 | 55,300 | 1899.40 | 923 | 972 | 699 | 88 |
| 2/23/2008 | 55,400 | 1899.50 | 942 | 938 | 663 | 92 |
| 2/24/2008 | 59,300 | 1901.50 | 4,731 | 2,910 | 3,240 | 89 |
| 2/25/2008 | 57,500 | 1900.60 | 3,358 | 4,490 | 4,530 | 145 |
| 2/26/2008 | 56,700 | 1900.20 | 1,877 | 2,400 | 2,490 | 248 |
| 2/27/2008 | 56,200 | 1899.90 | 1,382 | 1,720 | 1,720 | 247 |
| 2/28/2008 | 56,100 | 1899.80 | 1,375 | 1,500 | 1,350 | 246 |
| 2/29/2008 | 56,000 | 1899.80 | 1,299 | 1,420 | 1,180 | 246 |
| 3/1/2008 | 55,900 | 1899.70 | 1,242 | 1,360 | 1,060 | 245 |
| 3/2/2008 | 55,700 | 1899.60 | 1,087 | 1,250 | 913 | 245 |
| 3/3/2008 | 55,500 | 1899.50 | 973 | 1,130 | 741 | 246 |
| 3/4/2008 | 55,400 | 1899.50 | 928 | 1,030 | 755 | 114 |
| 3/5/2008 | 55,400 | 1899.50 | 911 | 959 | 724 | 44 |
| 3/6/2008 | 55,400 | 1899.50 | 884 | 931 | 689 | 45 |
| 3/7/2008 | 55,300 | 1899.40 | 787 | 882 | 636 | 47 |
| 3/8/2008 | 55,300 | 1899.40 | 795 | 837 | 596 | 45 |
| 3/9/2008 | 55,300 | 1899.40 | 776 | 817 | 569 | 44 |
| 3/10/2008 | 55,300 | 1899.40 | 795 | 837 | 574 | 44 |
| 3/11/2008 | 55,400 | 1899.50 | 925 | 921 | 635 | 46 |
| 3/12/2008 | 55,400 | 1899.50 | 889 | 936 | 651 | 46 |
| 3/13/2008 | 55,700 | 1899.60 | 1,139 | 1,040 | 730 | 46 |
| 3/14/2008 | 55,700 | 1899.60 | 1,083 | 1,140 | 881 | 46 |
| 3/15/2008 | 55,500 | 1899.50 | 906 | 1,060 | 818 | 46 |
| 3/16/2008 | 55,300 | 1899.40 | 781 | 928 | 667 | 45 |
| 3/17/2008 | 55,200 | 1899.30 | 718 | 809 | 554 | 45 |
| 3/18/2008 | 55,100 | 1899.30 | 655 | 743 | 497 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/19/2008 | 55,200 | 1899.30 | 754 | 741 | 490 | 47 |
| 3/20/2008 | 55,100 | 1899.30 | 654 | 742 | 466 | 72 |
| 3/21/2008 | 55,100 | 1899.30 | 664 | 699 | 444 | 66 |
| 3/22/2008 | 55,000 | 1899.20 | 572 | 655 | 426 | 50 |
| 3/23/2008 | 55,000 | 1899.20 | 601 | 633 | 403 | 50 |
| 3/24/2008 | 55,300 | 1899.40 | 599 | 471 | 352 | 50 |
| 3/25/2008 | 55,900 | 1899.70 | 531 | 241 | 169 | 47 |
| 3/26/2008 | 56,400 | 1900.00 | 501 | 262 | 178 | 45 |
| 3/27/2008 | 56,800 | 1900.20 | 471 | 283 | 178 | 59 |
| 3/28/2008 | 57,200 | 1900.40 | 488 | 301 | 182 | 66 |
| 3/29/2008 | 57,500 | 1900.60 | 454 | 319 | 201 | 59 |
| 3/30/2008 | 57,700 | 1900.70 | 417 | 333 | 196 | 70 |
| 3/31/2008 | 57,900 | 1900.80 | 396 | 311 | 184 | 76 |
| 4/1/2008 | 58,100 | 1900.90 | 372 | 285 | 194 | 76 |
| 4/2/2008 | 58,300 | 1901.00 | 368 | 281 | 193 | 71 |
| 4/3/2008 | 58,500 | 1901.10 | 361 | 274 | 196 | 60 |
| 4/4/2008 | 58,700 | 1901.20 | 360 | 273 | 194 | 60 |
| 4/5/2008 | 58,900 | 1901.30 | 358 | 271 | 192 | 60 |
| 4/6/2008 | 59,100 | 1901.40 | 358 | 271 | 191 | 60 |
| 4/7/2008 | 59,200 | 1901.50 | 315 | 278 | 192 | 60 |
| 4/8/2008 | 59,300 | 1901.50 | 327 | 291 | 193 | 69 |
| 4/9/2008 | 59,400 | 1901.60 | 330 | 294 | 190 | 73 |
| 4/10/2008 | 59,500 | 1901.60 | 330 | 294 | 189 | 73 |
| 4/11/2008 | 59,700 | 1901.70 | 382 | 296 | 188 | 73 |
| 4/12/2008 | 59,900 | 1901.80 | 374 | 288 | 194 | 63 |
| 4/13/2008 | 60,300 | 1902.00 | 467 | 279 | 192 | 58 |
| 4/14/2008 | 60,700 | 1902.20 | 468 | 280 | 191 | 58 |
| 4/15/2008 | 61,100 | 1902.40 | 467 | 279 | 187 | 65 |
| 4/16/2008 | 61,300 | 1902.50 | 383 | 297 | 184 | 75 |
| 4/17/2008 | 61,400 | 1902.60 | 346 | 311 | 182 | 87 |
| 4/18/2008 | 61,600 | 1902.70 | 392 | 307 | 184 | 85 |
| 4/19/2008 | 61,900 | 1902.80 | 437 | 301 | 184 | 80 |
| 4/20/2008 | 62,000 | 1902.90 | 334 | 299 | 182 | 80 |
| 4/21/2008 | 62,100 | 1902.90 | 333 | 297 | 181 | 79 |
| 4/22/2008 | 62,300 | 1903.00 | 385 | 299 | 187 | 79 |
| 4/23/2008 | 62,400 | 1903.10 | 334 | 299 | 194 | 79 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/24/2008 | 62,400 | 1903.10 | 284 | 299 | 188 | 78 |
| 4/25/2008 | 62,400 | 1903.10 | 286 | 301 | 183 | 80 |
| 4/26/2008 | 62,500 | 1903.10 | 338 | 303 | 184 | 80 |
| 4/27/2008 | 62,600 | 1903.20 | 339 | 304 | 183 | 79 |
| 4/28/2008 | 62,900 | 1903.30 | 431 | 295 | 182 | 75 |
| 4/29/2008 | 63,100 | 1903.40 | 377 | 291 | 182 | 70 |
| 4/30/2008 | 63,300 | 1903.50 | 382 | 296 | 182 | 72 |
| 5/1/2008 | 63,300 | 1903.50 | 290 | 305 | 185 | 77 |
| 5/2/2008 | 63,400 | 1903.60 | 336 | 301 | 183 | 79 |
| 5/3/2008 | 63,400 | 1903.60 | 283 | 298 | 184 | 79 |
| 5/4/2008 | 63,500 | 1903.60 | 327 | 291 | 184 | 79 |
| 5/5/2008 | 63,600 | 1903.70 | 319 | 283 | 184 | 76 |
| 5/6/2008 | 63,700 | 1903.70 | 313 | 276 | 184 | 72 |
| 5/7/2008 | 63,900 | 1903.80 | 358 | 271 | 184 | 72 |
| 5/8/2008 | 64,000 | 1903.90 | 305 | 268 | 187 | 72 |
| 5/9/2008 | 64,100 | 1903.90 | 299 | 262 | 188 | 72 |
| 5/10/2008 | 64,100 | 1903.90 | 241 | 254 | 185 | 73 |
| 5/11/2008 | 64,200 | 1904.00 | 288 | 250 | 186 | 73 |
| 5/12/2008 | 64,200 | 1904.00 | 234 | 246 | 186 | 72 |
| 5/13/2008 | 64,100 | 1903.90 | 179 | 241 | 186 | 72 |
| 5/14/2008 | 64,100 | 1903.90 | 236 | 248 | 187 | 72 |
| 5/15/2008 | 64,100 | 1903.90 | 277 | 292 | 184 | 109 |
| 5/16/2008 | 64,100 | 1903.90 | 278 | 293 | 190 | 111 |
| 5/17/2008 | 64,100 | 1903.90 | 273 | 287 | 187 | 113 |
| 5/18/2008 | 64,000 | 1903.90 | 220 | 285 | 188 | 113 |
| 5/19/2008 | 63,800 | 1903.80 | 177 | 292 | 188 | 121 |
| 5/20/2008 | 63,600 | 1903.70 | 180 | 296 | 183 | 130 |
| 5/21/2008 | 63,300 | 1903.50 | 147 | 314 | 186 | 130 |
| 5/22/2008 | 63,000 | 1903.40 | 181 | 350 | 212 | 129 |
| 5/23/2008 | 62,600 | 1903.20 | 135 | 354 | 215 | 130 |
| 5/24/2008 | 62,300 | 1903.00 | 164 | 332 | 199 | 132 |
| 5/25/2008 | 62,000 | 1902.90 | 140 | 307 | 186 | 132 |
| 5/26/2008 | 61,733 | 1902.80 | 117 | 265 | 157 | 131 |
| 5/27/2008 | 61,467 | 1902.60 | 104 | 251 | 147 | 130 |
| 5/28/2008 | 61,200 | 1902.50 | 112 | 259 | 140 | 127 |
| 5/29/2008 | 61,000 | 1902.40 | 169 | 284 | 140 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/30/2008 | 60,800 | 1902.30 | 174 | 289 | 129 | 130 |
| 5/31/2008 | 60,500 | 1902.10 | 134 | 300 | 108 | 130 |
| 6/1/2008 | 60,200 | 1902.00 | 135 | 301 | 106 | 130 |
| 6/2/2008 | 59,900 | 1901.80 | 129 | 295 | 103 | 131 |
| 6/3/2008 | 59,600 | 1901.70 | 113 | 278 | 99 | 131 |
| 6/4/2008 | 59,300 | 1901.50 | 94 | 258 | 88 | 129 |
| 6/5/2008 | 59,000 | 1901.40 | 81 | 244 | 81 | 130 |
| 6/6/2008 | 58,733 | 1901.20 | 82 | 228 | 72 | 132 |
| 6/7/2008 | 58,467 | 1901.10 | 72 | 217 | 67 | 131 |
| 6/8/2008 | 58,200 | 1900.90 | 59 | 204 | 59 | 131 |
| 6/9/2008 | 57,950 | 1900.80 | 65 | 201 | 56 | 131 |
| 6/10/2008 | 57,700 | 1900.70 | 61 | 197 | 55 | 131 |
| 6/11/2008 | 57,450 | 1900.50 | 54 | 189 | 51 | 131 |
| 6/12/2008 | 57,200 | 1900.40 | 53 | 188 | 49 | 132 |
| 6/13/2008 | 56,950 | 1900.30 | 57 | 193 | 44 | 131 |
| 6/14/2008 | 56,700 | 1900.20 | 54 | 189 | 41 | 131 |
| 6/15/2008 | 56,400 | 1900.00 | 25 | 185 | 39 | 131 |
| 6/16/2008 | 56,100 | 1899.80 | 56 | 218 | 52 | 130 |
| 6/17/2008 | 55,750 | 1899.60 | 60 | 249 | 110 | 136 |
| 6/18/2008 | 55,500 | 1899.50 | 39 | 174 | 40 | 130 |
| 6/19/2008 | 55,250 | 1899.40 | 35 | 170 | 29 | 130 |
| 6/20/2008 | 55,000 | 1899.20 | 31 | 165 | 27 | 130 |
| 6/21/2008 | 54,700 | 1899.10 | 3 | 162 | 25 | 129 |
| 6/22/2008 | 54,450 | 1898.90 | 24 | 158 | 23 | 129 |
| 6/23/2008 | 54,200 | 1898.80 | 20 | 154 | 20 | 130 |
| 6/24/2008 | 53,950 | 1898.70 | 19 | 153 | 19 | 130 |
| 6/25/2008 | 53,700 | 1898.50 | 18 | 152 | 18 | 130 |
| 6/26/2008 | 53,450 | 1898.40 | 17 | 151 | 18 | 130 |
| 6/27/2008 | 53,200 | 1898.30 | 16 | 150 | 18 | 130 |
| 6/28/2008 | 52,950 | 1898.10 | 16 | 149 | 18 | 130 |
| 6/29/2008 | 52,700 | 1898.00 | 14 | 147 | 17 | 130 |
| 6/30/2008 | 52,400 | 1897.80 | -11 | 148 | 14 | 131 |
| 7/1/2008 | 52,200 | 1897.70 | 38 | 146 | 14 | 130 |
| 7/2/2008 | 51,900 | 1897.50 | -13 | 145 | 14 | 131 |
| 7/3/2008 | 51,600 | 1897.40 | -13 | 146 | 13 | 130 |
| 7/4/2008 | 51,400 | 1897.30 | 39 | 147 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/5/2008 | 51,100 | 1897.10 | -11 | 148 | 13 | 130 |
| 7/6/2008 | 50,800 | 1896.90 | -12 | 147 | 13 | 130 |
| 7/7/2008 | 50,600 | 1896.80 | 37 | 145 | 13 | 130 |
| 7/8/2008 | 50,350 | 1896.70 | 14 | 147 | 13 | 130 |
| 7/9/2008 | 50,100 | 1896.50 | 14 | 147 | 13 | 130 |
| 7/10/2008 | 49,850 | 1896.40 | 11 | 144 | 13 | 130 |
| 7/11/2008 | 49,600 | 1896.20 | 10 | 143 | 13 | 130 |
| 7/12/2008 | 49,300 | 1896.10 | -15 | 143 | 13 | 130 |
| 7/13/2008 | 49,000 | 1895.90 | -15 | 143 | 12 | 130 |
| 7/14/2008 | 48,700 | 1895.70 | -14 | 144 | 12 | 130 |
| 7/15/2008 | 48,400 | 1895.60 | -12 | 147 | 12 | 130 |
| 7/16/2008 | 48,100 | 1895.40 | -10 | 149 | 12 | 130 |
| 7/17/2008 | 47,800 | 1895.20 | -10 | 149 | 13 | 130 |
| 7/18/2008 | 47,500 | 1895.00 | -10 | 149 | 12 | 130 |
| 7/19/2008 | 47,200 | 1894.90 | -10 | 149 | 12 | 130 |
| 7/20/2008 | 46,900 | 1894.70 | -8 | 151 | 12 | 130 |
| 7/21/2008 | 46,600 | 1894.50 | -8 | 151 | 12 | 131 |
| 7/22/2008 | 46,300 | 1894.30 | -9 | 150 | 12 | 130 |
| 7/23/2008 | 46,100 | 1894.20 | 42 | 150 | 12 | 130 |
| 7/24/2008 | 45,800 | 1894.00 | -9 | 150 | 13 | 129 |
| 7/25/2008 | 45,500 | 1893.90 | -9 | 150 | 12 | 130 |
| 7/26/2008 | 45,200 | 1893.70 | -11 | 148 | 12 | 130 |
| 7/27/2008 | 44,900 | 1893.50 | -7 | 152 | 12 | 130 |
| 7/28/2008 | 44,600 | 1893.30 | -8 | 151 | 12 | 130 |
| 7/29/2008 | 44,300 | 1893.10 | -8 | 151 | 12 | 130 |
| 7/30/2008 | 44,000 | 1893.00 | 8 | 168 | 12 | 130 |
| 7/31/2008 | 43,600 | 1892.70 | -27 | 184 | 12 | 133 |
| 8/1/2008 | 43,300 | 1892.50 | -7 | 152 | 12 | 130 |
| 8/2/2008 | 43,000 | 1892.30 | -7 | 152 | 12 | 130 |
| 8/3/2008 | 42,700 | 1892.20 | -8 | 151 | 12 | 130 |
| 8/4/2008 | 42,400 | 1892.00 | -8 | 151 | 12 | 130 |
| 8/5/2008 | 42,100 | 1891.80 | -8 | 151 | 12 | 130 |
| 8/6/2008 | 41,800 | 1891.60 | -9 | 150 | 12 | 129 |
| 8/7/2008 | 41,500 | 1891.40 | -9 | 150 | 12 | 129 |
| 8/8/2008 | 41,200 | 1891.20 | -9 | 150 | 12 | 129 |
| 8/9/2008 | 40,900 | 1891.00 | -9 | 150 | 12 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/10/2008 | 40,600 | 1890.80 | -9 | 150 | 12 | 130 |
| 8/11/2008 | 40,300 | 1890.60 | -9 | 150 | 12 | 130 |
| 8/12/2008 | 40,000 | 1890.50 | -7 | 152 | 12 | 129 |
| 8/13/2008 | 39,700 | 1890.30 | -7 | 152 | 12 | 130 |
| 8/14/2008 | 39,400 | 1890.10 | -6 | 153 | 12 | 129 |
| 8/15/2008 | 39,100 | 1889.90 | -5 | 154 | 12 | 130 |
| 8/16/2008 | 38,800 | 1889.70 | 0 | 159 | 14 | 129 |
| 8/17/2008 | 38,500 | 1889.50 | 9 | 169 | 17 | 130 |
| 8/18/2008 | 38,200 | 1889.30 | 7 | 167 | 16 | 131 |
| 8/19/2008 | 37,900 | 1889.10 | 6 | 166 | 17 | 130 |
| 8/20/2008 | 37,600 | 1888.90 | 7 | 167 | 17 | 130 |
| 8/21/2008 | 37,300 | 1888.70 | 7 | 167 | 17 | 130 |
| 8/22/2008 | 37,000 | 1888.50 | 6 | 166 | 16 | 130 |
| 8/23/2008 | 36,700 | 1888.30 | 6 | 165 | 16 | 130 |
| 8/24/2008 | 36,400 | 1888.10 | 6 | 165 | 16 | 130 |
| 8/25/2008 | 36,100 | 1887.90 | 6 | 165 | 15 | 130 |
| 8/26/2008 | 35,800 | 1887.70 | 6 | 165 | 15 | 130 |
| 8/27/2008 | 35,400 | 1887.40 | -45 | 165 | 15 | 130 |
| 8/28/2008 | 35,100 | 1887.20 | 6 | 165 | 14 | 130 |
| 8/29/2008 | 34,800 | 1887.00 | 6 | 165 | 14 | 130 |
| 8/30/2008 | 34,500 | 1886.80 | 6 | 166 | 14 | 131 |
| 8/31/2008 | 34,200 | 1886.60 | 8 | 168 | 15 | 130 |
| 9/1/2008 | 33,900 | 1886.30 | 6 | 166 | 14 | 130 |
| 9/2/2008 | 33,600 | 1886.10 | 7 | 167 | 14 | 130 |
| 9/3/2008 | 33,300 | 1885.90 | 25 | 185 | 15 | 130 |
| 9/4/2008 | 32,900 | 1885.60 | 0 | 212 | 44 | 135 |
| 9/5/2008 | 32,600 | 1885.40 | 9 | 169 | 16 | 130 |
| 9/6/2008 | 32,300 | 1885.20 | 10 | 170 | 14 | 130 |
| 9/7/2008 | 32,000 | 1885.00 | 11 | 171 | 14 | 130 |
| 9/8/2008 | 31,650 | 1884.70 | 2 | 188 | 15 | 129 |
| 9/9/2008 | 31,300 | 1884.40 | 23 | 210 | 46 | 132 |
| 9/10/2008 | 31,100 | 1884.30 | 58 | 167 | 15 | 130 |
| 9/11/2008 | 30,800 | 1884.10 | 7 | 167 | 13 | 130 |
| 9/12/2008 | 30,500 | 1883.80 | 7 | 167 | 13 | 130 |
| 9/13/2008 | 30,200 | 1883.60 | 7 | 167 | 13 | 130 |
| 9/14/2008 | 29,900 | 1883.40 | 9 | 169 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/15/2008 | 29,600 | 1883.10 | 9 | 169 | 13 | 130 |
| 9/16/2008 | 29,300 | 1882.90 | -2 | 157 | 31 | 103 |
| 9/17/2008 | 29,100 | 1882.70 | 48 | 157 | 35 | 99 |
| 9/18/2008 | 28,800 | 1882.50 | -11 | 148 | 35 | 97 |
| 9/19/2008 | 28,600 | 1882.30 | 22 | 129 | 23 | 91 |
| 9/20/2008 | 28,400 | 1882.20 | 16 | 123 | 17 | 90 |
| 9/21/2008 | 28,200 | 1882.00 | 14 | 121 | 14 | 90 |
| 9/22/2008 | 28,000 | 1881.90 | 14 | 121 | 14 | 90 |
| 9/23/2008 | 27,800 | 1881.70 | 13 | 120 | 13 | 90 |
| 9/24/2008 | 27,600 | 1881.50 | 13 | 120 | 13 | 90 |
| 9/25/2008 | 27,300 | 1881.30 | -37 | 120 | 13 | 90 |
| 9/26/2008 | 27,100 | 1881.10 | 12 | 119 | 13 | 90 |
| 9/27/2008 | 26,900 | 1880.90 | 13 | 120 | 13 | 90 |
| 9/28/2008 | 26,700 | 1880.80 | 13 | 120 | 12 | 90 |
| 9/29/2008 | 26,500 | 1880.60 | 12 | 119 | 12 | 90 |
| 9/30/2008 | 26,300 | 1880.40 | 12 | 119 | 12 | 90 |
| 10/1/2008 | 26,100 | 1880.30 | 8 | 115 | 14 | 90 |
| 10/2/2008 | 25,900 | 1880.10 | 11 | 118 | 15 | 90 |
| 10/3/2008 | 25,800 | 1880.00 | 65 | 122 | 32 | 91 |
| 10/4/2008 | 25,700 | 1879.90 | 60 | 116 | 30 | 91 |
| 10/5/2008 | 25,600 | 1879.80 | 60 | 116 | 18 | 90 |
| 10/6/2008 | 25,400 | 1879.70 | 10 | 117 | 20 | 90 |
| 10/7/2008 | 25,200 | 1879.50 | 13 | 120 | 22 | 90 |
| 10/8/2008 | 25,000 | 1879.30 | 16 | 123 | 25 | 90 |
| 10/9/2008 | 24,767 | 1879.10 | 3 | 127 | 29 | 90 |
| 10/10/2008 | 24,533 | 1878.90 | 7 | 131 | 31 | 90 |
| 10/11/2008 | 24,300 | 1878.70 | 4 | 128 | 35 | 91 |
| 10/12/2008 | 24,100 | 1878.50 | 17 | 124 | 32 | 90 |
| 10/13/2008 | 23,867 | 1878.30 | -1 | 123 | 30 | 90 |
| 10/14/2008 | 23,633 | 1878.10 | -2 | 122 | 30 | 90 |
| 10/15/2008 | 23,400 | 1877.90 | -2 | 122 | 30 | 90 |
| 10/16/2008 | 23,300 | 1877.80 | 47 | 103 | 52 | 60 |
| 10/17/2008 | 23,100 | 1877.60 | -15 | 90 | 38 | 45 |
| 10/18/2008 | 23,000 | 1877.50 | 32 | 87 | 33 | 45 |
| 10/19/2008 | 22,900 | 1877.40 | 31 | 86 | 30 | 45 |
| 10/20/2008 | 22,700 | 1877.20 | -19 | 86 | 30 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/21/2008 | 22,600 | 1877.10 | 31 | 86 | 30 | 45 |
| 10/22/2008 | 22,500 | 1877.00 | 31 | 86 | 30 | 45 |
| 10/23/2008 | 22,300 | 1876.80 | -19 | 86 | 30 | 45 |
| 10/24/2008 | 22,200 | 1876.70 | 32 | 87 | 32 | 45 |
| 10/25/2008 | 22,100 | 1876.60 | 32 | 87 | 33 | 45 |
| 10/26/2008 | 22,000 | 1876.50 | 31 | 86 | 33 | 45 |
| 10/27/2008 | 21,800 | 1876.30 | -19 | 86 | 33 | 45 |
| 10/28/2008 | 21,700 | 1876.20 | 31 | 86 | 33 | 45 |
| 10/29/2008 | 21,600 | 1876.10 | 32 | 87 | 34 | 45 |
| 10/30/2008 | 21,400 | 1875.90 | -18 | 87 | 35 | 44 |
| 10/31/2008 | 21,300 | 1875.80 | 33 | 88 | 38 | 45 |
| 11/1/2008 | 21,400 | 1875.90 | 135 | 89 | 45 | 45 |
| 11/2/2008 | 21,600 | 1876.10 | 205 | 110 | 69 | 45 |
| 11/3/2008 | 21,800 | 1876.30 | 221 | 126 | 131 | 49 |
| 11/4/2008 | 22,100 | 1876.60 | 261 | 116 | 126 | 46 |
| 11/5/2008 | 22,000 | 1876.50 | 77 | 134 | 109 | 45 |
| 11/6/2008 | 21,767 | 1876.30 | 46 | 172 | 134 | 44 |
| 11/7/2008 | 21,533 | 1876.10 | 63 | 190 | 150 | 44 |
| 11/8/2008 | 21,300 | 1875.80 | 73 | 201 | 158 | 44 |
| 11/9/2008 | 21,000 | 1875.50 | 42 | 203 | 156 | 45 |
| 11/10/2008 | 20,700 | 1875.20 | 33 | 194 | 150 | 45 |
| 11/11/2008 | 20,500 | 1875.00 | 73 | 183 | 140 | 45 |
| 11/12/2008 | 20,300 | 1874.80 | 52 | 161 | 127 | 45 |
| 11/13/2008 | 20,100 | 1874.60 | 30 | 138 | 104 | 45 |
| 11/14/2008 | 19,900 | 1874.40 | 15 | 122 | 89 | 45 |
| 11/15/2008 | 19,800 | 1874.30 | 58 | 114 | 73 | 45 |
| 11/16/2008 | 19,600 | 1874.00 | 27 | 135 | 73 | 45 |
| 11/17/2008 | 19,400 | 1873.80 | 36 | 144 | 113 | 52 |
| 11/18/2008 | 19,300 | 1873.70 | 49 | 105 | 59 | 46 |
| 11/19/2008 | 19,100 | 1873.50 | -7 | 99 | 47 | 46 |
| 11/20/2008 | 19,000 | 1873.40 | 41 | 96 | 43 | 45 |
| 11/21/2008 | 18,900 | 1873.30 | 40 | 95 | 37 | 46 |
| 11/22/2008 | 18,800 | 1873.20 | 40 | 95 | 37 | 46 |
| 11/23/2008 | 18,700 | 1873.00 | 38 | 93 | 36 | 45 |
| 11/24/2008 | 18,600 | 1872.90 | 35 | 90 | 32 | 46 |
| 11/25/2008 | 18,500 | 1872.80 | 34 | 89 | 29 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/26/2008 | 18,400 | 1872.70 | 34 | 89 | 28 | 45 |
| 11/27/2008 | 18,300 | 1872.60 | 34 | 89 | 28 | 45 |
| 11/28/2008 | 18,300 | 1872.60 | 85 | 89 | 28 | 46 |
| 11/29/2008 | 18,200 | 1872.50 | 34 | 89 | 28 | 45 |
| 11/30/2008 | 18,100 | 1872.40 | 34 | 89 | 28 | 46 |
| 12/1/2008 | 17,900 | 1872.10 | 28 | 136 | 57 | 46 |
| 12/2/2008 | 17,600 | 1871.80 | 16 | 176 | 130 | 44 |
| 12/3/2008 | 17,300 | 1871.40 | 15 | 175 | 130 | 45 |
| 12/4/2008 | 17,000 | 1871.00 | 14 | 174 | 130 | 45 |
| 12/5/2008 | 16,800 | 1870.80 | 62 | 171 | 128 | 45 |
| 12/6/2008 | 16,500 | 1870.40 | 10 | 170 | 126 | 44 |
| 12/7/2008 | 16,300 | 1870.20 | 60 | 169 | 126 | 44 |
| 12/8/2008 | 16,000 | 1869.80 | 9 | 169 | 125 | 44 |
| 12/9/2008 | 15,800 | 1869.50 | 59 | 168 | 124 | 44 |
| 12/10/2008 | 15,500 | 1869.10 | 7 | 167 | 124 | 44 |
| 12/11/2008 | 15,300 | 1868.80 | 57 | 166 | 124 | 44 |
| 12/12/2008 | 15,000 | 1868.40 | 6 | 166 | 124 | 44 |
| 12/13/2008 | 14,800 | 1868.20 | 59 | 168 | 124 | 44 |
| 12/14/2008 | 14,500 | 1867.70 | 9 | 169 | 126 | 45 |
| 12/15/2008 | 14,300 | 1867.50 | 60 | 169 | 130 | 45 |
| 12/16/2008 | 14,100 | 1867.20 | 59 | 168 | 125 | 45 |
| 12/17/2008 | 13,800 | 1866.70 | 6 | 166 | 124 | 44 |
| 12/18/2008 | 13,600 | 1866.40 | 57 | 166 | 127 | 44 |
| 12/19/2008 | 13,400 | 1866.10 | 56 | 165 | 140 | 44 |
| 12/20/2008 | 13,200 | 1865.80 | 55 | 164 | 130 | 44 |
| 12/21/2008 | 13,100 | 1865.70 | 106 | 165 | 154 | 45 |
| 12/22/2008 | 13,200 | 1865.80 | 207 | 165 | 187 | 45 |
| 12/23/2008 | 13,100 | 1865.70 | 105 | 164 | 145 | 45 |
| 12/24/2008 | 13,300 | 1866.00 | 262 | 170 | 181 | 45 |
| 12/25/2008 | 14,700 | 1868.00 | 870 | 173 | 449 | 45 |
| 12/26/2008 | 14,800 | 1868.20 | 222 | 181 | 190 | 45 |
| 12/27/2008 | 14,800 | 1868.20 | 177 | 186 | 167 | 45 |
| 12/28/2008 | 15,000 | 1868.40 | 278 | 187 | 241 | 45 |
| 12/29/2008 | 15,400 | 1869.00 | 380 | 188 | 244 | 44 |
| 12/30/2008 | 15,700 | 1869.40 | 331 | 189 | 207 | 44 |
| 12/31/2008 | 15,700 | 1869.40 | 179 | 188 | 175 | 45 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/1/2009 | 15,700 | 1869.40 | 179 | 188 | 165 | 44 |
| 1/2/2009 | 16,000 | 1869.80 | 331 | 189 | 230 | 44 |
| 1/3/2009 | 16,300 | 1870.20 | 332 | 190 | 203 | 45 |
| 1/4/2009 | 16,300 | 1870.20 | 180 | 190 | 173 | 45 |
| 1/5/2009 | 16,400 | 1870.30 | 231 | 190 | 184 | 45 |
| 1/6/2009 | 16,500 | 1870.40 | 231 | 190 | 184 | 44 |
| 1/7/2009 | 16,600 | 1870.50 | 231 | 190 | 173 | 44 |
| 1/8/2009 | 16,600 | 1870.50 | 183 | 193 | 168 | 44 |
| 1/9/2009 | 16,600 | 1870.50 | 189 | 199 | 167 | 44 |
| 1/10/2009 | 16,500 | 1870.40 | 138 | 198 | 161 | 44 |
| 1/11/2009 | 16,400 | 1870.30 | 137 | 197 | 158 | 44 |
| 1/12/2009 | 16,300 | 1870.20 | 137 | 197 | 156 | 44 |
| 1/13/2009 | 16,300 | 1870.20 | 187 | 197 | 155 | 44 |
| 1/14/2009 | 16,200 | 1870.00 | 137 | 197 | 153 | 44 |
| 1/15/2009 | 16,100 | 1869.90 | 133 | 193 | 150 | 44 |
| 1/16/2009 | 16,000 | 1869.80 | 123 | 183 | 143 | 44 |
| 1/17/2009 | 15,900 | 1869.60 | 110 | 169 | 133 | 44 |
| 1/18/2009 | 15,800 | 1869.50 | 102 | 160 | 125 | 44 |
| 1/19/2009 | 15,600 | 1869.20 | 45 | 154 | 119 | 44 |
| 1/20/2009 | 15,500 | 1869.10 | 92 | 150 | 115 | 44 |
| 1/21/2009 | 15,400 | 1869.00 | 93 | 151 | 114 | 44 |
| 1/22/2009 | 15,300 | 1868.80 | 93 | 151 | 117 | 44 |
| 1/23/2009 | 15,200 | 1868.70 | 93 | 151 | 116 | 44 |
| 1/24/2009 | 15,100 | 1868.60 | 95 | 153 | 115 | 44 |
| 1/25/2009 | 15,000 | 1868.40 | 95 | 153 | 117 | 44 |
| 1/26/2009 | 14,800 | 1868.20 | 45 | 153 | 116 | 44 |
| 1/27/2009 | 14,700 | 1868.00 | 95 | 153 | 116 | 44 |
| 1/28/2009 | 14,500 | 1867.70 | 45 | 153 | 115 | 44 |
| 1/29/2009 | 14,400 | 1867.60 | 96 | 154 | 115 | 44 |
| 1/30/2009 | 14,200 | 1867.30 | 47 | 156 | 117 | 44 |
| 1/31/2009 | 14,100 | 1867.20 | 74 | 131 | 114 | 17 |
| 2/1/2009 | 13,900 | 1866.90 | 29 | 137 | 120 | 0 |
| 2/2/2009 | 13,800 | 1866.70 | 80 | 137 | 118 | 0 |
| 2/3/2009 | 13,600 | 1866.40 | 28 | 136 | 117 | 0 |
| 2/4/2009 | 13,500 | 1866.30 | 79 | 136 | 117 | 0 |
| 2/5/2009 | 13,400 | 1866.10 | 79 | 136 | 123 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/6/2009 | 13,500 | 1866.30 | 181 | 137 | 143 | 0 |
| 2/7/2009 | 13,500 | 1866.30 | 130 | 137 | 139 | 0 |
| 2/8/2009 | 13,400 | 1866.10 | 80 | 137 | 134 | 0 |
| 2/9/2009 | 13,400 | 1866.10 | 130 | 137 | 133 | 0 |
| 2/10/2009 | 13,300 | 1866.00 | 81 | 138 | 129 | 0 |
| 2/11/2009 | 13,300 | 1866.00 | 156 | 164 | 147 | 26 |
| 2/12/2009 | 13,200 | 1865.80 | 103 | 161 | 150 | 45 |
| 2/13/2009 | 13,500 | 1866.30 | 304 | 161 | 183 | 44 |
| 2/14/2009 | 13,700 | 1866.60 | 254 | 161 | 183 | 44 |
| 2/15/2009 | 14,800 | 1868.20 | 720 | 174 | 361 | 44 |
| 2/16/2009 | 16,300 | 1870.20 | 934 | 187 | 461 | 44 |
| 2/17/2009 | 17,400 | 1871.50 | 735 | 190 | 437 | 43 |
| 2/18/2009 | 18,000 | 1872.20 | 469 | 175 | 327 | 43 |
| 2/19/2009 | 18,400 | 1872.70 | 343 | 149 | 237 | 43 |
| 2/20/2009 | 18,800 | 1873.20 | 333 | 138 | 205 | 43 |
| 2/21/2009 | 19,200 | 1873.60 | 329 | 134 | 199 | 44 |
| 2/22/2009 | 25,800 | 1880.00 | 3,443 | 122 | 752 | 45 |
| 2/23/2009 | 35,300 | 1887.30 | 4,887 | 103 | 1,220 | 44 |
| 2/24/2009 | 41,000 | 1891.10 | 2,954 | 84 | 887 | 43 |
| 2/25/2009 | 43,700 | 1892.80 | 1,435 | 78 | 398 | 43 |
| 2/26/2009 | 46,400 | 1894.40 | 1,451 | 95 | 446 | 43 |
| 2/27/2009 | 48,100 | 1895.40 | 989 | 139 | 382 | 43 |
| 2/28/2009 | 49,300 | 1896.10 | 737 | 139 | 303 | 43 |
| 3/1/2009 | 52,600 | 1897.90 | 1,783 | 126 | 315 | 43 |
| 3/2/2009 | 58,800 | 1901.30 | 5,605 | 2,610 | 2,650 | 43 |
| 3/3/2009 | 58,200 | 1900.90 | 4,144 | 4,680 | 5,240 | 142 |
| 3/4/2009 | 57,000 | 1900.30 | 2,492 | 3,260 | 3,630 | 184 |
| 3/5/2009 | 56,300 | 1899.90 | 1,766 | 2,230 | 2,160 | 189 |
| 3/6/2009 | 55,800 | 1899.70 | 1,334 | 1,670 | 1,410 | 183 |
| 3/7/2009 | 55,600 | 1899.60 | 1,191 | 1,360 | 974 | 184 |
| 3/8/2009 | 55,400 | 1899.50 | 1,011 | 1,170 | 859 | 101 |
| 3/9/2009 | 55,200 | 1899.30 | 868 | 1,020 | 761 | 46 |
| 3/10/2009 | 55,100 | 1899.30 | 773 | 867 | 635 | 46 |
| 3/11/2009 | 55,100 | 1899.30 | 643 | 677 | 541 | 45 |
| 3/12/2009 | 55,300 | 1899.40 | 466 | 384 | 379 | 44 |
| 3/13/2009 | 55,700 | 1899.60 | 385 | 193 | 205 | 43 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/14/2009 | 56,100 | 1899.80 | 384 | 192 | 196 | 43 |
| 3/15/2009 | 56,700 | 1900.20 | 487 | 194 | 221 | 43 |
| 3/16/2009 | 58,700 | 1901.20 | 1,196 | 198 | 433 | 44 |
| 3/17/2009 | 60,600 | 1902.20 | 1,147 | 199 | 426 | 45 |
| 3/18/2009 | 62,000 | 1902.90 | 889 | 193 | 354 | 44 |
| 3/19/2009 | 63,100 | 1903.40 | 739 | 194 | 321 | 44 |
| 3/20/2009 | 64,000 | 1903.90 | 639 | 195 | 299 | 44 |
| 3/21/2009 | 65,000 | 1904.40 | 675 | 180 | 281 | 45 |
| 3/22/2009 | 66,000 | 1904.90 | 649 | 152 | 302 | 46 |
| 3/23/2009 | 66,800 | 1905.30 | 541 | 145 | 251 | 46 |
| 3/24/2009 | 67,400 | 1905.50 | 458 | 164 | 255 | 46 |
| 3/25/2009 | 68,000 | 1905.80 | 466 | 172 | 214 | 81 |
| 3/26/2009 | 68,500 | 1906.10 | 432 | 189 | 191 | 100 |
| 3/27/2009 | 69,000 | 1906.30 | 446 | 204 | 208 | 88 |
| 3/28/2009 | 69,600 | 1906.60 | 492 | 200 | 189 | 100 |
| 3/29/2009 | 70,000 | 1906.80 | 393 | 201 | 178 | 100 |
| 3/30/2009 | 70,500 | 1907.00 | 444 | 202 | 175 | 99 |
| 3/31/2009 | 70,800 | 1907.20 | 348 | 207 | 173 | 96 |
| 4/1/2009 | 71,200 | 1907.40 | 427 | 237 | 186 | 96 |
| 4/2/2009 | 71,400 | 1907.40 | 341 | 253 | 243 | 67 |
| 4/3/2009 | 71,200 | 1907.40 | 293 | 415 | 323 | 46 |
| 4/4/2009 | 70,400 | 1907.00 | 538 | 991 | 636 | 75 |
| 4/5/2009 | 69,800 | 1906.70 | 469 | 812 | 621 | 89 |
| 4/6/2009 | 69,600 | 1906.60 | 235 | 353 | 301 | 90 |
| 4/7/2009 | 69,600 | 1906.60 | 282 | 297 | 243 | 91 |
| 4/8/2009 | 69,600 | 1906.60 | 267 | 281 | 247 | 77 |
| 4/9/2009 | 69,600 | 1906.60 | 259 | 273 | 253 | 66 |
| 4/10/2009 | 69,600 | 1906.60 | 259 | 273 | 255 | 65 |
| 4/11/2009 | 69,500 | 1906.60 | 215 | 279 | 249 | 66 |
| 4/12/2009 | 69,400 | 1906.50 | 209 | 273 | 255 | 57 |
| 4/13/2009 | 69,300 | 1906.50 | 210 | 274 | 246 | 51 |
| 4/14/2009 | 69,000 | 1906.30 | 234 | 406 | 318 | 77 |
| 4/15/2009 | 68,800 | 1906.20 | 150 | 264 | 255 | 64 |
| 4/16/2009 | 68,700 | 1906.20 | 188 | 251 | 225 | 64 |
| 4/17/2009 | 68,600 | 1906.10 | 181 | 244 | 215 | 65 |
| 4/18/2009 | 68,500 | 1906.10 | 176 | 238 | 210 | 64 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/19/2009 | 68,400 | 1906.00 | 163 | 225 | 201 | 64 |
| 4/20/2009 | 68,300 | 1906.00 | 153 | 214 | 188 | 64 |
| 4/21/2009 | 68,300 | 1906.00 | 195 | 205 | 179 | 64 |
| 4/22/2009 | 68,400 | 1906.00 | 238 | 197 | 171 | 64 |
| 4/23/2009 | 68,400 | 1906.00 | 192 | 202 | 169 | 64 |
| 4/24/2009 | 68,300 | 1906.00 | 151 | 212 | 172 | 71 |
| 4/25/2009 | 68,200 | 1905.90 | 157 | 218 | 174 | 74 |
| 4/26/2009 | 68,100 | 1905.90 | 155 | 216 | 173 | 74 |
| 4/27/2009 | 67,900 | 1905.80 | 104 | 216 | 173 | 74 |
| 4/28/2009 | 67,700 | 1905.70 | 118 | 230 | 173 | 81 |
| 4/29/2009 | 67,500 | 1905.60 | 121 | 233 | 171 | 90 |
| 4/30/2009 | 67,400 | 1905.50 | 165 | 227 | 167 | 89 |
| 5/1/2009 | 67,400 | 1905.50 | 209 | 220 | 170 | 90 |
| 5/2/2009 | 68,200 | 1905.90 | 613 | 221 | 207 | 90 |
| 5/3/2009 | 69,100 | 1906.40 | 670 | 228 | 250 | 90 |
| 5/4/2009 | 70,700 | 1907.10 | 1,053 | 259 | 281 | 89 |
| 5/5/2009 | 71,500 | 1907.50 | 2,683 | 2,400 | 2,460 | 89 |
| 5/6/2009 | 71,100 | 1907.30 | 1,299 | 1,580 | 1,880 | 90 |
| 5/7/2009 | 71,700 | 1907.60 | 799 | 523 | 486 | 90 |
| 5/8/2009 | 72,100 | 1907.80 | 648 | 470 | 446 | 90 |
| 5/9/2009 | 72,400 | 1907.90 | 459 | 324 | 320 | 89 |
| 5/10/2009 | 72,500 | 1908.00 | 372 | 339 | 315 | 90 |
| 5/11/2009 | 72,600 | 1908.00 | 342 | 307 | 289 | 91 |
| 5/12/2009 | 72,700 | 1908.10 | 329 | 293 | 269 | 91 |
| 5/13/2009 | 72,700 | 1908.10 | 275 | 289 | 267 | 82 |
| 5/14/2009 | 72,600 | 1908.00 | 229 | 294 | 254 | 91 |
| 5/15/2009 | 72,500 | 1908.00 | 255 | 322 | 249 | 108 |
| 5/16/2009 | 72,300 | 1907.90 | 227 | 345 | 246 | 130 |
| 5/17/2009 | 72,100 | 1907.80 | 226 | 344 | 242 | 129 |
| 5/18/2009 | 71,800 | 1907.60 | 174 | 342 | 237 | 130 |
| 5/19/2009 | 71,500 | 1907.50 | 168 | 336 | 229 | 129 |
| 5/20/2009 | 71,200 | 1907.40 | 162 | 330 | 225 | 129 |
| 5/21/2009 | 70,800 | 1907.20 | 217 | 441 | 281 | 135 |
| 5/22/2009 | 70,500 | 1907.00 | 136 | 302 | 214 | 130 |
| 5/23/2009 | 70,200 | 1906.90 | 126 | 292 | 188 | 130 |
| 5/24/2009 | 69,900 | 1906.70 | 111 | 276 | 175 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/25/2009 | 69,600 | 1906.60 | 101 | 265 | 165 | 130 |
| 5/26/2009 | 69,400 | 1906.50 | 143 | 257 | 157 | 130 |
| 5/27/2009 | 69,100 | 1906.40 | 90 | 254 | 153 | 130 |
| 5/28/2009 | 68,800 | 1906.20 | 80 | 243 | 147 | 129 |
| 5/29/2009 | 68,500 | 1906.10 | 73 | 236 | 139 | 129 |
| 5/30/2009 | 68,200 | 1905.90 | 70 | 233 | 135 | 129 |
| 5/31/2009 | 67,900 | 1905.80 | 61 | 223 | 129 | 130 |
| 6/1/2009 | 67,600 | 1905.60 | 55 | 217 | 121 | 130 |
| 6/2/2009 | 67,300 | 1905.50 | 54 | 216 | 121 | 130 |
| 6/3/2009 | 67,100 | 1905.40 | 99 | 210 | 117 | 130 |
| 6/4/2009 | 67,000 | 1905.40 | 149 | 210 | 117 | 130 |
| 6/5/2009 | 66,800 | 1905.30 | 99 | 210 | 115 | 131 |
| 6/6/2009 | 66,500 | 1905.10 | 49 | 211 | 113 | 131 |
| 6/7/2009 | 66,300 | 1905.00 | 95 | 206 | 109 | 130 |
| 6/8/2009 | 66,000 | 1904.90 | 37 | 198 | 102 | 130 |
| 6/9/2009 | 65,800 | 1904.80 | 81 | 191 | 95 | 130 |
| 6/10/2009 | 65,500 | 1904.60 | 24 | 184 | 88 | 130 |
| 6/11/2009 | 65,300 | 1904.50 | 67 | 177 | 79 | 130 |
| 6/12/2009 | 65,000 | 1904.40 | 11 | 171 | 72 | 130 |
| 6/13/2009 | 64,800 | 1904.30 | 59 | 168 | 64 | 130 |
| 6/14/2009 | 64,600 | 1904.20 | 53 | 162 | 52 | 130 |
| 6/15/2009 | 64,600 | 1904.20 | 153 | 161 | 50 | 131 |
| 6/16/2009 | 64,400 | 1904.10 | 48 | 157 | 46 | 130 |
| 6/17/2009 | 64,200 | 1904.00 | 42 | 150 | 37 | 130 |
| 6/18/2009 | 63,900 | 1903.80 | -7 | 152 | 31 | 130 |
| 6/19/2009 | 63,800 | 1903.80 | 91 | 149 | 27 | 131 |
| 6/20/2009 | 63,500 | 1903.60 | -12 | 147 | 26 | 130 |
| 6/21/2009 | 63,300 | 1903.50 | 39 | 147 | 26 | 130 |
| 6/22/2009 | 63,100 | 1903.40 | 40 | 148 | 26 | 130 |
| 6/23/2009 | 62,900 | 1903.30 | 35 | 143 | 23 | 131 |
| 6/24/2009 | 62,600 | 1903.20 | -17 | 141 | 19 | 130 |
| 6/25/2009 | 62,400 | 1903.10 | 32 | 140 | 19 | 130 |
| 6/26/2009 | 62,200 | 1903.00 | 31 | 139 | 16 | 130 |
| 6/27/2009 | 62,000 | 1902.90 | 28 | 136 | 14 | 130 |
| 6/28/2009 | 61,700 | 1902.70 | -23 | 135 | 12 | 130 |
| 6/29/2009 | 61,500 | 1902.60 | 27 | 135 | 12 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/30/2009 | 61,300 | 1902.50 | 30 | 138 | 12 | 129 |
| 7/1/2009 | 61,000 | 1902.40 | -16 | 142 | 15 | 129 |
| 7/2/2009 | 60,700 | 1902.20 | -14 | 144 | 15 | 130 |
| 7/3/2009 | 60,500 | 1902.10 | 32 | 140 | 15 | 129 |
| 7/4/2009 | 60,200 | 1902.00 | -18 | 140 | 13 | 129 |
| 7/5/2009 | 59,900 | 1901.80 | -18 | 140 | 13 | 129 |
| 7/6/2009 | 59,700 | 1901.70 | 32 | 140 | 12 | 130 |
| 7/7/2009 | 59,400 | 1901.60 | -17 | 141 | 12 | 130 |
| 7/8/2009 | 59,100 | 1901.40 | -16 | 142 | 13 | 130 |
| 7/9/2009 | 58,900 | 1901.30 | 34 | 142 | 13 | 130 |
| 7/10/2009 | 58,600 | 1901.10 | -15 | 143 | 13 | 130 |
| 7/11/2009 | 58,300 | 1901.00 | -16 | 142 | 13 | 130 |
| 7/12/2009 | 58,000 | 1900.80 | -16 | 142 | 13 | 129 |
| 7/13/2009 | 57,800 | 1900.70 | 32 | 140 | 13 | 129 |
| 7/14/2009 | 57,500 | 1900.60 | -18 | 140 | 11 | 129 |
| 7/15/2009 | 57,200 | 1900.40 | -15 | 143 | 11 | 129 |
| 7/16/2009 | 57,000 | 1900.30 | 37 | 145 | 13 | 130 |
| 7/17/2009 | 56,700 | 1900.20 | -13 | 145 | 12 | 130 |
| 7/18/2009 | 56,400 | 1900.00 | -11 | 148 | 12 | 131 |
| 7/19/2009 | 56,100 | 1899.80 | -10 | 149 | 12 | 130 |
| 7/20/2009 | 55,800 | 1899.70 | -11 | 148 | 11 | 130 |
| 7/21/2009 | 55,500 | 1899.50 | -9 | 150 | 11 | 130 |
| 7/22/2009 | 55,200 | 1899.30 | -7 | 152 | 13 | 130 |
| 7/23/2009 | 54,900 | 1899.20 | -7 | 152 | 14 | 131 |
| 7/24/2009 | 54,600 | 1899.00 | -7 | 152 | 14 | 131 |
| 7/25/2009 | 54,300 | 1898.90 | -6 | 153 | 14 | 131 |
| 7/26/2009 | 54,100 | 1898.70 | 46 | 155 | 13 | 130 |
| 7/27/2009 | 53,800 | 1898.60 | -3 | 156 | 13 | 130 |
| 7/28/2009 | 53,500 | 1898.40 | -5 | 154 | 13 | 130 |
| 7/29/2009 | 53,200 | 1898.30 | -4 | 155 | 13 | 130 |
| 7/30/2009 | 52,900 | 1898.10 | -3 | 156 | 13 | 131 |
| 7/31/2009 | 52,600 | 1897.90 | -1 | 158 | 13 | 130 |
| 8/1/2009 | 52,300 | 1897.80 | -1 | 158 | 13 | 129 |
| 8/2/2009 | 52,000 | 1897.60 | -1 | 158 | 13 | 130 |
| 8/3/2009 | 51,700 | 1897.40 | 2 | 161 | 13 | 130 |
| 8/4/2009 | 51,300 | 1897.20 | -48 | 162 | 13 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/5/2009 | 51,100 | 1897.10 | 53 | 162 | 13 | 130 |
| 8/6/2009 | 50,800 | 1896.90 | 4 | 163 | 14 | 130 |
| 8/7/2009 | 50,500 | 1896.80 | 5 | 164 | 14 | 130 |
| 8/8/2009 | 50,200 | 1896.60 | 5 | 164 | 13 | 130 |
| 8/9/2009 | 49,900 | 1896.40 | 5 | 164 | 12 | 129 |
| 8/10/2009 | 49,600 | 1896.20 | 6 | 165 | 12 | 129 |
| 8/11/2009 | 49,300 | 1896.10 | 6 | 166 | 12 | 129 |
| 8/12/2009 | 49,000 | 1895.90 | 7 | 167 | 12 | 129 |
| 8/13/2009 | 48,700 | 1895.70 | 6 | 165 | 11 | 130 |
| 8/14/2009 | 48,400 | 1895.60 | 6 | 166 | 10 | 130 |
| 8/15/2009 | 48,100 | 1895.40 | 8 | 168 | 11 | 130 |
| 8/16/2009 | 47,800 | 1895.20 | 11 | 171 | 10 | 130 |
| 8/17/2009 | 47,500 | 1895.00 | 12 | 172 | 10 | 130 |
| 8/18/2009 | 47,300 | 1894.90 | 64 | 173 | 9 | 130 |
| 8/19/2009 | 47,000 | 1894.70 | 2 | 161 | 9 | 130 |
| 8/20/2009 | 46,700 | 1894.60 | -17 | 141 | 9 | 130 |
| 8/21/2009 | 46,400 | 1894.40 | -16 | 142 | 9 | 130 |
| 8/22/2009 | 46,100 | 1894.20 | -17 | 141 | 9 | 130 |
| 8/23/2009 | 45,800 | 1894.00 | -16 | 142 | 10 | 130 |
| 8/24/2009 | 45,500 | 1893.90 | -15 | 143 | 11 | 130 |
| 8/25/2009 | 45,200 | 1893.70 | -13 | 145 | 11 | 130 |
| 8/26/2009 | 44,900 | 1893.50 | -13 | 146 | 12 | 130 |
| 8/27/2009 | 44,600 | 1893.30 | -10 | 149 | 10 | 130 |
| 8/28/2009 | 44,400 | 1893.20 | 42 | 150 | 10 | 130 |
| 8/29/2009 | 44,100 | 1893.00 | -13 | 146 | 11 | 130 |
| 8/30/2009 | 43,800 | 1892.80 | -13 | 145 | 10 | 130 |
| 8/31/2009 | 43,500 | 1892.60 | -13 | 146 | 10 | 131 |
| 9/1/2009 | 43,200 | 1892.50 | -12 | 147 | 11 | 130 |
| 9/2/2009 | 42,900 | 1892.30 | -13 | 146 | 12 | 129 |
| 9/3/2009 | 42,600 | 1892.10 | -12 | 147 | 10 | 131 |
| 9/4/2009 | 42,300 | 1891.90 | -11 | 148 | 10 | 130 |
| 9/5/2009 | 42,000 | 1891.70 | -11 | 148 | 11 | 130 |
| 9/6/2009 | 41,700 | 1891.50 | -12 | 147 | 11 | 130 |
| 9/7/2009 | 41,400 | 1891.30 | -11 | 148 | 11 | 130 |
| 9/8/2009 | 41,100 | 1891.20 | -11 | 148 | 11 | 130 |
| 9/9/2009 | 40,800 | 1891.00 | -12 | 147 | 10 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/10/2009 | 40,500 | 1890.80 | -12 | 147 | 10 | 130 |
| 9/11/2009 | 40,200 | 1890.60 | -15 | 143 | 10 | 130 |
| 9/12/2009 | 40,000 | 1890.50 | 34 | 142 | 10 | 130 |
| 9/13/2009 | 39,700 | 1890.30 | -15 | 143 | 10 | 130 |
| 9/14/2009 | 39,400 | 1890.10 | -14 | 144 | 11 | 130 |
| 9/15/2009 | 39,100 | 1889.90 | -13 | 145 | 10 | 130 |
| 9/16/2009 | 38,900 | 1889.70 | 18 | 125 | 22 | 104 |
| 9/17/2009 | 38,700 | 1889.60 | 6 | 112 | 16 | 92 |
| 9/18/2009 | 38,500 | 1889.50 | -2 | 104 | 12 | 92 |
| 9/19/2009 | 38,300 | 1889.40 | -3 | 103 | 10 | 92 |
| 9/20/2009 | 38,100 | 1889.20 | -2 | 104 | 11 | 92 |
| 9/21/2009 | 37,900 | 1889.10 | -3 | 103 | 11 | 92 |
| 9/22/2009 | 37,600 | 1888.90 | -54 | 102 | 11 | 91 |
| 9/23/2009 | 37,400 | 1888.80 | -4 | 102 | 11 | 92 |
| 9/24/2009 | 37,200 | 1888.60 | -5 | 101 | 11 | 92 |
| 9/25/2009 | 37,000 | 1888.50 | -4 | 102 | 10 | 92 |
| 9/26/2009 | 36,800 | 1888.40 | -4 | 102 | 10 | 91 |
| 9/27/2009 | 36,600 | 1888.20 | -4 | 102 | 10 | 91 |
| 9/28/2009 | 36,400 | 1888.10 | -4 | 102 | 10 | 91 |
| 9/29/2009 | 36,200 | 1887.90 | -4 | 102 | 10 | 92 |
| 9/30/2009 | 36,000 | 1887.80 | -4 | 102 | 11 | 92 |
| 10/1/2009 | 35,755 | 1887.60 | -2 | 101 | 8 | 92 |
| 10/2/2009 | 35,538 | 1887.50 | -6 | 102 | 9 | 92 |
| 10/3/2009 | 35,321 | 1887.30 | -11 | 102 | 9 | 90 |
| 10/4/2009 | 35,105 | 1887.20 | -9 | 102 | 11 | 89 |
| 10/5/2009 | 34,890 | 1887.00 | 1 | 108 | 13 | 89 |
| 10/6/2009 | 34,661 | 1886.90 | -2 | 113 | 20 | 89 |
| 10/7/2009 | 34,448 | 1886.70 | 5 | 114 | 20 | 91 |
| 10/8/2009 | 34,236 | 1886.60 | 4 | 114 | 18 | 92 |
| 10/9/2009 | 34,024 | 1886.40 | 5 | 113 | 19 | 91 |
| 10/10/2009 | 33,799 | 1886.30 | 8 | 118 | 19 | 91 |
| 10/11/2009 | 33,589 | 1886.10 | 9 | 118 | 20 | 90 |
| 10/12/2009 | 33,393 | 1886.00 | 15 | 119 | 20 | 90 |
| 10/13/2009 | 33,533 | 1886.10 | 188 | 122 | 42 | 90 |
| 10/14/2009 | 33,813 | 1886.30 | 257 | 122 | 31 | 90 |
| 10/15/2009 | 33,813 | 1886.30 | 140 | 131 | 28 | 90 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/16/2009 | 33,701 | 1886.20 | 72 | 128 | 75 | 59 |
| 10/17/2009 | 33,547 | 1886.10 | 47 | 125 | 73 | 46 |
| 10/18/2009 | 33,352 | 1886.00 | 38 | 138 | 97 | 44 |
| 10/19/2009 | 33,185 | 1885.80 | 55 | 140 | 102 | 44 |
| 10/20/2009 | 32,978 | 1885.70 | 33 | 143 | 102 | 45 |
| 10/21/2009 | 32,744 | 1885.50 | 31 | 146 | 113 | 46 |
| 10/22/2009 | 32,565 | 1885.40 | 34 | 128 | 92 | 45 |
| 10/23/2009 | 32,373 | 1885.20 | 27 | 117 | 80 | 45 |
| 10/24/2009 | 32,237 | 1885.10 | 33 | 104 | 67 | 45 |
| 10/25/2009 | 32,115 | 1885.00 | 30 | 96 | 44 | 43 |
| 10/26/2009 | 31,952 | 1884.90 | 28 | 97 | 42 | 44 |
| 10/27/2009 | 31,831 | 1884.80 | 23 | 95 | 39 | 46 |
| 10/28/2009 | 31,710 | 1884.70 | 28 | 92 | 34 | 45 |
| 10/29/2009 | 31,562 | 1884.60 | 20 | 96 | 34 | 45 |
| 10/30/2009 | 31,455 | 1884.60 | 26 | 90 | 41 | 45 |
| 10/31/2009 | 31,334 | 1884.50 | 23 | 91 | 32 | 44 |
| 11/1/2009 | 31,214 | 1884.40 | 27 | 92 | 36 | 44 |
| 11/2/2009 | 31,107 | 1884.30 | 25 | 92 | 36 | 44 |
| 11/3/2009 | 30,988 | 1884.20 | 27 | 93 | 35 | 45 |
| 11/4/2009 | 30,855 | 1884.10 | 23 | 95 | 35 | 45 |
| 11/5/2009 | 30,737 | 1884.00 | 42 | 107 | 62 | 45 |
| 11/6/2009 | 30,657 | 1884.00 | 38 | 92 | 43 | 45 |
| 11/7/2009 | 30,565 | 1883.90 | 39 | 92 | 35 | 45 |
| 11/8/2009 | 30,474 | 1883.80 | 38 | 92 | 35 | 45 |
| 11/9/2009 | 30,369 | 1883.70 | 35 | 92 | 36 | 45 |
| 11/10/2009 | 30,278 | 1883.70 | 36 | 92 | 36 | 45 |
| 11/11/2009 | 30,173 | 1883.60 | 34 | 93 | 36 | 45 |
| 11/12/2009 | 30,056 | 1883.50 | 29 | 93 | 36 | 45 |
| 11/13/2009 | 29,952 | 1883.40 | 30 | 93 | 36 | 45 |
| 11/14/2009 | 29,849 | 1883.30 | 31 | 93 | 37 | 44 |
| 11/15/2009 | 29,732 | 1883.20 | 30 | 93 | 36 | 44 |
| 11/16/2009 | 29,629 | 1883.20 | 30 | 92 | 36 | 44 |
| 11/17/2009 | 29,539 | 1883.10 | 45 | 93 | 39 | 44 |
| 11/18/2009 | 29,437 | 1883.00 | 32 | 93 | 39 | 44 |
| 11/19/2009 | 29,335 | 1882.90 | 35 | 92 | 38 | 44 |
| 11/20/2009 | 29,424 | 1883.00 | 126 | 94 | 59 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/21/2009 | 29,488 | 1883.00 | 116 | 93 | 58 | 44 |
| 11/22/2009 | 29,462 | 1883.00 | 80 | 97 | 55 | 44 |
| 11/23/2009 | 29,411 | 1883.00 | 73 | 107 | 76 | 45 |
| 11/24/2009 | 29,322 | 1882.90 | 54 | 111 | 77 | 45 |
| 11/25/2009 | 29,194 | 1882.80 | 49 | 119 | 82 | 44 |
| 11/26/2009 | 29,067 | 1882.70 | 46 | 119 | 89 | 44 |
| 11/27/2009 | 28,928 | 1882.60 | 49 | 125 | 95 | 43 |
| 11/28/2009 | 28,764 | 1882.50 | 34 | 134 | 109 | 44 |
| 11/29/2009 | 28,601 | 1882.30 | 40 | 134 | 108 | 45 |
| 11/30/2009 | 28,438 | 1882.20 | 37 | 126 | 106 | 45 |
| 12/1/2009 | 28,128 | 1882.00 | 38 | 142 | 88 | 45 |
| 12/2/2009 | 27,869 | 1881.70 | 40 | 189 | 157 | 45 |
| 12/3/2009 | 27,600 | 1881.50 | 37 | 178 | 137 | 45 |
| 12/4/2009 | 27,333 | 1881.30 | 38 | 178 | 136 | 45 |
| 12/5/2009 | 27,068 | 1881.10 | 37 | 176 | 135 | 45 |
| 12/6/2009 | 26,805 | 1880.90 | 39 | 173 | 132 | 45 |
| 12/7/2009 | 26,543 | 1880.60 | 37 | 173 | 132 | 45 |
| 12/8/2009 | 26,273 | 1880.40 | 29 | 172 | 131 | 45 |
| 12/9/2009 | 26,005 | 1880.20 | 34 | 172 | 133 | 44 |
| 12/10/2009 | 25,751 | 1880.00 | 37 | 171 | 132 | 43 |
| 12/11/2009 | 25,544 | 1879.80 | 61 | 172 | 134 | 43 |
| 12/12/2009 | 25,452 | 1879.70 | 120 | 173 | 141 | 43 |
| 12/13/2009 | 25,498 | 1879.70 | 191 | 172 | 157 | 43 |
| 12/14/2009 | 25,361 | 1879.60 | 100 | 172 | 142 | 44 |
| 12/15/2009 | 25,315 | 1879.60 | 133 | 173 | 142 | 44 |
| 12/16/2009 | 27,140 | 1881.10 | 1,086 | 178 | 327 | 44 |
| 12/17/2009 | 27,466 | 1881.40 | 354 | 185 | 177 | 44 |
| 12/18/2009 | 27,442 | 1881.40 | 196 | 211 | 174 | 45 |
| 12/19/2009 | 27,357 | 1881.30 | 154 | 210 | 165 | 46 |
| 12/20/2009 | 27,260 | 1881.20 | 159 | 211 | 166 | 46 |
| 12/21/2009 | 27,442 | 1881.40 | 283 | 211 | 205 | 46 |
| 12/22/2009 | 27,636 | 1881.60 | 306 | 211 | 207 | 46 |
| 12/23/2009 | 27,624 | 1881.50 | 199 | 211 | 176 | 46 |
| 12/24/2009 | 27,527 | 1881.50 | 155 | 210 | 170 | 43 |
| 12/25/2009 | 27,381 | 1881.30 | 131 | 211 | 162 | 47 |
| 12/26/2009 | 27,236 | 1881.20 | 131 | 211 | 160 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/27/2009 | 27,200 | 1881.20 | 182 | 210 | 174 | 46 |
| 12/28/2009 | 27,092 | 1881.10 | 146 | 209 | 169 | 46 |
| 12/29/2009 | 26,996 | 1881.00 | 152 | 210 | 166 | 46 |
| 12/30/2009 | 26,888 | 1880.90 | 140 | 204 | 169 | 46 |
| 12/31/2009 | 26,828 | 1880.90 | 158 | 195 | 161 | 46 |
| 1/1/2010 | 28,091 | 1881.90 | 795 | 194 | 264 | 46 |
| 1/2/2010 | 28,890 | 1882.60 | 612 | 195 | 229 | 46 |
| 1/3/2010 | 29,156 | 1882.80 | 324 | 195 | 180 | 46 |
| 1/4/2010 | 29,233 | 1882.80 | 237 | 196 | 165 | 46 |
| 1/5/2010 | 29,245 | 1882.90 | 190 | 195 | 159 | 46 |
| 1/6/2010 | 29,194 | 1882.80 | 163 | 194 | 155 | 46 |
| 1/7/2010 | 29,118 | 1882.80 | 150 | 194 | 152 | 46 |
| 1/8/2010 | 29,029 | 1882.70 | 140 | 193 | 150 | 46 |
| 1/9/2010 | 28,915 | 1882.60 | 129 | 192 | 148 | 46 |
| 1/10/2010 | 28,789 | 1882.50 | 120 | 191 | 147 | 46 |
| 1/11/2010 | 28,676 | 1882.40 | 120 | 190 | 147 | 46 |
| 1/12/2010 | 30,343 | 1883.70 | 978 | 195 | 302 | 46 |
| 1/13/2010 | 33,603 | 1886.10 | 1,869 | 203 | 463 | 45 |
| 1/14/2010 | 34,647 | 1886.90 | 730 | 200 | 262 | 45 |
| 1/15/2010 | 35,148 | 1887.20 | 454 | 201 | 209 | 46 |
| 1/16/2010 | 35,451 | 1887.40 | 346 | 201 | 189 | 46 |
| 1/17/2010 | 35,988 | 1887.80 | 444 | 203 | 200 | 50 |
| 1/18/2010 | 42,007 | 1891.70 | 3,125 | 222 | 793 | 96 |
| 1/19/2010 | 51,836 | 1897.50 | 5,204 | 245 | 1,500 | 132 |
| 1/20/2010 | 57,568 | 1900.60 | 4,244 | 1,530 | 2,140 | 114 |
| 1/21/2010 | 57,433 | 1900.50 | 3,500 | 3,900 | 4,660 | 47 |
| 1/22/2010 | 56,306 | 1899.90 | 1,757 | 2,670 | 3,020 | 80 |
| 1/23/2010 | 55,928 | 1899.70 | 1,284 | 1,880 | 1,770 | 174 |
| 1/24/2010 | 56,022 | 1899.80 | 1,267 | 1,630 | 1,380 | 224 |
| 1/25/2010 | 59,460 | 1901.60 | 4,746 | 3,620 | 4,030 | 204 |
| 1/26/2010 | 58,032 | 1900.90 | 4,279 | 5,170 | 6,000 | 74 |
| 1/27/2010 | 56,839 | 1900.20 | 2,209 | 2,920 | 3,370 | 211 |
| 1/28/2010 | 56,174 | 1899.90 | 1,492 | 1,920 | 1,980 | 249 |
| 1/29/2010 | 55,890 | 1899.70 | 1,190 | 1,460 | 1,360 | 250 |
| 1/30/2010 | 55,815 | 1899.70 | 1,190 | 1,350 | 1,200 | 251 |
| 1/31/2010 | 55,645 | 1899.60 | 985 | 1,170 | 1,030 | 249 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/1/2010 | 55,495 | 1899.50 | 819 | 995 | 849 | 249 |
| 2/2/2010 | 55,382 | 1899.40 | 735 | 921 | 739 | 249 |
| 2/3/2010 | 55,420 | 1899.50 | 772 | 894 | 693 | 248 |
| 2/4/2010 | 56,117 | 1899.80 | 1,080 | 952 | 744 | 248 |
| 2/5/2010 | 57,145 | 1900.40 | 2,994 | 2,500 | 2,720 | 244 |
| 2/6/2010 | 56,667 | 1900.10 | 1,872 | 2,150 | 2,450 | 241 |
| 2/7/2010 | 56,249 | 1899.90 | 1,492 | 1,730 | 1,910 | 241 |
| 2/8/2010 | 55,909 | 1899.70 | 1,162 | 1,380 | 1,430 | 241 |
| 2/9/2010 | 55,702 | 1899.60 | 996 | 1,180 | 1,140 | 240 |
| 2/10/2010 | 55,513 | 1899.50 | 832 | 1,030 | 898 | 241 |
| 2/11/2010 | 55,401 | 1899.50 | 743 | 927 | 779 | 240 |
| 2/12/2010 | 55,420 | 1899.50 | 778 | 901 | 735 | 240 |
| 2/13/2010 | 55,420 | 1899.50 | 772 | 910 | 732 | 240 |
| 2/14/2010 | 55,382 | 1899.40 | 738 | 892 | 687 | 240 |
| 2/15/2010 | 55,401 | 1899.50 | 742 | 883 | 651 | 240 |
| 2/16/2010 | 55,476 | 1899.50 | 837 | 928 | 692 | 240 |
| 2/17/2010 | 55,457 | 1899.50 | 833 | 960 | 775 | 186 |
| 2/18/2010 | 55,420 | 1899.50 | 813 | 950 | 755 | 189 |
| 2/19/2010 | 55,326 | 1899.40 | 719 | 900 | 644 | 235 |
| 2/20/2010 | 55,251 | 1899.40 | 647 | 842 | 574 | 235 |
| 2/21/2010 | 55,139 | 1899.30 | 556 | 777 | 507 | 236 |
| 2/22/2010 | 55,026 | 1899.30 | 445 | 700 | 432 | 236 |
| 2/23/2010 | 55,382 | 1899.40 | 628 | 691 | 417 | 236 |
| 2/24/2010 | 55,758 | 1899.60 | 1,243 | 1,130 | 1,080 | 140 |
| 2/25/2010 | 55,513 | 1899.50 | 871 | 1,070 | 1,020 | 125 |
| 2/26/2010 | 57,472 | 1900.60 | 2,216 | 1,440 | 1,230 | 240 |
| 2/27/2010 | 56,858 | 1900.20 | 2,316 | 2,610 | 2,940 | 239 |
| 2/28/2010 | 56,211 | 1899.90 | 1,453 | 1,800 | 1,900 | 239 |
| 3/1/2010 | 55,852 | 1899.70 | 1,114 | 1,350 | 1,280 | 239 |
| 3/2/2010 | 56,079 | 1899.80 | 1,321 | 1,290 | 1,200 | 239 |
| 3/3/2010 | 56,439 | 1900.00 | 1,765 | 1,650 | 1,770 | 240 |
| 3/4/2010 | 56,117 | 1899.80 | 1,375 | 1,580 | 1,700 | 240 |
| 3/5/2010 | 55,852 | 1899.70 | 1,138 | 1,320 | 1,310 | 240 |
| 3/6/2010 | 55,664 | 1899.60 | 963 | 1,140 | 1,040 | 240 |
| 3/7/2010 | 55,495 | 1899.50 | 832 | 1,010 | 979 | 114 |
| 3/8/2010 | 55,382 | 1899.40 | 760 | 931 | 919 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/9/2010 | 55,307 | 1899.40 | 664 | 858 | 825 | 46 |
| 3/10/2010 | 55,195 | 1899.30 | 585 | 804 | 757 | 46 |
| 3/11/2010 | 55,101 | 1899.30 | 505 | 738 | 681 | 46 |
| 3/12/2010 | 56,591 | 1900.10 | 1,539 | 1,010 | 1,140 | 46 |
| 3/13/2010 | 56,249 | 1899.90 | 1,604 | 1,780 | 2,250 | 46 |
| 3/14/2010 | 55,852 | 1899.70 | 1,115 | 1,360 | 1,660 | 46 |
| 3/15/2010 | 55,626 | 1899.60 | 925 | 1,110 | 1,260 | 46 |
| 3/16/2010 | 55,495 | 1899.50 | 840 | 1,000 | 1,050 | 46 |
| 3/17/2010 | 55,438 | 1899.50 | 796 | 948 | 1,150 | 46 |
| 3/18/2010 | 55,382 | 1899.40 | 758 | 918 | 1,090 | 45 |
| 3/19/2010 | 55,307 | 1899.40 | 690 | 870 | 991 | 45 |
| 3/20/2010 | 55,251 | 1899.40 | 649 | 831 | 915 | 45 |
| 3/21/2010 | 55,195 | 1899.30 | 591 | 789 | 858 | 45 |
| 3/22/2010 | 55,176 | 1899.30 | 567 | 764 | 805 | 59 |
| 3/23/2010 | 55,101 | 1899.30 | 513 | 731 | 743 | 65 |
| 3/24/2010 | 55,139 | 1899.30 | 536 | 699 | 697 | 65 |
| 3/25/2010 | 55,120 | 1899.30 | 640 | 735 | 765 | 65 |
| 3/26/2010 | 55,026 | 1899.30 | 545 | 693 | 724 | 65 |
| 3/27/2010 | 54,933 | 1899.20 | 472 | 619 | 626 | 65 |
| 3/28/2010 | 54,914 | 1899.20 | 449 | 561 | 559 | 65 |
| 3/29/2010 | 55,288 | 1899.40 | 730 | 662 | 709 | 65 |
| 3/30/2010 | 56,610 | 1900.10 | 2,034 | 1,510 | 1,950 | 65 |
| 3/31/2010 | 56,230 | 1899.90 | 1,451 | 1,720 | 2,240 | 65 |
| 4/1/2010 | 56,401 | 1900.00 | 1,173 | 1,170 | 1,610 | 65 |
| 4/2/2010 | 58,733 | 1901.20 | 1,683 | 657 | 1,200 | 52 |
| 4/3/2010 | 61,535 | 1902.70 | 1,626 | 275 | 879 | 44 |
| 4/4/2010 | 64,274 | 1904.00 | 1,497 | 232 | 750 | 44 |
| 4/5/2010 | 66,662 | 1905.20 | 1,390 | 236 | 729 | 45 |
| 4/6/2010 | 68,542 | 1906.10 | 1,128 | 248 | 652 | 45 |
| 4/7/2010 | 70,189 | 1906.90 | 1,028 | 281 | 585 | 55 |
| 4/8/2010 | 69,645 | 1906.60 | 1,144 | 1,530 | 1,700 | 60 |
| 4/9/2010 | 69,124 | 1906.40 | 1,079 | 1,440 | 1,780 | 55 |
| 4/10/2010 | 68,284 | 1906.00 | 1,000 | 1,530 | 1,840 | 48 |
| 4/11/2010 | 67,920 | 1905.80 | 1,246 | 1,560 | 2,010 | 44 |
| 4/12/2010 | 69,493 | 1906.50 | 2,857 | 2,200 | 3,070 | 45 |
| 4/13/2010 | 67,984 | 1905.80 | 1,872 | 2,790 | 3,670 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/14/2010 | 67,257 | 1905.50 | 1,551 | 2,080 | 2,700 | 44 |
| 4/15/2010 | 66,981 | 1905.30 | 1,300 | 1,570 | 2,030 | 72 |
| 4/16/2010 | 67,321 | 1905.50 | 1,198 | 1,170 | 1,510 | 88 |
| 4/17/2010 | 68,069 | 1905.90 | 1,155 | 907 | 1,040 | 89 |
| 4/18/2010 | 68,649 | 1906.10 | 1,130 | 968 | 1,070 | 89 |
| 4/19/2010 | 68,951 | 1906.30 | 1,088 | 1,080 | 1,160 | 89 |
| 4/20/2010 | 69,406 | 1906.50 | 1,336 | 1,250 | 1,400 | 89 |
| 4/21/2010 | 69,319 | 1906.50 | 1,079 | 1,270 | 1,450 | 89 |
| 4/22/2010 | 69,298 | 1906.50 | 931 | 1,090 | 1,200 | 89 |
| 4/23/2010 | 69,298 | 1906.50 | 864 | 1,020 | 1,030 | 89 |
| 4/24/2010 | 69,124 | 1906.40 | 835 | 1,070 | 1,060 | 89 |
| 4/25/2010 | 68,995 | 1906.30 | 857 | 1,140 | 1,130 | 89 |
| 4/26/2010 | 68,305 | 1906.00 | 594 | 1,170 | 1,160 | 89 |
| 4/27/2010 | 69,471 | 1906.50 | 2,852 | 2,510 | 2,680 | 89 |
| 4/28/2010 | 68,177 | 1905.90 | 2,574 | 3,360 | 4,210 | 89 |
| 4/29/2010 | 67,642 | 1905.70 | 1,832 | 2,240 | 2,750 | 89 |
| 4/30/2010 | 67,343 | 1905.50 | 1,433 | 1,720 | 2,120 | 88 |
| 5/1/2010 | 67,706 | 1905.70 | 1,238 | 1,190 | 1,390 | 88 |
| 5/2/2010 | 67,856 | 1905.80 | 1,126 | 1,200 | 1,310 | 89 |
| 5/3/2010 | 68,005 | 1905.80 | 1,052 | 1,110 | 1,210 | 89 |
| 5/4/2010 | 68,327 | 1906.00 | 1,041 | 1,020 | 1,030 | 90 |
| 5/5/2010 | 68,327 | 1906.00 | 898 | 1,030 | 1,020 | 89 |
| 5/6/2010 | 68,262 | 1906.00 | 800 | 991 | 973 | 89 |
| 5/7/2010 | 68,198 | 1905.90 | 730 | 908 | 855 | 90 |
| 5/8/2010 | 68,305 | 1906.00 | 688 | 772 | 731 | 89 |
| 5/9/2010 | 68,585 | 1906.10 | 663 | 664 | 579 | 90 |
| 5/10/2010 | 69,038 | 1906.30 | 754 | 627 | 585 | 90 |
| 5/11/2010 | 69,298 | 1906.50 | 616 | 586 | 541 | 90 |
| 5/12/2010 | 69,536 | 1906.60 | 567 | 543 | 480 | 93 |
| 5/13/2010 | 69,819 | 1906.70 | 533 | 496 | 443 | 96 |
| 5/14/2010 | 69,971 | 1906.80 | 512 | 451 | 378 | 116 |
| 5/15/2010 | 70,539 | 1907.00 | 485 | 387 | 309 | 129 |
| 5/16/2010 | 71,021 | 1907.30 | 504 | 356 | 270 | 130 |
| 5/17/2010 | 71,594 | 1907.50 | 566 | 388 | 284 | 130 |
| 5/18/2010 | 72,059 | 1907.80 | 632 | 549 | 381 | 130 |
| 5/19/2010 | 72,014 | 1907.70 | 551 | 743 | 567 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/20/2010 | 71,926 | 1907.70 | 531 | 742 | 569 | 129 |
| 5/21/2010 | 71,704 | 1907.60 | 484 | 763 | 574 | 129 |
| 5/22/2010 | 71,329 | 1907.40 | 445 | 786 | 621 | 129 |
| 5/23/2010 | 71,131 | 1907.30 | 395 | 607 | 497 | 129 |
| 5/24/2010 | 71,153 | 1907.30 | 327 | 438 | 340 | 129 |
| 5/25/2010 | 71,373 | 1907.40 | 403 | 391 | 300 | 129 |
| 5/26/2010 | 71,594 | 1907.50 | 491 | 486 | 367 | 130 |
| 5/27/2010 | 71,881 | 1907.70 | 621 | 591 | 487 | 130 |
| 5/28/2010 | 71,881 | 1907.70 | 651 | 758 | 679 | 129 |
| 5/29/2010 | 71,771 | 1907.60 | 537 | 709 | 616 | 129 |
| 5/30/2010 | 71,749 | 1907.60 | 474 | 596 | 502 | 129 |
| 5/31/2010 | 71,771 | 1907.60 | 477 | 567 | 459 | 130 |
| 6/1/2010 | 71,771 | 1907.60 | 459 | 564 | 451 | 130 |
| 6/2/2010 | 71,837 | 1907.70 | 483 | 564 | 447 | 129 |
| 6/3/2010 | 71,837 | 1907.70 | 539 | 667 | 487 | 130 |
| 6/4/2010 | 71,970 | 1907.70 | 768 | 809 | 703 | 129 |
| 6/5/2010 | 72,059 | 1907.80 | 678 | 716 | 658 | 129 |
| 6/6/2010 | 72,369 | 1907.90 | 524 | 473 | 416 | 130 |
| 6/7/2010 | 72,657 | 1908.00 | 436 | 382 | 324 | 129 |
| 6/8/2010 | 72,902 | 1908.20 | 369 | 361 | 293 | 129 |
| 6/9/2010 | 73,058 | 1908.20 | 343 | 360 | 288 | 130 |
| 6/10/2010 | 73,170 | 1908.30 | 304 | 358 | 284 | 129 |
| 6/11/2010 | 73,170 | 1908.30 | 263 | 355 | 278 | 129 |
| 6/12/2010 | 73,170 | 1908.30 | 230 | 351 | 270 | 129 |
| 6/13/2010 | 73,125 | 1908.30 | 215 | 349 | 264 | 130 |
| 6/14/2010 | 73,080 | 1908.20 | 209 | 349 | 257 | 130 |
| 6/15/2010 | 72,991 | 1908.20 | 185 | 337 | 251 | 130 |
| 6/16/2010 | 72,924 | 1908.20 | 157 | 291 | 219 | 129 |
| 6/17/2010 | 72,924 | 1908.20 | 151 | 221 | 153 | 129 |
| 6/18/2010 | 72,969 | 1908.20 | 162 | 187 | 102 | 130 |
| 6/19/2010 | 72,991 | 1908.20 | 154 | 179 | 105 | 129 |
| 6/20/2010 | 73,014 | 1908.20 | 149 | 179 | 102 | 130 |
| 6/21/2010 | 73,014 | 1908.20 | 141 | 171 | 96 | 129 |
| 6/22/2010 | 73,014 | 1908.20 | 140 | 161 | 82 | 130 |
| 6/23/2010 | 73,014 | 1908.20 | 130 | 158 | 76 | 129 |
| 6/24/2010 | 73,014 | 1908.20 | 123 | 175 | 74 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/25/2010 | 72,991 | 1908.20 | 114 | 166 | 58 | 130 |
| 6/26/2010 | 72,947 | 1908.20 | 107 | 159 | 51 | 130 |
| 6/27/2010 | 72,902 | 1908.20 | 95 | 159 | 50 | 129 |
| 6/28/2010 | 72,835 | 1908.10 | 78 | 157 | 45 | 130 |
| 6/29/2010 | 72,724 | 1908.10 | 78 | 153 | 38 | 130 |
| 6/30/2010 | 72,635 | 1908.00 | 75 | 155 | 37 | 130 |
| 7/1/2010 | 72,524 | 1908.00 | 72 | 155 | 37 | 130 |
| 7/2/2010 | 72,435 | 1907.90 | 68 | 151 | 34 | 130 |
| 7/3/2010 | 72,302 | 1907.90 | 61 | 148 | 31 | 129 |
| 7/4/2010 | 72,192 | 1907.80 | 57 | 147 | 31 | 129 |
| 7/5/2010 | 72,037 | 1907.70 | 48 | 149 | 29 | 129 |
| 7/6/2010 | 71,881 | 1907.70 | 47 | 154 | 34 | 129 |
| 7/7/2010 | 71,726 | 1907.60 | 43 | 155 | 34 | 129 |
| 7/8/2010 | 71,550 | 1907.50 | 39 | 151 | 30 | 130 |
| 7/9/2010 | 71,395 | 1907.40 | 37 | 148 | 27 | 129 |
| 7/10/2010 | 71,197 | 1907.40 | 34 | 155 | 27 | 130 |
| 7/11/2010 | 71,021 | 1907.30 | 37 | 161 | 25 | 130 |
| 7/12/2010 | 70,845 | 1907.20 | 33 | 156 | 25 | 130 |
| 7/13/2010 | 70,626 | 1907.10 | 27 | 153 | 26 | 129 |
| 7/14/2010 | 70,429 | 1907.00 | 29 | 151 | 25 | 130 |
| 7/15/2010 | 70,233 | 1906.90 | 34 | 146 | 24 | 130 |
| 7/16/2010 | 69,993 | 1906.80 | 36 | 151 | 25 | 130 |
| 7/17/2010 | 69,776 | 1906.70 | 27 | 151 | 27 | 130 |
| 7/18/2010 | 69,536 | 1906.60 | 23 | 150 | 26 | 130 |
| 7/19/2010 | 69,298 | 1906.50 | 22 | 151 | 26 | 129 |
| 7/20/2010 | 69,038 | 1906.30 | 20 | 151 | 26 | 129 |
| 7/21/2010 | 68,779 | 1906.20 | 15 | 150 | 25 | 130 |
| 7/22/2010 | 68,542 | 1906.10 | 19 | 152 | 25 | 129 |
| 7/23/2010 | 68,284 | 1906.00 | 18 | 152 | 24 | 130 |
| 7/24/2010 | 68,027 | 1905.80 | 13 | 151 | 22 | 130 |
| 7/25/2010 | 67,749 | 1905.70 | 11 | 148 | 22 | 130 |
| 7/26/2010 | 67,492 | 1905.60 | 6 | 147 | 22 | 130 |
| 7/27/2010 | 67,215 | 1905.50 | 5 | 147 | 22 | 129 |
| 7/28/2010 | 66,959 | 1905.30 | 4 | 147 | 21 | 129 |
| 7/29/2010 | 66,684 | 1905.20 | 4 | 149 | 20 | 130 |
| 7/30/2010 | 66,387 | 1905.10 | 4 | 152 | 22 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/31/2010 | 66,113 | 1904.90 | 6 | 153 | 22 | 130 |
| 8/1/2010 | 65,840 | 1904.80 | 3 | 152 | 22 | 130 |
| 8/2/2010 | 65,567 | 1904.70 | 3 | 152 | 22 | 129 |
| 8/3/2010 | 65,274 | 1904.50 | 4 | 154 | 21 | 129 |
| 8/4/2010 | 64,960 | 1904.40 | 7 | 175 | 22 | 130 |
| 8/5/2010 | 64,585 | 1904.20 | 16 | 209 | 69 | 135 |
| 8/6/2010 | 64,315 | 1904.00 | 7 | 160 | 33 | 130 |
| 8/7/2010 | 64,005 | 1903.90 | 5 | 158 | 20 | 129 |
| 8/8/2010 | 63,737 | 1903.80 | 4 | 156 | 19 | 129 |
| 8/9/2010 | 63,449 | 1903.60 | 4 | 154 | 19 | 129 |
| 8/10/2010 | 63,161 | 1903.50 | -1 | 151 | 19 | 129 |
| 8/11/2010 | 62,854 | 1903.30 | -7 | 149 | 19 | 129 |
| 8/12/2010 | 62,569 | 1903.20 | -4 | 149 | 19 | 129 |
| 8/13/2010 | 62,284 | 1903.00 | -2 | 148 | 19 | 130 |
| 8/14/2010 | 62,000 | 1902.90 | -3 | 149 | 18 | 130 |
| 8/15/2010 | 61,676 | 1902.70 | -2 | 176 | 21 | 130 |
| 8/16/2010 | 61,293 | 1902.50 | 15 | 213 | 96 | 131 |
| 8/17/2010 | 60,992 | 1902.40 | -4 | 149 | 24 | 130 |
| 8/18/2010 | 60,671 | 1902.20 | -9 | 168 | 17 | 130 |
| 8/19/2010 | 60,292 | 1902.00 | 10 | 205 | 80 | 134 |
| 8/20/2010 | 59,994 | 1901.90 | -5 | 153 | 21 | 130 |
| 8/21/2010 | 59,697 | 1901.70 | -11 | 153 | 19 | 131 |
| 8/22/2010 | 59,400 | 1901.60 | -4 | 156 | 19 | 131 |
| 8/23/2010 | 59,105 | 1901.40 | 4 | 158 | 19 | 131 |
| 8/24/2010 | 58,811 | 1901.30 | -2 | 152 | 18 | 130 |
| 8/25/2010 | 58,518 | 1901.10 | -6 | 149 | 17 | 130 |
| 8/26/2010 | 58,226 | 1901.00 | -6 | 151 | 17 | 130 |
| 8/27/2010 | 57,916 | 1900.80 | -10 | 151 | 16 | 130 |
| 8/28/2010 | 57,587 | 1900.60 | -15 | 151 | 16 | 130 |
| 8/29/2010 | 57,299 | 1900.50 | -7 | 151 | 16 | 130 |
| 8/30/2010 | 57,011 | 1900.30 | -1 | 151 | 15 | 130 |
| 8/31/2010 | 56,725 | 1900.20 | 1 | 151 | 15 | 130 |
| 9/1/2010 | 56,458 | 1900.00 | 2 | 151 | 15 | 130 |
| 9/2/2010 | 56,155 | 1899.90 | -4 | 151 | 16 | 130 |
| 9/3/2010 | 55,833 | 1899.70 | 3 | 151 | 23 | 131 |
| 9/4/2010 | 55,476 | 1899.50 | -2 | 151 | 35 | 144 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/5/2010 | 55,139 | 1899.30 | 5 | 173 | 27 | 152 |
| 9/6/2010 | 54,765 | 1899.10 | -16 | 193 | 27 | 152 |
| 9/7/2010 | 54,413 | 1898.90 | -16 | 193 | 26 | 152 |
| 9/8/2010 | 54,024 | 1898.70 | -12 | 217 | 23 | 163 |
| 9/9/2010 | 53,601 | 1898.50 | -2 | 239 | 20 | 188 |
| 9/10/2010 | 53,197 | 1898.30 | 3 | 243 | 21 | 193 |
| 9/11/2010 | 52,795 | 1898.00 | 5 | 245 | 17 | 196 |
| 9/12/2010 | 52,378 | 1897.80 | 4 | 250 | 19 | 198 |
| 9/13/2010 | 51,963 | 1897.60 | 0 | 245 | 18 | 199 |
| 9/14/2010 | 51,530 | 1897.30 | -7 | 244 | 20 | 197 |
| 9/15/2010 | 51,100 | 1897.10 | -2 | 246 | 25 | 196 |
| 9/16/2010 | 50,797 | 1896.90 | -1 | 181 | 91 | 117 |
| 9/17/2010 | 50,566 | 1896.80 | -3 | 120 | 48 | 79 |
| 9/18/2010 | 50,353 | 1896.70 | -3 | 117 | 49 | 74 |
| 9/19/2010 | 50,177 | 1896.60 | 16 | 118 | 32 | 90 |
| 9/20/2010 | 49,964 | 1896.40 | 2 | 119 | 29 | 90 |
| 9/21/2010 | 49,753 | 1896.30 | 9 | 118 | 27 | 90 |
| 9/22/2010 | 49,542 | 1896.20 | 6 | 117 | 23 | 90 |
| 9/23/2010 | 49,331 | 1896.10 | 7 | 116 | 23 | 90 |
| 9/24/2010 | 49,121 | 1896.00 | 3 | 115 | 23 | 90 |
| 9/25/2010 | 48,912 | 1895.80 | 6 | 115 | 22 | 90 |
| 9/26/2010 | 48,704 | 1895.70 | 5 | 116 | 22 | 90 |
| 9/27/2010 | 48,513 | 1895.60 | 8 | 116 | 22 | 90 |
| 9/28/2010 | 48,288 | 1895.50 | 6 | 116 | 22 | 91 |
| 9/29/2010 | 48,063 | 1895.40 | -8 | 117 | 21 | 91 |
| 9/30/2010 | 47,856 | 1895.20 | 67 | 119 | 22 | 91 |
| 10/1/2010 | 47,633 | 1895.10 | 2 | 120 | 25 | 91 |
| 10/2/2010 | 47,427 | 1895.00 | 10 | 120 | 24 | 91 |
| 10/3/2010 | 47,205 | 1894.90 | 3 | 121 | 25 | 91 |
| 10/4/2010 | 47,001 | 1894.70 | 13 | 122 | 26 | 91 |
| 10/5/2010 | 46,780 | 1894.60 | 5 | 122 | 27 | 91 |
| 10/6/2010 | 46,577 | 1894.50 | 14 | 122 | 28 | 91 |
| 10/7/2010 | 46,357 | 1894.40 | 5 | 122 | 28 | 91 |
| 10/8/2010 | 46,154 | 1894.20 | 14 | 122 | 28 | 91 |
| 10/9/2010 | 45,952 | 1894.10 | 14 | 122 | 27 | 91 |
| 10/10/2010 | 45,735 | 1894.00 | 6 | 122 | 27 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/11/2010 | 45,517 | 1893.90 | 7 | 123 | 28 | 91 |
| 10/12/2010 | 45,301 | 1893.70 | 9 | 124 | 29 | 91 |
| 10/13/2010 | 45,085 | 1893.60 | 12 | 127 | 30 | 91 |
| 10/14/2010 | 44,886 | 1893.50 | 21 | 128 | 32 | 91 |
| 10/15/2010 | 44,637 | 1893.30 | -3 | 129 | 32 | 91 |
| 10/16/2010 | 44,406 | 1893.20 | 5 | 128 | 32 | 91 |
| 10/17/2010 | 44,192 | 1893.10 | 14 | 128 | 32 | 91 |
| 10/18/2010 | 43,995 | 1892.90 | 12 | 117 | 34 | 83 |
| 10/19/2010 | 43,832 | 1892.80 | 18 | 105 | 37 | 61 |
| 10/20/2010 | 43,653 | 1892.70 | 9 | 105 | 36 | 60 |
| 10/21/2010 | 43,490 | 1892.60 | 18 | 105 | 38 | 60 |
| 10/22/2010 | 43,344 | 1892.60 | 24 | 103 | 48 | 52 |
| 10/23/2010 | 43,523 | 1892.70 | 185 | 100 | 64 | 45 |
| 10/24/2010 | 46,984 | 1894.70 | 1,848 | 108 | 483 | 45 |
| 10/25/2010 | 47,616 | 1895.10 | 440 | 128 | 189 | 45 |
| 10/26/2010 | 47,547 | 1895.10 | 127 | 170 | 139 | 44 |
| 10/27/2010 | 47,393 | 1895.00 | 114 | 202 | 162 | 44 |
| 10/28/2010 | 48,288 | 1895.50 | 642 | 201 | 275 | 45 |
| 10/29/2010 | 50,300 | 1896.60 | 1,206 | 202 | 397 | 46 |
| 10/30/2010 | 50,815 | 1896.90 | 443 | 193 | 249 | 46 |
| 10/31/2010 | 50,815 | 1896.90 | 208 | 219 | 199 | 46 |
| 11/1/2010 | 50,601 | 1896.80 | 144 | 265 | 235 | 49 |
| 11/2/2010 | 50,353 | 1896.70 | 121 | 259 | 189 | 87 |
| 11/3/2010 | 50,000 | 1896.50 | 96 | 289 | 145 | 131 |
| 11/4/2010 | 49,612 | 1896.20 | 107 | 318 | 206 | 120 |
| 11/5/2010 | 49,384 | 1896.10 | 66 | 190 | 144 | 83 |
| 11/6/2010 | 48,999 | 1895.90 | 60 | 267 | 155 | 98 |
| 11/7/2010 | 49,349 | 1896.10 | 503 | 344 | 299 | 168 |
| 11/8/2010 | 49,139 | 1896.00 | 200 | 322 | 231 | 171 |
| 11/9/2010 | 48,860 | 1895.80 | 163 | 320 | 203 | 171 |
| 11/10/2010 | 48,548 | 1895.60 | 147 | 320 | 211 | 165 |
| 11/11/2010 | 48,167 | 1895.40 | 115 | 323 | 204 | 162 |
| 11/12/2010 | 47,753 | 1895.20 | 94 | 318 | 197 | 162 |
| 11/13/2010 | 47,325 | 1894.90 | 80 | 311 | 193 | 162 |
| 11/14/2010 | 46,865 | 1894.70 | 62 | 309 | 192 | 161 |
| 11/15/2010 | 46,407 | 1894.40 | 59 | 305 | 191 | 161 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/16/2010 | 45,919 | 1894.10 | 45 | 306 | 181 | 170 |
| 11/17/2010 | 45,434 | 1893.80 | 49 | 309 | 174 | 174 |
| 11/18/2010 | 44,952 | 1893.50 | 52 | 310 | 166 | 180 |
| 11/19/2010 | 44,455 | 1893.20 | 43 | 309 | 155 | 192 |
| 11/20/2010 | 44,127 | 1893.00 | 131 | 312 | 161 | 196 |
| 11/21/2010 | 43,799 | 1892.80 | 131 | 312 | 171 | 197 |
| 11/22/2010 | 43,555 | 1892.70 | 174 | 313 | 200 | 196 |
| 11/23/2010 | 44,077 | 1893.00 | 563 | 316 | 483 | 196 |
| 11/24/2010 | 43,995 | 1892.90 | 257 | 314 | 275 | 197 |
| 11/25/2010 | 43,718 | 1892.80 | 158 | 313 | 215 | 192 |
| 11/26/2010 | 43,376 | 1892.60 | 123 | 311 | 197 | 187 |
| 11/27/2010 | 43,604 | 1892.70 | 416 | 317 | 295 | 193 |
| 11/28/2010 | 43,653 | 1892.70 | 324 | 315 | 262 | 200 |
| 11/29/2010 | 43,458 | 1892.60 | 200 | 314 | 209 | 201 |
| 11/30/2010 | 43,165 | 1892.40 | 149 | 312 | 187 | 201 |
| 12/1/2010 | 42,873 | 1892.30 | 148 | 311 | 176 | 201 |
| 12/2/2010 | 42,922 | 1892.30 | 325 | 316 | 196 | 200 |
| 12/3/2010 | 44,571 | 1893.30 | 1,137 | 322 | 299 | 200 |
| 12/4/2010 | 45,251 | 1893.70 | 651 | 324 | 245 | 200 |
| 12/5/2010 | 46,020 | 1894.20 | 699 | 328 | 244 | 200 |
| 12/6/2010 | 50,371 | 1896.70 | 2,516 | 339 | 620 | 199 |
| 12/7/2010 | 51,945 | 1897.60 | 1,114 | 337 | 393 | 195 |
| 12/8/2010 | 53,234 | 1898.30 | 976 | 343 | 359 | 197 |
| 12/9/2010 | 54,505 | 1899.00 | 986 | 363 | 426 | 124 |
| 12/10/2010 | 55,045 | 1899.30 | 1,006 | 772 | 553 | 165 |
| 12/11/2010 | 55,008 | 1899.20 | 915 | 983 | 688 | 210 |
| 12/12/2010 | 54,859 | 1899.20 | 770 | 889 | 600 | 210 |
| 12/13/2010 | 54,691 | 1899.10 | 618 | 740 | 463 | 210 |
| 12/14/2010 | 55,739 | 1899.60 | 1,668 | 1,200 | 1,070 | 210 |
| 12/15/2010 | 55,457 | 1899.50 | 1,254 | 1,470 | 1,440 | 210 |
| 12/16/2010 | 55,139 | 1899.30 | 960 | 1,180 | 1,020 | 210 |
| 12/17/2010 | 55,064 | 1899.30 | 869 | 954 | 721 | 210 |
| 12/18/2010 | 56,136 | 1899.90 | 2,042 | 1,580 | 1,450 | 210 |
| 12/19/2010 | 58,323 | 1901.00 | 4,152 | 3,210 | 3,800 | 195 |
| 12/20/2010 | 57,529 | 1900.60 | 3,200 | 3,790 | 4,560 | 174 |
| 12/21/2010 | 56,820 | 1900.20 | 2,397 | 2,900 | 3,250 | 174 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/22/2010 | 57,011 | 1900.30 | 2,861 | 2,910 | 3,240 | 172 |
| 12/23/2010 | 56,306 | 1899.90 | 1,982 | 2,460 | 2,560 | 198 |
| 12/24/2010 | 55,796 | 1899.70 | 1,481 | 1,830 | 1,800 | 199 |
| 12/25/2010 | 56,003 | 1899.80 | 1,586 | 1,560 | 1,500 | 199 |
| 12/26/2010 | 56,439 | 1900.00 | 2,072 | 1,950 | 2,030 | 200 |
| 12/27/2010 | 56,173 | 1899.90 | 1,908 | 2,150 | 2,300 | 200 |
| 12/28/2010 | 58,968 | 1901.30 | 3,566 | 2,270 | 2,630 | 199 |
| 12/29/2010 | 58,968 | 1901.30 | 6,669 | 7,020 | 9,010 | 11 |
| 12/30/2010 | 57,260 | 1900.40 | 2,664 | 3,710 | 5,020 | 67 |
| 12/31/2010 | 56,477 | 1900.00 | 1,847 | 2,360 | 2,990 | 82 |
| 1/1/2011 | 56,022 | 1899.80 | 1,500 | 1,820 | 2,160 | 83 |
| 1/2/2011 | 55,777 | 1899.70 | 1,301 | 1,500 | 1,640 | 155 |
| 1/3/2011 | 55,532 | 1899.50 | 1,093 | 1,280 | 1,310 | 200 |
| 1/4/2011 | 55,307 | 1899.40 | 893 | 1,060 | 1,040 | 200 |
| 1/5/2011 | 55,082 | 1899.30 | 757 | 916 | 876 | 190 |
| 1/6/2011 | 55,008 | 1899.20 | 724 | 801 | 708 | 200 |
| 1/7/2011 | 54,896 | 1899.20 | 624 | 716 | 615 | 200 |
| 1/8/2011 | 54,821 | 1899.10 | 594 | 665 | 547 | 200 |
| 1/9/2011 | 54,747 | 1899.10 | 547 | 615 | 502 | 206 |
| 1/10/2011 | 54,672 | 1899.10 | 492 | 557 | 459 | 210 |
| 1/11/2011 | 54,710 | 1899.10 | 526 | 534 | 425 | 209 |
| 1/12/2011 | 54,747 | 1899.10 | 578 | 589 | 419 | 216 |
| 1/13/2011 | 55,607 | 1899.60 | 1,220 | 827 | 504 | 221 |
| 1/14/2011 | 55,664 | 1899.60 | 1,292 | 1,330 | 979 | 220 |
| 1/15/2011 | 55,607 | 1899.60 | 1,159 | 1,250 | 954 | 220 |
| 1/16/2011 | 55,796 | 1899.70 | 1,311 | 1,280 | 969 | 220 |
| 1/17/2011 | 55,871 | 1899.70 | 1,416 | 1,450 | 1,014 | 219 |
| 1/18/2011 | 55,645 | 1899.60 | 1,169 | 1,350 | 979 | 219 |
| 1/19/2011 | 55,438 | 1899.50 | 1,026 | 1,190 | 864 | 220 |
| 1/20/2011 | 55,232 | 1899.40 | 884 | 1,040 | 744 | 220 |
| 1/21/2011 | 55,101 | 1899.30 | 797 | 908 | 674 | 220 |
| 1/22/2011 | 55,008 | 1899.20 | 733 | 821 | 394 | 220 |
| 1/23/2011 | 54,896 | 1899.20 | 649 | 743 | 319 | 220 |
| 1/24/2011 | 54,803 | 1899.10 | 587 | 667 | 354 | 214 |
| 1/25/2011 | 54,710 | 1899.10 | 523 | 600 | 381 | 223 |
| 1/26/2011 | 54,635 | 1899.00 | 463 | 527 | 343 | 223 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/27/2011 | 54,561 | 1899.00 | 416 | 477 | 311 | 223 |
| 1/28/2011 | 54,487 | 1899.00 | 374 | 433 | 313 | 188 |
| 1/29/2011 | 54,468 | 1899.00 | 375 | 405 | 278 | 201 |
| 1/30/2011 | 54,524 | 1899.00 | 437 | 430 | 284 | 224 |
| 1/31/2011 | 54,431 | 1898.90 | 351 | 419 | 270 | 224 |
| 2/1/2011 | 54,302 | 1898.90 | 297 | 381 | 241 | 224 |
| 2/2/2011 | 54,191 | 1898.80 | 279 | 353 | 219 | 224 |
| 2/3/2011 | 54,043 | 1898.70 | 257 | 349 | 209 | 224 |
| 2/4/2011 | 53,877 | 1898.60 | 250 | 351 | 205 | 224 |
| 2/5/2011 | 53,693 | 1898.50 | 240 | 350 | 203 | 223 |
| 2/6/2011 | 53,509 | 1898.40 | 239 | 349 | 201 | 223 |
| 2/7/2011 | 53,307 | 1898.30 | 231 | 350 | 200 | 223 |
| 2/8/2011 | 53,105 | 1898.20 | 231 | 350 | 199 | 223 |
| 2/9/2011 | 52,850 | 1898.10 | 204 | 350 | 197 | 223 |
| 2/10/2011 | 52,595 | 1897.90 | 204 | 350 | 196 | 223 |
| 2/11/2011 | 52,324 | 1897.80 | 195 | 350 | 194 | 222 |
| 2/12/2011 | 52,071 | 1897.60 | 189 | 333 | 193 | 221 |
| 2/13/2011 | 52,053 | 1897.60 | 183 | 202 | 210 | 49 |
| 2/14/2011 | 52,378 | 1897.80 | 357 | 203 | 255 | 1 |
| 2/15/2011 | 57,819 | 1900.70 | 3,304 | 590 | 1,150 | 1 |
| 2/16/2011 | 58,032 | 1900.90 | 4,192 | 4,300 | 5,580 | 1 |
| 2/17/2011 | 56,992 | 1900.30 | 1,936 | 2,590 | 3,430 | 1 |
| 2/18/2011 | 56,401 | 1900.00 | 1,479 | 1,870 | 2,290 | 1 |
| 2/19/2011 | 56,079 | 1899.80 | 1,234 | 1,470 | 1,690 | 57 |
| 2/20/2011 | 55,758 | 1899.60 | 1,007 | 1,230 | 1,190 | 224 |
| 2/21/2011 | 55,495 | 1899.50 | 884 | 1,070 | 973 | 232 |
| 2/22/2011 | 55,307 | 1899.40 | 796 | 937 | 799 | 237 |
| 2/23/2011 | 55,213 | 1899.40 | 758 | 848 | 677 | 234 |
| 2/24/2011 | 55,232 | 1899.40 | 774 | 805 | 623 | 232 |
| 2/25/2011 | 55,232 | 1899.40 | 796 | 838 | 673 | 233 |
| 2/26/2011 | 55,139 | 1899.30 | 709 | 796 | 615 | 233 |
| 2/27/2011 | 55,045 | 1899.30 | 639 | 722 | 533 | 232 |
| 2/28/2011 | 55,026 | 1899.30 | 640 | 684 | 493 | 232 |
| 3/1/2011 | 55,045 | 1899.30 | 655 | 680 | 495 | 232 |
| 3/2/2011 | 56,534 | 1900.10 | 1,891 | 1,200 | 1,100 | 231 |
| 3/3/2011 | 56,496 | 1900.00 | 1,843 | 1,960 | 2,110 | 231 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/4/2011 | 56,136 | 1899.90 | 1,376 | 1,640 | 1,750 | 231 |
| 3/5/2011 | 55,984 | 1899.80 | 1,244 | 1,390 | 1,400 | 231 |
| 3/6/2011 | 56,686 | 1900.10 | 1,940 | 1,670 | 1,780 | 230 |
| 3/7/2011 | 56,686 | 1900.10 | 1,966 | 2,070 | 2,260 | 230 |
| 3/8/2011 | 56,401 | 1900.00 | 1,585 | 1,820 | 2,020 | 150 |
| 3/9/2011 | 56,230 | 1899.90 | 1,424 | 1,590 | 1,780 | 46 |
| 3/10/2011 | 56,344 | 1900.00 | 1,492 | 1,510 | 1,690 | 46 |
| 3/11/2011 | 56,211 | 1899.90 | 1,386 | 1,530 | 1,740 | 46 |
| 3/12/2011 | 56,003 | 1899.80 | 1,197 | 1,370 | 1,520 | 45 |
| 3/13/2011 | 56,629 | 1900.10 | 1,589 | 1,340 | 1,470 | 45 |
| 3/14/2011 | 56,858 | 1900.20 | 2,177 | 2,170 | 2,470 | 45 |
| 3/15/2011 | 59,007 | 1901.40 | 3,449 | 2,490 | 2,960 | 45 |
| 3/16/2011 | 58,948 | 1901.30 | 4,891 | 5,180 | 6,730 | 45 |
| 3/17/2011 | 57,819 | 1900.70 | 2,927 | 3,680 | 4,740 | 45 |
| 3/18/2011 | 58,752 | 1901.20 | 4,033 | 3,750 | 4,980 | 45 |
| 3/19/2011 | 58,245 | 1901.00 | 3,326 | 3,770 | 5,030 | 45 |
| 3/20/2011 | 59,046 | 1901.40 | 4,575 | 4,390 | 5,880 | 45 |
| 3/21/2011 | 57,780 | 1900.70 | 2,791 | 3,610 | 4,840 | 45 |
| 3/22/2011 | 57,145 | 1900.40 | 2,178 | 2,630 | 3,410 | 45 |
| 3/23/2011 | 57,606 | 1900.60 | 2,684 | 2,580 | 3,400 | 45 |
| 3/24/2011 | 58,421 | 1901.10 | 3,650 | 3,410 | 4,800 | 45 |
| 3/25/2011 | 58,460 | 1901.10 | 3,582 | 3,750 | 5,460 | 46 |
| 3/26/2011 | 58,694 | 1901.20 | 4,117 | 4,210 | 6,270 | 45 |
| 3/27/2011 | 58,071 | 1900.90 | 3,172 | 3,670 | 5,300 | 45 |
| 3/28/2011 | 57,645 | 1900.70 | 2,787 | 3,160 | 4,190 | 45 |
| 3/29/2011 | 57,299 | 1900.50 | 2,390 | 2,700 | 3,480 | 45 |
| 3/30/2011 | 57,780 | 1900.70 | 2,789 | 2,680 | 3,310 | 45 |
| 3/31/2011 | 58,246 | 1901.00 | 3,436 | 3,370 | 4,120 | 45 |
| 4/1/2011 | 58,323 | 1901.00 | 3,564 | 3,710 | 4,500 | 46 |
| 4/2/2011 | 59,341 | 1901.50 | 3,610 | 3,260 | 4,270 | 45 |
| 4/3/2011 | 61,576 | 1902.70 | 2,846 | 1,810 | 2,360 | 45 |
| 4/4/2011 | 64,212 | 1904.00 | 2,336 | 1,060 | 1,230 | 51 |
| 4/5/2011 | 65,483 | 1904.60 | 2,313 | 1,760 | 1,820 | 49 |
| 4/6/2011 | 65,106 | 1904.40 | 2,023 | 2,330 | 2,590 | 46 |
| 4/7/2011 | 64,440 | 1904.10 | 1,745 | 2,190 | 2,400 | 46 |
| 4/8/2011 | 64,026 | 1903.90 | 1,492 | 1,790 | 1,950 | 55 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/9/2011 | 64,026 | 1903.90 | 1,301 | 1,370 | 1,510 | 61 |
| 4/10/2011 | 64,523 | 1904.10 | 1,153 | 950 | 941 | 61 |
| 4/11/2011 | 64,793 | 1904.30 | 1,096 | 1,010 | 982 | 61 |
| 4/12/2011 | 65,588 | 1904.70 | 1,003 | 634 | 761 | 67 |
| 4/13/2011 | 66,917 | 1905.30 | 1,015 | 292 | 390 | 70 |
| 4/14/2011 | 67,984 | 1905.80 | 879 | 295 | 375 | 70 |
| 4/15/2011 | 69,059 | 1906.30 | 875 | 297 | 360 | 84 |
| 4/16/2011 | 70,386 | 1907.00 | 1,013 | 300 | 356 | 90 |
| 4/17/2011 | 71,021 | 1907.30 | 1,280 | 1,010 | 814 | 89 |
| 4/18/2011 | 71,043 | 1907.30 | 1,425 | 1,510 | 1,350 | 89 |
| 4/19/2011 | 70,364 | 1907.00 | 1,349 | 1,790 | 1,750 | 89 |
| 4/20/2011 | 69,841 | 1906.70 | 1,187 | 1,540 | 1,530 | 89 |
| 4/21/2011 | 69,993 | 1906.80 | 1,312 | 1,300 | 1,230 | 89 |
| 4/22/2011 | 69,885 | 1906.70 | 1,123 | 1,240 | 1,310 | 89 |
| 4/23/2011 | 70,015 | 1906.80 | 1,006 | 967 | 982 | 90 |
| 4/24/2011 | 70,758 | 1907.10 | 932 | 576 | 509 | 89 |
| 4/25/2011 | 70,977 | 1907.30 | 976 | 908 | 741 | 89 |
| 4/26/2011 | 71,109 | 1907.30 | 925 | 899 | 890 | 90 |
| 4/27/2011 | 71,700 | 1907.60 | 827 | 527 | 466 | 90 |
| 4/28/2011 | 72,147 | 1907.80 | 865 | 535 | 466 | 90 |
| 4/29/2011 | 72,457 | 1907.90 | 672 | 519 | 457 | 90 |
| 4/30/2011 | 72,724 | 1908.10 | 624 | 503 | 443 | 90 |
| 5/1/2011 | 72,835 | 1908.10 | 600 | 573 | 467 | 90 |
| 5/2/2011 | 72,880 | 1908.10 | 603 | 617 | 501 | 90 |
| 5/3/2011 | 73,058 | 1908.20 | 670 | 616 | 502 | 90 |
| 5/4/2011 | 73,125 | 1908.30 | 608 | 606 | 492 | 90 |
| 5/5/2011 | 73,259 | 1908.30 | 639 | 604 | 488 | 89 |
| 5/6/2011 | 73,371 | 1908.40 | 678 | 651 | 500 | 89 |
| 5/7/2011 | 73,170 | 1908.30 | 698 | 845 | 650 | 89 |
| 5/8/2011 | 72,902 | 1908.20 | 654 | 837 | 717 | 89 |
| 5/9/2011 | 72,924 | 1908.20 | 565 | 576 | 505 | 89 |
| 5/10/2011 | 73,036 | 1908.20 | 492 | 434 | 373 | 89 |
| 5/11/2011 | 73,125 | 1908.30 | 482 | 435 | 373 | 89 |
| 5/12/2011 | 73,058 | 1908.20 | 497 | 554 | 410 | 90 |
| 5/13/2011 | 72,902 | 1908.20 | 510 | 631 | 479 | 98 |
| 5/14/2011 | 72,769 | 1908.10 | 481 | 584 | 430 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/15/2011 | 72,724 | 1908.10 | 551 | 598 | 430 | 129 |
| 5/16/2011 | 72,657 | 1908.00 | 498 | 565 | 417 | 130 |
| 5/17/2011 | 72,746 | 1908.10 | 512 | 491 | 376 | 130 |
| 5/18/2011 | 72,813 | 1908.10 | 528 | 510 | 391 | 130 |
| 5/19/2011 | 72,791 | 1908.10 | 462 | 485 | 375 | 130 |
| 5/20/2011 | 72,835 | 1908.10 | 440 | 415 | 331 | 130 |
| 5/21/2011 | 72,969 | 1908.20 | 423 | 341 | 262 | 130 |
| 5/22/2011 | 73,103 | 1908.20 | 417 | 333 | 251 | 130 |
| 5/23/2011 | 73,192 | 1908.30 | 396 | 328 | 245 | 130 |
| 5/24/2011 | 73,237 | 1908.30 | 383 | 342 | 246 | 130 |
| 5/25/2011 | 73,505 | 1908.40 | 538 | 403 | 273 | 130 |
| 5/26/2011 | 73,483 | 1908.40 | 484 | 513 | 380 | 131 |
| 5/27/2011 | 73,505 | 1908.40 | 402 | 384 | 282 | 131 |
| 5/28/2011 | 73,528 | 1908.40 | 396 | 384 | 277 | 131 |
| 5/29/2011 | 73,483 | 1908.40 | 364 | 377 | 272 | 131 |
| 5/30/2011 | 73,416 | 1908.40 | 347 | 366 | 256 | 131 |
| 5/31/2011 | 73,573 | 1908.50 | 446 | 369 | 282 | 131 |
| 6/1/2011 | 73,148 | 1908.30 | 538 | 810 | 543 | 131 |
| 6/2/2011 | 72,325 | 1907.90 | 591 | 1,070 | 975 | 131 |
| 6/3/2011 | 72,192 | 1907.80 | 446 | 537 | 412 | 133 |
| 6/4/2011 | 72,924 | 1908.20 | 807 | 506 | 384 | 131 |
| 6/5/2011 | 73,036 | 1908.20 | 1,723 | 1,870 | 1,440 | 130 |
| 6/6/2011 | 71,771 | 1907.60 | 1,345 | 1,730 | 1,800 | 129 |
| 6/7/2011 | 71,837 | 1907.70 | 935 | 949 | 827 | 129 |
| 6/8/2011 | 71,881 | 1907.70 | 797 | 819 | 626 | 129 |
| 6/9/2011 | 72,037 | 1907.70 | 689 | 638 | 522 | 129 |
| 6/10/2011 | 72,258 | 1907.90 | 598 | 486 | 388 | 130 |
| 6/11/2011 | 72,480 | 1908.00 | 528 | 414 | 335 | 130 |
| 6/12/2011 | 72,613 | 1908.00 | 480 | 409 | 327 | 130 |
| 6/13/2011 | 72,746 | 1908.10 | 466 | 401 | 319 | 129 |
| 6/14/2011 | 72,791 | 1908.10 | 427 | 399 | 311 | 129 |
| 6/15/2011 | 72,880 | 1908.10 | 366 | 333 | 276 | 129 |
| 6/16/2011 | 73,014 | 1908.20 | 327 | 286 | 218 | 129 |
| 6/17/2011 | 72,991 | 1908.20 | 292 | 304 | 225 | 129 |
| 6/18/2011 | 72,947 | 1908.20 | 274 | 307 | 236 | 130 |
| 6/19/2011 | 72,924 | 1908.20 | 257 | 281 | 213 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/20/2011 | 72,924 | 1908.20 | 239 | 271 | 193 | 129 |
| 6/21/2011 | 72,924 | 1908.20 | 227 | 253 | 179 | 129 |
| 6/22/2011 | 72,947 | 1908.20 | 200 | 221 | 143 | 129 |
| 6/23/2011 | 72,991 | 1908.20 | 187 | 193 | 108 | 129 |
| 6/24/2011 | 73,014 | 1908.20 | 184 | 180 | 89 | 129 |
| 6/25/2011 | 72,991 | 1908.20 | 170 | 180 | 87 | 129 |
| 6/26/2011 | 72,969 | 1908.20 | 160 | 180 | 85 | 130 |
| 6/27/2011 | 72,924 | 1908.20 | 149 | 179 | 82 | 130 |
| 6/28/2011 | 73,036 | 1908.20 | 240 | 197 | 98 | 130 |
| 6/29/2011 | 73,036 | 1908.20 | 215 | 228 | 140 | 131 |
| 6/30/2011 | 72,969 | 1908.20 | 165 | 209 | 119 | 130 |
| 7/1/2011 | 72,924 | 1908.20 | 150 | 198 | 95 | 130 |
| 7/2/2011 | 72,835 | 1908.10 | 129 | 186 | 81 | 129 |
| 7/3/2011 | 72,724 | 1908.10 | 117 | 187 | 78 | 129 |
| 7/4/2011 | 72,613 | 1908.00 | 177 | 190 | 77 | 129 |
| 7/5/2011 | 72,480 | 1908.00 | 105 | 188 | 75 | 130 |
| 7/6/2011 | 72,325 | 1907.90 | 101 | 187 | 73 | 130 |
| 7/7/2011 | 72,170 | 1907.80 | 79 | 186 | 72 | 130 |
| 7/8/2011 | 71,992 | 1907.70 | 85 | 185 | 71 | 130 |
| 7/9/2011 | 71,815 | 1907.60 | 71 | 181 | 70 | 130 |
| 7/10/2011 | 71,616 | 1907.60 | 64 | 181 | 69 | 130 |
| 7/11/2011 | 71,395 | 1907.40 | 64 | 183 | 68 | 130 |
| 7/12/2011 | 71,197 | 1907.40 | 64 | 183 | 68 | 130 |
| 7/13/2011 | 70,977 | 1907.30 | 53 | 171 | 63 | 130 |
| 7/14/2011 | 70,779 | 1907.20 | 52 | 157 | 53 | 130 |
| 7/15/2011 | 70,604 | 1907.10 | 49 | 154 | 48 | 130 |
| 7/16/2011 | 70,429 | 1907.00 | 48 | 153 | 47 | 130 |
| 7/17/2011 | 70,233 | 1906.90 | 47 | 152 | 47 | 130 |
| 7/18/2011 | 70,059 | 1906.80 | 47 | 152 | 47 | 130 |
| 7/19/2011 | 69,885 | 1906.70 | 48 | 152 | 46 | 130 |
| 7/20/2011 | 69,667 | 1906.60 | 36 | 152 | 45 | 130 |
| 7/21/2011 | 69,471 | 1906.50 | 41 | 152 | 44 | 130 |
| 7/22/2011 | 69,276 | 1906.40 | 37 | 152 | 43 | 130 |
| 7/23/2011 | 69,059 | 1906.30 | 34 | 152 | 42 | 130 |
| 7/24/2011 | 68,843 | 1906.20 | 26 | 151 | 42 | 130 |
| 7/25/2011 | 68,606 | 1906.10 | 24 | 151 | 41 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/26/2011 | 68,370 | 1906.00 | 19 | 151 | 40 | 130 |
| 7/27/2011 | 68,155 | 1905.90 | 24 | 151 | 40 | 130 |
| 7/28/2011 | 67,877 | 1905.80 | 16 | 172 | 39 | 130 |
| 7/29/2011 | 67,449 | 1905.60 | 25 | 252 | 136 | 132 |
| 7/30/2011 | 67,194 | 1905.40 | 12 | 160 | 67 | 131 |
| 7/31/2011 | 66,938 | 1905.30 | 13 | 155 | 41 | 131 |
| 8/1/2011 | 66,684 | 1905.20 | 9 | 152 | 37 | 130 |
| 8/2/2011 | 66,409 | 1905.10 | 4 | 152 | 36 | 130 |
| 8/3/2011 | 66,134 | 1904.90 | 1 | 152 | 36 | 130 |
| 8/4/2011 | 65,882 | 1904.80 | 5 | 153 | 35 | 130 |
| 8/5/2011 | 65,609 | 1904.70 | 7 | 153 | 35 | 130 |
| 8/6/2011 | 65,357 | 1904.60 | 6 | 153 | 35 | 130 |
| 8/7/2011 | 65,085 | 1904.40 | 5 | 155 | 34 | 130 |
| 8/8/2011 | 64,814 | 1904.30 | 4 | 156 | 34 | 130 |
| 8/9/2011 | 64,544 | 1904.20 | 11 | 162 | 33 | 130 |
| 8/10/2011 | 64,253 | 1904.00 | 6 | 162 | 36 | 131 |
| 8/11/2011 | 63,964 | 1903.90 | -4 | 153 | 35 | 131 |
| 8/12/2011 | 63,675 | 1903.70 | -10 | 152 | 34 | 131 |
| 8/13/2011 | 63,387 | 1903.60 | -7 | 152 | 34 | 130 |
| 8/14/2011 | 63,100 | 1903.40 | -11 | 152 | 34 | 129 |
| 8/15/2011 | 62,813 | 1903.30 | -7 | 152 | 34 | 130 |
| 8/16/2011 | 62,528 | 1903.20 | -9 | 151 | 33 | 130 |
| 8/17/2011 | 62,223 | 1903.00 | -11 | 151 | 32 | 130 |
| 8/18/2011 | 61,939 | 1902.90 | -8 | 151 | 32 | 130 |
| 8/19/2011 | 61,656 | 1902.70 | -10 | 151 | 32 | 130 |
| 8/20/2011 | 61,374 | 1902.60 | -8 | 151 | 32 | 129 |
| 8/21/2011 | 61,072 | 1902.40 | -9 | 151 | 31 | 129 |
| 8/22/2011 | 60,791 | 1902.30 | -5 | 151 | 31 | 129 |
| 8/23/2011 | 60,451 | 1902.10 | -1 | 171 | 32 | 138 |
| 8/24/2011 | 60,212 | 1902.00 | 2 | 182 | 31 | 155 |
| 8/25/2011 | 59,875 | 1901.80 | -3 | 181 | 28 | 158 |
| 8/26/2011 | 59,499 | 1901.60 | -3 | 191 | 29 | 158 |
| 8/27/2011 | 59,105 | 1901.40 | -1 | 209 | 39 | 172 |
| 8/28/2011 | 58,713 | 1901.20 | -2 | 209 | 28 | 181 |
| 8/29/2011 | 58,323 | 1901.00 | -1 | 209 | 29 | 180 |
| 8/30/2011 | 57,935 | 1900.80 | -10 | 209 | 28 | 180 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/31/2011 | 57,529 | 1900.60 | -11 | 210 | 29 | 180 |
| 9/1/2011 | 57,126 | 1900.40 | -4 | 210 | 31 | 180 |
| 9/2/2011 | 56,725 | 1900.20 | -9 | 218 | 33 | 182 |
| 9/3/2011 | 56,287 | 1899.90 | -9 | 225 | 33 | 193 |
| 9/4/2011 | 55,852 | 1899.70 | -10 | 227 | 34 | 195 |
| 9/5/2011 | 55,420 | 1899.50 | -11 | 227 | 32 | 196 |
| 9/6/2011 | 54,989 | 1899.20 | -9 | 226 | 31 | 196 |
| 9/7/2011 | 54,561 | 1899.00 | -8 | 226 | 30 | 196 |
| 9/8/2011 | 54,135 | 1898.80 | -12 | 225 | 29 | 196 |
| 9/9/2011 | 53,693 | 1898.50 | -8 | 226 | 29 | 196 |
| 9/10/2011 | 53,252 | 1898.30 | -17 | 230 | 34 | 197 |
| 9/11/2011 | 52,814 | 1898.00 | -9 | 229 | 32 | 198 |
| 9/12/2011 | 52,378 | 1897.80 | -8 | 229 | 31 | 198 |
| 9/13/2011 | 51,927 | 1897.60 | -14 | 228 | 30 | 198 |
| 9/14/2011 | 51,512 | 1897.30 | -6 | 228 | 29 | 199 |
| 9/15/2011 | 51,064 | 1897.10 | -9 | 228 | 29 | 199 |
| 9/16/2011 | 50,637 | 1896.80 | -8 | 228 | 29 | 199 |
| 9/17/2011 | 50,212 | 1896.60 | -6 | 228 | 28 | 199 |
| 9/18/2011 | 49,788 | 1896.30 | -3 | 227 | 28 | 199 |
| 9/19/2011 | 49,349 | 1896.10 | -5 | 227 | 27 | 199 |
| 9/20/2011 | 48,947 | 1895.90 | -6 | 228 | 26 | 199 |
| 9/21/2011 | 48,513 | 1895.60 | -5 | 229 | 29 | 199 |
| 9/22/2011 | 48,080 | 1895.40 | -6 | 231 | 29 | 199 |
| 9/23/2011 | 47,633 | 1895.10 | -6 | 232 | 32 | 198 |
| 9/24/2011 | 47,205 | 1894.90 | -7 | 231 | 30 | 199 |
| 9/25/2011 | 46,763 | 1894.60 | -6 | 231 | 30 | 199 |
| 9/26/2011 | 46,323 | 1894.30 | -6 | 231 | 30 | 200 |
| 9/27/2011 | 45,919 | 1894.10 | -4 | 231 | 29 | 200 |
| 9/28/2011 | 45,401 | 1893.80 | -46 | 230 | 29 | 200 |
| 9/29/2011 | 44,969 | 1893.50 | -2 | 233 | 30 | 200 |
| 9/30/2011 | 44,521 | 1893.30 | -3 | 235 | 34 | 200 |
| 10/1/2011 | 44,077 | 1893.00 | 28 | 235 | 34 | 200 |
| 10/2/2011 | 43,636 | 1892.70 | -42 | 234 | 34 | 200 |
| 10/3/2011 | 43,441 | 1892.60 | 120 | 236 | 45 | 200 |
| 10/4/2011 | 43,197 | 1892.50 | 92 | 236 | 44 | 200 |
| 10/5/2011 | 43,100 | 1892.40 | 184 | 237 | 69 | 201 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/6/2011 | 42,776 | 1892.20 | 68 | 243 | 51 | 201 |
| 10/7/2011 | 42,342 | 1891.90 | 36 | 265 | 71 | 196 |
| 10/8/2011 | 41,880 | 1891.60 | 27 | 274 | 82 | 202 |
| 10/9/2011 | 41,387 | 1891.30 | 20 | 279 | 83 | 203 |
| 10/10/2011 | 41,134 | 1891.20 | 148 | 283 | 119 | 204 |
| 10/11/2011 | 40,866 | 1891.00 | 144 | 286 | 122 | 204 |
| 10/12/2011 | 40,412 | 1890.70 | 59 | 308 | 113 | 204 |
| 10/13/2011 | 39,837 | 1890.40 | 38 | 349 | 131 | 228 |
| 10/14/2011 | 39,250 | 1890.00 | 31 | 347 | 122 | 241 |
| 10/15/2011 | 38,716 | 1889.60 | 30 | 311 | 102 | 231 |
| 10/16/2011 | 38,185 | 1889.30 | 25 | 306 | 92 | 231 |
| 10/17/2011 | 37,643 | 1888.90 | 24 | 306 | 91 | 231 |
| 10/18/2011 | 37,107 | 1888.60 | 23 | 304 | 88 | 231 |
| 10/19/2011 | 36,603 | 1888.20 | 22 | 297 | 80 | 232 |
| 10/20/2011 | 36,104 | 1887.90 | 24 | 282 | 74 | 223 |
| 10/21/2011 | 35,668 | 1887.60 | 22 | 260 | 70 | 206 |
| 10/22/2011 | 35,263 | 1887.30 | 23 | 239 | 53 | 197 |
| 10/23/2011 | 34,890 | 1887.00 | 21 | 229 | 52 | 185 |
| 10/24/2011 | 34,519 | 1886.80 | 22 | 226 | 52 | 180 |
| 10/25/2011 | 34,151 | 1886.50 | 19 | 221 | 54 | 174 |
| 10/26/2011 | 33,897 | 1886.30 | 19 | 156 | 107 | 84 |
| 10/27/2011 | 33,729 | 1886.20 | 14 | 109 | 82 | 45 |
| 10/28/2011 | 33,575 | 1886.10 | 17 | 102 | 71 | 45 |
| 10/29/2011 | 33,435 | 1886.00 | 19 | 99 | 58 | 51 |
| 10/30/2011 | 33,296 | 1885.90 | 18 | 99 | 51 | 58 |
| 10/31/2011 | 33,144 | 1885.80 | 19 | 99 | 49 | 60 |
| 11/1/2011 | 32,992 | 1885.70 | 5 | 103 | 49 | 60 |
| 11/2/2011 | 32,785 | 1885.50 | 10 | 117 | 59 | 69 |
| 11/3/2011 | 32,579 | 1885.40 | 14 | 126 | 59 | 83 |
| 11/4/2011 | 32,373 | 1885.20 | 8 | 125 | 51 | 90 |
| 11/5/2011 | 32,237 | 1885.10 | 60 | 139 | 61 | 93 |
| 11/6/2011 | 32,088 | 1885.00 | 53 | 145 | 66 | 100 |
| 11/7/2011 | 31,763 | 1884.80 | -15 | 161 | 59 | 112 |
| 11/8/2011 | 31,508 | 1884.60 | 18 | 167 | 54 | 127 |
| 11/9/2011 | 31,227 | 1884.40 | 20 | 174 | 51 | 132 |
| 11/10/2011 | 30,935 | 1884.20 | 21 | 187 | 58 | 134 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/11/2011 | 30,657 | 1884.00 | 32 | 191 | 60 | 138 |
| 11/12/2011 | 30,356 | 1883.70 | 25 | 190 | 59 | 139 |
| 11/13/2011 | 30,069 | 1883.50 | 27 | 189 | 58 | 139 |
| 11/14/2011 | 29,771 | 1883.30 | 23 | 189 | 57 | 138 |
| 11/15/2011 | 29,475 | 1883.00 | 23 | 189 | 56 | 139 |
| 11/16/2011 | 29,194 | 1882.80 | 27 | 183 | 53 | 139 |
| 11/17/2011 | 28,928 | 1882.60 | 17 | 170 | 54 | 129 |
| 11/18/2011 | 28,701 | 1882.40 | 26 | 151 | 65 | 105 |
| 11/19/2011 | 28,601 | 1882.30 | 55 | 108 | 80 | 57 |
| 11/20/2011 | 28,526 | 1882.30 | 56 | 100 | 76 | 46 |
| 11/21/2011 | 28,414 | 1882.20 | 36 | 100 | 70 | 46 |
| 11/22/2011 | 28,314 | 1882.10 | 44 | 100 | 74 | 46 |
| 11/23/2011 | 28,289 | 1882.10 | 82 | 100 | 82 | 46 |
| 11/24/2011 | 28,701 | 1882.40 | 303 | 100 | 151 | 46 |
| 11/25/2011 | 28,777 | 1882.50 | 151 | 119 | 109 | 46 |
| 11/26/2011 | 28,676 | 1882.40 | 97 | 155 | 136 | 46 |
| 11/27/2011 | 28,501 | 1882.30 | 68 | 164 | 148 | 46 |
| 11/28/2011 | 28,314 | 1882.10 | 61 | 164 | 144 | 46 |
| 11/29/2011 | 28,091 | 1881.90 | 58 | 179 | 143 | 53 |
| 11/30/2011 | 27,832 | 1881.70 | 47 | 186 | 139 | 67 |
| 12/1/2011 | 27,624 | 1881.50 | 72 | 186 | 128 | 74 |
| 12/2/2011 | 27,345 | 1881.30 | 21 | 170 | 141 | 55 |
| 12/3/2011 | 27,128 | 1881.10 | 44 | 162 | 139 | 45 |
| 12/4/2011 | 26,900 | 1880.90 | 33 | 162 | 137 | 45 |
| 12/5/2011 | 26,496 | 1880.60 | -56 | 167 | 136 | 45 |
| 12/6/2011 | 26,261 | 1880.40 | 43 | 170 | 136 | 45 |
| 12/7/2011 | 26,040 | 1880.20 | 44 | 164 | 132 | 46 |
| 12/8/2011 | 25,831 | 1880.00 | 47 | 160 | 127 | 45 |
| 12/9/2011 | 25,612 | 1879.80 | 40 | 159 | 126 | 46 |
| 12/10/2011 | 25,395 | 1879.70 | 42 | 160 | 125 | 46 |
| 12/11/2011 | 25,180 | 1879.50 | 33 | 160 | 125 | 46 |
| 12/12/2011 | 24,966 | 1879.30 | 43 | 158 | 125 | 46 |
| 12/13/2011 | 24,743 | 1879.10 | 29 | 161 | 125 | 46 |
| 12/14/2011 | 24,510 | 1878.90 | 32 | 165 | 131 | 46 |
| 12/15/2011 | 24,312 | 1878.70 | 54 | 162 | 132 | 46 |
| 12/16/2011 | 24,093 | 1878.50 | 42 | 160 | 127 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/17/2011 | 23,878 | 1878.30 | 43 | 159 | 127 | 46 |
| 12/18/2011 | 23,664 | 1878.10 | 30 | 159 | 126 | 46 |
| 12/19/2011 | 23,440 | 1877.90 | 27 | 158 | 126 | 46 |
| 12/20/2011 | 23,219 | 1877.70 | 28 | 159 | 126 | 46 |
| 12/21/2011 | 23,000 | 1877.50 | 30 | 160 | 125 | 46 |
| 12/22/2011 | 22,773 | 1877.30 | 24 | 159 | 124 | 46 |
| 12/23/2011 | 22,538 | 1877.00 | 25 | 159 | 124 | 46 |
| 12/24/2011 | 22,314 | 1876.80 | 27 | 159 | 123 | 46 |
| 12/25/2011 | 22,093 | 1876.60 | 27 | 158 | 123 | 46 |
| 12/26/2011 | 21,875 | 1876.40 | 26 | 158 | 123 | 46 |
| 12/27/2011 | 21,649 | 1876.20 | 27 | 162 | 125 | 46 |
| 12/28/2011 | 21,414 | 1875.90 | 29 | 164 | 130 | 46 |
| 12/29/2011 | 21,192 | 1875.70 | 30 | 164 | 131 | 46 |
| 12/30/2011 | 20,990 | 1875.50 | 42 | 164 | 136 | 46 |
| 12/31/2011 | 20,791 | 1875.30 | 46 | 163 | 134 | 46 |
| 1/1/2012 | 20,575 | 1875.10 | 38 | 163 | 132 | 46 |
| 1/2/2012 | 20,369 | 1874.90 | 34 | 162 | 131 | 46 |
| 1/3/2012 | 20,146 | 1874.60 | 32 | 162 | 131 | 46 |
| 1/4/2012 | 19,918 | 1874.40 | 26 | 162 | 129 | 46 |
| 1/5/2012 | 19,691 | 1874.10 | 30 | 162 | 128 | 46 |
| 1/6/2012 | 19,466 | 1873.90 | 28 | 161 | 127 | 46 |
| 1/7/2012 | 19,243 | 1873.70 | 29 | 163 | 127 | 46 |
| 1/8/2012 | 19,006 | 1873.40 | 30 | 163 | 126 | 46 |
| 1/9/2012 | 18,788 | 1873.10 | 29 | 163 | 126 | 46 |
| 1/10/2012 | 18,564 | 1872.90 | 28 | 159 | 123 | 46 |
| 1/11/2012 | 18,342 | 1872.60 | 26 | 156 | 118 | 46 |
| 1/12/2012 | 18,123 | 1872.40 | 28 | 156 | 118 | 46 |
| 1/13/2012 | 17,905 | 1872.10 | 25 | 156 | 117 | 45 |
| 1/14/2012 | 17,905 | 1872.10 | 26 | 156 | 117 | 45 |
| 1/15/2012 | 17,461 | 1871.60 | 25 | 155 | 116 | 45 |
| 1/16/2012 | 17,234 | 1871.30 | 23 | 159 | 116 | 45 |
| 1/17/2012 | 16,985 | 1871.00 | 21 | 165 | 126 | 45 |
| 1/18/2012 | 16,770 | 1870.80 | 36 | 165 | 126 | 45 |
| 1/19/2012 | 16,739 | 1870.70 | 134 | 167 | 160 | 45 |
| 1/20/2012 | 19,181 | 1873.60 | 1,242 | 180 | 543 | 45 |
| 1/21/2012 | 23,409 | 1877.90 | 2,591 | 182 | 886 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/22/2012 | 24,754 | 1879.10 | 868 | 182 | 372 | 43 |
| 1/23/2012 | 28,852 | 1882.50 | 2,212 | 195 | 908 | 44 |
| 1/24/2012 | 29,849 | 1883.30 | 858 | 186 | 362 | 45 |
| 1/25/2012 | 30,750 | 1884.00 | 635 | 186 | 277 | 45 |
| 1/26/2012 | 32,088 | 1885.00 | 875 | 187 | 263 | 45 |
| 1/27/2012 | 32,881 | 1885.60 | 611 | 198 | 237 | 45 |
| 1/28/2012 | 33,227 | 1885.90 | 377 | 204 | 220 | 45 |
| 1/29/2012 | 33,407 | 1886.00 | 284 | 204 | 206 | 45 |
| 1/30/2012 | 33,505 | 1886.10 | 231 | 204 | 198 | 45 |
| 1/31/2012 | 33,519 | 1886.10 | 199 | 204 | 191 | 45 |
| 2/1/2012 | 33,547 | 1886.10 | 193 | 205 | 192 | 45 |
| 2/2/2012 | 33,505 | 1886.10 | 166 | 204 | 186 | 45 |
| 2/3/2012 | 33,435 | 1886.00 | 146 | 204 | 182 | 45 |
| 2/4/2012 | 33,338 | 1885.90 | 132 | 204 | 179 | 45 |
| 2/5/2012 | 33,213 | 1885.90 | 125 | 204 | 177 | 45 |
| 2/6/2012 | 33,088 | 1885.80 | 124 | 204 | 175 | 45 |
| 2/7/2012 | 33,282 | 1885.90 | 282 | 205 | 192 | 45 |
| 2/8/2012 | 33,533 | 1886.10 | 334 | 204 | 202 | 45 |
| 2/9/2012 | 33,589 | 1886.10 | 224 | 204 | 188 | 45 |
| 2/10/2012 | 33,631 | 1886.20 | 208 | 202 | 185 | 45 |
| 2/11/2012 | 33,687 | 1886.20 | 219 | 202 | 188 | 45 |
| 2/12/2012 | 33,729 | 1886.20 | 211 | 202 | 184 | 45 |
| 2/13/2012 | 33,827 | 1886.30 | 248 | 203 | 219 | 45 |
| 2/14/2012 | 33,925 | 1886.40 | 237 | 202 | 215 | 45 |
| 2/15/2012 | 33,925 | 1886.40 | 197 | 202 | 202 | 45 |
| 2/16/2012 | 33,897 | 1886.30 | 175 | 204 | 193 | 45 |
| 2/17/2012 | 33,841 | 1886.30 | 163 | 204 | 188 | 45 |
| 2/18/2012 | 33,771 | 1886.30 | 158 | 204 | 186 | 44 |
| 2/19/2012 | 33,701 | 1886.20 | 149 | 205 | 183 | 45 |
| 2/20/2012 | 33,603 | 1886.10 | 140 | 204 | 181 | 44 |
| 2/21/2012 | 33,491 | 1886.10 | 133 | 204 | 179 | 44 |
| 2/22/2012 | 33,366 | 1886.00 | 127 | 203 | 177 | 44 |
| 2/23/2012 | 33,241 | 1885.90 | 119 | 197 | 172 | 44 |
| 2/24/2012 | 33,116 | 1885.80 | 116 | 192 | 167 | 44 |
| 2/25/2012 | 32,992 | 1885.70 | 102 | 182 | 158 | 44 |
| 2/26/2012 | 32,868 | 1885.60 | 104 | 175 | 150 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/27/2012 | 32,744 | 1885.50 | 91 | 167 | 147 | 45 |
| 2/28/2012 | 32,716 | 1885.50 | 131 | 162 | 146 | 46 |
| 2/29/2012 | 33,061 | 1885.70 | 313 | 150 | 219 | 46 |
| 3/1/2012 | 33,407 | 1886.00 | 327 | 150 | 262 | 46 |
| 3/2/2012 | 33,631 | 1886.20 | 268 | 149 | 223 | 46 |
| 3/3/2012 | 33,785 | 1886.30 | 224 | 149 | 194 | 45 |
| 3/4/2012 | 34,010 | 1886.40 | 263 | 159 | 193 | 45 |
| 3/5/2012 | 34,222 | 1886.60 | 268 | 170 | 194 | 45 |
| 3/6/2012 | 34,335 | 1886.70 | 225 | 183 | 195 | 45 |
| 3/7/2012 | 34,377 | 1886.70 | 190 | 189 | 191 | 45 |
| 3/8/2012 | 34,363 | 1886.70 | 167 | 189 | 184 | 45 |
| 3/9/2012 | 34,335 | 1886.70 | 157 | 189 | 180 | 45 |
| 3/10/2012 | 34,307 | 1886.60 | 155 | 189 | 177 | 45 |
| 3/11/2012 | 34,278 | 1886.60 | 155 | 189 | 179 | 45 |
| 3/12/2012 | 34,250 | 1886.60 | 150 | 189 | 176 | 45 |
| 3/13/2012 | 38,534 | 1889.50 | 2,327 | 203 | 671 | 45 |
| 3/14/2012 | 44,028 | 1893.00 | 3,142 | 206 | 912 | 45 |
| 3/15/2012 | 47,273 | 1894.90 | 1,909 | 206 | 563 | 45 |
| 3/16/2012 | 54,672 | 1899.10 | 3,941 | 217 | 1,166 | 45 |
| 3/17/2012 | 57,107 | 1900.40 | 3,701 | 2,406 | 2,987 | 45 |
| 3/18/2012 | 56,325 | 1900.00 | 1,883 | 2,341 | 2,826 | 45 |
| 3/19/2012 | 55,890 | 1899.70 | 1,215 | 1,429 | 1,682 | 45 |
| 3/20/2012 | 55,720 | 1899.60 | 934 | 974 | 1,088 | 45 |
| 3/21/2012 | 55,909 | 1899.70 | 907 | 770 | 933 | 45 |
| 3/22/2012 | 56,915 | 1900.30 | 942 | 414 | 574 | 45 |
| 3/23/2012 | 57,858 | 1900.80 | 696 | 207 | 324 | 45 |
| 3/24/2012 | 58,792 | 1901.20 | 689 | 207 | 324 | 45 |
| 3/25/2012 | 59,875 | 1901.80 | 760 | 209 | 350 | 45 |
| 3/26/2012 | 60,791 | 1902.30 | 684 | 209 | 341 | 45 |
| 3/27/2012 | 62,528 | 1903.20 | 1,136 | 1,242 | 1,151 | 55 |
| 3/28/2012 | 63,161 | 1903.50 | 6,403 | 6,431 | 7,819 | 60 |
| 3/29/2012 | 60,731 | 1902.20 | 2,459 | 3,811 | 4,640 | 51 |
| 3/30/2012 | 60,571 | 1902.20 | 2,890 | 3,052 | 3,803 | 45 |
| 3/31/2012 | 61,959 | 1902.90 | 4,085 | 3,563 | 4,386 | 45 |
| 4/1/2012 | 61,899 | 1902.80 | 3,524 | 3,692 | 4,677 | 45 |
| 4/2/2012 | 62,406 | 1903.10 | 2,322 | 2,005 | 2,689 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/3/2012 | 63,881 | 1903.80 | 1,777 | 938 | 1,301 | 44 |
| 4/4/2012 | 64,710 | 1904.20 | 1,459 | 988 | 1,066 | 44 |
| 4/5/2012 | 64,898 | 1904.30 | 1,226 | 1,065 | 1,224 | 52 |
| 4/6/2012 | 64,835 | 1904.30 | 1,032 | 999 | 1,052 | 63 |
| 4/7/2012 | 64,752 | 1904.30 | 902 | 892 | 959 | 66 |
| 4/8/2012 | 64,898 | 1904.30 | 780 | 669 | 738 | 55 |
| 4/9/2012 | 65,106 | 1904.40 | 662 | 544 | 573 | 45 |
| 4/10/2012 | 65,546 | 1904.60 | 714 | 577 | 632 | 45 |
| 4/11/2012 | 66,366 | 1905.00 | 1,037 | 518 | 609 | 45 |
| 4/12/2012 | 67,130 | 1905.40 | 1,764 | 1,396 | 1,508 | 45 |
| 4/13/2012 | 67,321 | 1905.50 | 1,931 | 1,797 | 2,110 | 45 |
| 4/14/2012 | 67,321 | 1905.50 | 1,507 | 1,455 | 1,779 | 57 |
| 4/15/2012 | 67,620 | 1905.70 | 1,304 | 1,087 | 1,204 | 90 |
| 4/16/2012 | 67,941 | 1905.80 | 1,209 | 977 | 1,078 | 90 |
| 4/17/2012 | 68,219 | 1905.90 | 1,120 | 918 | 965 | 90 |
| 4/18/2012 | 68,606 | 1906.10 | 1,069 | 802 | 859 | 90 |
| 4/19/2012 | 69,059 | 1906.30 | 1,016 | 737 | 751 | 90 |
| 4/20/2012 | 69,623 | 1906.60 | 1,108 | 748 | 741 | 89 |
| 4/21/2012 | 70,189 | 1906.90 | 1,110 | 763 | 739 | 91 |
| 4/22/2012 | 70,342 | 1907.00 | 1,099 | 957 | 859 | 91 |
| 4/23/2012 | 70,211 | 1906.90 | 1,040 | 1,059 | 987 | 91 |
| 4/24/2012 | 69,863 | 1906.70 | 922 | 1,050 | 969 | 91 |
| 4/25/2012 | 69,667 | 1906.60 | 833 | 881 | 854 | 91 |
| 4/26/2012 | 69,754 | 1906.70 | 829 | 730 | 680 | 91 |
| 4/27/2012 | 69,580 | 1906.60 | 690 | 728 | 659 | 91 |
| 4/28/2012 | 69,471 | 1906.50 | 589 | 618 | 585 | 90 |
| 4/29/2012 | 69,493 | 1906.50 | 527 | 521 | 458 | 90 |
| 4/30/2012 | 69,558 | 1906.60 | 491 | 470 | 437 | 90 |
| 5/1/2012 | 69,732 | 1906.70 | 429 | 375 | 346 | 91 |
| 5/2/2012 | 69,798 | 1906.70 | 376 | 374 | 340 | 91 |
| 5/3/2012 | 69,928 | 1906.80 | 407 | 367 | 340 | 91 |
| 5/4/2012 | 70,037 | 1906.80 | 384 | 340 | 316 | 91 |
| 5/5/2012 | 70,059 | 1906.80 | 330 | 321 | 285 | 91 |
| 5/6/2012 | 70,059 | 1906.80 | 300 | 321 | 279 | 91 |
| 5/7/2012 | 70,037 | 1906.80 | 294 | 322 | 275 | 91 |
| 5/8/2012 | 69,972 | 1906.80 | 275 | 322 | 273 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/9/2012 | 69,907 | 1906.70 | 270 | 322 | 271 | 90 |
| 5/10/2012 | 69,819 | 1906.70 | 262 | 322 | 269 | 90 |
| 5/11/2012 | 69,689 | 1906.60 | 240 | 322 | 264 | 90 |
| 5/12/2012 | 69,536 | 1906.60 | 228 | 321 | 262 | 90 |
| 5/13/2012 | 69,384 | 1906.50 | 221 | 321 | 260 | 90 |
| 5/14/2012 | 69,146 | 1906.40 | 210 | 349 | 260 | 108 |
| 5/15/2012 | 68,887 | 1906.30 | 202 | 362 | 258 | 130 |
| 5/16/2012 | 68,585 | 1906.10 | 192 | 363 | 256 | 90 |
| 5/17/2012 | 68,262 | 1906.00 | 113 | 352 | 250 | 131 |
| 5/18/2012 | 67,449 | 1905.60 | 163 | 644 | 394 | 131 |
| 5/19/2012 | 66,324 | 1905.00 | 164 | 808 | 687 | 129 |
| 5/20/2012 | 65,525 | 1904.60 | 160 | 610 | 479 | 129 |
| 5/21/2012 | 64,918 | 1904.30 | 144 | 492 | 378 | 129 |
| 5/22/2012 | 64,441 | 1904.10 | 126 | 405 | 303 | 129 |
| 5/23/2012 | 63,964 | 1903.90 | 88 | 365 | 261 | 130 |
| 5/24/2012 | 63,552 | 1903.70 | 91 | 318 | 225 | 130 |
| 5/25/2012 | 63,285 | 1903.50 | 106 | 270 | 192 | 130 |
| 5/26/2012 | 63,100 | 1903.40 | 132 | 237 | 152 | 130 |
| 5/27/2012 | 62,854 | 1903.30 | 106 | 237 | 150 | 130 |
| 5/28/2012 | 62,650 | 1903.20 | 95 | 225 | 140 | 130 |
| 5/29/2012 | 62,426 | 1903.10 | 93 | 216 | 122 | 127 |
| 5/30/2012 | 62,182 | 1903.00 | 84 | 232 | 134 | 130 |
| 5/31/2012 | 61,919 | 1902.80 | 82 | 232 | 135 | 130 |
| 6/1/2012 | 61,636 | 1902.70 | 70 | 228 | 129 | 130 |
| 6/2/2012 | 61,354 | 1902.60 | 63 | 228 | 127 | 130 |
| 6/3/2012 | 61,052 | 1902.40 | 79 | 228 | 126 | 130 |
| 6/4/2012 | 60,791 | 1902.30 | 58 | 226 | 127 | 130 |
| 6/5/2012 | 60,531 | 1902.10 | 59 | 220 | 120 | 130 |
| 6/6/2012 | 60,272 | 1902.00 | 69 | 217 | 114 | 130 |
| 6/7/2012 | 59,994 | 1901.90 | 60 | 217 | 112 | 130 |
| 6/8/2012 | 59,697 | 1901.70 | 57 | 217 | 109 | 130 |
| 6/9/2012 | 59,302 | 1901.50 | 54 | 266 | 157 | 132 |
| 6/10/2012 | 58,948 | 1901.30 | 46 | 247 | 125 | 136 |
| 6/11/2012 | 58,655 | 1901.20 | 46 | 215 | 112 | 130 |
| 6/12/2012 | 58,343 | 1901.00 | 33 | 211 | 99 | 130 |
| 6/13/2012 | 58,051 | 1900.90 | 38 | 204 | 94 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/14/2012 | 57,742 | 1900.70 | 30 | 194 | 83 | 130 |
| 6/15/2012 | 57,472 | 1900.60 | 40 | 185 | 72 | 130 |
| 6/16/2012 | 57,222 | 1900.40 | 47 | 182 | 68 | 130 |
| 6/17/2012 | 56,954 | 1900.30 | 27 | 178 | 65 | 129 |
| 6/18/2012 | 56,686 | 1900.10 | 20 | 171 | 59 | 129 |
| 6/19/2012 | 56,420 | 1900.00 | 15 | 166 | 53 | 129 |
| 6/20/2012 | 56,136 | 1899.90 | 15 | 163 | 49 | 129 |
| 6/21/2012 | 55,871 | 1899.70 | 22 | 163 | 50 | 129 |
| 6/22/2012 | 55,626 | 1899.60 | 31 | 163 | 52 | 130 |
| 6/23/2012 | 55,382 | 1899.40 | 29 | 160 | 51 | 130 |
| 6/24/2012 | 55,120 | 1899.30 | 16 | 157 | 45 | 130 |
| 6/25/2012 | 54,877 | 1899.20 | 14 | 154 | 39 | 130 |
| 6/26/2012 | 54,635 | 1899.00 | 17 | 149 | 34 | 130 |
| 6/27/2012 | 54,394 | 1898.90 | 15 | 148 | 28 | 130 |
| 6/28/2012 | 54,154 | 1898.80 | 12 | 148 | 27 | 129 |
| 6/29/2012 | 53,914 | 1898.60 | 19 | 147 | 27 | 129 |
| 6/30/2012 | 53,693 | 1898.50 | 28 | 147 | 27 | 129 |
| 7/1/2012 | 53,435 | 1898.40 | 12 | 147 | 27 | 129 |
| 7/2/2012 | 53,179 | 1898.20 | 2 | 147 | 26 | 129 |
| 7/3/2012 | 52,923 | 1898.10 | 7 | 147 | 26 | 130 |
| 7/4/2012 | 52,668 | 1898.00 | 3 | 147 | 25 | 130 |
| 7/5/2012 | 52,396 | 1897.80 | -2 | 147 | 25 | 130 |
| 7/6/2012 | 52,143 | 1897.70 | -2 | 146 | 25 | 130 |
| 7/7/2012 | 51,872 | 1897.50 | 0 | 146 | 25 | 130 |
| 7/8/2012 | 51,602 | 1897.40 | -5 | 146 | 24 | 130 |
| 7/9/2012 | 51,315 | 1897.20 | -6 | 146 | 24 | 129 |
| 7/10/2012 | 51,046 | 1897.10 | -3 | 146 | 25 | 129 |
| 7/11/2012 | 50,779 | 1896.90 | 4 | 146 | 25 | 128 |
| 7/12/2012 | 50,460 | 1896.70 | -5 | 146 | 24 | 129 |
| 7/13/2012 | 50,230 | 1896.60 | 0 | 146 | 23 | 130 |
| 7/14/2012 | 49,947 | 1896.40 | -17 | 146 | 23 | 130 |
| 7/15/2012 | 49,682 | 1896.30 | -8 | 145 | 23 | 130 |
| 7/16/2012 | 49,384 | 1896.10 | -15 | 145 | 23 | 130 |
| 7/17/2012 | 49,086 | 1895.90 | -12 | 145 | 23 | 130 |
| 7/18/2012 | 48,825 | 1895.80 | -6 | 145 | 23 | 130 |
| 7/19/2012 | 48,530 | 1895.60 | -10 | 145 | 23 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/20/2012 | 48,271 | 1895.50 | -10 | 145 | 22 | 130 |
| 7/21/2012 | 47,977 | 1895.30 | -8 | 145 | 22 | 130 |
| 7/22/2012 | 47,701 | 1895.10 | -11 | 146 | 22 | 129 |
| 7/23/2012 | 47,461 | 1895.00 | 11 | 147 | 23 | 129 |
| 7/24/2012 | 47,154 | 1894.80 | -11 | 147 | 24 | 129 |
| 7/25/2012 | 46,882 | 1894.70 | -12 | 147 | 23 | 129 |
| 7/26/2012 | 46,577 | 1894.50 | -14 | 148 | 22 | 129 |
| 7/27/2012 | 46,272 | 1894.30 | -13 | 148 | 22 | 130 |
| 7/28/2012 | 45,986 | 1894.10 | -5 | 147 | 22 | 130 |
| 7/29/2012 | 45,684 | 1894.00 | -19 | 147 | 22 | 130 |
| 7/30/2012 | 45,384 | 1893.80 | -13 | 149 | 21 | 131 |
| 7/31/2012 | 45,085 | 1893.60 | 10 | 153 | 26 | 129 |
| 8/1/2012 | 44,786 | 1893.40 | -40 | 153 | 26 | 129 |
| 8/2/2012 | 44,472 | 1893.20 | -16 | 153 | 25 | 130 |
| 8/3/2012 | 44,176 | 1893.10 | -11 | 153 | 24 | 131 |
| 8/4/2012 | 43,865 | 1892.90 | -15 | 152 | 24 | 131 |
| 8/5/2012 | 43,555 | 1892.70 | -14 | 152 | 24 | 130 |
| 8/6/2012 | 43,246 | 1892.50 | -16 | 152 | 24 | 131 |
| 8/7/2012 | 42,938 | 1892.30 | -17 | 151 | 23 | 131 |
| 8/8/2012 | 42,631 | 1892.10 | -26 | 149 | 23 | 131 |
| 8/9/2012 | 42,326 | 1891.90 | -22 | 148 | 23 | 131 |
| 8/10/2012 | 42,023 | 1891.70 | -22 | 148 | 23 | 131 |
| 8/11/2012 | 41,705 | 1891.50 | -24 | 148 | 23 | 130 |
| 8/12/2012 | 41,403 | 1891.30 | -22 | 148 | 23 | 130 |
| 8/13/2012 | 41,086 | 1891.10 | -22 | 148 | 23 | 130 |
| 8/14/2012 | 40,787 | 1891.00 | -21 | 147 | 22 | 131 |
| 8/15/2012 | 40,475 | 1890.80 | -23 | 147 | 21 | 132 |
| 8/16/2012 | 40,163 | 1890.60 | -23 | 147 | 21 | 132 |
| 8/17/2012 | 39,852 | 1890.40 | -26 | 147 | 21 | 132 |
| 8/18/2012 | 39,543 | 1890.20 | -27 | 146 | 21 | 132 |
| 8/19/2012 | 39,235 | 1890.00 | -24 | 146 | 21 | 131 |
| 8/20/2012 | 38,929 | 1889.80 | -24 | 146 | 21 | 131 |
| 8/21/2012 | 38,625 | 1889.60 | -23 | 146 | 21 | 130 |
| 8/22/2012 | 38,276 | 1889.30 | -26 | 161 | 28 | 125 |
| 8/23/2012 | 37,883 | 1889.10 | -24 | 195 | 70 | 134 |
| 8/24/2012 | 37,584 | 1888.90 | -19 | 145 | 33 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/25/2012 | 37,286 | 1888.70 | -20 | 145 | 19 | 130 |
| 8/26/2012 | 36,973 | 1888.50 | -25 | 145 | 19 | 131 |
| 8/27/2012 | 36,677 | 1888.30 | -19 | 146 | 19 | 131 |
| 8/28/2012 | 36,383 | 1888.10 | -17 | 146 | 19 | 130 |
| 8/29/2012 | 36,090 | 1887.90 | -20 | 146 | 19 | 130 |
| 8/30/2012 | 35,784 | 1887.70 | -22 | 145 | 19 | 129 |
| 8/31/2012 | 35,480 | 1887.50 | -18 | 146 | 19 | 130 |
| 9/1/2012 | 35,191 | 1887.30 | -12 | 147 | 19 | 130 |
| 9/2/2012 | 34,904 | 1887.10 | -14 | 147 | 19 | 130 |
| 9/3/2012 | 34,604 | 1886.80 | -13 | 147 | 19 | 130 |
| 9/4/2012 | 34,321 | 1886.60 | -12 | 147 | 19 | 130 |
| 9/5/2012 | 34,038 | 1886.40 | -9 | 147 | 19 | 130 |
| 9/6/2012 | 33,743 | 1886.20 | -14 | 147 | 19 | 130 |
| 9/7/2012 | 33,449 | 1886.00 | -10 | 147 | 19 | 130 |
| 9/8/2012 | 33,171 | 1885.80 | -11 | 147 | 19 | 128 |
| 9/9/2012 | 32,868 | 1885.60 | -13 | 147 | 19 | 129 |
| 9/10/2012 | 32,592 | 1885.40 | -10 | 148 | 19 | 130 |
| 9/11/2012 | 32,305 | 1885.20 | -7 | 147 | 19 | 130 |
| 9/12/2012 | 32,020 | 1885.00 | -8 | 148 | 19 | 130 |
| 9/13/2012 | 31,736 | 1884.80 | -1 | 148 | 19 | 130 |
| 9/14/2012 | 31,455 | 1884.60 | -11 | 148 | 19 | 130 |
| 9/15/2012 | 31,174 | 1884.30 | -6 | 148 | 19 | 130 |
| 9/16/2012 | 30,935 | 1884.20 | -8 | 129 | 30 | 105 |
| 9/17/2012 | 30,710 | 1884.00 | -13 | 116 | 21 | 90 |
| 9/18/2012 | 30,487 | 1883.80 | -11 | 116 | 21 | 90 |
| 9/19/2012 | 30,264 | 1883.60 | -12 | 115 | 21 | 90 |
| 9/20/2012 | 30,056 | 1883.50 | -11 | 114 | 20 | 90 |
| 9/21/2012 | 29,849 | 1883.30 | -9 | 112 | 20 | 90 |
| 9/22/2012 | 29,629 | 1883.20 | -9 | 112 | 19 | 90 |
| 9/23/2012 | 29,411 | 1883.00 | -11 | 111 | 19 | 90 |
| 9/24/2012 | 29,220 | 1882.80 | -8 | 111 | 19 | 90 |
| 9/25/2012 | 29,004 | 1882.70 | -8 | 111 | 19 | 90 |
| 9/26/2012 | 28,802 | 1882.50 | -7 | 111 | 19 | 90 |
| 9/27/2012 | 28,601 | 1882.30 | 5 | 111 | 19 | 90 |
| 9/28/2012 | 28,389 | 1882.20 | -1 | 111 | 19 | 90 |
| 9/29/2012 | 28,177 | 1882.00 | -1 | 111 | 19 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/30/2012 | 27,980 | 1881.80 | -7 | 111 | 19 | 90 |
| 10/1/2012 | 27,783 | 1881.70 | -6 | 111 | 19 | 90 |
| 10/2/2012 | 27,563 | 1881.50 | 7 | 118 | 20 | 90 |
| 10/3/2012 | 27,333 | 1881.30 | -26 | 122 | 26 | 90 |
| 10/4/2012 | 27,104 | 1881.10 | -10 | 122 | 27 | 90 |
| 10/5/2012 | 26,876 | 1880.90 | -9 | 122 | 27 | 90 |
| 10/6/2012 | 26,650 | 1880.70 | -7 | 121 | 26 | 90 |
| 10/7/2012 | 26,437 | 1880.60 | -9 | 120 | 26 | 90 |
| 10/8/2012 | 26,203 | 1880.40 | -7 | 123 | 28 | 91 |
| 10/9/2012 | 25,970 | 1880.20 | -7 | 125 | 33 | 91 |
| 10/10/2012 | 25,750 | 1880.00 | -5 | 125 | 33 | 91 |
| 10/11/2012 | 25,521 | 1879.80 | -7 | 125 | 33 | 90 |
| 10/12/2012 | 25,293 | 1879.60 | -6 | 125 | 33 | 91 |
| 10/13/2012 | 25,078 | 1879.40 | -2 | 125 | 33 | 91 |
| 10/14/2012 | 24,854 | 1879.20 | -2 | 125 | 32 | 90 |
| 10/15/2012 | 24,632 | 1879.00 | -3 | 125 | 32 | 91 |
| 10/16/2012 | 24,433 | 1878.80 | -2 | 116 | 42 | 78 |
| 10/17/2012 | 24,246 | 1878.60 | 0 | 108 | 32 | 72 |
| 10/18/2012 | 24,061 | 1878.50 | -1 | 108 | 32 | 72 |
| 10/19/2012 | 23,889 | 1878.30 | 0 | 104 | 38 | 66 |
| 10/20/2012 | 23,707 | 1878.10 | 0 | 100 | 36 | 61 |
| 10/21/2012 | 23,547 | 1878.00 | 4 | 100 | 37 | 60 |
| 10/22/2012 | 23,462 | 1877.90 | 51 | 96 | 52 | 51 |
| 10/23/2012 | 23,366 | 1877.80 | 25 | 93 | 45 | 46 |
| 10/24/2012 | 23,292 | 1877.80 | 49 | 93 | 52 | 46 |
| 10/25/2012 | 23,177 | 1877.60 | 19 | 93 | 44 | 46 |
| 10/26/2012 | 23,041 | 1877.50 | 17 | 93 | 42 | 46 |
| 10/27/2012 | 22,927 | 1877.40 | 20 | 92 | 42 | 46 |
| 10/28/2012 | 22,804 | 1877.30 | 18 | 92 | 42 | 46 |
| 10/29/2012 | 22,671 | 1877.20 | 18 | 92 | 42 | 46 |
| 10/30/2012 | 22,548 | 1877.00 | 26 | 92 | 42 | 46 |
| 10/31/2012 | 22,477 | 1877.00 | 47 | 93 | 48 | 46 |
| 11/1/2012 | 22,365 | 1876.90 | 28 | 93 | 50 | 46 |
| 11/2/2012 | 22,254 | 1876.80 | 26 | 93 | 45 | 46 |
| 11/3/2012 | 22,143 | 1876.70 | 23 | 92 | 45 | 46 |
| 11/4/2012 | 22,023 | 1876.50 | 19 | 92 | 44 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/5/2012 | 21,895 | 1876.40 | 20 | 93 | 45 | 46 |
| 11/6/2012 | 21,688 | 1876.20 | 10 | 134 | 87 | 53 |
| 11/7/2012 | 21,561 | 1876.10 | 17 | 96 | 51 | 46 |
| 11/8/2012 | 21,453 | 1876.00 | 32 | 94 | 49 | 46 |
| 11/9/2012 | 21,327 | 1875.80 | 19 | 93 | 45 | 46 |
| 11/10/2012 | 21,201 | 1875.70 | 15 | 92 | 42 | 46 |
| 11/11/2012 | 21,076 | 1875.60 | 17 | 92 | 41 | 46 |
| 11/12/2012 | 20,952 | 1875.50 | 18 | 92 | 41 | 46 |
| 11/13/2012 | 20,857 | 1875.40 | 31 | 92 | 42 | 46 |
| 11/14/2012 | 20,744 | 1875.30 | 23 | 92 | 41 | 46 |
| 11/15/2012 | 20,622 | 1875.10 | 23 | 92 | 40 | 46 |
| 11/16/2012 | 20,546 | 1875.00 | 41 | 92 | 42 | 46 |
| 11/17/2012 | 20,612 | 1875.10 | 119 | 93 | 60 | 46 |
| 11/18/2012 | 20,820 | 1875.30 | 191 | 92 | 71 | 46 |
| 11/19/2012 | 20,810 | 1875.30 | 91 | 102 | 65 | 47 |
| 11/20/2012 | 22,722 | 1877.20 | 1,008 | 116 | 334 | 47 |
| 11/21/2012 | 24,202 | 1878.60 | 1,015 | 142 | 296 | 47 |
| 11/22/2012 | 24,356 | 1878.70 | 268 | 191 | 201 | 46 |
| 11/23/2012 | 24,246 | 1878.60 | 143 | 209 | 209 | 46 |
| 11/24/2012 | 24,072 | 1878.50 | 100 | 208 | 201 | 47 |
| 11/25/2012 | 23,846 | 1878.30 | 79 | 208 | 197 | 47 |
| 11/26/2012 | 23,568 | 1878.00 | 67 | 229 | 195 | 47 |
| 11/27/2012 | 23,177 | 1877.60 | 68 | 288 | 278 | 50 |
| 11/28/2012 | 23,942 | 1878.40 | 556 | 215 | 264 | 46 |
| 11/29/2012 | 28,513 | 1882.30 | 2,407 | 226 | 642 | 46 |
| 11/30/2012 | 43,767 | 1892.80 | 8,490 | 255 | 1,954 | 46 |
| 12/1/2012 | 49,911 | 1896.40 | 3,531 | 254 | 859 | 46 |
| 12/2/2012 | 61,717 | 1902.70 | 15,574 | 10,101 | 12,102 | 7 |
| 12/3/2012 | 57,626 | 1900.60 | 2,833 | 4,770 | 6,850 | 73 |
| 12/4/2012 | 58,071 | 1900.90 | 2,580 | 2,480 | 3,168 | 150 |
| 12/5/2012 | 58,518 | 1901.10 | 4,429 | 4,425 | 6,043 | 95 |
| 12/6/2012 | 56,973 | 1900.30 | 1,964 | 2,803 | 3,858 | 151 |
| 12/7/2012 | 56,117 | 1899.80 | 1,232 | 1,690 | 2,127 | 151 |
| 12/8/2012 | 55,589 | 1899.60 | 873 | 1,154 | 1,352 | 151 |
| 12/9/2012 | 55,213 | 1899.40 | 673 | 878 | 921 | 170 |
| 12/10/2012 | 54,933 | 1899.20 | 542 | 698 | 632 | 199 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/11/2012 | 54,728 | 1899.10 | 470 | 583 | 470 | 199 |
| 12/12/2012 | 54,561 | 1899.00 | 412 | 508 | 386 | 199 |
| 12/13/2012 | 54,413 | 1898.90 | 342 | 433 | 311 | 200 |
| 12/14/2012 | 54,265 | 1898.80 | 295 | 391 | 262 | 201 |
| 12/15/2012 | 54,228 | 1898.80 | 326 | 373 | 241 | 202 |
| 12/16/2012 | 54,302 | 1898.90 | 399 | 382 | 281 | 202 |
| 12/17/2012 | 56,174 | 1899.90 | 1,866 | 1,143 | 1,389 | 201 |
| 12/18/2012 | 55,626 | 1899.60 | 978 | 1,265 | 1,509 | 200 |
| 12/19/2012 | 55,213 | 1899.40 | 690 | 912 | 976 | 200 |
| 12/20/2012 | 55,195 | 1899.30 | 715 | 749 | 769 | 200 |
| 12/21/2012 | 60,431 | 1902.10 | 7,097 | 4,888 | 6,593 | 82 |
| 12/22/2012 | 59,282 | 1901.50 | 5,139 | 5,925 | 8,404 | 1 |
| 12/23/2012 | 60,411 | 1902.10 | 5,776 | 5,482 | 7,965 | 16 |
| 12/24/2012 | 58,032 | 1900.90 | 3,216 | 4,542 | 6,687 | 24 |
| 12/25/2012 | 57,241 | 1900.40 | 2,124 | 2,664 | 3,797 | 46 |
| 12/26/2012 | 56,992 | 1900.30 | 2,040 | 2,277 | 3,172 | 109 |
| 12/27/2012 | 56,420 | 1900.00 | 1,572 | 1,940 | 2,524 | 153 |
| 12/28/2012 | 56,003 | 1899.80 | 1,204 | 1,489 | 1,825 | 153 |
| 12/29/2012 | 55,702 | 1899.60 | 1,004 | 1,217 | 1,396 | 154 |
| 12/30/2012 | 55,457 | 1899.50 | 851 | 1,027 | 1,107 | 155 |
| 12/31/2012 | 55,251 | 1899.40 | 736 | 882 | 902 | 155 |
| 1/1/2013 | 55,101 | 1899.30 | 647 | 766 | 740 | 155 |
| 1/2/2013 | 54,970 | 1899.20 | 589 | 682 | 623 | 155 |
| 1/3/2013 | 54,877 | 1899.20 | 541 | 624 | 539 | 155 |
| 1/4/2013 | 54,803 | 1899.10 | 508 | 578 | 481 | 155 |
| 1/5/2013 | 54,896 | 1899.20 | 578 | 565 | 473 | 155 |
| 1/6/2013 | 55,101 | 1899.30 | 734 | 676 | 641 | 155 |
| 1/7/2013 | 55,082 | 1899.30 | 679 | 713 | 666 | 155 |
| 1/8/2013 | 55,120 | 1899.30 | 704 | 717 | 654 | 155 |
| 1/9/2013 | 55,157 | 1899.30 | 763 | 749 | 670 | 155 |
| 1/10/2013 | 55,101 | 1899.30 | 730 | 758 | 678 | 155 |
| 1/11/2013 | 55,008 | 1899.20 | 645 | 705 | 615 | 155 |
| 1/12/2013 | 54,914 | 1899.20 | 588 | 651 | 543 | 155 |
| 1/13/2013 | 54,784 | 1899.10 | 531 | 596 | 479 | 156 |
| 1/14/2013 | 54,710 | 1899.10 | 486 | 546 | 428 | 156 |
| 1/15/2013 | 54,635 | 1899.00 | 444 | 497 | 383 | 156 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/16/2013 | 54,580 | 1899.00 | 428 | 465 | 351 | 155 |
| 1/17/2013 | 54,580 | 1899.00 | 425 | 451 | 336 | 155 |
| 1/18/2013 | 54,561 | 1899.00 | 428 | 448 | 330 | 155 |
| 1/19/2013 | 54,561 | 1899.00 | 419 | 444 | 324 | 155 |
| 1/20/2013 | 54,524 | 1899.00 | 402 | 432 | 313 | 155 |
| 1/21/2013 | 54,450 | 1898.90 | 392 | 438 | 310 | 155 |
| 1/22/2013 | 54,394 | 1898.90 | 387 | 429 | 305 | 155 |
| 1/23/2013 | 54,821 | 1899.10 | 559 | 418 | 297 | 155 |
| 1/24/2013 | 55,589 | 1899.60 | 1,243 | 901 | 681 | 155 |
| 1/25/2013 | 55,476 | 1899.50 | 899 | 1,006 | 859 | 170 |
| 1/26/2013 | 55,326 | 1899.40 | 800 | 922 | 727 | 202 |
| 1/27/2013 | 55,157 | 1899.30 | 727 | 815 | 616 | 202 |
| 1/28/2013 | 55,008 | 1899.20 | 613 | 704 | 500 | 201 |
| 1/29/2013 | 54,859 | 1899.20 | 540 | 621 | 418 | 201 |
| 1/30/2013 | 54,747 | 1899.10 | 492 | 560 | 361 | 201 |
| 1/31/2013 | 54,672 | 1899.10 | 453 | 508 | 320 | 201 |
| 2/1/2013 | 54,598 | 1899.00 | 420 | 470 | 288 | 201 |
| 2/2/2013 | 54,524 | 1899.00 | 385 | 437 | 261 | 201 |
| 2/3/2013 | 54,450 | 1898.90 | 367 | 412 | 242 | 201 |
| 2/4/2013 | 54,394 | 1898.90 | 346 | 393 | 228 | 201 |
| 2/5/2013 | 54,339 | 1898.90 | 335 | 377 | 214 | 201 |
| 2/6/2013 | 54,265 | 1898.80 | 312 | 363 | 203 | 201 |
| 2/7/2013 | 54,283 | 1898.80 | 345 | 358 | 203 | 201 |
| 2/8/2013 | 54,209 | 1898.80 | 295 | 351 | 197 | 201 |
| 2/9/2013 | 54,172 | 1898.80 | 278 | 309 | 187 | 176 |
| 2/10/2013 | 54,135 | 1898.80 | 261 | 290 | 182 | 151 |
| 2/11/2013 | 54,117 | 1898.80 | 245 | 271 | 174 | 143 |
| 2/12/2013 | 54,135 | 1898.80 | 234 | 239 | 171 | 115 |
| 2/13/2013 | 54,154 | 1898.80 | 225 | 227 | 166 | 100 |
| 2/14/2013 | 54,154 | 1898.80 | 212 | 228 | 166 | 99 |
| 2/15/2013 | 54,172 | 1898.80 | 221 | 227 | 164 | 99 |
| 2/16/2013 | 54,172 | 1898.80 | 212 | 227 | 163 | 99 |
| 2/17/2013 | 54,172 | 1898.80 | 211 | 227 | 162 | 99 |
| 2/18/2013 | 54,154 | 1898.80 | 203 | 227 | 162 | 99 |
| 2/19/2013 | 54,209 | 1898.80 | 237 | 211 | 195 | 22 |
| 2/20/2013 | 54,265 | 1898.80 | 199 | 195 | 199 | 22 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/21/2013 | 54,265 | 1898.80 | 177 | 187 | 195 | 22 |
| 2/22/2013 | 54,339 | 1898.90 | 212 | 184 | 188 | 22 |
| 2/23/2013 | 54,320 | 1898.90 | 170 | 188 | 192 | 22 |
| 2/24/2013 | 54,339 | 1898.90 | 177 | 183 | 188 | 22 |
| 2/25/2013 | 54,339 | 1898.90 | 172 | 181 | 185 | 22 |
| 2/26/2013 | 54,320 | 1898.90 | 160 | 179 | 183 | 22 |
| 2/27/2013 | 54,320 | 1898.90 | 168 | 177 | 180 | 22 |
| 2/28/2013 | 54,302 | 1898.90 | 152 | 175 | 177 | 22 |
| 3/1/2013 | 54,302 | 1898.90 | 162 | 174 | 177 | 22 |
| 3/2/2013 | 54,320 | 1898.90 | 169 | 176 | 177 | 22 |
| 3/3/2013 | 54,339 | 1898.90 | 171 | 177 | 177 | 22 |
| 3/4/2013 | 54,357 | 1898.90 | 176 | 180 | 180 | 22 |
| 3/5/2013 | 54,431 | 1898.90 | 224 | 210 | 197 | 22 |
| 3/6/2013 | 54,989 | 1899.20 | 647 | 365 | 384 | 22 |
| 3/7/2013 | 54,877 | 1899.20 | 418 | 460 | 486 | 22 |
| 3/8/2013 | 54,710 | 1899.10 | 300 | 379 | 396 | 22 |
| 3/9/2013 | 54,617 | 1899.00 | 266 | 321 | 322 | 22 |
| 3/10/2013 | 54,561 | 1899.00 | 255 | 288 | 280 | 46 |
| 3/11/2013 | 54,542 | 1899.00 | 248 | 271 | 260 | 46 |
| 3/12/2013 | 54,542 | 1899.00 | 280 | 272 | 255 | 45 |
| 3/13/2013 | 54,580 | 1899.00 | 284 | 279 | 255 | 46 |
| 3/14/2013 | 54,561 | 1899.00 | 272 | 280 | 255 | 46 |
| 3/15/2013 | 54,524 | 1899.00 | 259 | 275 | 251 | 46 |
| 3/16/2013 | 54,505 | 1899.00 | 253 | 269 | 245 | 46 |
| 3/17/2013 | 54,487 | 1899.00 | 240 | 256 | 235 | 46 |
| 3/18/2013 | 54,450 | 1898.90 | 225 | 246 | 227 | 46 |
| 3/19/2013 | 54,487 | 1899.00 | 235 | 225 | 214 | 46 |
| 3/20/2013 | 55,064 | 1899.30 | 518 | 227 | 232 | 59 |
| 3/21/2013 | 55,420 | 1899.50 | 427 | 223 | 215 | 66 |
| 3/22/2013 | 55,626 | 1899.60 | 325 | 219 | 201 | 66 |
| 3/23/2013 | 55,777 | 1899.70 | 283 | 213 | 199 | 60 |
| 3/24/2013 | 55,871 | 1899.70 | 258 | 209 | 195 | 56 |
| 3/25/2013 | 55,947 | 1899.70 | 247 | 210 | 193 | 55 |
| 3/26/2013 | 55,984 | 1899.80 | 226 | 209 | 191 | 55 |
| 3/27/2013 | 56,060 | 1899.80 | 227 | 199 | 188 | 52 |
| 3/28/2013 | 56,136 | 1899.90 | 217 | 188 | 179 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/29/2013 | 56,192 | 1899.90 | 207 | 187 | 177 | 45 |
| 3/30/2013 | 56,287 | 1899.90 | 226 | 188 | 177 | 45 |
| 3/31/2013 | 56,534 | 1900.10 | 296 | 180 | 175 | 45 |
| 4/1/2013 | 56,725 | 1900.20 | 296 | 202 | 177 | 45 |
| 4/2/2013 | 56,744 | 1900.20 | 242 | 227 | 209 | 45 |
| 4/3/2013 | 56,801 | 1900.20 | 239 | 223 | 203 | 45 |
| 4/4/2013 | 57,587 | 1900.60 | 626 | 225 | 237 | 45 |
| 4/5/2013 | 58,382 | 1901.00 | 643 | 226 | 246 | 45 |
| 4/6/2013 | 58,870 | 1901.30 | 485 | 227 | 235 | 45 |
| 4/7/2013 | 59,223 | 1901.50 | 420 | 228 | 230 | 45 |
| 4/8/2013 | 59,499 | 1901.60 | 396 | 247 | 246 | 50 |
| 4/9/2013 | 59,677 | 1901.70 | 335 | 257 | 238 | 60 |
| 4/10/2013 | 59,756 | 1901.70 | 307 | 258 | 236 | 60 |
| 4/11/2013 | 59,835 | 1901.80 | 282 | 251 | 228 | 60 |
| 4/12/2013 | 59,855 | 1901.80 | 266 | 253 | 226 | 60 |
| 4/13/2013 | 59,835 | 1901.80 | 242 | 252 | 224 | 60 |
| 4/14/2013 | 59,776 | 1901.80 | 225 | 265 | 224 | 60 |
| 4/15/2013 | 59,637 | 1901.70 | 208 | 294 | 236 | 81 |
| 4/16/2013 | 59,479 | 1901.60 | 211 | 289 | 224 | 90 |
| 4/17/2013 | 59,341 | 1901.50 | 209 | 286 | 220 | 89 |
| 4/18/2013 | 59,145 | 1901.40 | 175 | 288 | 217 | 90 |
| 4/19/2013 | 58,909 | 1901.30 | 157 | 290 | 219 | 91 |
| 4/20/2013 | 58,713 | 1901.20 | 165 | 278 | 210 | 91 |
| 4/21/2013 | 58,518 | 1901.10 | 137 | 264 | 198 | 91 |
| 4/22/2013 | 58,362 | 1901.00 | 133 | 253 | 187 | 90 |
| 4/23/2013 | 58,168 | 1900.90 | 116 | 243 | 174 | 90 |
| 4/24/2013 | 58,013 | 1900.80 | 117 | 236 | 167 | 90 |
| 4/25/2013 | 57,838 | 1900.80 | 109 | 237 | 159 | 90 |
| 4/26/2013 | 57,645 | 1900.70 | 108 | 243 | 166 | 90 |
| 4/27/2013 | 57,472 | 1900.60 | 105 | 237 | 158 | 90 |
| 4/28/2013 | 57,279 | 1900.50 | 108 | 230 | 148 | 90 |
| 4/29/2013 | 57,107 | 1900.40 | 104 | 227 | 142 | 89 |
| 4/30/2013 | 56,915 | 1900.30 | 112 | 220 | 135 | 89 |
| 5/1/2013 | 56,725 | 1900.20 | 71 | 215 | 124 | 89 |
| 5/2/2013 | 56,515 | 1900.10 | 83 | 215 | 121 | 90 |
| 5/3/2013 | 56,344 | 1900.00 | 118 | 215 | 120 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/4/2013 | 56,098 | 1899.80 | 82 | 217 | 120 | 89 |
| 5/5/2013 | 55,890 | 1899.70 | 75 | 218 | 121 | 90 |
| 5/6/2013 | 55,777 | 1899.70 | 126 | 218 | 126 | 91 |
| 5/7/2013 | 55,626 | 1899.60 | 96 | 212 | 120 | 91 |
| 5/8/2013 | 55,438 | 1899.50 | 89 | 213 | 118 | 89 |
| 5/9/2013 | 55,139 | 1899.30 | 87 | 270 | 160 | 95 |
| 5/10/2013 | 54,933 | 1899.20 | 75 | 224 | 130 | 91 |
| 5/11/2013 | 54,710 | 1899.10 | 71 | 218 | 122 | 90 |
| 5/12/2013 | 54,450 | 1898.90 | 57 | 216 | 119 | 90 |
| 5/13/2013 | 54,209 | 1898.80 | 54 | 217 | 122 | 90 |
| 5/14/2013 | 53,895 | 1898.60 | 75 | 246 | 124 | 105 |
| 5/15/2013 | 53,693 | 1898.50 | 131 | 264 | 122 | 131 |
| 5/16/2013 | 53,325 | 1898.30 | 44 | 260 | 124 | 131 |
| 5/17/2013 | 52,978 | 1898.10 | 39 | 255 | 117 | 132 |
| 5/18/2013 | 52,650 | 1898.00 | 36 | 240 | 109 | 131 |
| 5/19/2013 | 52,342 | 1897.80 | 35 | 227 | 97 | 129 |
| 5/20/2013 | 52,035 | 1897.60 | 29 | 223 | 94 | 129 |
| 5/21/2013 | 51,674 | 1897.40 | 10 | 222 | 93 | 129 |
| 5/22/2013 | 51,351 | 1897.20 | 7 | 216 | 89 | 129 |
| 5/23/2013 | 51,046 | 1897.10 | 22 | 214 | 85 | 129 |
| 5/24/2013 | 50,761 | 1896.90 | 22 | 207 | 82 | 129 |
| 5/25/2013 | 50,477 | 1896.70 | 19 | 198 | 75 | 129 |
| 5/26/2013 | 50,212 | 1896.60 | 18 | 193 | 69 | 129 |
| 5/27/2013 | 49,982 | 1896.50 | 29 | 187 | 68 | 129 |
| 5/28/2013 | 49,753 | 1896.30 | 24 | 183 | 66 | 129 |
| 5/29/2013 | 49,489 | 1896.20 | 15 | 183 | 65 | 128 |
| 5/30/2013 | 49,226 | 1896.00 | 14 | 184 | 64 | 129 |
| 5/31/2013 | 48,947 | 1895.90 | 6 | 186 | 64 | 129 |
| 6/1/2013 | 48,704 | 1895.70 | 33 | 172 | 76 | 107 |
| 6/2/2013 | 48,496 | 1895.60 | 39 | 155 | 68 | 96 |
| 6/3/2013 | 48,288 | 1895.50 | 30 | 147 | 55 | 96 |
| 6/4/2013 | 48,080 | 1895.40 | 25 | 145 | 46 | 96 |
| 6/5/2013 | 47,891 | 1895.30 | 29 | 143 | 47 | 95 |
| 6/6/2013 | 47,684 | 1895.10 | 23 | 142 | 44 | 95 |
| 6/7/2013 | 47,478 | 1895.00 | 25 | 142 | 44 | 94 |
| 6/8/2013 | 47,256 | 1894.90 | 20 | 142 | 44 | 94 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/9/2013 | 47,052 | 1894.80 | 17 | 142 | 45 | 94 |
| 6/10/2013 | 46,814 | 1894.60 | 18 | 146 | 46 | 94 |
| 6/11/2013 | 46,594 | 1894.50 | 20 | 143 | 48 | 94 |
| 6/12/2013 | 46,357 | 1894.40 | 7 | 139 | 44 | 94 |
| 6/13/2013 | 46,137 | 1894.20 | 12 | 137 | 37 | 94 |
| 6/14/2013 | 45,919 | 1894.10 | 16 | 142 | 35 | 94 |
| 6/15/2013 | 45,701 | 1894.00 | 22 | 149 | 38 | 95 |
| 6/16/2013 | 45,480 | 1893.80 | 20 | 148 | 33 | 95 |
| 6/17/2013 | 45,270 | 1893.70 | 25 | 150 | 33 | 95 |
| 6/18/2013 | 45,052 | 1893.60 | 13 | 132 | 31 | 95 |
| 6/19/2013 | 44,819 | 1893.40 | -5 | 119 | 29 | 95 |
| 6/20/2013 | 44,604 | 1893.30 | 3 | 119 | 29 | 95 |
| 6/21/2013 | 44,406 | 1893.20 | 4 | 116 | 28 | 95 |
| 6/22/2013 | 44,209 | 1893.10 | 5 | 114 | 25 | 95 |
| 6/23/2013 | 44,012 | 1893.00 | 9 | 116 | 25 | 95 |
| 6/24/2013 | 43,832 | 1892.80 | 27 | 120 | 31 | 95 |
| 6/25/2013 | 43,946 | 1892.90 | 171 | 119 | 49 | 95 |
| 6/26/2013 | 43,881 | 1892.90 | 75 | 118 | 39 | 95 |
| 6/27/2013 | 43,750 | 1892.80 | 46 | 118 | 31 | 94 |
| 6/28/2013 | 43,571 | 1892.70 | 26 | 119 | 29 | 94 |
| 6/29/2013 | 43,393 | 1892.60 | 20 | 119 | 27 | 95 |
| 6/30/2013 | 43,181 | 1892.50 | 9 | 120 | 27 | 94 |
| 7/1/2013 | 43,003 | 1892.30 | 14 | 121 | 27 | 94 |
| 7/2/2013 | 42,793 | 1892.20 | 8 | 121 | 26 | 94 |
| 7/3/2013 | 42,567 | 1892.10 | 0 | 122 | 26 | 94 |
| 7/4/2013 | 42,358 | 1891.90 | 1 | 118 | 25 | 95 |
| 7/5/2013 | 42,166 | 1891.80 | 0 | 118 | 23 | 95 |
| 7/6/2013 | 41,943 | 1891.70 | -3 | 118 | 23 | 95 |
| 7/7/2013 | 41,721 | 1891.50 | 0 | 118 | 22 | 95 |
| 7/8/2013 | 41,514 | 1891.40 | 0 | 118 | 22 | 95 |
| 7/9/2013 | 41,292 | 1891.30 | -10 | 118 | 22 | 95 |
| 7/10/2013 | 41,071 | 1891.10 | -5 | 119 | 22 | 95 |
| 7/11/2013 | 40,835 | 1891.00 | -7 | 118 | 22 | 95 |
| 7/12/2013 | 40,599 | 1890.80 | -10 | 120 | 22 | 95 |
| 7/13/2013 | 40,366 | 1890.70 | -7 | 123 | 25 | 95 |
| 7/14/2013 | 40,132 | 1890.50 | -9 | 120 | 24 | 95 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/15/2013 | 39,899 | 1890.40 | -12 | 119 | 24 | 95 |
| 7/16/2013 | 39,666 | 1890.20 | -12 | 120 | 24 | 95 |
| 7/17/2013 | 39,435 | 1890.10 | -10 | 121 | 24 | 95 |
| 7/18/2013 | 39,199 | 1889.90 | -10 | 122 | 24 | 95 |
| 7/19/2013 | 38,970 | 1889.80 | -6 | 123 | 23 | 95 |
| 7/20/2013 | 38,736 | 1889.60 | -6 | 123 | 23 | 95 |
| 7/21/2013 | 38,503 | 1889.50 | -8 | 123 | 23 | 95 |
| 7/22/2013 | 38,270 | 1889.30 | -9 | 123 | 23 | 94 |
| 7/23/2013 | 38,034 | 1889.20 | -9 | 124 | 23 | 94 |
| 7/24/2013 | 37,793 | 1889.00 | -10 | 125 | 23 | 94 |
| 7/25/2013 | 37,548 | 1888.90 | -13 | 123 | 23 | 94 |
| 7/26/2013 | 37,305 | 1888.70 | -13 | 126 | 22 | 94 |
| 7/27/2013 | 37,061 | 1888.50 | -13 | 124 | 22 | 94 |
| 7/28/2013 | 36,817 | 1888.40 | -13 | 126 | 21 | 95 |
| 7/29/2013 | 36,573 | 1888.20 | -13 | 126 | 21 | 96 |
| 7/30/2013 | 36,333 | 1888.00 | -13 | 121 | 21 | 96 |
| 7/31/2013 | 36,094 | 1887.90 | -17 | 119 | 21 | 96 |
| 8/1/2013 | 35,861 | 1887.70 | -15 | 118 | 21 | 96 |
| 8/2/2013 | 35,625 | 1887.60 | -16 | 118 | 21 | 96 |
| 8/3/2013 | 35,393 | 1887.40 | -12 | 117 | 21 | 96 |
| 8/4/2013 | 35,162 | 1887.20 | -14 | 117 | 21 | 96 |
| 8/5/2013 | 34,932 | 1887.10 | -12 | 117 | 21 | 96 |
| 8/6/2013 | 34,704 | 1886.90 | -10 | 118 | 20 | 96 |
| 8/7/2013 | 34,462 | 1886.70 | -12 | 120 | 20 | 96 |
| 8/8/2013 | 34,222 | 1886.60 | -9 | 120 | 21 | 96 |
| 8/9/2013 | 33,996 | 1886.40 | -10 | 119 | 21 | 96 |
| 8/10/2013 | 33,757 | 1886.20 | -12 | 118 | 20 | 95 |
| 8/11/2013 | 33,533 | 1886.10 | -8 | 116 | 20 | 95 |
| 8/12/2013 | 33,296 | 1885.90 | -8 | 118 | 20 | 95 |
| 8/13/2013 | 33,074 | 1885.80 | -8 | 117 | 20 | 95 |
| 8/14/2013 | 32,840 | 1885.60 | -8 | 117 | 20 | 95 |
| 8/15/2013 | 32,606 | 1885.40 | -8 | 114 | 20 | 95 |
| 8/16/2013 | 32,373 | 1885.20 | -6 | 115 | 20 | 95 |
| 8/17/2013 | 32,155 | 1885.10 | -7 | 116 | 20 | 95 |
| 8/18/2013 | 31,925 | 1884.90 | -2 | 118 | 20 | 95 |
| 8/19/2013 | 31,710 | 1884.70 | -1 | 110 | 20 | 95 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/20/2013 | 31,484 | 1884.60 | -1 | 115 | 20 | 95 |
| 8/21/2013 | 31,254 | 1884.40 | -2 | 116 | 19 | 95 |
| 8/22/2013 | 31,028 | 1884.20 | -4 | 116 | 19 | 95 |
| 8/23/2013 | 30,789 | 1884.10 | -6 | 115 | 19 | 95 |
| 8/24/2013 | 30,558 | 1883.90 | -3 | 115 | 19 | 95 |
| 8/25/2013 | 30,330 | 1883.70 | -3 | 113 | 19 | 95 |
| 8/26/2013 | 30,098 | 1883.50 | -3 | 116 | 19 | 95 |
| 8/27/2013 | 29,875 | 1883.30 | -8 | 114 | 19 | 95 |
| 8/28/2013 | 29,655 | 1883.20 | -11 | 114 | 19 | 95 |
| 8/29/2013 | 29,424 | 1883.00 | -9 | 114 | 19 | 95 |
| 8/30/2013 | 29,201 | 1882.80 | -11 | 114 | 19 | 95 |
| 8/31/2013 | 28,966 | 1882.60 | -12 | 117 | 19 | 95 |
| 9/1/2013 | 28,739 | 1882.50 | -10 | 118 | 20 | 95 |
| 9/2/2013 | 28,501 | 1882.30 | -12 | 118 | 20 | 95 |
| 9/3/2013 | 28,265 | 1882.10 | -11 | 118 | 20 | 95 |
| 9/4/2013 | 28,029 | 1881.90 | -13 | 118 | 20 | 95 |
| 9/5/2013 | 27,801 | 1881.70 | -12 | 117 | 20 | 95 |
| 9/6/2013 | 27,575 | 1881.50 | -10 | 117 | 20 | 95 |
| 9/7/2013 | 27,345 | 1881.30 | -9 | 117 | 20 | 95 |
| 9/8/2013 | 27,116 | 1881.10 | -7 | 118 | 20 | 95 |
| 9/9/2013 | 26,888 | 1880.90 | -9 | 118 | 20 | 95 |
| 9/10/2013 | 26,662 | 1880.70 | -9 | 118 | 20 | 95 |
| 9/11/2013 | 26,432 | 1880.60 | -8 | 118 | 20 | 95 |
| 9/12/2013 | 26,215 | 1880.40 | -6 | 118 | 20 | 95 |
| 9/13/2013 | 25,993 | 1880.20 | -6 | 118 | 20 | 95 |
| 9/14/2013 | 25,762 | 1880.00 | -8 | 118 | 20 | 95 |
| 9/15/2013 | 25,532 | 1879.80 | -7 | 118 | 20 | 95 |
| 9/16/2013 | 25,315 | 1879.60 | -7 | 118 | 20 | 92 |
| 9/17/2013 | 25,078 | 1879.40 | -14 | 113 | 21 | 90 |
| 9/18/2013 | 24,866 | 1879.20 | -5 | 108 | 20 | 90 |
| 9/19/2013 | 24,654 | 1879.00 | -5 | 110 | 19 | 90 |
| 9/20/2013 | 24,460 | 1878.80 | 2 | 111 | 20 | 90 |
| 9/21/2013 | 24,284 | 1878.70 | 17 | 111 | 24 | 91 |
| 9/22/2013 | 24,072 | 1878.50 | -2 | 112 | 20 | 91 |
| 9/23/2013 | 23,867 | 1878.30 | 1 | 111 | 20 | 91 |
| 9/24/2013 | 23,681 | 1878.10 | 58 | 111 | 20 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/25/2013 | 23,494 | 1877.90 | -60 | 111 | 20 | 91 |
| 9/26/2013 | 23,239 | 1877.70 | -4 | 112 | 20 | 91 |
| 9/27/2013 | 23,036 | 1877.50 | 2 | 112 | 20 | 91 |
| 9/28/2013 | 22,835 | 1877.30 | 5 | 113 | 20 | 91 |
| 9/29/2013 | 22,680 | 1877.20 | 26 | 114 | 21 | 91 |
| 9/30/2013 | 22,507 | 1877.00 | 27 | 115 | 29 | 91 |
| 10/1/2013 | 22,334 | 1876.80 | 19 | 115 | 20 | 91 |
| 10/2/2013 | 22,128 | 1876.60 | 14 | 114 | 20 | 91 |
| 10/3/2013 | 21,939 | 1876.50 | 6 | 113 | 20 | 91 |
| 10/4/2013 | 21,717 | 1876.20 | -9 | 116 | 20 | 91 |
| 10/5/2013 | 21,521 | 1876.00 | 7 | 119 | 23 | 91 |
| 10/6/2013 | 21,317 | 1875.80 | 4 | 119 | 24 | 91 |
| 10/7/2013 | 21,105 | 1875.60 | 4 | 123 | 27 | 91 |
| 10/8/2013 | 20,891 | 1875.40 | 5 | 123 | 30 | 90 |
| 10/9/2013 | 20,683 | 1875.20 | -1 | 119 | 28 | 90 |
| 10/10/2013 | 20,490 | 1875.00 | 2 | 117 | 24 | 89 |
| 10/11/2013 | 20,285 | 1874.80 | 2 | 117 | 24 | 90 |
| 10/12/2013 | 20,082 | 1874.60 | 1 | 116 | 24 | 90 |
| 10/13/2013 | 19,890 | 1874.40 | 3 | 117 | 24 | 90 |
| 10/14/2013 | 19,691 | 1874.10 | 1 | 117 | 25 | 90 |
| 10/15/2013 | 19,484 | 1873.90 | 0 | 116 | 25 | 90 |
| 10/16/2013 | 19,309 | 1873.70 | 6 | 102 | 46 | 67 |
| 10/17/2013 | 19,163 | 1873.60 | 11 | 85 | 45 | 51 |
| 10/18/2013 | 19,027 | 1873.40 | 11 | 80 | 33 | 51 |
| 10/19/2013 | 18,888 | 1873.30 | 11 | 83 | 30 | 51 |
| 10/20/2013 | 18,736 | 1873.10 | 9 | 86 | 40 | 51 |
| 10/21/2013 | 18,500 | 1872.80 | -64 | 86 | 40 | 51 |
| 10/22/2013 | 18,309 | 1872.60 | 12 | 85 | 39 | 52 |
| 10/23/2013 | 18,173 | 1872.40 | 12 | 83 | 34 | 52 |
| 10/24/2013 | 18,031 | 1872.30 | 22 | 82 | 34 | 52 |
| 10/25/2013 | 17,889 | 1872.10 | 1 | 82 | 33 | 52 |
| 10/26/2013 | 17,752 | 1871.90 | 22 | 82 | 33 | 52 |
| 10/27/2013 | 17,607 | 1871.80 | 4 | 82 | 34 | 52 |
| 10/28/2013 | 17,469 | 1871.60 | 20 | 82 | 34 | 52 |
| 10/29/2013 | 17,331 | 1871.40 | 3 | 82 | 35 | 52 |
| 10/30/2013 | 17,189 | 1871.30 | 11 | 81 | 35 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/31/2013 | 17,049 | 1871.10 | 12 | 81 | 34 | 52 |
| 11/1/2013 | 16,933 | 1871.00 | 18 | 75 | 37 | 46 |
| 11/2/2013 | 16,794 | 1870.80 | 17 | 74 | 31 | 46 |
| 11/3/2013 | 16,668 | 1870.60 | 8 | 74 | 31 | 46 |
| 11/4/2013 | 16,546 | 1870.50 | 8 | 74 | 31 | 46 |
| 11/5/2013 | 16,429 | 1870.30 | 17 | 74 | 32 | 46 |
| 11/6/2013 | 16,313 | 1870.20 | 18 | 73 | 32 | 46 |
| 11/7/2013 | 16,178 | 1870.00 | 16 | 73 | 32 | 46 |
| 11/8/2013 | 16,067 | 1869.90 | 16 | 73 | 32 | 46 |
| 11/9/2013 | 15,941 | 1869.70 | 17 | 73 | 31 | 46 |
| 11/10/2013 | 15,831 | 1869.50 | 18 | 73 | 31 | 46 |
| 11/11/2013 | 15,706 | 1869.40 | 16 | 73 | 31 | 46 |
| 11/12/2013 | 15,586 | 1869.20 | 17 | 73 | 31 | 46 |
| 11/13/2013 | 15,467 | 1869.10 | 18 | 74 | 31 | 46 |
| 11/14/2013 | 15,281 | 1868.80 | 10 | 108 | 58 | 47 |
| 11/15/2013 | 15,156 | 1868.60 | 19 | 82 | 49 | 46 |
| 11/16/2013 | 15,031 | 1868.50 | 19 | 77 | 29 | 46 |
| 11/17/2013 | 14,907 | 1868.30 | 20 | 77 | 29 | 46 |
| 11/18/2013 | 14,794 | 1868.20 | 24 | 77 | 29 | 46 |
| 11/19/2013 | 14,806 | 1868.20 | 89 | 79 | 46 | 46 |
| 11/20/2013 | 14,798 | 1868.20 | 82 | 80 | 52 | 46 |
| 11/21/2013 | 14,728 | 1868.10 | 47 | 80 | 39 | 46 |
| 11/22/2013 | 14,610 | 1867.90 | 28 | 79 | 35 | 45 |
| 11/23/2013 | 14,495 | 1867.70 | 28 | 84 | 37 | 44 |
| 11/24/2013 | 14,381 | 1867.60 | 30 | 85 | 47 | 45 |
| 11/25/2013 | 14,264 | 1867.40 | 27 | 82 | 39 | 45 |
| 11/26/2013 | 14,143 | 1867.20 | 24 | 84 | 38 | 45 |
| 11/27/2013 | 14,024 | 1867.10 | 23 | 86 | 42 | 45 |
| 11/28/2013 | 13,898 | 1866.90 | 12 | 85 | 40 | 45 |
| 11/29/2013 | 13,770 | 1866.70 | 12 | 85 | 38 | 46 |
| 11/30/2013 | 13,607 | 1866.50 | 3 | 103 | 39 | 46 |
| 12/1/2013 | 13,330 | 1866.00 | -1 | 179 | 121 | 46 |
| 12/2/2013 | 13,039 | 1865.60 | 0 | 182 | 130 | 46 |
| 12/3/2013 | 12,747 | 1865.20 | -2 | 183 | 132 | 46 |
| 12/4/2013 | 12,458 | 1864.70 | 1 | 182 | 130 | 46 |
| 12/5/2013 | 12,177 | 1864.30 | 1 | 181 | 129 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/6/2013 | 11,936 | 1863.90 | 19 | 180 | 132 | 46 |
| 12/7/2013 | 11,660 | 1863.40 | 10 | 181 | 131 | 46 |
| 12/8/2013 | 11,383 | 1863.00 | 73 | 181 | 129 | 46 |
| 12/9/2013 | 11,117 | 1862.60 | 8 | 179 | 128 | 46 |
| 12/10/2013 | 10,846 | 1862.10 | 13 | 178 | 127 | 46 |
| 12/11/2013 | 10,586 | 1861.70 | 9 | 177 | 126 | 45 |
| 12/12/2013 | 10,417 | 1861.40 | 17 | 129 | 117 | 38 |
| 12/13/2013 | 10,361 | 1861.30 | 29 | 54 | 47 | 15 |
| 12/14/2013 | 10,307 | 1861.20 | 27 | 53 | 39 | 15 |
| 12/15/2013 | 10,255 | 1861.10 | 29 | 53 | 39 | 15 |
| 12/16/2013 | 10,203 | 1861.00 | 28 | 52 | 39 | 15 |
| 12/17/2013 | 10,126 | 1860.80 | 14 | 52 | 38 | 15 |
| 12/18/2013 | 10,094 | 1860.80 | 36 | 52 | 38 | 15 |
| 12/19/2013 | 10,024 | 1860.70 | 49 | 52 | 38 | 15 |
| 12/20/2013 | 9,957 | 1860.50 | 22 | 52 | 38 | 15 |
| 12/21/2013 | 9,913 | 1860.50 | 22 | 53 | 37 | 16 |
| 12/22/2013 | 9,860 | 1860.40 | 21 | 53 | 37 | 16 |
| 12/23/2013 | 9,804 | 1860.30 | 25 | 59 | 37 | 16 |
| 12/24/2013 | 9,749 | 1860.20 | 28 | 62 | 37 | 16 |
| 12/25/2013 | 9,693 | 1860.10 | 30 | 65 | 37 | 16 |
| 12/26/2013 | 9,638 | 1860.00 | 32 | 64 | 37 | 16 |
| 12/27/2013 | 9,583 | 1859.90 | 31 | 63 | 37 | 16 |
| 12/28/2013 | 9,523 | 1859.80 | 32 | 60 | 37 | 16 |
| 12/29/2013 | 9,471 | 1859.70 | 31 | 59 | 35 | 16 |
| 12/30/2013 | 9,409 | 1859.50 | 31 | 59 | 40 | 16 |
| 12/31/2013 | 9,347 | 1859.40 | 32 | 55 | 38 | 16 |
| 1/1/2014 | 9,288 | 1859.30 | 32 | 52 | 38 | 16 |
| 1/2/2014 | 9,227 | 1859.20 | 31 | 51 | 38 | 15 |
| 1/3/2014 | 9,171 | 1859.10 | 24 | 50 | 37 | 15 |
| 1/4/2014 | 9,113 | 1859.00 | 20 | 49 | 36 | 15 |
| 1/5/2014 | 9,060 | 1858.90 | 14 | 49 | 36 | 15 |
| 1/6/2014 | 9,005 | 1858.80 | 16 | 49 | 36 | 14 |
| 1/7/2014 | 8,950 | 1858.70 | 17 | 48 | 36 | 13 |
| 1/8/2014 | 8,898 | 1858.60 | 16 | 47 | 35 | 13 |
| 1/9/2014 | 8,846 | 1858.50 | 17 | 47 | 35 | 13 |
| 1/10/2014 | 8,806 | 1858.40 | 17 | 45 | 36 | 12 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/11/2014 | 8,773 | 1858.30 | 23 | 43 | 37 | 10 |
| 1/12/2014 | 8,739 | 1858.20 | 16 | 42 | 34 | 10 |
| 1/13/2014 | 8,706 | 1858.20 | 19 | 42 | 33 | 10 |
| 1/14/2014 | 8,664 | 1858.10 | 18 | 42 | 33 | 10 |
| 1/15/2014 | 8,638 | 1858.00 | 21 | 42 | 33 | 10 |
| 1/16/2014 | 8,585 | 1857.90 | 18 | 42 | 33 | 10 |
| 1/17/2014 | 8,542 | 1857.90 | 18 | 42 | 33 | 10 |
| 1/18/2014 | 8,502 | 1857.80 | 18 | 42 | 33 | 10 |
| 1/19/2014 | 8,457 | 1857.70 | 17 | 42 | 33 | 10 |
| 1/20/2014 | 8,413 | 1857.60 | 16 | 42 | 33 | 10 |
| 1/21/2014 | 8,373 | 1857.50 | 16 | 41 | 32 | 10 |
| 1/22/2014 | 8,329 | 1857.40 | 18 | 41 | 32 | 10 |
| 1/23/2014 | 8,288 | 1857.30 | 14 | 41 | 33 | 10 |
| 1/24/2014 | 8,240 | 1857.20 | 16 | 40 | 31 | 10 |
| 1/25/2014 | 8,196 | 1857.10 | 16 | 42 | 32 | 10 |
| 1/26/2014 | 8,148 | 1857.00 | 16 | 42 | 33 | 10 |
| 1/27/2014 | 8,105 | 1857.00 | 17 | 42 | 33 | 10 |
| 1/28/2014 | 8,061 | 1856.90 | 17 | 42 | 33 | 10 |
| 1/29/2014 | 8,044 | 1856.80 | 31 | 43 | 37 | 10 |
| 1/30/2014 | 8,015 | 1856.80 | 24 | 42 | 35 | 10 |
| 1/31/2014 | 7,975 | 1856.70 | 26 | 45 | 34 | 10 |
| 2/1/2014 | 7,944 | 1856.60 | 21 | 40 | 37 | 10 |
| 2/2/2014 | 7,908 | 1856.50 | 19 | 40 | 33 | 10 |
| 2/3/2014 | 7,870 | 1856.50 | 18 | 40 | 32 | 10 |
| 2/4/2014 | 7,833 | 1856.40 | 17 | 39 | 32 | 10 |
| 2/5/2014 | 7,809 | 1856.30 | 21 | 37 | 32 | 9 |
| 2/6/2014 | 7,816 | 1856.30 | 39 | 37 | 35 | 9 |
| 2/7/2014 | 7,927 | 1856.60 | 94 | 38 | 62 | 9 |
| 2/8/2014 | 8,588 | 1857.90 | 881 | 46 | 483 | 9 |
| 2/9/2014 | 14,105 | 1867.20 | 2,500 | 49 | 570 | 9 |
| 2/10/2014 | 16,374 | 1870.30 | 1,265 | 73 | 376 | 9 |
| 2/11/2014 | 16,857 | 1870.90 | 404 | 87 | 176 | 9 |
| 2/12/2014 | 17,129 | 1871.20 | 191 | 47 | 98 | 9 |
| 2/13/2014 | 17,339 | 1871.50 | 154 | 45 | 71 | 9 |
| 2/14/2014 | 17,513 | 1871.70 | 135 | 45 | 64 | 9 |
| 2/15/2014 | 17,674 | 1871.90 | 129 | 45 | 62 | 9 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/16/2014 | 18,013 | 1872.30 | 216 | 42 | 117 | 9 |
| 2/17/2014 | 18,232 | 1872.50 | 161 | 42 | 80 | 9 |
| 2/18/2014 | 18,388 | 1872.70 | 122 | 42 | 65 | 9 |
| 2/19/2014 | 18,508 | 1872.80 | 105 | 42 | 60 | 9 |
| 2/20/2014 | 18,607 | 1872.90 | 93 | 43 | 55 | 9 |
| 2/21/2014 | 18,684 | 1873.00 | 82 | 43 | 51 | 9 |
| 2/22/2014 | 18,754 | 1873.10 | 77 | 43 | 49 | 9 |
| 2/23/2014 | 18,801 | 1873.20 | 68 | 43 | 48 | 9 |
| 2/24/2014 | 18,844 | 1873.20 | 64 | 44 | 46 | 9 |
| 2/25/2014 | 18,875 | 1873.20 | 60 | 45 | 45 | 9 |
| 2/26/2014 | 18,910 | 1873.30 | 81 | 45 | 46 | 9 |
| 2/27/2014 | 21,929 | 1876.40 | 1,578 | 48 | 229 | 9 |
| 2/28/2014 | 24,165 | 1878.60 | 1,222 | 50 | 259 | 9 |
| 3/1/2014 | 25,964 | 1880.20 | 1,032 | 49 | 221 | 9 |
| 3/2/2014 | 26,916 | 1881.00 | 559 | 48 | 144 | 9 |
| 3/3/2014 | 28,128 | 1882.00 | 1,374 | 52 | 390 | 9 |
| 3/4/2014 | 35,912 | 1887.80 | 3,462 | 54 | 599 | 9 |
| 3/5/2014 | 37,300 | 1888.70 | 933 | 51 | 288 | 9 |
| 3/6/2014 | 42,530 | 1892.10 | 2,668 | 55 | 486 | 9 |
| 3/7/2014 | 44,363 | 1893.20 | 1,062 | 54 | 291 | 9 |
| 3/8/2014 | 45,550 | 1893.90 | 690 | 55 | 213 | 9 |
| 3/9/2014 | 46,003 | 1894.20 | 813 | 55 | 180 | 9 |
| 3/10/2014 | 50,749 | 1896.90 | 2,027 | 57 | 400 | 9 |
| 3/11/2014 | 52,595 | 1897.90 | 1,055 | 49 | 269 | 9 |
| 3/12/2014 | 53,811 | 1898.60 | 691 | 45 | 206 | 9 |
| 3/13/2014 | 54,662 | 1899.10 | 533 | 59 | 174 | 11 |
| 3/14/2014 | 55,307 | 1899.40 | 434 | 73 | 149 | 13 |
| 3/15/2014 | 55,739 | 1899.60 | 397 | 105 | 167 | 15 |
| 3/16/2014 | 56,061 | 1899.80 | 343 | 108 | 158 | 15 |
| 3/17/2014 | 56,307 | 1899.90 | 308 | 110 | 147 | 14 |
| 3/18/2014 | 56,496 | 1900.00 | 285 | 127 | 151 | 14 |
| 3/19/2014 | 56,580 | 1900.10 | 273 | 147 | 177 | 14 |
| 3/20/2014 | 56,858 | 1900.20 | 337 | 158 | 174 | 14 |
| 3/21/2014 | 56,820 | 1900.20 | 159 | 185 | 189 | 22 |
| 3/22/2014 | 56,774 | 1900.20 | 152 | 187 | 184 | 24 |
| 3/23/2014 | 56,706 | 1900.20 | 140 | 183 | 181 | 20 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/24/2014 | 56,639 | 1900.10 | 131 | 182 | 177 | 20 |
| 3/25/2014 | 56,667 | 1900.10 | 192 | 183 | 183 | 17 |
| 3/26/2014 | 56,810 | 1900.20 | 241 | 181 | 204 | 15 |
| 3/27/2014 | 56,975 | 1900.30 | 262 | 180 | 219 | 15 |
| 3/28/2014 | 57,472 | 1900.60 | 411 | 180 | 252 | 15 |
| 3/29/2014 | 67,445 | 1905.60 | 5,119 | 198 | 950 | 15 |
| 3/30/2014 | 69,808 | 1906.70 | 1,794 | 525 | 774 | 15 |
| 3/31/2014 | 70,102 | 1906.80 | 1,496 | 1,404 | 1,548 | 15 |
| 4/1/2014 | 70,037 | 1906.80 | 1,475 | 1,511 | 1,939 | 36 |
| 4/2/2014 | 70,234 | 1906.90 | 1,339 | 1,281 | 1,616 | 45 |
| 4/3/2014 | 70,322 | 1906.90 | 1,091 | 1,052 | 1,207 | 46 |
| 4/4/2014 | 70,384 | 1907.00 | 928 | 890 | 1,007 | 45 |
| 4/5/2014 | 70,485 | 1907.00 | 795 | 725 | 816 | 45 |
| 4/6/2014 | 70,714 | 1907.10 | 673 | 556 | 652 | 45 |
| 4/7/2014 | 71,032 | 1907.30 | 575 | 443 | 506 | 45 |
| 4/8/2014 | 71,285 | 1907.40 | 490 | 406 | 465 | 45 |
| 4/9/2014 | 71,505 | 1907.50 | 418 | 358 | 410 | 44 |
| 4/10/2014 | 71,671 | 1907.60 | 359 | 321 | 353 | 44 |
| 4/11/2014 | 71,815 | 1907.60 | 320 | 291 | 317 | 44 |
| 4/12/2014 | 71,837 | 1907.70 | 265 | 280 | 293 | 44 |
| 4/13/2014 | 71,904 | 1907.70 | 281 | 279 | 282 | 44 |
| 4/14/2014 | 71,859 | 1907.70 | 230 | 279 | 271 | 44 |
| 4/15/2014 | 71,771 | 1907.60 | 225 | 300 | 262 | 62 |
| 4/16/2014 | 71,594 | 1907.50 | 200 | 306 | 245 | 80 |
| 4/17/2014 | 71,505 | 1907.50 | 208 | 308 | 243 | 80 |
| 4/18/2014 | 71,258 | 1907.40 | 141 | 304 | 238 | 80 |
| 4/19/2014 | 71,011 | 1907.30 | 141 | 303 | 234 | 80 |
| 4/20/2014 | 70,764 | 1907.20 | 135 | 297 | 227 | 80 |
| 4/21/2014 | 70,518 | 1907.00 | 124 | 288 | 218 | 80 |
| 4/22/2014 | 70,298 | 1906.90 | 110 | 269 | 202 | 80 |
| 4/23/2014 | 70,081 | 1906.80 | 105 | 257 | 191 | 80 |
| 4/24/2014 | 69,885 | 1906.70 | 116 | 255 | 187 | 80 |
| 4/25/2014 | 69,689 | 1906.60 | 112 | 255 | 186 | 80 |
| 4/26/2014 | 69,471 | 1906.50 | 107 | 257 | 184 | 80 |
| 4/27/2014 | 69,254 | 1906.40 | 109 | 258 | 183 | 80 |
| 4/28/2014 | 69,015 | 1906.30 | 89 | 256 | 180 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/29/2014 | 68,757 | 1906.20 | 86 | 255 | 177 | 80 |
| 4/30/2014 | 68,499 | 1906.10 | 69 | 242 | 172 | 80 |
| 5/1/2014 | 68,252 | 1906.00 | 60 | 233 | 165 | 80 |
| 5/2/2014 | 67,995 | 1905.80 | 50 | 226 | 161 | 80 |
| 5/3/2014 | 67,728 | 1905.70 | 41 | 222 | 157 | 80 |
| 5/4/2014 | 67,461 | 1905.60 | 36 | 222 | 155 | 81 |
| 5/5/2014 | 67,215 | 1905.50 | 35 | 211 | 154 | 81 |
| 5/6/2014 | 66,954 | 1905.30 | 230 | 188 | 135 | 81 |
| 5/7/2014 | 66,796 | 1905.30 | 105 | 187 | 132 | 80 |
| 5/8/2014 | 66,595 | 1905.20 | 29 | 186 | 131 | 81 |
| 5/9/2014 | 66,368 | 1905.00 | 47 | 186 | 130 | 81 |
| 5/10/2014 | 66,131 | 1904.90 | 58 | 185 | 127 | 81 |
| 5/11/2014 | 65,890 | 1904.80 | 64 | 185 | 125 | 81 |
| 5/12/2014 | 65,675 | 1904.70 | 64 | 186 | 123 | 81 |
| 5/13/2014 | 65,428 | 1904.60 | 56 | 187 | 121 | 80 |
| 5/14/2014 | 65,179 | 1904.50 | 59 | 188 | 119 | 80 |
| 5/15/2014 | 64,939 | 1904.30 | 57 | 186 | 117 | 80 |
| 5/16/2014 | 64,702 | 1904.20 | 43 | 174 | 106 | 80 |
| 5/17/2014 | 64,458 | 1904.10 | 45 | 168 | 90 | 80 |
| 5/18/2014 | 64,235 | 1904.00 | 46 | 167 | 86 | 80 |
| 5/19/2014 | 64,053 | 1903.90 | 53 | 160 | 79 | 81 |
| 5/20/2014 | 63,885 | 1903.80 | 55 | 149 | 68 | 80 |
| 5/21/2014 | 63,730 | 1903.80 | 44 | 141 | 58 | 78 |
| 5/22/2014 | 63,563 | 1903.70 | 33 | 135 | 53 | 79 |
| 5/23/2014 | 63,394 | 1903.60 | 25 | 134 | 49 | 79 |
| 5/24/2014 | 63,224 | 1903.50 | 22 | 133 | 49 | 79 |
| 5/25/2014 | 63,032 | 1903.40 | 13 | 134 | 49 | 79 |
| 5/26/2014 | 62,844 | 1903.30 | 15 | 133 | 50 | 79 |
| 5/27/2014 | 62,661 | 1903.20 | 8 | 127 | 48 | 79 |
| 5/28/2014 | 62,450 | 1903.10 | -2 | 124 | 42 | 79 |
| 5/29/2014 | 62,267 | 1903.00 | 15 | 124 | 42 | 79 |
| 5/30/2014 | 62,073 | 1902.90 | 13 | 123 | 43 | 80 |
| 5/31/2014 | 61,922 | 1902.80 | 14 | 117 | 39 | 80 |
| 6/1/2014 | 61,758 | 1902.80 | 11 | 115 | 34 | 80 |
| 6/2/2014 | 61,572 | 1902.70 | 4 | 115 | 34 | 80 |
| 6/3/2014 | 61,377 | 1902.60 | 9 | 114 | 34 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/4/2014 | 61,203 | 1902.50 | -2 | 113 | 33 | 80 |
| 6/5/2014 | 61,021 | 1902.40 | 5 | 112 | 32 | 80 |
| 6/6/2014 | 60,819 | 1902.30 | 1 | 112 | 30 | 80 |
| 6/7/2014 | 60,644 | 1902.20 | 6 | 112 | 30 | 80 |
| 6/8/2014 | 60,439 | 1902.10 | 3 | 109 | 29 | 79 |
| 6/9/2014 | 60,261 | 1902.00 | 1 | 107 | 25 | 80 |
| 6/10/2014 | 60,073 | 1901.90 | 1 | 106 | 24 | 80 |
| 6/11/2014 | 59,883 | 1901.80 | -4 | 103 | 22 | 80 |
| 6/12/2014 | 59,670 | 1901.70 | -17 | 100 | 18 | 80 |
| 6/13/2014 | 59,480 | 1901.60 | -5 | 96 | 16 | 80 |
| 6/14/2014 | 59,320 | 1901.50 | -6 | 94 | 13 | 80 |
| 6/15/2014 | 59,129 | 1901.40 | -12 | 92 | 11 | 80 |
| 6/16/2014 | 58,912 | 1901.30 | -19 | 91 | 10 | 79 |
| 6/17/2014 | 58,749 | 1901.20 | -8 | 91 | 10 | 79 |
| 6/18/2014 | 58,567 | 1901.10 | -7 | 90 | 10 | 79 |
| 6/19/2014 | 58,375 | 1901.00 | -13 | 93 | 10 | 79 |
| 6/20/2014 | 58,194 | 1900.90 | -12 | 89 | 14 | 79 |
| 6/21/2014 | 58,019 | 1900.80 | -12 | 89 | 10 | 79 |
| 6/22/2014 | 57,832 | 1900.70 | -12 | 89 | 10 | 79 |
| 6/23/2014 | 57,635 | 1900.60 | -14 | 89 | 9 | 79 |
| 6/24/2014 | 57,437 | 1900.50 | -22 | 88 | 9 | 79 |
| 6/25/2014 | 57,279 | 1900.50 | -4 | 90 | 9 | 79 |
| 6/26/2014 | 57,097 | 1900.40 | -10 | 91 | 10 | 79 |
| 6/27/2014 | 56,918 | 1900.30 | -4 | 91 | 10 | 79 |
| 6/28/2014 | 56,734 | 1900.20 | -16 | 90 | 9 | 79 |
| 6/29/2014 | 56,544 | 1900.10 | -12 | 91 | 9 | 79 |
| 6/30/2014 | 56,356 | 1900.00 | -13 | 92 | 10 | 79 |
| 7/1/2014 | 56,155 | 1899.90 | -18 | 92 | 10 | 80 |
| 7/2/2014 | 55,953 | 1899.80 | -14 | 92 | 9 | 80 |
| 7/3/2014 | 55,761 | 1899.70 | -18 | 92 | 9 | 80 |
| 7/4/2014 | 55,535 | 1899.50 | -25 | 92 | 9 | 80 |
| 7/5/2014 | 55,330 | 1899.40 | -23 | 93 | 9 | 80 |
| 7/6/2014 | 55,145 | 1899.30 | -18 | 92 | 9 | 80 |
| 7/7/2014 | 54,939 | 1899.20 | -16 | 91 | 9 | 80 |
| 7/8/2014 | 54,779 | 1899.10 | -10 | 91 | 9 | 80 |
| 7/9/2014 | 54,570 | 1899.00 | -16 | 92 | 9 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/10/2014 | 54,351 | 1898.90 | -19 | 95 | 9 | 80 |
| 7/11/2014 | 54,138 | 1898.80 | -23 | 96 | 11 | 80 |
| 7/12/2014 | 53,950 | 1898.70 | -18 | 95 | 10 | 80 |
| 7/13/2014 | 53,716 | 1898.50 | -23 | 95 | 10 | 80 |
| 7/14/2014 | 53,512 | 1898.40 | -20 | 94 | 10 | 80 |
| 7/15/2014 | 53,316 | 1898.30 | -20 | 94 | 10 | 80 |
| 7/16/2014 | 53,124 | 1898.20 | -8 | 93 | 10 | 80 |
| 7/17/2014 | 52,917 | 1898.10 | -20 | 92 | 10 | 80 |
| 7/18/2014 | 52,722 | 1898.00 | -19 | 90 | 10 | 80 |
| 7/19/2014 | 52,518 | 1897.90 | -18 | 90 | 10 | 80 |
| 7/20/2014 | 52,305 | 1897.80 | -28 | 89 | 9 | 80 |
| 7/21/2014 | 52,110 | 1897.70 | -19 | 90 | 9 | 80 |
| 7/22/2014 | 51,912 | 1897.50 | -18 | 90 | 10 | 80 |
| 7/23/2014 | 51,706 | 1897.40 | -27 | 89 | 9 | 80 |
| 7/24/2014 | 51,494 | 1897.30 | -19 | 88 | 9 | 80 |
| 7/25/2014 | 51,278 | 1897.20 | -29 | 89 | 9 | 80 |
| 7/26/2014 | 51,100 | 1897.10 | -14 | 89 | 10 | 80 |
| 7/27/2014 | 50,868 | 1897.00 | -24 | 90 | 10 | 80 |
| 7/28/2014 | 50,643 | 1896.80 | -38 | 89 | 10 | 80 |
| 7/29/2014 | 50,437 | 1896.70 | -27 | 89 | 9 | 80 |
| 7/30/2014 | 50,250 | 1896.60 | -24 | 89 | 10 | 80 |
| 7/31/2014 | 50,124 | 1896.50 | 36 | 89 | 9 | 80 |
| 8/1/2014 | 49,917 | 1896.40 | -27 | 89 | 9 | 80 |
| 8/2/2014 | 49,709 | 1896.30 | -24 | 89 | 9 | 80 |
| 8/3/2014 | 49,530 | 1896.20 | -22 | 89 | 10 | 80 |
| 8/4/2014 | 49,332 | 1896.10 | -15 | 90 | 10 | 80 |
| 8/5/2014 | 49,159 | 1896.00 | -4 | 90 | 10 | 80 |
| 8/6/2014 | 48,982 | 1895.90 | -6 | 90 | 10 | 81 |
| 8/7/2014 | 48,843 | 1895.80 | -24 | 91 | 10 | 80 |
| 8/8/2014 | 48,571 | 1895.70 | -10 | 91 | 9 | 80 |
| 8/9/2014 | 48,363 | 1895.50 | -41 | 90 | 9 | 80 |
| 8/10/2014 | 48,149 | 1895.40 | -24 | 90 | 9 | 80 |
| 8/11/2014 | 47,943 | 1895.30 | -31 | 89 | 9 | 80 |
| 8/12/2014 | 47,751 | 1895.20 | -26 | 90 | 9 | 80 |
| 8/13/2014 | 47,542 | 1895.10 | -23 | 89 | 10 | 80 |
| 8/14/2014 | 47,343 | 1894.90 | -18 | 90 | 9 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/15/2014 | 47,118 | 1894.80 | -25 | 103 | 14 | 80 |
| 8/16/2014 | 46,880 | 1894.70 | -23 | 108 | 28 | 80 |
| 8/17/2014 | 46,646 | 1894.50 | -25 | 107 | 29 | 80 |
| 8/18/2014 | 46,417 | 1894.40 | -20 | 107 | 27 | 80 |
| 8/19/2014 | 46,138 | 1894.20 | -46 | 108 | 27 | 80 |
| 8/20/2014 | 45,905 | 1894.10 | -19 | 109 | 29 | 80 |
| 8/21/2014 | 45,681 | 1894.00 | -16 | 109 | 29 | 80 |
| 8/22/2014 | 45,446 | 1893.80 | -23 | 109 | 29 | 80 |
| 8/23/2014 | 45,223 | 1893.70 | -17 | 107 | 28 | 80 |
| 8/24/2014 | 44,984 | 1893.50 | -25 | 106 | 26 | 80 |
| 8/25/2014 | 44,763 | 1893.40 | -22 | 106 | 27 | 80 |
| 8/26/2014 | 44,533 | 1893.30 | -19 | 105 | 26 | 80 |
| 8/27/2014 | 44,300 | 1893.10 | -22 | 105 | 26 | 80 |
| 8/28/2014 | 44,074 | 1893.00 | -21 | 105 | 26 | 80 |
| 8/29/2014 | 43,849 | 1892.90 | -20 | 105 | 26 | 80 |
| 8/30/2014 | 43,611 | 1892.70 | -27 | 104 | 26 | 80 |
| 8/31/2014 | 43,381 | 1892.60 | -21 | 105 | 28 | 80 |
| 9/1/2014 | 43,160 | 1892.40 | -19 | 105 | 28 | 80 |
| 9/2/2014 | 42,925 | 1892.30 | -21 | 105 | 27 | 80 |
| 9/3/2014 | 42,700 | 1892.20 | -19 | 105 | 27 | 80 |
| 9/4/2014 | 42,458 | 1892.00 | -26 | 105 | 27 | 80 |
| 9/5/2014 | 42,232 | 1891.90 | -21 | 104 | 27 | 80 |
| 9/6/2014 | 42,006 | 1891.70 | -23 | 105 | 27 | 80 |
| 9/7/2014 | 41,768 | 1891.60 | -21 | 105 | 27 | 80 |
| 9/8/2014 | 41,540 | 1891.40 | -22 | 105 | 26 | 80 |
| 9/9/2014 | 41,316 | 1891.30 | -19 | 104 | 26 | 80 |
| 9/10/2014 | 41,085 | 1891.10 | -18 | 105 | 27 | 80 |
| 9/11/2014 | 40,848 | 1891.00 | -27 | 107 | 28 | 80 |
| 9/12/2014 | 40,619 | 1890.90 | -18 | 109 | 28 | 80 |
| 9/13/2014 | 40,401 | 1890.70 | -13 | 107 | 28 | 80 |
| 9/14/2014 | 40,179 | 1890.60 | -17 | 106 | 28 | 80 |
| 9/15/2014 | 39,947 | 1890.40 | -21 | 108 | 25 | 82 |
| 9/16/2014 | 39,696 | 1890.30 | -32 | 110 | 21 | 89 |
| 9/17/2014 | 39,450 | 1890.10 | -22 | 111 | 21 | 89 |
| 9/18/2014 | 39,239 | 1890.00 | -6 | 112 | 23 | 89 |
| 9/19/2014 | 39,027 | 1889.80 | -9 | 111 | 22 | 89 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/20/2014 | 38,800 | 1889.70 | -14 | 113 | 21 | 89 |
| 9/21/2014 | 38,562 | 1889.50 | -19 | 115 | 24 | 89 |
| 9/22/2014 | 38,329 | 1889.40 | -16 | 115 | 24 | 90 |
| 9/23/2014 | 38,104 | 1889.20 | -11 | 115 | 22 | 90 |
| 9/24/2014 | 37,889 | 1889.10 | -1 | 115 | 24 | 91 |
| 9/25/2014 | 37,711 | 1889.00 | 7 | 115 | 25 | 91 |
| 9/26/2014 | 37,488 | 1888.80 | -10 | 115 | 26 | 91 |
| 9/27/2014 | 37,266 | 1888.70 | -7 | 114 | 23 | 91 |
| 9/28/2014 | 37,041 | 1888.50 | -14 | 113 | 23 | 90 |
| 9/29/2014 | 36,814 | 1888.40 | -13 | 113 | 21 | 91 |
| 9/30/2014 | 36,582 | 1888.20 | -14 | 113 | 21 | 90 |
| 10/1/2014 | 36,354 | 1888.10 | -17 | 113 | 21 | 91 |
| 10/2/2014 | 36,118 | 1887.90 | -18 | 115 | 23 | 91 |
| 10/3/2014 | 35,889 | 1887.70 | -15 | 115 | 24 | 91 |
| 10/4/2014 | 35,660 | 1887.60 | -12 | 114 | 24 | 91 |
| 10/5/2014 | 35,431 | 1887.40 | -13 | 114 | 24 | 91 |
| 10/6/2014 | 35,198 | 1887.30 | -14 | 114 | 23 | 90 |
| 10/7/2014 | 34,964 | 1887.10 | -17 | 115 | 23 | 91 |
| 10/8/2014 | 34,744 | 1886.90 | -11 | 115 | 23 | 91 |
| 10/9/2014 | 34,518 | 1886.80 | -12 | 115 | 23 | 91 |
| 10/10/2014 | 34,286 | 1886.60 | -11 | 114 | 23 | 90 |
| 10/11/2014 | 34,064 | 1886.50 | -10 | 114 | 24 | 90 |
| 10/12/2014 | 33,837 | 1886.30 | -12 | 114 | 24 | 90 |
| 10/13/2014 | 33,603 | 1886.10 | -14 | 115 | 25 | 90 |
| 10/14/2014 | 33,384 | 1886.00 | -7 | 116 | 27 | 90 |
| 10/15/2014 | 33,238 | 1885.90 | 25 | 111 | 33 | 90 |
| 10/16/2014 | 33,106 | 1885.80 | 2 | 72 | 31 | 51 |
| 10/17/2014 | 32,984 | 1885.70 | 4 | 72 | 26 | 45 |
| 10/18/2014 | 32,867 | 1885.60 | 12 | 74 | 26 | 45 |
| 10/19/2014 | 32,731 | 1885.50 | 2 | 76 | 31 | 45 |
| 10/20/2014 | 32,606 | 1885.40 | 8 | 76 | 34 | 45 |
| 10/21/2014 | 32,469 | 1885.30 | 3 | 77 | 31 | 45 |
| 10/22/2014 | 32,334 | 1885.20 | 6 | 77 | 31 | 45 |
| 10/23/2014 | 32,170 | 1885.10 | -14 | 77 | 34 | 45 |
| 10/24/2014 | 32,042 | 1885.00 | 4 | 76 | 33 | 45 |
| 10/25/2014 | 32,037 | 1885.00 | 70 | 77 | 44 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/26/2014 | 31,967 | 1884.90 | 35 | 76 | 38 | 45 |
| 10/27/2014 | 31,869 | 1884.90 | 18 | 76 | 33 | 45 |
| 10/28/2014 | 31,745 | 1884.80 | 9 | 76 | 32 | 45 |
| 10/29/2014 | 31,629 | 1884.70 | 10 | 76 | 31 | 45 |
| 10/30/2014 | 31,510 | 1884.60 | 7 | 76 | 31 | 45 |
| 10/31/2014 | 31,439 | 1884.50 | 33 | 77 | 37 | 45 |
| 11/1/2014 | 31,308 | 1884.40 | 3 | 77 | 34 | 45 |
| 11/2/2014 | 31,183 | 1884.40 | 5 | 77 | 33 | 45 |
| 11/3/2014 | 31,048 | 1884.20 | 4 | 77 | 34 | 45 |
| 11/4/2014 | 30,925 | 1884.20 | 6 | 77 | 33 | 45 |
| 11/5/2014 | 30,806 | 1884.10 | 10 | 77 | 33 | 45 |
| 11/6/2014 | 30,672 | 1884.00 | 3 | 77 | 33 | 45 |
| 11/7/2014 | 30,548 | 1883.90 | 7 | 77 | 33 | 45 |
| 11/8/2014 | 30,426 | 1883.80 | 4 | 77 | 33 | 45 |
| 11/9/2014 | 30,297 | 1883.70 | 8 | 78 | 33 | 45 |
| 11/10/2014 | 30,172 | 1883.60 | 4 | 78 | 33 | 45 |
| 11/11/2014 | 30,043 | 1883.50 | 5 | 78 | 33 | 45 |
| 11/12/2014 | 29,925 | 1883.40 | 16 | 78 | 33 | 45 |
| 11/13/2014 | 29,875 | 1883.30 | 41 | 79 | 39 | 45 |
| 11/14/2014 | 29,758 | 1883.30 | 13 | 77 | 34 | 45 |
| 11/15/2014 | 29,642 | 1883.20 | 8 | 75 | 39 | 25 |
| 11/16/2014 | 29,539 | 1883.10 | 10 | 69 | 46 | 0 |
| 11/17/2014 | 29,439 | 1883.00 | 8 | 69 | 44 | 0 |
| 11/18/2014 | 29,340 | 1882.90 | 13 | 69 | 42 | 0 |
| 11/19/2014 | 29,335 | 1882.90 | 62 | 69 | 48 | 0 |
| 11/20/2014 | 29,373 | 1883.00 | 84 | 69 | 57 | 0 |
| 11/21/2014 | 29,417 | 1883.00 | 90 | 68 | 59 | 0 |
| 11/22/2014 | 30,276 | 1883.70 | 502 | 68 | 211 | 0 |
| 11/23/2014 | 30,500 | 1883.80 | 174 | 68 | 89 | 0 |
| 11/24/2014 | 30,396 | 1883.80 | 25 | 90 | 60 | 0 |
| 11/25/2014 | 30,162 | 1883.60 | 19 | 162 | 160 | 0 |
| 11/26/2014 | 29,900 | 1883.40 | 34 | 190 | 171 | 0 |
| 11/27/2014 | 29,627 | 1883.20 | 31 | 190 | 169 | 0 |
| 11/28/2014 | 29,452 | 1883.00 | 77 | 191 | 172 | 0 |
| 11/29/2014 | 29,373 | 1883.00 | 132 | 191 | 194 | 0 |
| 11/30/2014 | 29,574 | 1883.10 | 275 | 192 | 246 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/1/2014 | 29,646 | 1883.20 | 216 | 192 | 205 | 0 |
| 12/2/2014 | 29,778 | 1883.30 | 247 | 194 | 200 | 0 |
| 12/3/2014 | 33,972 | 1886.40 | 2,335 | 206 | 581 | 0 |
| 12/4/2014 | 35,600 | 1887.50 | 1,112 | 205 | 432 | 0 |
| 12/5/2014 | 36,866 | 1888.40 | 808 | 208 | 398 | 0 |
| 12/6/2014 | 39,428 | 1890.10 | 1,593 | 208 | 601 | 0 |
| 12/7/2014 | 40,098 | 1890.50 | 561 | 208 | 307 | 0 |
| 12/8/2014 | 41,063 | 1891.10 | 682 | 208 | 336 | 0 |
| 12/9/2014 | 41,927 | 1891.70 | 671 | 208 | 349 | 0 |
| 12/10/2014 | 43,243 | 1892.50 | 837 | 211 | 363 | 0 |
| 12/11/2014 | 61,154 | 1902.50 | 12,782 | 3,541 | 5,772 | 0 |
| 12/12/2014 | 57,962 | 1900.80 | 4,102 | 5,322 | 7,946 | 0 |
| 12/13/2014 | 56,477 | 1900.00 | 1,669 | 2,284 | 3,270 | 0 |
| 12/14/2014 | 55,772 | 1899.70 | 1,085 | 1,310 | 1,694 | 0 |
| 12/15/2014 | 55,584 | 1899.60 | 977 | 975 | 1,153 | 0 |
| 12/16/2014 | 56,065 | 1899.80 | 1,271 | 971 | 1,186 | 0 |
| 12/17/2014 | 56,420 | 1900.00 | 1,913 | 1,577 | 2,279 | 0 |
| 12/18/2014 | 56,177 | 1899.90 | 1,550 | 1,537 | 2,191 | 0 |
| 12/19/2014 | 57,396 | 1900.50 | 2,817 | 2,091 | 2,963 | 0 |
| 12/20/2014 | 56,999 | 1900.30 | 2,211 | 2,273 | 3,294 | 0 |
| 12/21/2014 | 56,995 | 1900.30 | 2,355 | 2,207 | 3,084 | 0 |
| 12/22/2014 | 56,498 | 1900.00 | 1,859 | 1,946 | 2,661 | 0 |
| 12/23/2014 | 55,994 | 1899.80 | 1,321 | 1,444 | 1,896 | 0 |
| 12/24/2014 | 55,718 | 1899.60 | 1,093 | 1,126 | 1,385 | 0 |
| 12/25/2014 | 55,463 | 1899.50 | 903 | 932 | 1,080 | 0 |
| 12/26/2014 | 55,251 | 1899.40 | 718 | 740 | 852 | 0 |
| 12/27/2014 | 55,064 | 1899.30 | 571 | 604 | 699 | 0 |
| 12/28/2014 | 54,948 | 1899.20 | 476 | 504 | 590 | 0 |
| 12/29/2014 | 54,837 | 1899.20 | 388 | 429 | 512 | 0 |
| 12/30/2014 | 54,750 | 1899.10 | 335 | 378 | 454 | 0 |
| 12/31/2014 | 54,680 | 1899.10 | 286 | 324 | 394 | 0 |
| 1/1/2015 | 54,613 | 1899.00 | 251 | 296 | 358 | 0 |
| 1/2/2015 | 54,545 | 1899.00 | 222 | 271 | 327 | 0 |
| 1/3/2015 | 54,488 | 1899.00 | 203 | 251 | 300 | 0 |
| 1/4/2015 | 54,418 | 1898.90 | 177 | 233 | 275 | 0 |
| 1/5/2015 | 54,364 | 1898.90 | 166 | 219 | 254 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/6/2015 | 54,309 | 1898.90 | 154 | 207 | 236 | 0 |
| 1/7/2015 | 54,261 | 1898.80 | 146 | 198 | 221 | 0 |
| 1/8/2015 | 54,186 | 1898.80 | 125 | 193 | 210 | 0 |
| 1/9/2015 | 54,116 | 1898.80 | 126 | 190 | 203 | 0 |
| 1/10/2015 | 54,025 | 1898.70 | 116 | 189 | 198 | 0 |
| 1/11/2015 | 53,934 | 1898.70 | 111 | 189 | 196 | 0 |
| 1/12/2015 | 53,831 | 1898.60 | 106 | 188 | 194 | 0 |
| 1/13/2015 | 53,718 | 1898.50 | 101 | 188 | 193 | 0 |
| 1/14/2015 | 53,578 | 1898.50 | 90 | 188 | 192 | 0 |
| 1/15/2015 | 53,429 | 1898.40 | 85 | 188 | 191 | 0 |
| 1/16/2015 | 53,409 | 1898.40 | 149 | 188 | 202 | 0 |
| 1/17/2015 | 53,317 | 1898.30 | 114 | 187 | 198 | 0 |
| 1/18/2015 | 53,279 | 1898.30 | 137 | 187 | 210 | 0 |
| 1/19/2015 | 53,258 | 1898.30 | 150 | 188 | 207 | 0 |
| 1/20/2015 | 53,178 | 1898.20 | 116 | 189 | 198 | 0 |
| 1/21/2015 | 53,063 | 1898.20 | 100 | 189 | 195 | 0 |
| 1/22/2015 | 52,935 | 1898.10 | 93 | 189 | 192 | 0 |
| 1/23/2015 | 52,783 | 1898.00 | 79 | 188 | 191 | 0 |
| 1/24/2015 | 52,641 | 1897.90 | 87 | 188 | 190 | 0 |
| 1/25/2015 | 52,480 | 1897.90 | 77 | 187 | 190 | 0 |
| 1/26/2015 | 52,306 | 1897.80 | 67 | 186 | 187 | 0 |
| 1/27/2015 | 52,130 | 1897.70 | 62 | 184 | 185 | 0 |
| 1/28/2015 | 51,963 | 1897.60 | 64 | 181 | 181 | 0 |
| 1/29/2015 | 51,790 | 1897.50 | 61 | 179 | 178 | 0 |
| 1/30/2015 | 51,621 | 1897.40 | 52 | 176 | 175 | 0 |
| 1/31/2015 | 51,441 | 1897.30 | 56 | 176 | 173 | 0 |
| 2/1/2015 | 51,255 | 1897.20 | 48 | 176 | 173 | 0 |
| 2/2/2015 | 51,071 | 1897.10 | 47 | 175 | 173 | 0 |
| 2/3/2015 | 50,896 | 1897.00 | 50 | 173 | 171 | 0 |
| 2/4/2015 | 50,716 | 1896.90 | 49 | 173 | 170 | 0 |
| 2/5/2015 | 50,538 | 1896.80 | 56 | 174 | 170 | 0 |
| 2/6/2015 | 56,926 | 1900.30 | 3,299 | 287 | 615 | 0 |
| 2/7/2015 | 58,117 | 1900.90 | 5,301 | 4,273 | 5,709 | 0 |
| 2/8/2015 | 57,473 | 1900.60 | 2,346 | 2,545 | 3,422 | 0 |
| 2/9/2015 | 58,578 | 1901.10 | 4,216 | 3,675 | 5,131 | 0 |
| 2/10/2015 | 57,259 | 1900.40 | 2,157 | 2,934 | 4,194 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/11/2015 | 56,453 | 1900.00 | 1,367 | 1,857 | 2,466 | 0 |
| 2/12/2015 | 55,948 | 1899.80 | 970 | 1,288 | 1,624 | 0 |
| 2/13/2015 | 55,625 | 1899.60 | 776 | 991 | 1,139 | 0 |
| 2/14/2015 | 55,399 | 1899.50 | 638 | 792 | 886 | 0 |
| 2/15/2015 | 55,215 | 1899.40 | 532 | 651 | 730 | 0 |
| 2/16/2015 | 55,081 | 1899.30 | 464 | 561 | 621 | 0 |
| 2/17/2015 | 54,970 | 1899.20 | 403 | 483 | 539 | 0 |
| 2/18/2015 | 54,869 | 1899.20 | 345 | 413 | 476 | 0 |
| 2/19/2015 | 54,786 | 1899.10 | 310 | 367 | 427 | 0 |
| 2/20/2015 | 54,724 | 1899.10 | 283 | 331 | 387 | 0 |
| 2/21/2015 | 54,631 | 1899.00 | 254 | 314 | 354 | 14 |
| 2/22/2015 | 54,531 | 1899.00 | 226 | 290 | 320 | 46 |
| 2/23/2015 | 54,410 | 1898.90 | 209 | 284 | 288 | 57 |
| 2/24/2015 | 54,306 | 1898.90 | 197 | 269 | 255 | 71 |
| 2/25/2015 | 54,220 | 1898.80 | 184 | 252 | 235 | 73 |
| 2/26/2015 | 54,120 | 1898.80 | 179 | 244 | 222 | 70 |
| 2/27/2015 | 54,039 | 1898.70 | 178 | 242 | 218 | 70 |
| 2/28/2015 | 53,937 | 1898.70 | 163 | 242 | 215 | 70 |
| 3/1/2015 | 53,785 | 1898.60 | 159 | 240 | 212 | 69 |
| 3/2/2015 | 53,693 | 1898.50 | 145 | 220 | 215 | 46 |
| 3/3/2015 | 53,582 | 1898.50 | 141 | 213 | 203 | 46 |
| 3/4/2015 | 53,463 | 1898.40 | 129 | 207 | 194 | 45 |
| 3/5/2015 | 53,343 | 1898.30 | 118 | 206 | 191 | 45 |
| 3/6/2015 | 53,206 | 1898.30 | 118 | 205 | 190 | 45 |
| 3/7/2015 | 53,069 | 1898.20 | 111 | 205 | 190 | 45 |
| 3/8/2015 | 52,932 | 1898.10 | 109 | 201 | 187 | 45 |
| 3/9/2015 | 52,777 | 1898.00 | 100 | 199 | 183 | 45 |
| 3/10/2015 | 52,650 | 1898.00 | 102 | 194 | 183 | 45 |
| 3/11/2015 | 52,468 | 1897.90 | 107 | 195 | 182 | 45 |
| 3/12/2015 | 52,324 | 1897.80 | 104 | 193 | 180 | 45 |
| 3/13/2015 | 52,152 | 1897.70 | 99 | 188 | 176 | 45 |
| 3/14/2015 | 52,017 | 1897.60 | 98 | 179 | 169 | 45 |
| 3/15/2015 | 51,863 | 1897.50 | 82 | 172 | 161 | 45 |
| 3/16/2015 | 51,728 | 1897.40 | 90 | 172 | 160 | 45 |
| 3/17/2015 | 51,566 | 1897.30 | 82 | 172 | 160 | 45 |
| 3/18/2015 | 51,404 | 1897.30 | 77 | 174 | 155 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/19/2015 | 51,216 | 1897.20 | 71 | 176 | 153 | 55 |
| 3/20/2015 | 51,037 | 1897.10 | 71 | 172 | 151 | 56 |
| 3/21/2015 | 50,868 | 1897.00 | 70 | 168 | 147 | 55 |
| 3/22/2015 | 50,779 | 1896.90 | 100 | 164 | 150 | 49 |
| 3/23/2015 | 50,672 | 1896.80 | 99 | 161 | 155 | 44 |
| 3/24/2015 | 50,601 | 1896.80 | 109 | 161 | 153 | 44 |
| 3/25/2015 | 50,513 | 1896.80 | 105 | 161 | 152 | 45 |
| 3/26/2015 | 50,371 | 1896.70 | 81 | 162 | 149 | 45 |
| 3/27/2015 | 50,212 | 1896.60 | 65 | 162 | 148 | 45 |
| 3/28/2015 | 50,053 | 1896.50 | 67 | 161 | 147 | 45 |
| 3/29/2015 | 49,894 | 1896.40 | 59 | 161 | 147 | 45 |
| 3/30/2015 | 49,700 | 1896.30 | 55 | 162 | 145 | 46 |
| 3/31/2015 | 49,506 | 1896.20 | 39 | 162 | 145 | 46 |
| 4/1/2015 | 49,296 | 1896.10 | 51 | 166 | 142 | 52 |
| 4/2/2015 | 49,069 | 1895.90 | 40 | 172 | 143 | 60 |
| 4/3/2015 | 48,843 | 1895.80 | 47 | 176 | 133 | 76 |
| 4/4/2015 | 48,600 | 1895.70 | 45 | 176 | 128 | 81 |
| 4/5/2015 | 48,444 | 1895.60 | 73 | 171 | 136 | 72 |
| 4/6/2015 | 48,288 | 1895.50 | 80 | 166 | 133 | 68 |
| 4/7/2015 | 48,409 | 1895.60 | 205 | 168 | 159 | 68 |
| 4/8/2015 | 48,340 | 1895.50 | 126 | 167 | 150 | 68 |
| 4/9/2015 | 48,219 | 1895.40 | 94 | 167 | 147 | 63 |
| 4/10/2015 | 48,063 | 1895.40 | 80 | 167 | 150 | 54 |
| 4/11/2015 | 47,925 | 1895.30 | 79 | 167 | 150 | 50 |
| 4/12/2015 | 47,770 | 1895.20 | 80 | 167 | 149 | 50 |
| 4/13/2015 | 47,598 | 1895.10 | 63 | 167 | 147 | 50 |
| 4/14/2015 | 47,359 | 1894.90 | 52 | 183 | 151 | 49 |
| 4/15/2015 | 47,103 | 1894.80 | 62 | 206 | 158 | 77 |
| 4/16/2015 | 46,814 | 1894.60 | 54 | 204 | 145 | 90 |
| 4/17/2015 | 46,560 | 1894.50 | 53 | 200 | 141 | 90 |
| 4/18/2015 | 46,306 | 1894.30 | 52 | 196 | 137 | 90 |
| 4/19/2015 | 46,053 | 1894.20 | 54 | 196 | 135 | 90 |
| 4/20/2015 | 45,785 | 1894.00 | 52 | 194 | 134 | 90 |
| 4/21/2015 | 45,384 | 1893.80 | 43 | 257 | 164 | 90 |
| 4/22/2015 | 45,002 | 1893.60 | 43 | 243 | 194 | 90 |
| 4/23/2015 | 44,753 | 1893.40 | 38 | 193 | 136 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/24/2015 | 44,488 | 1893.20 | 46 | 193 | 135 | 90 |
| 4/25/2015 | 44,241 | 1893.10 | 67 | 193 | 139 | 90 |
| 4/26/2015 | 43,946 | 1892.90 | 52 | 193 | 135 | 90 |
| 4/27/2015 | 43,701 | 1892.80 | 48 | 193 | 134 | 90 |
| 4/28/2015 | 43,409 | 1892.60 | 37 | 194 | 133 | 90 |
| 4/29/2015 | 43,116 | 1892.40 | 35 | 194 | 133 | 90 |
| 4/30/2015 | 42,825 | 1892.20 | 30 | 194 | 132 | 90 |
| 5/1/2015 | 42,503 | 1892.00 | 26 | 194 | 131 | 90 |
| 5/2/2015 | 42,198 | 1891.80 | 26 | 193 | 130 | 90 |
| 5/3/2015 | 41,880 | 1891.60 | 23 | 193 | 129 | 90 |
| 5/4/2015 | 41,561 | 1891.40 | 20 | 193 | 128 | 90 |
| 5/5/2015 | 41,102 | 1891.20 | 16 | 255 | 157 | 90 |
| 5/6/2015 | 40,662 | 1890.90 | 14 | 244 | 189 | 90 |
| 5/7/2015 | 40,365 | 1890.70 | 10 | 198 | 136 | 90 |
| 5/8/2015 | 40,007 | 1890.50 | 18 | 197 | 135 | 90 |
| 5/9/2015 | 39,682 | 1890.30 | 19 | 198 | 135 | 90 |
| 5/10/2015 | 39,343 | 1890.00 | 20 | 198 | 134 | 90 |
| 5/11/2015 | 39,006 | 1889.80 | 6 | 191 | 134 | 86 |
| 5/12/2015 | 38,655 | 1889.60 | 3 | 186 | 134 | 79 |
| 5/13/2015 | 38,351 | 1889.40 | 19 | 186 | 133 | 79 |
| 5/14/2015 | 37,883 | 1889.10 | 29 | 287 | 167 | 96 |
| 5/15/2015 | 37,449 | 1888.80 | 24 | 272 | 191 | 118 |
| 5/16/2015 | 37,122 | 1888.60 | 24 | 201 | 118 | 112 |
| 5/17/2015 | 36,884 | 1888.40 | 71 | 190 | 113 | 105 |
| 5/18/2015 | 36,633 | 1888.20 | 30 | 173 | 110 | 95 |
| 5/19/2015 | 36,412 | 1888.10 | 24 | 148 | 92 | 77 |
| 5/20/2015 | 36,236 | 1888.00 | 34 | 137 | 78 | 77 |
| 5/21/2015 | 36,090 | 1887.90 | 48 | 131 | 67 | 77 |
| 5/22/2015 | 35,959 | 1887.80 | 49 | 130 | 64 | 77 |
| 5/23/2015 | 35,784 | 1887.70 | 27 | 130 | 63 | 76 |
| 5/24/2015 | 35,596 | 1887.50 | 20 | 130 | 62 | 77 |
| 5/25/2015 | 35,393 | 1887.40 | 14 | 130 | 61 | 76 |
| 5/26/2015 | 35,205 | 1887.30 | 17 | 126 | 59 | 76 |
| 5/27/2015 | 35,004 | 1887.10 | 17 | 124 | 51 | 78 |
| 5/28/2015 | 34,832 | 1887.00 | 21 | 116 | 53 | 71 |
| 5/29/2015 | 34,675 | 1886.90 | 17 | 109 | 45 | 67 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/30/2015 | 34,533 | 1886.80 | 19 | 102 | 43 | 61 |
| 5/31/2015 | 34,377 | 1886.70 | 17 | 100 | 41 | 59 |
| 6/1/2015 | 34,250 | 1886.60 | 20 | 86 | 38 | 56 |
| 6/2/2015 | 34,123 | 1886.50 | 17 | 91 | 38 | 52 |
| 6/3/2015 | 33,967 | 1886.40 | 8 | 100 | 40 | 56 |
| 6/4/2015 | 33,827 | 1886.30 | 14 | 99 | 37 | 67 |
| 6/5/2015 | 33,672 | 1886.20 | 13 | 96 | 31 | 66 |
| 6/6/2015 | 33,519 | 1886.10 | 9 | 96 | 31 | 66 |
| 6/7/2015 | 33,393 | 1886.00 | 13 | 87 | 31 | 63 |
| 6/8/2015 | 33,268 | 1885.90 | 11 | 80 | 26 | 56 |
| 6/9/2015 | 33,102 | 1885.80 | 3 | 98 | 29 | 60 |
| 6/10/2015 | 32,923 | 1885.60 | 6 | 104 | 27 | 77 |
| 6/11/2015 | 32,757 | 1885.50 | 8 | 102 | 24 | 77 |
| 6/12/2015 | 32,565 | 1885.40 | 0 | 99 | 22 | 77 |
| 6/13/2015 | 32,387 | 1885.20 | -2 | 101 | 20 | 77 |
| 6/14/2015 | 32,210 | 1885.10 | -5 | 98 | 20 | 77 |
| 6/15/2015 | 32,033 | 1885.00 | -4 | 97 | 18 | 77 |
| 6/16/2015 | 31,844 | 1884.80 | -6 | 96 | 18 | 77 |
| 6/17/2015 | 31,656 | 1884.70 | -9 | 99 | 17 | 77 |
| 6/18/2015 | 31,468 | 1884.60 | -13 | 99 | 21 | 78 |
| 6/19/2015 | 31,267 | 1884.40 | -8 | 95 | 17 | 77 |
| 6/20/2015 | 31,094 | 1884.30 | -9 | 95 | 17 | 77 |
| 6/21/2015 | 30,895 | 1884.10 | -14 | 95 | 17 | 77 |
| 6/22/2015 | 30,710 | 1884.00 | -11 | 95 | 17 | 77 |
| 6/23/2015 | 30,526 | 1883.90 | -9 | 95 | 16 | 76 |
| 6/24/2015 | 30,343 | 1883.70 | -5 | 96 | 16 | 76 |
| 6/25/2015 | 30,173 | 1883.60 | -5 | 97 | 15 | 76 |
| 6/26/2015 | 29,978 | 1883.40 | -8 | 100 | 18 | 76 |
| 6/27/2015 | 29,797 | 1883.30 | -9 | 91 | 19 | 71 |
| 6/28/2015 | 29,616 | 1883.10 | -12 | 87 | 18 | 65 |
| 6/29/2015 | 29,462 | 1883.00 | -9 | 84 | 17 | 65 |
| 6/30/2015 | 29,283 | 1882.90 | -9 | 84 | 15 | 65 |
| 7/1/2015 | 29,118 | 1882.80 | -12 | 85 | 17 | 65 |
| 7/2/2015 | 28,941 | 1882.60 | -9 | 85 | 16 | 65 |
| 7/3/2015 | 28,777 | 1882.50 | -3 | 94 | 19 | 67 |
| 7/4/2015 | 28,576 | 1882.30 | -12 | 96 | 19 | 73 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/5/2015 | 28,389 | 1882.20 | -14 | 94 | 16 | 73 |
| 7/6/2015 | 28,190 | 1882.00 | -14 | 94 | 17 | 73 |
| 7/7/2015 | 28,004 | 1881.90 | -14 | 94 | 17 | 73 |
| 7/8/2015 | 27,857 | 1881.70 | 3 | 93 | 18 | 73 |
| 7/9/2015 | 27,758 | 1881.70 | 37 | 92 | 19 | 73 |
| 7/10/2015 | 27,636 | 1881.60 | 18 | 92 | 18 | 73 |
| 7/11/2015 | 27,478 | 1881.40 | 7 | 92 | 17 | 73 |
| 7/12/2015 | 27,309 | 1881.30 | -3 | 92 | 16 | 73 |
| 7/13/2015 | 27,176 | 1881.20 | 0 | 77 | 18 | 65 |
| 7/14/2015 | 27,032 | 1881.10 | -5 | 73 | 15 | 53 |
| 7/15/2015 | 26,888 | 1880.90 | -5 | 76 | 19 | 54 |
| 7/16/2015 | 26,733 | 1880.80 | -7 | 77 | 17 | 57 |
| 7/17/2015 | 26,567 | 1880.70 | -10 | 79 | 15 | 58 |
| 7/18/2015 | 26,414 | 1880.50 | -12 | 80 | 17 | 62 |
| 7/19/2015 | 26,250 | 1880.40 | -7 | 79 | 16 | 62 |
| 7/20/2015 | 26,121 | 1880.30 | 0 | 76 | 16 | 59 |
| 7/21/2015 | 25,982 | 1880.20 | -7 | 73 | 19 | 52 |
| 7/22/2015 | 25,797 | 1880.00 | -18 | 84 | 20 | 55 |
| 7/23/2015 | 25,612 | 1879.80 | -15 | 85 | 19 | 68 |
| 7/24/2015 | 25,441 | 1879.70 | -12 | 84 | 15 | 68 |
| 7/25/2015 | 25,270 | 1879.50 | -15 | 84 | 15 | 67 |
| 7/26/2015 | 25,090 | 1879.40 | -13 | 84 | 16 | 67 |
| 7/27/2015 | 24,921 | 1879.20 | -12 | 90 | 16 | 68 |
| 7/28/2015 | 24,732 | 1879.10 | -10 | 91 | 17 | 73 |
| 7/29/2015 | 24,554 | 1878.90 | -13 | 90 | 14 | 73 |
| 7/30/2015 | 24,378 | 1878.80 | -9 | 90 | 14 | 73 |
| 7/31/2015 | 24,202 | 1878.60 | -7 | 92 | 14 | 73 |
| 8/1/2015 | 24,029 | 1878.40 | -6 | 92 | 14 | 73 |
| 8/2/2015 | 23,856 | 1878.30 | -9 | 92 | 14 | 73 |
| 8/3/2015 | 23,674 | 1878.10 | -7 | 92 | 14 | 73 |
| 8/4/2015 | 23,493 | 1877.90 | -5 | 92 | 14 | 73 |
| 8/5/2015 | 23,335 | 1877.80 | -6 | 89 | 15 | 71 |
| 8/6/2015 | 23,155 | 1877.60 | -7 | 87 | 16 | 67 |
| 8/7/2015 | 22,989 | 1877.50 | -11 | 87 | 15 | 66 |
| 8/8/2015 | 22,824 | 1877.30 | -9 | 84 | 17 | 64 |
| 8/9/2015 | 22,660 | 1877.20 | -10 | 80 | 21 | 58 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/10/2015 | 22,517 | 1877.00 | -8 | 76 | 18 | 56 |
| 8/11/2015 | 22,385 | 1876.90 | -8 | 72 | 18 | 52 |
| 8/12/2015 | 22,244 | 1876.80 | -11 | 71 | 15 | 52 |
| 8/13/2015 | 22,093 | 1876.60 | -11 | 79 | 17 | 54 |
| 8/14/2015 | 21,934 | 1876.40 | -5 | 83 | 17 | 60 |
| 8/15/2015 | 21,786 | 1876.30 | -2 | 80 | 17 | 60 |
| 8/16/2015 | 21,639 | 1876.20 | -6 | 79 | 13 | 60 |
| 8/17/2015 | 21,492 | 1876.00 | -7 | 79 | 12 | 60 |
| 8/18/2015 | 21,337 | 1875.90 | -6 | 80 | 12 | 60 |
| 8/19/2015 | 21,182 | 1875.70 | -5 | 82 | 16 | 60 |
| 8/20/2015 | 21,029 | 1875.50 | -4 | 82 | 17 | 60 |
| 8/21/2015 | 20,886 | 1875.40 | -2 | 81 | 15 | 60 |
| 8/22/2015 | 20,725 | 1875.20 | -2 | 85 | 16 | 61 |
| 8/23/2015 | 20,556 | 1875.10 | -5 | 87 | 17 | 64 |
| 8/24/2015 | 20,397 | 1874.90 | -4 | 86 | 16 | 64 |
| 8/25/2015 | 20,248 | 1874.70 | -4 | 82 | 17 | 62 |
| 8/26/2015 | 20,091 | 1874.60 | -5 | 78 | 18 | 56 |
| 8/27/2015 | 19,954 | 1874.40 | -3 | 79 | 17 | 56 |
| 8/28/2015 | 19,808 | 1874.30 | 1 | 80 | 17 | 56 |
| 8/29/2015 | 19,645 | 1874.10 | -3 | 85 | 18 | 58 |
| 8/30/2015 | 19,483 | 1873.90 | -1 | 88 | 15 | 65 |
| 8/31/2015 | 19,307 | 1873.70 | 1 | 87 | 16 | 65 |
| 9/1/2015 | 19,148 | 1873.50 | -3 | 81 | 16 | 61 |
| 9/2/2015 | 18,998 | 1873.40 | -4 | 77 | 15 | 55 |
| 9/3/2015 | 18,858 | 1873.20 | -4 | 76 | 15 | 55 |
| 9/4/2015 | 18,710 | 1873.10 | -8 | 75 | 15 | 55 |
| 9/5/2015 | 18,571 | 1872.90 | -1 | 75 | 18 | 52 |
| 9/6/2015 | 18,425 | 1872.70 | -5 | 73 | 18 | 50 |
| 9/7/2015 | 18,297 | 1872.60 | -2 | 70 | 16 | 50 |
| 9/8/2015 | 18,170 | 1872.40 | -3 | 70 | 14 | 50 |
| 9/9/2015 | 18,035 | 1872.30 | 0 | 78 | 15 | 52 |
| 9/10/2015 | 17,884 | 1872.10 | 0 | 82 | 14 | 59 |
| 9/11/2015 | 17,742 | 1871.90 | 0 | 81 | 14 | 59 |
| 9/12/2015 | 17,601 | 1871.80 | 0 | 81 | 14 | 59 |
| 9/13/2015 | 17,462 | 1871.60 | 7 | 81 | 15 | 59 |
| 9/14/2015 | 17,331 | 1871.40 | 5 | 80 | 17 | 57 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/15/2015 | 17,185 | 1871.30 | -23 | 76 | 20 | 53 |
| 9/16/2015 | 17,097 | 1871.20 | 24 | 73 | 24 | 53 |
| 9/17/2015 | 16,969 | 1871.00 | 1 | 71 | 22 | 52 |
| 9/18/2015 | 16,849 | 1870.90 | 1 | 67 | 23 | 47 |
| 9/19/2015 | 16,731 | 1870.70 | -2 | 64 | 22 | 43 |
| 9/20/2015 | 16,613 | 1870.60 | 0 | 61 | 18 | 43 |
| 9/21/2015 | 16,503 | 1870.40 | 0 | 60 | 15 | 43 |
| 9/22/2015 | 16,387 | 1870.30 | -2 | 59 | 14 | 42 |
| 9/23/2015 | 16,278 | 1870.10 | 1 | 58 | 14 | 42 |
| 9/24/2015 | 16,170 | 1870.00 | 1 | 58 | 14 | 43 |
| 9/25/2015 | 16,055 | 1869.80 | 0 | 57 | 14 | 42 |
| 9/26/2015 | 15,941 | 1869.70 | 0 | 56 | 14 | 43 |
| 9/27/2015 | 15,835 | 1869.60 | 1 | 56 | 14 | 43 |
| 9/28/2015 | 15,729 | 1869.40 | 0 | 56 | 13 | 43 |
| 9/29/2015 | 15,601 | 1869.20 | -3 | 56 | 13 | 43 |
| 9/30/2015 | 15,497 | 1869.10 | 4 | 56 | 13 | 43 |
| 10/1/2015 | 15,400 | 1869.00 | 4 | 57 | 15 | 43 |
| 10/2/2015 | 15,296 | 1868.80 | 4 | 55 | 15 | 41 |
| 10/3/2015 | 15,185 | 1868.70 | 0 | 54 | 17 | 38 |
| 10/4/2015 | 15,090 | 1868.60 | 2 | 53 | 15 | 38 |
| 10/5/2015 | 14,972 | 1868.40 | 0 | 59 | 16 | 39 |
| 10/6/2015 | 14,849 | 1868.20 | 1 | 63 | 19 | 45 |
| 10/7/2015 | 14,747 | 1868.10 | 2 | 62 | 15 | 45 |
| 10/8/2015 | 14,625 | 1867.90 | 1 | 62 | 15 | 45 |
| 10/9/2015 | 14,517 | 1867.80 | 1 | 62 | 15 | 45 |
| 10/10/2015 | 14,403 | 1867.60 | 1 | 62 | 15 | 45 |
| 10/11/2015 | 14,281 | 1867.40 | 3 | 62 | 15 | 45 |
| 10/12/2015 | 14,182 | 1867.30 | 3 | 62 | 15 | 45 |
| 10/13/2015 | 14,063 | 1867.10 | -3 | 62 | 14 | 45 |
| 10/14/2015 | 13,950 | 1867.00 | 10 | 62 | 14 | 45 |
| 10/15/2015 | 13,846 | 1866.80 | 4 | 62 | 14 | 45 |
| 10/16/2015 | 13,749 | 1866.70 | 4 | 56 | 21 | 38 |
| 10/17/2015 | 13,666 | 1866.50 | 8 | 45 | 16 | 33 |
| 10/18/2015 | 13,597 | 1866.40 | 20 | 50 | 12 | 32 |
| 10/19/2015 | 13,507 | 1866.30 | 8 | 49 | 18 | 33 |
| 10/20/2015 | 13,425 | 1866.20 | 9 | 49 | 12 | 33 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/21/2015 | 13,330 | 1866.00 | 4 | 51 | 15 | 33 |
| 10/22/2015 | 13,228 | 1865.90 | -1 | 52 | 17 | 33 |
| 10/23/2015 | 13,134 | 1865.70 | 0 | 52 | 16 | 33 |
| 10/24/2015 | 13,053 | 1865.60 | 11 | 51 | 16 | 33 |
| 10/25/2015 | 12,959 | 1865.50 | 3 | 49 | 14 | 33 |
| 10/26/2015 | 12,879 | 1865.40 | 8 | 49 | 13 | 33 |
| 10/27/2015 | 12,800 | 1865.20 | 8 | 49 | 15 | 33 |
| 10/28/2015 | 12,720 | 1865.10 | 9 | 50 | 15 | 33 |
| 10/29/2015 | 12,654 | 1865.00 | 9 | 39 | 30 | 19 |
| 10/30/2015 | 12,615 | 1865.00 | 14 | 27 | 20 | 11 |
| 10/31/2015 | 12,576 | 1864.90 | 15 | 28 | 14 | 11 |
| 11/1/2015 | 12,563 | 1864.90 | 23 | 28 | 16 | 11 |
| 11/2/2015 | 12,523 | 1864.80 | 1 | 28 | 17 | 11 |
| 11/3/2015 | 12,484 | 1864.80 | 35 | 29 | 16 | 11 |
| 11/4/2015 | 12,452 | 1864.70 | 15 | 29 | 17 | 11 |
| 11/5/2015 | 12,419 | 1864.70 | -9 | 27 | 16 | 11 |
| 11/6/2015 | 12,380 | 1864.60 | -1 | 27 | 13 | 11 |
| 11/7/2015 | 12,348 | 1864.50 | 31 | 28 | 14 | 11 |
| 11/8/2015 | 12,322 | 1864.50 | 12 | 28 | 18 | 11 |
| 11/9/2015 | 12,329 | 1864.50 | 28 | 29 | 22 | 11 |
| 11/10/2015 | 12,290 | 1864.40 | 13 | 28 | 18 | 11 |
| 11/11/2015 | 12,284 | 1864.40 | 14 | 28 | 17 | 11 |
| 11/12/2015 | 12,252 | 1864.40 | 9 | 28 | 16 | 11 |
| 11/13/2015 | 12,219 | 1864.30 | 9 | 28 | 16 | 11 |
| 11/14/2015 | 12,187 | 1864.30 | 10 | 28 | 15 | 11 |
| 11/15/2015 | 12,207 | 1864.30 | 36 | 31 | 28 | 4 |
| 11/16/2015 | 12,168 | 1864.30 | 4 | 28 | 21 | 0 |
| 11/17/2015 | 12,149 | 1864.20 | 16 | 28 | 20 | 0 |
| 11/18/2015 | 12,124 | 1864.20 | 13 | 28 | 19 | 0 |
| 11/19/2015 | 12,104 | 1864.20 | 16 | 28 | 18 | 0 |
| 11/20/2015 | 12,073 | 1864.10 | 14 | 29 | 18 | 0 |
| 11/21/2015 | 12,047 | 1864.10 | 12 | 29 | 19 | 0 |
| 11/22/2015 | 12,022 | 1864.00 | 12 | 29 | 19 | 0 |
| 11/23/2015 | 11,977 | 1864.00 | 14 | 39 | 20 | 0 |
| 11/24/2015 | 11,933 | 1863.90 | 22 | 48 | 38 | 0 |
| 11/25/2015 | 11,877 | 1863.80 | 3 | 50 | 38 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/26/2015 | 11,820 | 1863.70 | 35 | 51 | 39 | 0 |
| 11/27/2015 | 11,752 | 1863.60 | 21 | 51 | 39 | 0 |
| 11/28/2015 | 11,620 | 1863.40 | 16 | 80 | 51 | 0 |
| 11/29/2015 | 11,484 | 1863.20 | 32 | 88 | 86 | 0 |
| 11/30/2015 | 11,349 | 1862.90 | 28 | 83 | 66 | 0 |
| 12/1/2015 | 11,209 | 1862.70 | 45 | 90 | 77 | 0 |
| 12/2/2015 | 11,102 | 1862.50 | 20 | 78 | 74 | 0 |
| 12/3/2015 | 11,105 | 1862.50 | 48 | 53 | 58 | 0 |
| 12/4/2015 | 11,166 | 1862.60 | 87 | 53 | 44 | 0 |
| 12/5/2015 | 11,159 | 1862.60 | 56 | 66 | 58 | 0 |
| 12/6/2015 | 11,318 | 1862.90 | 151 | 67 | 90 | 0 |
| 12/7/2015 | 11,490 | 1863.20 | 147 | 66 | 77 | 0 |
| 12/8/2015 | 11,502 | 1863.20 | 73 | 66 | 63 | 0 |
| 12/9/2015 | 11,533 | 1863.20 | 82 | 67 | 67 | 0 |
| 12/10/2015 | 13,981 | 1867.00 | 1,304 | 71 | 358 | 0 |
| 12/11/2015 | 15,046 | 1868.50 | 680 | 72 | 234 | 0 |
| 12/12/2015 | 15,360 | 1868.90 | 240 | 71 | 124 | 0 |
| 12/13/2015 | 17,942 | 1872.20 | 1,386 | 75 | 378 | 0 |
| 12/14/2015 | 18,754 | 1873.10 | 535 | 74 | 182 | 0 |
| 12/15/2015 | 19,084 | 1873.50 | 263 | 75 | 117 | 0 |
| 12/16/2015 | 19,216 | 1873.60 | 160 | 84 | 102 | 0 |
| 12/17/2015 | 19,305 | 1873.70 | 128 | 87 | 102 | 0 |
| 12/18/2015 | 21,316 | 1875.80 | 1,035 | 81 | 320 | 0 |
| 12/19/2015 | 23,145 | 1877.60 | 1,128 | 74 | 310 | 0 |
| 12/20/2015 | 25,000 | 1879.30 | 1,003 | 74 | 187 | 0 |
| 12/21/2015 | 32,730 | 1885.50 | 3,981 | 93 | 1,488 | 0 |
| 12/22/2015 | 41,000 | 1891.10 | 4,300 | 149 | 1,964 | 0 |
| 12/23/2015 | 44,862 | 1893.50 | 2,105 | 175 | 620 | 0 |
| 12/24/2015 | 46,289 | 1894.30 | 947 | 176 | 448 | 0 |
| 12/25/2015 | 47,069 | 1894.80 | 605 | 181 | 356 | 0 |
| 12/26/2015 | 47,564 | 1895.10 | 429 | 180 | 291 | 0 |
| 12/27/2015 | 47,908 | 1895.30 | 359 | 180 | 259 | 0 |
| 12/28/2015 | 48,201 | 1895.40 | 314 | 180 | 245 | 0 |
| 12/29/2015 | 48,340 | 1895.50 | 264 | 180 | 228 | 0 |
| 12/30/2015 | 48,461 | 1895.60 | 233 | 180 | 216 | 0 |
| 12/31/2015 | 48,530 | 1895.60 | 199 | 180 | 207 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/1/2016 | 48,513 | 1895.60 | 172 | 179 | 199 | 0 |
| 1/2/2016 | 48,496 | 1895.60 | 163 | 178 | 194 | 0 |
| 1/3/2016 | 48,548 | 1895.60 | 197 | 179 | 203 | 0 |
| 1/4/2016 | 48,999 | 1895.90 | 391 | 181 | 234 | 0 |
| 1/5/2016 | 53,241 | 1898.30 | 2,356 | 189 | 564 | 0 |
| 1/6/2016 | 55,758 | 1899.60 | 1,938 | 629 | 782 | 0 |
| 1/7/2016 | 55,758 | 1899.60 | 1,429 | 1,472 | 1,628 | 0 |
| 1/8/2016 | 55,420 | 1899.50 | 971 | 1,196 | 1,249 | 0 |
| 1/9/2016 | 55,364 | 1899.40 | 939 | 1,024 | 997 | 0 |
| 1/10/2016 | 55,251 | 1899.40 | 878 | 975 | 936 | 0 |
| 1/11/2016 | 55,101 | 1899.30 | 715 | 831 | 796 | 0 |
| 1/12/2016 | 55,131 | 1899.30 | 721 | 744 | 704 | 0 |
| 1/13/2016 | 57,780 | 1900.70 | 3,655 | 2,502 | 2,691 | 0 |
| 1/14/2016 | 57,646 | 1900.70 | 2,921 | 3,162 | 4,060 | 0 |
| 1/15/2016 | 57,107 | 1900.40 | 2,828 | 3,197 | 4,240 | 0 |
| 1/16/2016 | 57,819 | 1900.70 | 3,530 | 3,371 | 4,232 | 0 |
| 1/17/2016 | 63,408 | 1903.60 | 8,393 | 6,405 | 7,432 | 0 |
| 1/18/2016 | 59,302 | 1901.50 | 7,373 | 9,401 | 11,809 | 0 |
| 1/19/2016 | 59,519 | 1901.60 | 5,210 | 5,407 | 6,926 | 0 |
| 1/20/2016 | 57,846 | 1900.80 | 3,370 | 4,378 | 5,649 | 0 |
| 1/21/2016 | 56,954 | 1900.30 | 2,249 | 2,835 | 3,409 | 0 |
| 1/22/2016 | 57,318 | 1900.50 | 2,728 | 2,684 | 2,965 | 0 |
| 1/23/2016 | 58,071 | 1900.90 | 3,479 | 3,297 | 4,075 | 0 |
| 1/24/2016 | 57,318 | 1900.50 | 2,683 | 3,202 | 4,018 | 0 |
| 1/25/2016 | 56,744 | 1900.20 | 2,078 | 2,471 | 2,895 | 0 |
| 1/26/2016 | 56,325 | 1900.00 | 1,644 | 1,951 | 2,155 | 0 |
| 1/27/2016 | 56,003 | 1899.80 | 1,340 | 1,578 | 1,671 | 0 |
| 1/28/2016 | 55,909 | 1899.70 | 1,261 | 1,376 | 1,369 | 0 |
| 1/29/2016 | 58,129 | 1900.90 | 3,157 | 2,231 | 2,627 | 0 |
| 1/30/2016 | 57,337 | 1900.50 | 2,842 | 3,344 | 4,265 | 0 |
| 1/31/2016 | 56,667 | 1900.10 | 1,965 | 2,420 | 2,865 | 0 |
| 2/1/2016 | 56,192 | 1899.90 | 1,496 | 1,815 | 2,013 | 0 |
| 2/2/2016 | 55,890 | 1899.70 | 1,250 | 1,469 | 1,548 | 0 |
| 2/3/2016 | 55,720 | 1899.60 | 1,095 | 1,250 | 1,213 | 0 |
| 2/4/2016 | 55,532 | 1899.50 | 971 | 1,116 | 1,042 | 0 |
| 2/5/2016 | 55,382 | 1899.40 | 838 | 971 | 889 | 0 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/6/2016 | 55,232 | 1899.40 | 735 | 844 | 772 | 0 |
| 2/7/2016 | 55,101 | 1899.30 | 652 | 740 | 681 | 0 |
| 2/8/2016 | 55,045 | 1899.30 | 595 | 667 | 615 | 0 |
| 2/9/2016 | 54,970 | 1899.20 | 540 | 604 | 568 | 0 |
| 2/10/2016 | 54,914 | 1899.20 | 485 | 545 | 525 | 0 |
| 2/11/2016 | 54,859 | 1899.20 | 444 | 496 | 487 | 0 |
| 2/12/2016 | 54,821 | 1899.10 | 415 | 459 | 457 | 0 |
| 2/13/2016 | 54,765 | 1899.10 | 377 | 427 | 431 | 0 |
| 2/14/2016 | 54,728 | 1899.10 | 356 | 398 | 406 | 0 |
| 2/15/2016 | 54,710 | 1899.10 | 354 | 378 | 386 | 0 |
| 2/16/2016 | 54,672 | 1899.10 | 331 | 364 | 372 | 0 |
| 2/17/2016 | 54,765 | 1899.10 | 394 | 359 | 368 | 0 |
| 2/18/2016 | 55,344 | 1899.40 | 839 | 604 | 563 | 0 |
| 2/19/2016 | 56,117 | 1899.80 | 1,519 | 1,193 | 1,080 | 37 |
| 2/20/2016 | 55,645 | 1899.60 | 1,197 | 1,489 | 1,327 | 147 |
| 2/21/2016 | 55,288 | 1899.40 | 891 | 1,147 | 924 | 148 |
| 2/22/2016 | 55,045 | 1899.30 | 754 | 909 | 685 | 148 |
| 2/23/2016 | 54,914 | 1899.20 | 666 | 767 | 567 | 148 |
| 2/24/2016 | 54,822 | 1899.10 | 604 | 685 | 498 | 148 |
| 2/25/2016 | 54,765 | 1899.10 | 560 | 623 | 451 | 147 |
| 2/26/2016 | 54,784 | 1899.10 | 567 | 601 | 425 | 147 |
| 2/27/2016 | 54,728 | 1899.10 | 552 | 595 | 416 | 147 |
| 2/28/2016 | 54,654 | 1899.10 | 490 | 554 | 391 | 148 |
| 2/29/2016 | 54,580 | 1899.00 | 444 | 507 | 357 | 147 |
| 3/1/2016 | 54,524 | 1899.00 | 409 | 463 | 325 | 148 |
| 3/2/2016 | 54,524 | 1899.00 | 410 | 440 | 305 | 147 |
| 3/3/2016 | 54,598 | 1899.00 | 496 | 475 | 340 | 147 |
| 3/4/2016 | 54,561 | 1899.00 | 455 | 483 | 338 | 147 |
| 3/5/2016 | 59,756 | 1901.70 | 3,584 | 1,532 | 1,265 | 146 |
| 3/6/2016 | 59,716 | 1901.70 | 6,754 | 6,776 | 8,457 | 11 |
| 3/7/2016 | 58,323 | 1901.00 | 4,189 | 5,043 | 6,657 | 78 |
| 3/8/2016 | 57,241 | 1900.40 | 2,564 | 3,268 | 3,928 | 147 |
| 3/9/2016 | 57,126 | 1900.40 | 2,447 | 2,646 | 2,858 | 147 |
| 3/10/2016 | 60,073 | 1901.90 | 5,795 | 4,615 | 5,536 | 61 |
| 3/11/2016 | 59,460 | 1901.60 | 5,360 | 5,932 | 7,613 | 1 |
| 3/12/2016 | 60,591 | 1902.20 | 5,849 | 5,627 | 7,282 | 36 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/13/2016 | 62,936 | 1903.40 | 9,568 | 9,076 | 11,528 | 1 |
| 3/14/2016 | 60,155 | 1902.00 | 7,822 | 9,386 | 11,879 | 0 |
| 3/15/2016 | 58,421 | 1901.10 | 3,809 | 4,897 | 6,528 | 26 |
| 3/16/2016 | 57,452 | 1900.50 | 2,665 | 3,291 | 4,069 | 45 |
| 3/17/2016 | 56,877 | 1900.20 | 2,120 | 2,532 | 2,897 | 45 |
| 3/18/2016 | 56,458 | 1900.00 | 1,747 | 2,053 | 2,247 | 45 |
| 3/19/2016 | 56,173 | 1899.90 | 1,485 | 1,717 | 1,812 | 44 |
| 3/20/2016 | 56,514 | 1900.10 | 1,684 | 1,625 | 1,641 | 45 |
| 3/21/2016 | 57,761 | 1900.70 | 2,726 | 2,249 | 2,484 | 45 |
| 3/22/2016 | 57,222 | 1900.40 | 2,600 | 2,964 | 3,669 | 45 |
| 3/23/2016 | 56,667 | 1900.10 | 1,912 | 2,297 | 2,635 | 44 |
| 3/24/2016 | 56,325 | 1900.00 | 1,613 | 1,871 | 2,034 | 45 |
| 3/25/2016 | 56,060 | 1899.80 | 1,408 | 1,615 | 1,696 | 45 |
| 3/26/2016 | 55,909 | 1899.70 | 1,255 | 1,421 | 1,411 | 45 |
| 3/27/2016 | 55,720 | 1899.60 | 1,139 | 1,282 | 1,207 | 45 |
| 3/28/2016 | 55,551 | 1899.50 | 1,010 | 1,146 | 1,048 | 45 |
| 3/29/2016 | 55,438 | 1899.50 | 916 | 1,032 | 927 | 45 |
| 3/30/2016 | 55,344 | 1899.40 | 827 | 927 | 828 | 45 |
| 3/31/2016 | 55,213 | 1899.40 | 750 | 946 | 817 | 45 |
| 4/1/2016 | 55,495 | 1899.50 | 699 | 632 | 648 | 45 |
| 4/2/2016 | 56,060 | 1899.80 | 539 | 286 | 386 | 45 |
| 4/3/2016 | 56,553 | 1900.10 | 502 | 276 | 362 | 45 |
| 4/4/2016 | 56,992 | 1900.30 | 475 | 267 | 343 | 45 |
| 4/5/2016 | 57,414 | 1900.50 | 461 | 261 | 326 | 45 |
| 4/6/2016 | 57,742 | 1900.70 | 435 | 262 | 318 | 45 |
| 4/7/2016 | 58,071 | 1900.90 | 420 | 263 | 312 | 45 |
| 4/8/2016 | 58,362 | 1901.00 | 405 | 264 | 307 | 45 |
| 4/9/2016 | 58,655 | 1901.20 | 404 | 264 | 304 | 45 |
| 4/10/2016 | 58,929 | 1901.30 | 392 | 264 | 300 | 45 |
| 4/11/2016 | 59,145 | 1901.40 | 370 | 265 | 296 | 45 |
| 4/12/2016 | 59,282 | 1901.50 | 341 | 266 | 292 | 45 |
| 4/13/2016 | 59,499 | 1901.60 | 341 | 266 | 290 | 45 |
| 4/14/2016 | 59,637 | 1901.70 | 361 | 278 | 306 | 45 |
| 4/15/2016 | 59,690 | 1901.70 | 313 | 305 | 287 | 75 |
| 4/16/2016 | 59,697 | 1901.70 | 293 | 303 | 260 | 90 |
| 4/17/2016 | 59,677 | 1901.70 | 270 | 297 | 247 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/18/2016 | 59,617 | 1901.70 | 258 | 299 | 243 | 90 |
| 4/19/2016 | 59,558 | 1901.60 | 250 | 305 | 248 | 90 |
| 4/20/2016 | 59,440 | 1901.60 | 236 | 305 | 246 | 89 |
| 4/21/2016 | 59,381 | 1901.60 | 235 | 305 | 245 | 89 |
| 4/22/2016 | 59,657 | 1901.70 | 449 | 308 | 318 | 90 |
| 4/23/2016 | 59,795 | 1901.80 | 370 | 306 | 306 | 90 |
| 4/24/2016 | 59,756 | 1901.70 | 282 | 306 | 266 | 89 |
| 4/25/2016 | 59,697 | 1901.70 | 243 | 304 | 254 | 90 |
| 4/26/2016 | 59,598 | 1901.70 | 241 | 304 | 248 | 90 |
| 4/27/2016 | 59,519 | 1901.60 | 258 | 305 | 250 | 90 |
| 4/28/2016 | 59,460 | 1901.60 | 253 | 305 | 247 | 90 |
| 4/29/2016 | 59,302 | 1901.50 | 198 | 305 | 241 | 89 |
| 4/30/2016 | 59,105 | 1901.40 | 192 | 305 | 236 | 89 |
| 5/1/2016 | 58,935 | 1901.30 | 201 | 305 | 233 | 89 |
| 5/2/2016 | 58,733 | 1901.20 | 180 | 300 | 227 | 89 |
| 5/3/2016 | 58,499 | 1901.10 | 185 | 337 | 252 | 89 |
| 5/4/2016 | 57,916 | 1900.80 | 249 | 579 | 459 | 89 |
| 5/5/2016 | 57,606 | 1900.60 | 325 | 530 | 474 | 89 |
| 5/6/2016 | 57,299 | 1900.50 | 263 | 436 | 385 | 89 |
| 5/7/2016 | 57,011 | 1900.30 | 217 | 391 | 328 | 89 |
| 5/8/2016 | 56,705 | 1900.20 | 205 | 378 | 306 | 89 |
| 5/9/2016 | 56,420 | 1900.00 | 188 | 361 | 288 | 89 |
| 5/10/2016 | 56,117 | 1899.80 | 171 | 352 | 268 | 89 |
| 5/11/2016 | 55,780 | 1899.70 | 155 | 345 | 276 | 89 |
| 5/12/2016 | 55,462 | 1899.50 | 141 | 330 | 262 | 89 |
| 5/13/2016 | 55,127 | 1899.30 | 131 | 315 | 245 | 89 |
| 5/14/2016 | 54,830 | 1899.10 | 121 | 303 | 235 | 89 |
| 5/15/2016 | 54,553 | 1899.00 | 119 | 287 | 193 | 117 |
| 5/16/2016 | 54,278 | 1898.80 | 117 | 274 | 171 | 130 |
| 5/17/2016 | 54,002 | 1898.70 | 96 | 253 | 158 | 129 |
| 5/18/2016 | 53,747 | 1898.60 | 90 | 245 | 147 | 129 |
| 5/19/2016 | 53,473 | 1898.40 | 72 | 245 | 146 | 130 |
| 5/20/2016 | 53,145 | 1898.20 | 87 | 245 | 146 | 129 |
| 5/21/2016 | 52,928 | 1898.10 | 109 | 249 | 154 | 127 |
| 5/22/2016 | 52,657 | 1898.00 | 96 | 245 | 148 | 129 |
| 5/23/2016 | 52,405 | 1897.80 | 88 | 238 | 142 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/24/2016 | 52,280 | 1897.70 | 127 | 231 | 137 | 130 |
| 5/25/2016 | 52,066 | 1897.60 | 118 | 225 | 130 | 130 |
| 5/26/2016 | 51,869 | 1897.50 | 93 | 223 | 125 | 130 |
| 5/27/2016 | 51,619 | 1897.40 | 80 | 222 | 120 | 130 |
| 5/28/2016 | 51,369 | 1897.20 | 72 | 221 | 118 | 130 |
| 5/29/2016 | 51,103 | 1897.10 | 64 | 222 | 116 | 130 |
| 5/30/2016 | 50,819 | 1896.90 | 66 | 224 | 119 | 130 |
| 5/31/2016 | 50,555 | 1896.80 | 57 | 221 | 116 | 130 |
| 6/1/2016 | 50,256 | 1896.60 | 50 | 218 | 112 | 129 |
| 6/2/2016 | 49,957 | 1896.40 | 45 | 217 | 110 | 129 |
| 6/3/2016 | 49,677 | 1896.30 | 42 | 217 | 109 | 130 |
| 6/4/2016 | 49,381 | 1896.10 | 45 | 213 | 106 | 130 |
| 6/5/2016 | 49,085 | 1895.90 | 41 | 210 | 101 | 129 |
| 6/6/2016 | 48,809 | 1895.80 | 36 | 210 | 99 | 129 |
| 6/7/2016 | 48,498 | 1895.60 | 30 | 209 | 98 | 129 |
| 6/8/2016 | 48,189 | 1895.40 | 24 | 200 | 90 | 130 |
| 6/9/2016 | 47,914 | 1895.30 | 20 | 190 | 81 | 130 |
| 6/10/2016 | 47,590 | 1895.10 | 11 | 184 | 74 | 129 |
| 6/11/2016 | 47,318 | 1894.90 | 17 | 179 | 69 | 130 |
| 6/12/2016 | 47,047 | 1894.80 | 10 | 173 | 63 | 130 |
| 6/13/2016 | 46,743 | 1894.60 | 10 | 171 | 60 | 130 |
| 6/14/2016 | 46,474 | 1894.40 | -3 | 172 | 60 | 130 |
| 6/15/2016 | 46,189 | 1894.30 | 15 | 179 | 60 | 130 |
| 6/16/2016 | 45,938 | 1894.10 | 29 | 179 | 62 | 130 |
| 6/17/2016 | 45,772 | 1894.00 | 67 | 180 | 63 | 130 |
| 6/18/2016 | 45,557 | 1893.90 | 59 | 180 | 65 | 130 |
| 6/19/2016 | 45,309 | 1893.70 | 35 | 176 | 61 | 130 |
| 6/20/2016 | 45,062 | 1893.60 | 24 | 174 | 56 | 130 |
| 6/21/2016 | 44,781 | 1893.40 | 18 | 179 | 55 | 130 |
| 6/22/2016 | 44,469 | 1893.20 | 14 | 208 | 81 | 131 |
| 6/23/2016 | 44,142 | 1893.00 | 8 | 192 | 69 | 132 |
| 6/24/2016 | 43,849 | 1892.90 | 13 | 189 | 64 | 133 |
| 6/25/2016 | 43,541 | 1892.70 | 11 | 182 | 63 | 129 |
| 6/26/2016 | 43,251 | 1892.50 | 5 | 166 | 45 | 131 |
| 6/27/2016 | 42,993 | 1892.30 | 2 | 160 | 39 | 131 |
| 6/28/2016 | 42,736 | 1892.20 | -2 | 155 | 34 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/29/2016 | 42,464 | 1892.00 | 0 | 154 | 31 | 131 |
| 6/30/2016 | 42,226 | 1891.90 | 3 | 146 | 31 | 125 |
| 7/1/2016 | 41,972 | 1891.70 | -2 | 142 | 33 | 115 |
| 7/2/2016 | 41,736 | 1891.60 | -4 | 141 | 33 | 115 |
| 7/3/2016 | 41,483 | 1891.40 | -8 | 141 | 33 | 115 |
| 7/4/2016 | 41,215 | 1891.20 | -6 | 141 | 32 | 115 |
| 7/5/2016 | 40,949 | 1891.10 | -11 | 141 | 32 | 115 |
| 7/6/2016 | 40,699 | 1890.90 | -8 | 141 | 31 | 115 |
| 7/7/2016 | 40,435 | 1890.70 | -15 | 140 | 31 | 115 |
| 7/8/2016 | 40,172 | 1890.60 | -12 | 139 | 31 | 115 |
| 7/9/2016 | 39,894 | 1890.40 | -15 | 137 | 28 | 115 |
| 7/10/2016 | 39,648 | 1890.20 | -14 | 136 | 26 | 115 |
| 7/11/2016 | 39,403 | 1890.10 | -9 | 135 | 26 | 115 |
| 7/12/2016 | 39,158 | 1889.90 | 52 | 136 | 27 | 115 |
| 7/13/2016 | 38,914 | 1889.80 | -4 | 135 | 25 | 115 |
| 7/14/2016 | 38,655 | 1889.60 | -4 | 137 | 25 | 115 |
| 7/15/2016 | 38,412 | 1889.40 | -10 | 128 | 28 | 110 |
| 7/16/2016 | 38,230 | 1889.30 | 0 | 104 | 41 | 65 |
| 7/17/2016 | 38,049 | 1889.20 | -4 | 100 | 34 | 65 |
| 7/18/2016 | 37,853 | 1889.10 | -2 | 99 | 31 | 65 |
| 7/19/2016 | 37,688 | 1889.00 | -3 | 99 | 30 | 65 |
| 7/20/2016 | 37,539 | 1888.90 | 5 | 96 | 26 | 65 |
| 7/21/2016 | 37,315 | 1888.70 | -7 | 107 | 26 | 70 |
| 7/22/2016 | 37,122 | 1888.60 | -4 | 109 | 33 | 79 |
| 7/23/2016 | 36,929 | 1888.40 | -2 | 104 | 24 | 78 |
| 7/24/2016 | 36,736 | 1888.30 | -1 | 104 | 19 | 78 |
| 7/25/2016 | 36,559 | 1888.20 | -2 | 103 | 22 | 78 |
| 7/26/2016 | 36,353 | 1888.10 | -1 | 103 | 19 | 78 |
| 7/27/2016 | 36,163 | 1887.90 | -5 | 102 | 19 | 77 |
| 7/28/2016 | 35,973 | 1887.80 | 1 | 104 | 18 | 79 |
| 7/29/2016 | 35,770 | 1887.70 | -6 | 105 | 23 | 79 |
| 7/30/2016 | 35,552 | 1887.50 | -7 | 104 | 20 | 79 |
| 7/31/2016 | 35,349 | 1887.40 | -9 | 103 | 18 | 79 |
| 8/1/2016 | 35,148 | 1887.20 | -9 | 102 | 19 | 78 |
| 8/2/2016 | 34,947 | 1887.10 | -10 | 103 | 20 | 78 |
| 8/3/2016 | 34,747 | 1886.90 | -9 | 102 | 20 | 78 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/4/2016 | 34,547 | 1886.80 | -12 | 102 | 23 | 74 |
| 8/5/2016 | 34,335 | 1886.70 | -12 | 100 | 24 | 72 |
| 8/6/2016 | 34,151 | 1886.50 | -9 | 101 | 24 | 72 |
| 8/7/2016 | 33,925 | 1886.40 | -13 | 100 | 24 | 72 |
| 8/8/2016 | 33,743 | 1886.20 | -9 | 99 | 22 | 72 |
| 8/9/2016 | 33,561 | 1886.10 | -5 | 97 | 25 | 67 |
| 8/10/2016 | 33,366 | 1886.00 | -6 | 99 | 23 | 68 |
| 8/11/2016 | 33,171 | 1885.80 | -5 | 100 | 19 | 74 |
| 8/12/2016 | 32,992 | 1885.70 | -5 | 100 | 18 | 74 |
| 8/13/2016 | 32,799 | 1885.50 | -7 | 102 | 20 | 74 |
| 8/14/2016 | 32,606 | 1885.40 | -7 | 99 | 21 | 72 |
| 8/15/2016 | 32,435 | 1885.30 | -3 | 97 | 23 | 67 |
| 8/16/2016 | 32,246 | 1885.10 | -2 | 97 | 22 | 67 |
| 8/17/2016 | 32,074 | 1885.00 | -2 | 96 | 21 | 67 |
| 8/18/2016 | 31,898 | 1884.90 | 0 | 98 | 19 | 67 |
| 8/19/2016 | 31,709 | 1884.70 | -4 | 99 | 24 | 67 |
| 8/20/2016 | 31,522 | 1884.60 | -1 | 99 | 23 | 67 |
| 8/21/2016 | 31,347 | 1884.50 | -5 | 99 | 23 | 68 |
| 8/22/2016 | 31,159 | 1884.30 | -3 | 99 | 22 | 68 |
| 8/23/2016 | 30,944 | 1884.20 | -11 | 106 | 26 | 70 |
| 8/24/2016 | 30,717 | 1884.00 | -12 | 109 | 29 | 78 |
| 8/25/2016 | 30,517 | 1883.80 | -6 | 106 | 27 | 78 |
| 8/26/2016 | 30,318 | 1883.70 | -6 | 106 | 24 | 77 |
| 8/27/2016 | 30,107 | 1883.50 | -6 | 106 | 24 | 77 |
| 8/28/2016 | 29,910 | 1883.40 | -3 | 101 | 24 | 74 |
| 8/29/2016 | 29,739 | 1883.20 | -3 | 98 | 22 | 67 |
| 8/30/2016 | 29,556 | 1883.10 | 1 | 97 | 22 | 68 |
| 8/31/2016 | 29,387 | 1883.00 | 0 | 94 | 21 | 66 |
| 9/1/2016 | 29,219 | 1882.80 | 0 | 93 | 20 | 63 |
| 9/2/2016 | 29,039 | 1882.70 | -1 | 93 | 20 | 63 |
| 9/3/2016 | 28,884 | 1882.60 | -1 | 93 | 19 | 63 |
| 9/4/2016 | 28,706 | 1882.40 | -1 | 93 | 20 | 63 |
| 9/5/2016 | 28,540 | 1882.30 | 1 | 92 | 20 | 63 |
| 9/6/2016 | 28,376 | 1882.20 | 4 | 92 | 20 | 63 |
| 9/7/2016 | 28,212 | 1882.00 | 5 | 93 | 19 | 63 |
| 9/8/2016 | 28,061 | 1881.90 | 3 | 90 | 21 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/9/2016 | 27,910 | 1881.80 | 2 | 88 | 21 | 58 |
| 9/10/2016 | 27,748 | 1881.70 | 4 | 88 | 21 | 58 |
| 9/11/2016 | 27,587 | 1881.50 | -1 | 88 | 21 | 58 |
| 9/12/2016 | 27,415 | 1881.40 | -1 | 88 | 21 | 58 |
| 9/13/2016 | 27,267 | 1881.30 | 0 | 87 | 22 | 58 |
| 9/14/2016 | 27,108 | 1881.10 | 1 | 87 | 22 | 58 |
| 9/15/2016 | 26,962 | 1881.00 | 2 | 86 | 22 | 58 |
| 9/16/2016 | 26,805 | 1880.90 | 1 | 84 | 22 | 56 |
| 9/17/2016 | 26,648 | 1880.70 | 1 | 82 | 25 | 53 |
| 9/18/2016 | 26,515 | 1880.60 | 2 | 81 | 22 | 53 |
| 9/19/2016 | 26,360 | 1880.50 | 3 | 85 | 22 | 54 |
| 9/20/2016 | 26,194 | 1880.30 | -1 | 89 | 22 | 60 |
| 9/21/2016 | 26,005 | 1880.20 | -7 | 94 | 23 | 62 |
| 9/22/2016 | 25,818 | 1880.00 | -10 | 98 | 24 | 71 |
| 9/23/2016 | 25,643 | 1879.90 | 0 | 98 | 24 | 71 |
| 9/24/2016 | 25,468 | 1879.70 | 5 | 97 | 23 | 71 |
| 9/25/2016 | 25,307 | 1879.60 | 1 | 97 | 20 | 71 |
| 9/26/2016 | 25,135 | 1879.40 | 5 | 94 | 20 | 69 |
| 9/27/2016 | 24,976 | 1879.30 | 4 | 92 | 20 | 64 |
| 9/28/2016 | 24,817 | 1879.10 | 4 | 91 | 21 | 63 |
| 9/29/2016 | 24,659 | 1879.00 | -1 | 89 | 19 | 63 |
| 9/30/2016 | 24,502 | 1878.90 | -1 | 85 | 21 | 60 |
| 10/1/2016 | 24,350 | 1878.70 | 1 | 75 | 24 | 54 |
| 10/2/2016 | 24,220 | 1878.60 | 14 | 74 | 25 | 54 |
| 10/3/2016 | 24,113 | 1878.50 | 25 | 74 | 25 | 54 |
| 10/4/2016 | 23,997 | 1878.40 | 10 | 68 | 24 | 51 |
| 10/5/2016 | 23,881 | 1878.30 | 7 | 64 | 20 | 44 |
| 10/6/2016 | 23,775 | 1878.20 | 11 | 64 | 20 | 44 |
| 10/7/2016 | 23,671 | 1878.10 | 12 | 63 | 27 | 37 |
| 10/8/2016 | 23,566 | 1878.00 | 9 | 59 | 30 | 33 |
| 10/9/2016 | 23,483 | 1877.90 | 13 | 57 | 24 | 33 |
| 10/10/2016 | 23,379 | 1877.80 | 7 | 56 | 21 | 33 |
| 10/11/2016 | 23,286 | 1877.70 | 6 | 55 | 20 | 33 |
| 10/12/2016 | 23,187 | 1877.70 | 9 | 55 | 19 | 33 |
| 10/13/2016 | 23,124 | 1877.60 | 12 | 54 | 20 | 33 |
| 10/14/2016 | 23,198 | 1877.70 | 94 | 55 | 42 | 33 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/15/2016 | 23,261 | 1877.70 | 84 | 55 | 33 | 33 |
| 10/16/2016 | 23,377 | 1877.80 | 114 | 55 | 39 | 33 |
| 10/17/2016 | 23,579 | 1878.00 | 160 | 55 | 56 | 33 |
| 10/18/2016 | 23,632 | 1878.10 | 79 | 54 | 36 | 33 |
| 10/19/2016 | 23,621 | 1878.10 | 48 | 54 | 30 | 33 |
| 10/20/2016 | 23,579 | 1878.00 | 32 | 54 | 29 | 33 |
| 10/21/2016 | 23,536 | 1878.00 | 30 | 53 | 25 | 33 |
| 10/22/2016 | 23,493 | 1877.90 | 25 | 52 | 23 | 33 |
| 10/23/2016 | 23,430 | 1877.90 | 21 | 51 | 20 | 33 |
| 10/24/2016 | 23,462 | 1877.90 | 70 | 60 | 31 | 34 |
| 10/25/2016 | 23,728 | 1878.20 | 221 | 88 | 89 | 34 |
| 10/26/2016 | 23,856 | 1878.30 | 189 | 123 | 103 | 33 |
| 10/27/2016 | 24,213 | 1878.60 | 317 | 140 | 159 | 33 |
| 10/28/2016 | 24,532 | 1878.90 | 326 | 146 | 164 | 33 |
| 10/29/2016 | 25,022 | 1879.30 | 398 | 149 | 179 | 33 |
| 10/30/2016 | 26,472 | 1880.60 | 870 | 152 | 303 | 33 |
| 10/31/2016 | 27,893 | 1881.80 | 956 | 170 | 307 | 41 |
| 11/1/2016 | 29,719 | 1883.20 | 1,158 | 182 | 372 | 44 |
| 11/2/2016 | 30,238 | 1883.60 | 479 | 182 | 214 | 44 |
| 11/3/2016 | 30,421 | 1883.80 | 287 | 182 | 178 | 44 |
| 11/4/2016 | 30,460 | 1883.80 | 203 | 183 | 166 | 44 |
| 11/5/2016 | 30,434 | 1883.80 | 169 | 183 | 159 | 44 |
| 11/6/2016 | 30,382 | 1883.70 | 161 | 184 | 162 | 44 |
| 11/7/2016 | 30,277 | 1883.70 | 135 | 190 | 157 | 44 |
| 11/8/2016 | 30,134 | 1883.50 | 124 | 202 | 168 | 44 |
| 11/9/2016 | 29,952 | 1883.40 | 114 | 203 | 167 | 44 |
| 11/10/2016 | 29,758 | 1883.30 | 92 | 202 | 164 | 44 |
| 11/11/2016 | 29,578 | 1883.10 | 80 | 200 | 163 | 44 |
| 11/12/2016 | 29,398 | 1883.00 | 85 | 196 | 160 | 44 |
| 11/13/2016 | 29,220 | 1882.80 | 71 | 187 | 153 | 44 |
| 11/14/2016 | 29,042 | 1882.70 | 70 | 178 | 146 | 44 |
| 11/15/2016 | 28,865 | 1882.60 | 70 | 172 | 141 | 44 |
| 11/16/2016 | 28,701 | 1882.40 | 61 | 167 | 136 | 44 |
| 11/17/2016 | 28,538 | 1882.30 | 53 | 162 | 131 | 45 |
| 11/18/2016 | 28,376 | 1882.20 | 56 | 160 | 126 | 44 |
| 11/19/2016 | 29,849 | 1883.30 | 870 | 163 | 315 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/20/2016 | 31,307 | 1884.40 | 935 | 174 | 314 | 44 |
| 11/21/2016 | 31,979 | 1884.90 | 552 | 193 | 245 | 45 |
| 11/22/2016 | 32,278 | 1885.20 | 356 | 203 | 209 | 45 |
| 11/23/2016 | 32,689 | 1885.50 | 410 | 205 | 270 | 45 |
| 11/24/2016 | 32,936 | 1885.60 | 318 | 204 | 230 | 45 |
| 11/25/2016 | 33,061 | 1885.70 | 266 | 203 | 207 | 45 |
| 11/26/2016 | 33,925 | 1886.40 | 613 | 208 | 327 | 45 |
| 11/27/2016 | 35,364 | 1887.40 | 971 | 209 | 546 | 45 |
| 11/28/2016 | 36,515 | 1888.20 | 799 | 209 | 370 | 45 |
| 11/29/2016 | 37,167 | 1888.60 | 552 | 209 | 295 | 45 |
| 11/30/2016 | 37,554 | 1888.90 | 408 | 209 | 252 | 45 |
| 12/1/2016 | 37,793 | 1889.00 | 327 | 209 | 226 | 45 |
| 12/2/2016 | 37,928 | 1889.10 | 270 | 209 | 208 | 45 |
| 12/3/2016 | 38,004 | 1889.20 | 231 | 209 | 196 | 45 |
| 12/4/2016 | 38,049 | 1889.20 | 222 | 209 | 190 | 45 |
| 12/5/2016 | 38,079 | 1889.20 | 210 | 210 | 187 | 45 |
| 12/6/2016 | 38,064 | 1889.20 | 189 | 209 | 184 | 45 |
| 12/7/2016 | 38,155 | 1889.30 | 241 | 209 | 184 | 45 |
| 12/8/2016 | 38,823 | 1889.70 | 531 | 211 | 247 | 45 |
| 12/9/2016 | 40,334 | 1890.70 | 1,004 | 212 | 298 | 45 |
| 12/10/2016 | 55,176 | 1899.30 | 7,952 | 271 | 2,274 | 44 |
| 12/11/2016 | 56,515 | 1900.10 | 2,816 | 1,705 | 2,107 | 44 |
| 12/12/2016 | 55,928 | 1899.70 | 1,573 | 1,559 | 1,980 | 67 |
| 12/13/2016 | 55,495 | 1899.50 | 1,118 | 1,113 | 1,231 | 118 |
| 12/14/2016 | 55,965 | 1899.80 | 1,612 | 1,171 | 1,127 | 189 |
| 12/15/2016 | 62,650 | 1903.20 | 11,412 | 8,592 | 9,784 | 55 |
| 12/16/2016 | 58,460 | 1901.10 | 5,454 | 7,537 | 9,481 | 4 |
| 12/17/2016 | 56,801 | 1900.20 | 2,343 | 2,919 | 3,760 | 133 |
| 12/18/2016 | 56,003 | 1899.80 | 1,606 | 1,703 | 1,992 | 200 |
| 12/19/2016 | 55,532 | 1899.50 | 1,239 | 1,230 | 1,291 | 200 |
| 12/20/2016 | 55,213 | 1899.40 | 1,024 | 1,000 | 934 | 200 |
| 12/21/2016 | 54,970 | 1899.20 | 872 | 846 | 748 | 199 |
| 12/22/2016 | 54,803 | 1899.10 | 741 | 718 | 613 | 200 |
| 12/23/2016 | 55,645 | 1899.60 | 1,453 | 901 | 845 | 200 |
| 12/24/2016 | 55,326 | 1899.40 | 1,193 | 1,113 | 1,107 | 200 |
| 12/25/2016 | 55,026 | 1899.30 | 890 | 895 | 825 | 200 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/26/2016 | 54,803 | 1899.10 | 720 | 725 | 645 | 200 |
| 12/27/2016 | 54,635 | 1899.00 | 615 | 611 | 516 | 200 |
| 12/28/2016 | 54,524 | 1899.00 | 548 | 540 | 433 | 200 |
| 12/29/2016 | 54,431 | 1898.90 | 491 | 485 | 369 | 200 |
| 12/30/2016 | 54,339 | 1898.90 | 448 | 445 | 320 | 200 |
| 12/31/2016 | 54,246 | 1898.80 | 419 | 418 | 280 | 200 |
| 1/1/2017 | 54,172 | 1898.80 | 391 | 397 | 248 | 200 |
| 1/2/2017 | 54,154 | 1898.80 | 397 | 384 | 233 | 201 |
| 1/3/2017 | 55,045 | 1899.30 | 845 | 418 | 306 | 202 |
| 1/4/2017 | 56,915 | 1900.30 | 3,401 | 2,136 | 2,013 | 206 |
| 1/5/2017 | 56,060 | 1899.80 | 1,386 | 1,827 | 1,746 | 207 |
| 1/6/2017 | 55,551 | 1899.50 | 873 | 1,250 | 1,165 | 201 |
| 1/7/2017 | 56,820 | 1900.20 | 1,877 | 1,413 | 1,300 | 201 |
| 1/8/2017 | 66,599 | 1905.20 | 15,618 | 12,062 | 10,499 | 89 |
| 1/9/2017 | 61,656 | 1902.70 | 11,521 | 14,155 | 15,188 | 1 |
| 1/10/2017 | 68,262 | 1906.00 | 19,834 | 18,208 | 18,006 | 1 |
| 1/11/2017 | 61,374 | 1902.60 | 13,329 | 16,735 | 18,766 | 1 |
| 1/12/2017 | 58,831 | 1901.30 | 4,676 | 6,211 | 8,774 | 1 |
| 1/13/2017 | 57,549 | 1900.60 | 2,701 | 3,577 | 5,308 | 25 |
| 1/14/2017 | 56,858 | 1900.20 | 1,950 | 2,509 | 3,700 | 48 |
| 1/15/2017 | 56,344 | 1900.00 | 1,526 | 1,930 | 2,676 | 43 |
| 1/16/2017 | 56,022 | 1899.80 | 1,224 | 1,541 | 2,112 | 43 |
| 1/17/2017 | 55,758 | 1899.60 | 1,063 | 1,315 | 1,745 | 43 |
| 1/18/2017 | 58,577 | 1901.10 | 2,712 | 1,683 | 2,247 | 43 |
| 1/19/2017 | 58,752 | 1901.20 | 4,618 | 4,655 | 6,393 | 43 |
| 1/20/2017 | 59,460 | 1901.60 | 5,092 | 5,069 | 6,706 | 30 |
| 1/21/2017 | 59,007 | 1901.40 | 4,393 | 4,895 | 6,949 | 20 |
| 1/22/2017 | 58,674 | 1901.20 | 4,359 | 4,787 | 6,679 | 43 |
| 1/23/2017 | 57,544 | 1900.60 | 2,657 | 3,457 | 4,890 | 45 |
| 1/24/2017 | 56,819 | 1900.20 | 1,883 | 2,447 | 3,467 | 79 |
| 1/25/2017 | 56,365 | 1900.00 | 1,457 | 1,858 | 2,497 | 105 |
| 1/26/2017 | 56,085 | 1899.80 | 1,195 | 1,497 | 1,992 | 106 |
| 1/27/2017 | 55,871 | 1899.70 | 1,125 | 1,304 | 1,710 | 105 |
| 1/28/2017 | 55,758 | 1899.60 | 1,028 | 1,200 | 1,526 | 106 |
| 1/29/2017 | 55,701 | 1899.60 | 1,023 | 1,155 | 1,413 | 105 |
| 1/30/2017 | 55,701 | 1899.60 | 1,023 | 1,135 | 1,370 | 105 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/31/2017 | 55,739 | 1899.60 | 1,052 | 1,146 | 1,369 | 105 |
| 2/1/2017 | 55,683 | 1899.60 | 1,007 | 1,136 | 1,358 | 105 |
| 2/2/2017 | 56,022 | 1899.80 | 1,223 | 1,182 | 1,406 | 105 |
| 2/3/2017 | 58,577 | 1901.10 | 3,555 | 2,543 | 3,066 | 105 |
| 2/4/2017 | 58,988 | 1901.40 | 4,323 | 4,366 | 5,446 | 105 |
| 2/5/2017 | 58,596 | 1901.10 | 3,538 | 4,022 | 5,193 | 105 |
| 2/6/2017 | 59,697 | 1901.70 | 5,133 | 4,883 | 6,250 | 70 |
| 2/7/2017 | 64,336 | 1904.10 | 19,373 | 18,165 | 18,233 | 3 |
| 2/8/2017 | 62,854 | 1903.30 | 9,996 | 11,291 | 12,874 | 1 |
| 2/9/2017 | 65,693 | 1904.70 | 14,495 | 13,993 | 14,630 | 1 |
| 2/10/2017 | 61,253 | 1902.50 | 9,840 | 12,304 | 14,165 | 1 |
| 2/11/2017 | 59,105 | 1901.40 | 4,650 | 6,034 | 7,986 | 13 |
| 2/12/2017 | 57,974 | 1900.80 | 2,977 | 3,833 | 5,097 | 44 |
| 2/13/2017 | 57,299 | 1900.50 | 2,263 | 2,876 | 3,663 | 65 |
| 2/14/2017 | 56,839 | 1900.20 | 1,845 | 2,325 | 2,655 | 144 |
| 2/15/2017 | 56,534 | 1900.10 | 1,598 | 1,959 | 2,127 | 172 |
| 2/16/2017 | 58,090 | 1900.90 | 3,439 | 2,944 | 3,504 | 172 |
| 2/17/2017 | 58,479 | 1901.10 | 3,205 | 3,256 | 3,922 | 172 |
| 2/18/2017 | 58,323 | 1901.00 | 3,773 | 4,060 | 4,705 | 172 |
| 2/19/2017 | 58,499 | 1901.10 | 3,411 | 3,578 | 4,326 | 172 |
| 2/20/2017 | 64,585 | 1904.20 | 8,713 | 6,552 | 7,226 | 35 |
| 2/21/2017 | 62,223 | 1903.00 | 13,153 | 14,616 | 15,442 | 1 |
| 2/22/2017 | 59,558 | 1901.60 | 5,445 | 7,067 | 8,491 | 0 |
| 2/23/2017 | 58,265 | 1901.00 | 3,356 | 4,237 | 5,149 | 43 |
| 2/24/2017 | 57,510 | 1900.60 | 2,516 | 3,097 | 3,726 | 44 |
| 2/25/2017 | 56,992 | 1900.30 | 2,062 | 2,492 | 2,838 | 66 |
| 2/26/2017 | 56,648 | 1900.10 | 1,764 | 2,093 | 2,265 | 85 |
| 2/27/2017 | 56,344 | 1900.00 | 1,508 | 1,804 | 1,940 | 85 |
| 2/28/2017 | 56,079 | 1899.80 | 1,287 | 1,541 | 1,699 | 85 |
| 3/1/2017 | 55,852 | 1899.70 | 1,139 | 1,323 | 1,472 | 85 |
| 3/2/2017 | 55,701 | 1899.60 | 1,056 | 1,175 | 1,303 | 84 |
| 3/3/2017 | 55,589 | 1899.60 | 994 | 1,072 | 1,185 | 83 |
| 3/4/2017 | 55,796 | 1899.70 | 1,131 | 1,048 | 1,169 | 85 |
| 3/5/2017 | 55,965 | 1899.80 | 1,348 | 1,330 | 1,530 | 86 |
| 3/6/2017 | 55,947 | 1899.70 | 1,192 | 1,276 | 1,474 | 86 |
| 3/7/2017 | 55,890 | 1899.70 | 1,200 | 1,281 | 1,510 | 72 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/8/2017 | 55,984 | 1899.80 | 1,247 | 1,278 | 1,537 | 44 |
| 3/9/2017 | 56,060 | 1899.80 | 1,351 | 1,392 | 1,649 | 45 |
| 3/10/2017 | 56,003 | 1899.80 | 1,294 | 1,410 | 1,645 | 45 |
| 3/11/2017 | 55,890 | 1899.70 | 1,204 | 1,337 | 1,542 | 45 |
| 3/12/2017 | 55,796 | 1899.70 | 1,132 | 1,253 | 1,426 | 44 |
| 3/13/2017 | 55,664 | 1899.60 | 1,061 | 1,154 | 1,302 | 44 |
| 3/14/2017 | 55,570 | 1899.50 | 1,014 | 1,079 | 1,205 | 44 |
| 3/15/2017 | 55,589 | 1899.60 | 987 | 980 | 1,109 | 44 |
| 3/16/2017 | 55,645 | 1899.60 | 964 | 898 | 1,005 | 43 |
| 3/17/2017 | 55,551 | 1899.50 | 891 | 933 | 1,037 | 44 |
| 3/18/2017 | 55,513 | 1899.50 | 893 | 884 | 981 | 44 |
| 3/19/2017 | 55,476 | 1899.50 | 882 | 871 | 965 | 44 |
| 3/20/2017 | 55,570 | 1899.50 | 909 | 855 | 933 | 44 |
| 3/21/2017 | 56,363 | 1900.00 | 1,619 | 1,281 | 1,267 | 45 |
| 3/22/2017 | 57,896 | 1900.80 | 1,876 | 1,140 | 1,463 | 44 |
| 3/23/2017 | 59,954 | 1901.80 | 1,325 | 228 | 445 | 44 |
| 3/24/2017 | 64,544 | 1904.20 | 2,577 | 237 | 667 | 44 |
| 3/25/2017 | 67,941 | 1905.80 | 2,089 | 246 | 642 | 44 |
| 3/26/2017 | 69,081 | 1906.40 | 1,641 | 1,082 | 1,045 | 44 |
| 3/27/2017 | 69,124 | 1906.40 | 1,550 | 1,624 | 1,760 | 68 |
| 3/28/2017 | 69,168 | 1906.40 | 1,264 | 1,297 | 1,334 | 80 |
| 3/29/2017 | 68,951 | 1906.30 | 1,124 | 1,299 | 1,322 | 80 |
| 3/30/2017 | 69,146 | 1906.40 | 1,200 | 1,125 | 1,191 | 59 |
| 3/31/2017 | 69,319 | 1906.50 | 1,050 | 936 | 1,011 | 45 |
| 4/1/2017 | 69,580 | 1906.60 | 960 | 787 | 902 | 45 |
| 4/2/2017 | 69,754 | 1906.70 | 906 | 756 | 822 | 45 |
| 4/3/2017 | 69,602 | 1906.60 | 830 | 914 | 928 | 44 |
| 4/4/2017 | 68,800 | 1906.20 | 718 | 1,160 | 1,097 | 44 |
| 4/5/2017 | 68,112 | 1905.90 | 702 | 1,100 | 1,135 | 44 |
| 4/6/2017 | 69,103 | 1906.40 | 1,401 | 987 | 1,048 | 44 |
| 4/7/2017 | 68,348 | 1906.00 | 2,936 | 3,530 | 3,684 | 44 |
| 4/8/2017 | 68,520 | 1906.10 | 2,127 | 2,145 | 2,562 | 44 |
| 4/9/2017 | 68,995 | 1906.30 | 1,553 | 1,371 | 1,526 | 43 |
| 4/10/2017 | 69,103 | 1906.40 | 1,312 | 1,327 | 1,441 | 43 |
| 4/11/2017 | 69,298 | 1906.50 | 1,341 | 1,309 | 1,404 | 43 |
| 4/12/2017 | 69,493 | 1906.50 | 1,498 | 1,522 | 1,558 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/13/2017 | 69,776 | 1906.70 | 2,273 | 2,292 | 2,384 | 44 |
| 4/14/2017 | 68,951 | 1906.30 | 1,646 | 2,242 | 2,407 | 44 |
| 4/15/2017 | 68,370 | 1906.00 | 1,390 | 1,832 | 1,871 | 88 |
| 4/16/2017 | 68,649 | 1906.10 | 1,368 | 1,279 | 1,447 | 88 |
| 4/17/2017 | 68,995 | 1906.30 | 1,293 | 1,136 | 1,209 | 88 |
| 4/18/2017 | 69,298 | 1906.50 | 1,282 | 1,154 | 1,245 | 88 |
| 4/19/2017 | 69,493 | 1906.50 | 1,225 | 1,160 | 1,219 | 89 |
| 4/20/2017 | 70,080 | 1906.80 | 1,444 | 1,178 | 1,277 | 91 |
| 4/21/2017 | 70,233 | 1906.90 | 1,189 | 1,132 | 1,210 | 90 |
| 4/22/2017 | 70,582 | 1907.10 | 1,122 | 895 | 1,010 | 89 |
| 4/23/2017 | 71,021 | 1907.30 | 1,026 | 764 | 857 | 90 |
| 4/24/2017 | 71,175 | 1907.30 | 966 | 875 | 914 | 91 |
| 4/25/2017 | 71,131 | 1907.30 | 898 | 864 | 905 | 95 |
| 4/26/2017 | 71,219 | 1907.40 | 878 | 806 | 833 | 126 |
| 4/27/2017 | 71,550 | 1907.50 | 855 | 640 | 753 | 91 |
| 4/28/2017 | 72,081 | 1907.80 | 744 | 444 | 550 | 90 |
| 4/29/2017 | 72,502 | 1908.00 | 676 | 431 | 500 | 90 |
| 4/30/2017 | 72,769 | 1908.10 | 650 | 483 | 542 | 90 |
| 5/1/2017 | 72,880 | 1908.10 | 650 | 568 | 582 | 90 |
| 5/2/2017 | 72,880 | 1908.10 | 692 | 639 | 668 | 90 |
| 5/3/2017 | 72,880 | 1908.10 | 687 | 625 | 646 | 90 |
| 5/4/2017 | 72,902 | 1908.20 | 662 | 612 | 640 | 90 |
| 5/5/2017 | 72,880 | 1908.10 | 579 | 573 | 597 | 90 |
| 5/6/2017 | 72,969 | 1908.20 | 510 | 488 | 539 | 90 |
| 5/7/2017 | 73,170 | 1908.30 | 465 | 364 | 389 | 90 |
| 5/8/2017 | 73,326 | 1908.30 | 429 | 378 | 380 | 90 |
| 5/9/2017 | 73,326 | 1908.30 | 407 | 420 | 421 | 89 |
| 5/10/2017 | 73,326 | 1908.30 | 385 | 405 | 419 | 89 |
| 5/11/2017 | 73,393 | 1908.40 | 371 | 359 | 364 | 89 |
| 5/12/2017 | 73,461 | 1908.40 | 348 | 337 | 324 | 90 |
| 5/13/2017 | 73,461 | 1908.40 | 314 | 335 | 323 | 90 |
| 5/14/2017 | 73,461 | 1908.40 | 325 | 346 | 327 | 91 |
| 5/15/2017 | 73,393 | 1908.40 | 303 | 364 | 306 | 130 |
| 5/16/2017 | 73,304 | 1908.30 | 292 | 361 | 303 | 130 |
| 5/17/2017 | 73,214 | 1908.30 | 295 | 354 | 291 | 130 |
| 5/18/2017 | 73,103 | 1908.20 | 284 | 355 | 291 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/19/2017 | 72,924 | 1908.20 | 270 | 356 | 287 | 130 |
| 5/20/2017 | 72,769 | 1908.10 | 255 | 357 | 285 | 130 |
| 5/21/2017 | 72,591 | 1908.00 | 251 | 356 | 282 | 130 |
| 5/22/2017 | 72,147 | 1907.80 | 227 | 474 | 337 | 129 |
| 5/23/2017 | 71,461 | 1907.50 | 212 | 605 | 543 | 129 |
| 5/24/2017 | 70,823 | 1907.20 | 204 | 566 | 493 | 129 |
| 5/25/2017 | 70,233 | 1906.90 | 195 | 527 | 459 | 129 |
| 5/26/2017 | 69,754 | 1906.70 | 190 | 469 | 396 | 130 |
| 5/27/2017 | 69,341 | 1906.50 | 188 | 426 | 342 | 130 |
| 5/28/2017 | 68,930 | 1906.30 | 182 | 404 | 309 | 130 |
| 5/29/2017 | 68,520 | 1906.10 | 170 | 395 | 291 | 131 |
| 5/30/2017 | 68,112 | 1905.90 | 159 | 401 | 293 | 131 |
| 5/31/2017 | 67,685 | 1905.70 | 154 | 387 | 283 | 131 |
| 6/1/2017 | 67,300 | 1905.50 | 151 | 367 | 252 | 131 |
| 6/2/2017 | 66,981 | 1905.30 | 149 | 322 | 213 | 131 |
| 6/3/2017 | 66,726 | 1905.20 | 140 | 282 | 168 | 130 |
| 6/4/2017 | 66,472 | 1905.10 | 125 | 269 | 158 | 129 |
| 6/5/2017 | 66,240 | 1905.00 | 125 | 253 | 147 | 129 |
| 6/6/2017 | 66,008 | 1904.90 | 112 | 239 | 135 | 129 |
| 6/7/2017 | 65,777 | 1904.80 | 114 | 230 | 126 | 130 |
| 6/8/2017 | 65,714 | 1904.70 | 182 | 225 | 128 | 130 |
| 6/9/2017 | 65,525 | 1904.60 | 122 | 214 | 113 | 130 |
| 6/10/2017 | 65,357 | 1904.60 | 107 | 206 | 101 | 131 |
| 6/11/2017 | 65,169 | 1904.50 | 113 | 204 | 99 | 131 |
| 6/12/2017 | 65,002 | 1904.40 | 119 | 204 | 97 | 130 |
| 6/13/2017 | 64,856 | 1904.30 | 115 | 197 | 93 | 130 |
| 6/14/2017 | 64,689 | 1904.20 | 104 | 191 | 81 | 130 |
| 6/15/2017 | 64,523 | 1904.10 | 95 | 189 | 79 | 130 |
| 6/16/2017 | 64,336 | 1904.10 | 89 | 185 | 73 | 130 |
| 6/17/2017 | 64,150 | 1904.00 | 83 | 180 | 68 | 129 |
| 6/18/2017 | 63,964 | 1903.90 | 79 | 178 | 64 | 129 |
| 6/19/2017 | 63,778 | 1903.80 | 73 | 178 | 64 | 130 |
| 6/20/2017 | 63,573 | 1903.70 | 67 | 178 | 62 | 130 |
| 6/21/2017 | 63,367 | 1903.60 | 57 | 174 | 61 | 130 |
| 6/22/2017 | 63,038 | 1903.40 | 58 | 231 | 92 | 142 |
| 6/23/2017 | 62,813 | 1903.30 | 40 | 172 | 62 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/24/2017 | 62,568 | 1903.20 | 39 | 171 | 55 | 131 |
| 6/25/2017 | 62,324 | 1903.10 | 35 | 171 | 54 | 131 |
| 6/26/2017 | 62,040 | 1902.90 | 22 | 169 | 55 | 130 |
| 6/27/2017 | 61,798 | 1902.80 | 25 | 167 | 56 | 131 |
| 6/28/2017 | 61,535 | 1902.70 | 21 | 167 | 55 | 131 |
| 6/29/2017 | 61,253 | 1902.50 | 17 | 166 | 54 | 131 |
| 6/30/2017 | 60,992 | 1902.40 | 19 | 166 | 53 | 131 |
| 7/1/2017 | 60,731 | 1902.20 | 18 | 166 | 53 | 130 |
| 7/2/2017 | 60,451 | 1902.10 | 17 | 166 | 52 | 130 |
| 7/3/2017 | 60,172 | 1902.00 | 15 | 166 | 51 | 130 |
| 7/4/2017 | 59,895 | 1901.80 | 8 | 166 | 52 | 130 |
| 7/5/2017 | 59,598 | 1901.70 | 6 | 165 | 50 | 130 |
| 7/6/2017 | 59,302 | 1901.50 | 7 | 169 | 51 | 130 |
| 7/7/2017 | 59,007 | 1901.40 | 6 | 172 | 54 | 130 |
| 7/8/2017 | 58,674 | 1901.20 | 2 | 171 | 54 | 130 |
| 7/9/2017 | 58,382 | 1901.00 | -2 | 171 | 52 | 130 |
| 7/10/2017 | 58,071 | 1900.90 | 2 | 171 | 52 | 130 |
| 7/11/2017 | 57,761 | 1900.70 | 2 | 171 | 52 | 130 |
| 7/12/2017 | 57,452 | 1900.50 | 3 | 171 | 52 | 130 |
| 7/13/2017 | 57,145 | 1900.40 | 0 | 171 | 48 | 130 |
| 7/14/2017 | 56,820 | 1900.20 | -6 | 166 | 47 | 130 |
| 7/15/2017 | 56,496 | 1900.00 | -7 | 163 | 44 | 130 |
| 7/16/2017 | 56,192 | 1899.90 | -11 | 163 | 44 | 130 |
| 7/17/2017 | 55,890 | 1899.70 | -12 | 162 | 44 | 130 |
| 7/18/2017 | 55,551 | 1899.50 | -18 | 162 | 44 | 130 |
| 7/19/2017 | 55,232 | 1899.40 | -14 | 162 | 43 | 130 |
| 7/20/2017 | 54,933 | 1899.20 | -12 | 162 | 43 | 130 |
| 7/21/2017 | 54,617 | 1899.00 | -10 | 162 | 42 | 130 |
| 7/22/2017 | 54,302 | 1898.90 | -8 | 161 | 42 | 130 |
| 7/23/2017 | 53,987 | 1898.70 | -10 | 161 | 41 | 130 |
| 7/24/2017 | 53,674 | 1898.50 | -14 | 163 | 42 | 130 |
| 7/25/2017 | 53,362 | 1898.30 | -18 | 164 | 44 | 130 |
| 7/26/2017 | 53,014 | 1898.20 | -13 | 164 | 44 | 130 |
| 7/27/2017 | 52,704 | 1898.00 | -13 | 163 | 43 | 130 |
| 7/28/2017 | 52,360 | 1897.80 | -20 | 163 | 43 | 130 |
| 7/29/2017 | 52,053 | 1897.60 | -17 | 163 | 43 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/30/2017 | 51,728 | 1897.40 | -15 | 164 | 43 | 130 |
| 7/31/2017 | 51,404 | 1897.30 | -14 | 164 | 42 | 130 |
| 8/1/2017 | 51,082 | 1897.10 | -13 | 164 | 42 | 130 |
| 8/2/2017 | 50,743 | 1896.90 | -18 | 164 | 42 | 130 |
| 8/3/2017 | 50,406 | 1896.70 | -22 | 164 | 41 | 129 |
| 8/4/2017 | 50,070 | 1896.50 | -15 | 166 | 43 | 130 |
| 8/5/2017 | 49,735 | 1896.30 | -19 | 165 | 42 | 130 |
| 8/6/2017 | 49,401 | 1896.10 | -19 | 165 | 42 | 130 |
| 8/7/2017 | 49,069 | 1895.90 | -17 | 166 | 42 | 130 |
| 8/8/2017 | 48,738 | 1895.70 | -17 | 166 | 41 | 130 |
| 8/9/2017 | 48,427 | 1895.60 | -8 | 166 | 40 | 130 |
| 8/10/2017 | 48,115 | 1895.40 | -7 | 166 | 40 | 130 |
| 8/11/2017 | 47,787 | 1895.20 | -6 | 166 | 39 | 131 |
| 8/12/2017 | 47,461 | 1895.00 | -15 | 166 | 39 | 132 |
| 8/13/2017 | 47,120 | 1894.80 | -17 | 166 | 40 | 131 |
| 8/14/2017 | 46,797 | 1894.60 | -12 | 167 | 40 | 131 |
| 8/15/2017 | 46,475 | 1894.40 | -10 | 167 | 40 | 130 |
| 8/16/2017 | 46,154 | 1894.20 | -15 | 166 | 40 | 130 |
| 8/17/2017 | 45,818 | 1894.00 | -13 | 170 | 44 | 130 |
| 8/18/2017 | 45,484 | 1893.80 | -12 | 171 | 43 | 130 |
| 8/19/2017 | 45,151 | 1893.60 | -13 | 169 | 41 | 130 |
| 8/20/2017 | 44,803 | 1893.40 | -16 | 168 | 40 | 130 |
| 8/21/2017 | 44,472 | 1893.20 | -12 | 169 | 40 | 130 |
| 8/22/2017 | 44,159 | 1893.00 | -10 | 169 | 40 | 130 |
| 8/23/2017 | 43,816 | 1892.80 | -12 | 168 | 40 | 130 |
| 8/24/2017 | 43,474 | 1892.60 | -16 | 168 | 39 | 130 |
| 8/25/2017 | 43,132 | 1892.40 | -13 | 167 | 39 | 130 |
| 8/26/2017 | 42,793 | 1892.20 | -17 | 169 | 39 | 130 |
| 8/27/2017 | 42,471 | 1892.00 | -14 | 169 | 40 | 130 |
| 8/28/2017 | 42,135 | 1891.80 | -13 | 168 | 39 | 129 |
| 8/29/2017 | 41,800 | 1891.60 | -21 | 168 | 38 | 129 |
| 8/30/2017 | 41,450 | 1891.40 | -21 | 167 | 38 | 130 |
| 8/31/2017 | 41,118 | 1891.20 | -13 | 167 | 38 | 130 |
| 9/1/2017 | 40,787 | 1891.00 | -19 | 169 | 38 | 129 |
| 9/2/2017 | 40,428 | 1890.70 | -12 | 172 | 41 | 129 |
| 9/3/2017 | 40,099 | 1890.50 | -12 | 171 | 40 | 130 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/4/2017 | 39,753 | 1890.30 | -17 | 172 | 40 | 130 |
| 9/5/2017 | 39,409 | 1890.10 | -12 | 172 | 40 | 130 |
| 9/6/2017 | 39,068 | 1889.90 | -12 | 173 | 40 | 131 |
| 9/7/2017 | 38,728 | 1889.60 | -12 | 173 | 40 | 131 |
| 9/8/2017 | 38,450 | 1889.50 | -6 | 172 | 39 | 131 |
| 9/9/2017 | 38,053 | 1889.20 | -11 | 171 | 39 | 131 |
| 9/10/2017 | 37,718 | 1889.00 | -12 | 170 | 38 | 131 |
| 9/11/2017 | 37,385 | 1888.70 | -9 | 169 | 39 | 131 |
| 9/12/2017 | 37,039 | 1888.50 | -18 | 169 | 40 | 131 |
| 9/13/2017 | 36,709 | 1888.30 | -11 | 168 | 40 | 131 |
| 9/14/2017 | 36,352 | 1888.10 | -20 | 168 | 40 | 131 |
| 9/15/2017 | 36,012 | 1887.80 | -14 | 167 | 39 | 131 |
| 9/16/2017 | 35,717 | 1887.60 | -18 | 147 | 59 | 90 |
| 9/17/2017 | 35,452 | 1887.40 | -19 | 144 | 54 | 91 |
| 9/18/2017 | 35,131 | 1887.20 | -25 | 143 | 52 | 91 |
| 9/19/2017 | 34,840 | 1887.00 | -24 | 140 | 49 | 91 |
| 9/20/2017 | 34,580 | 1886.80 | -12 | 138 | 48 | 91 |
| 9/21/2017 | 34,307 | 1886.60 | -20 | 135 | 45 | 92 |
| 9/22/2017 | 34,048 | 1886.50 | -10 | 133 | 43 | 92 |
| 9/23/2017 | 33,805 | 1886.30 | -15 | 129 | 40 | 91 |
| 9/24/2017 | 33,564 | 1886.10 | -13 | 131 | 40 | 91 |
| 9/25/2017 | 33,309 | 1885.90 | -15 | 132 | 42 | 91 |
| 9/26/2017 | 33,056 | 1885.70 | -13 | 132 | 41 | 91 |
| 9/27/2017 | 32,804 | 1885.60 | -13 | 132 | 41 | 91 |
| 9/28/2017 | 32,553 | 1885.40 | -15 | 132 | 41 | 91 |
| 9/29/2017 | 32,290 | 1885.20 | -17 | 132 | 41 | 91 |
| 9/30/2017 | 33,483 | 1886.00 | -14 | 132 | 41 | 91 |
| 10/1/2017 | 33,216 | 1885.90 | -19 | 118 | 41 | 91 |
| 10/2/2017 | 32,951 | 1885.70 | -15 | 118 | 41 | 92 |
| 10/3/2017 | 32,687 | 1885.50 | -24 | 117 | 41 | 91 |
| 10/4/2017 | 32,424 | 1885.30 | -16 | 117 | 42 | 90 |
| 10/5/2017 | 32,163 | 1885.10 | -18 | 117 | 42 | 90 |
| 10/6/2017 | 31,916 | 1884.90 | -15 | 116 | 42 | 90 |
| 10/7/2017 | 31,658 | 1884.70 | -15 | 116 | 41 | 90 |
| 10/8/2017 | 31,387 | 1884.50 | -23 | 117 | 41 | 90 |
| 10/9/2017 | 31,397 | 1884.50 | -20 | 119 | 47 | 78 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/10/2017 | 30,879 | 1884.10 | -19 | 119 | 43 | 50 |
| 10/11/2017 | 30,708 | 1884.00 | -16 | 122 | 39 | 92 |
| 10/12/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/13/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/14/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/15/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/16/2017 | 30,708 | 1884.00 | -12 | 126 | 38 | 91 |
| 10/17/2017 | 29,557 | 1883.10 | -11 | 94 | 42 | 91 |
| 10/18/2017 | 28,999 | 1882.70 | -20 | 84 | 56 | 47 |
| 10/19/2017 | 28,861 | 1882.50 | 4 | 84 | 57 | 45 |
| 10/20/2017 | 28,698 | 1882.40 | -2 | 79 | 58 | 45 |
| 10/21/2017 | 28,561 | 1882.30 | -4 | 70 | 49 | 45 |
| 10/22/2017 | 28,412 | 1882.20 | -9 | 66 | 45 | 45 |
| 10/23/2017 | 28,276 | 1882.10 | 7 | 78 | 43 | 45 |
| 10/24/2017 | 28,165 | 1882.00 | 14 | 83 | 41 | 44 |
| 10/25/2017 | 28,017 | 1881.90 | 14 | 84 | 41 | 44 |
| 10/26/2017 | 27,895 | 1881.80 | 13 | 84 | 41 | 41 |
| 10/27/2017 | 27,748 | 1881.60 | 9 | 85 | 41 | 45 |
| 10/28/2017 | 27,614 | 1881.50 | 19 | 89 | 40 | 45 |
| 10/29/2017 | 27,468 | 1881.40 | 17 | 90 | 41 | 45 |
| 10/30/2017 | 27,299 | 1881.30 | 16 | 90 | 41 | 45 |
| 10/31/2017 | 27,191 | 1881.20 | 15 | 90 | 41 | 45 |
| 11/1/2017 | 27,059 | 1881.10 | 17 | 90 | 41 | 45 |
| 11/2/2017 | 26,916 | 1881.00 | 16 | 90 | 41 | 45 |
| 11/3/2017 | 26,844 | 1880.90 | 50 | 91 | 45 | 45 |
| 11/4/2017 | 26,725 | 1880.80 | 34 | 91 | 45 | 45 |
| 11/5/2017 | 26,630 | 1880.70 | 32 | 91 | 43 | 45 |
| 11/6/2017 | 26,500 | 1880.60 | 27 | 91 | 43 | 45 |
| 11/7/2017 | 26,477 | 1880.60 | 76 | 92 | 42 | 45 |
| 11/8/2017 | 26,382 | 1880.50 | 58 | 92 | 47 | 45 |
| 11/9/2017 | 26,500 | 1880.60 | 151 | 91 | 59 | 45 |
| 11/10/2017 | 26,701 | 1880.80 | 212 | 91 | 61 | 45 |
| 11/11/2017 | 26,618 | 1880.70 | 110 | 155 | 71 | 71 |
| 11/12/2017 | 26,335 | 1880.50 | 61 | 216 | 67 | 149 |
| 11/13/2017 | 26,008 | 1880.20 | 87 | 250 | 95 | 150 |
| 11/14/2017 | 25,672 | 1879.90 | 139 | 313 | 225 | 95 |

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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/15/2017 | 25,857 | 1880.10 | 368 | 319 | 331 | 71 |
| 11/16/2017 | 26,559 | 1880.70 | 711 | 302 | 437 | 57 |
| 11/17/2017 | 26,595 | 1880.70 | 235 | 213 | 200 | 65 |
| 11/18/2017 | 26,382 | 1880.50 | 132 | 245 | 172 | 99 |
| 11/19/2017 | 26,090 | 1880.30 | 98 | 259 | 166 | 99 |
| 11/20/2017 | 26,785 | 1880.80 | 580 | 256 | 309 | 99 |
| 11/21/2017 | 27,275 | 1881.30 | 553 | 257 | 266 | 99 |
| 11/22/2017 | 27,203 | 1881.20 | 221 | 256 | 187 | 99 |
| 11/23/2017 | 26,999 | 1881.00 | 152 | 256 | 176 | 99 |
| 11/24/2017 | 26,737 | 1880.80 | 118 | 256 | 171 | 99 |
| 11/25/2017 | 26,435 | 1880.60 | 101 | 254 | 167 | 99 |
| 11/26/2017 | 26,808 | 1880.90 | 406 | 257 | 264 | 99 |
| 11/27/2017 | 27,456 | 1881.40 | 635 | 259 | 358 | 100 |
| 11/28/2017 | 27,505 | 1881.40 | 287 | 258 | 219 | 100 |
| 11/29/2017 | 27,372 | 1881.30 | 195 | 258 | 188 | 100 |
| 11/30/2017 | 27,167 | 1881.20 | 153 | 258 | 179 | 100 |
| 12/1/2017 | 26,987 | 1881.00 | 128 | 258 | 174 | 100 |
| 12/2/2017 | 26,654 | 1880.70 | 114 | 259 | 172 | 100 |
| 12/3/2017 | 26,335 | 1880.50 | 101 | 260 | 171 | 100 |
| 12/4/2017 | 26,020 | 1880.20 | 87 | 259 | 169 | 100 |
| 12/5/2017 | 25,695 | 1879.90 | 87 | 259 | 167 | 100 |
| 12/6/2017 | 25,339 | 1879.60 | 64 | 252 | 164 | 100 |
| 12/7/2017 | 25,027 | 1879.30 | 66 | 231 | 154 | 100 |
| 12/8/2017 | 24,729 | 1879.10 | 58 | 219 | 142 | 101 |
| 12/9/2017 | 24,495 | 1878.90 | 58 | 186 | 152 | 59 |
| 12/10/2017 | 24,295 | 1878.70 | 55 | 160 | 139 | 44 |
| 12/11/2017 | 24,119 | 1878.50 | 52 | 155 | 134 | 44 |
| 12/12/2017 | 23,912 | 1878.30 | 51 | 151 | 130 | 44 |
| 12/13/2017 | 23,728 | 1878.20 | 49 | 149 | 125 | 44 |
| 12/14/2017 | 23,555 | 1878.00 | 48 | 149 | 125 | 44 |
| 12/15/2017 | 23,351 | 1877.80 | 50 | 145 | 124 | 44 |
| 12/16/2017 | 23,138 | 1877.60 | 44 | 145 | 124 | 44 |
| 12/17/2017 | 22,959 | 1877.40 | 45 | 144 | 123 | 44 |
| 12/18/2017 | 22,749 | 1877.20 | 45 | 144 | 122 | 44 |
| 12/19/2017 | 22,583 | 1877.10 | 50 | 144 | 123 | 44 |
| 12/20/2017 | 22,375 | 1876.90 | 45 | 145 | 124 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/21/2017 | 22,170 | 1876.70 | 42 | 144 | 122 | 44 |
| 12/22/2017 | 21,976 | 1876.50 | 41 | 143 | 121 | 44 |
| 12/23/2017 | 21,773 | 1876.30 | 42 | 143 | 120 | 44 |
| 12/24/2017 | 21,572 | 1876.10 | 40 | 143 | 120 | 44 |
| 12/25/2017 | 21,382 | 1875.90 | 41 | 143 | 119 | 44 |
| 12/26/2017 | 21,174 | 1875.70 | 38 | 143 | 119 | 44 |
| 12/27/2017 | 20,987 | 1875.50 | 38 | 142 | 119 | 44 |
| 12/28/2017 | 20,773 | 1875.30 | 37 | 142 | 118 | 44 |
| 12/29/2017 | 20,560 | 1875.10 | 35 | 146 | 122 | 44 |
| 12/30/2017 | 20,339 | 1874.80 | 37 | 144 | 123 | 44 |
| 12/31/2017 | 20,139 | 1874.60 | 35 | 143 | 119 | 44 |
| 1/1/2018 | 19,933 | 1874.40 | 38 | 143 | 119 | 44 |
| 1/2/2018 | 19,718 | 1874.20 | 35 | 143 | 119 | 44 |
| 1/3/2018 | 19,515 | 1874.00 | 38 | 143 | 118 | 44 |
| 1/4/2018 | 19,332 | 1873.80 | 46 | 142 | 120 | 44 |
| 1/5/2018 | 19,323 | 1873.70 | 130 | 143 | 127 | 44 |
| 1/6/2018 | 19,286 | 1873.70 | 139 | 142 | 128 | 44 |
| 1/7/2018 | 19,168 | 1873.60 | 80 | 142 | 124 | 44 |
| 1/8/2018 | 20,017 | 1874.50 | 526 | 145 | 231 | 44 |
| 1/9/2018 | 22,293 | 1876.80 | 1,399 | 148 | 399 | 44 |
| 1/10/2018 | 22,802 | 1877.30 | 452 | 171 | 206 | 44 |
| 1/11/2018 | 22,970 | 1877.50 | 285 | 179 | 188 | 44 |
| 1/12/2018 | 23,022 | 1877.50 | 208 | 179 | 178 | 44 |
| 1/13/2018 | 22,980 | 1877.50 | 163 | 179 | 172 | 44 |
| 1/14/2018 | 22,906 | 1877.40 | 140 | 178 | 168 | 44 |
| 1/15/2018 | 22,802 | 1877.30 | 127 | 178 | 165 | 44 |
| 1/16/2018 | 22,728 | 1877.20 | 139 | 178 | 166 | 44 |
| 1/17/2018 | 22,624 | 1877.10 | 122 | 177 | 163 | 44 |
| 1/18/2018 | 22,666 | 1877.20 | 192 | 177 | 209 | 44 |
| 1/19/2018 | 23,043 | 1877.50 | 385 | 179 | 313 | 44 |
| 1/20/2018 | 23,149 | 1877.60 | 242 | 178 | 221 | 44 |
| 1/21/2018 | 23,544 | 1878.00 | 366 | 179 | 235 | 44 |
| 1/22/2018 | 26,230 | 1880.40 | 1,605 | 186 | 704 | 44 |
| 1/23/2018 | 26,951 | 1881.00 | 582 | 186 | 350 | 44 |
| 1/24/2018 | 28,214 | 1882.00 | 809 | 187 | 459 | 44 |
| 1/25/2018 | 29,955 | 1883.40 | 1,131 | 189 | 651 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/26/2018 | 30,918 | 1884.10 | 731 | 188 | 517 | 45 |
| 1/27/2018 | 31,502 | 1884.60 | 499 | 188 | 400 | 45 |
| 1/28/2018 | 32,186 | 1885.10 | 540 | 188 | 372 | 45 |
| 1/29/2018 | 32,758 | 1885.50 | 499 | 188 | 335 | 46 |
| 1/30/2018 | 33,088 | 1885.80 | 433 | 248 | 342 | 45 |
| 1/31/2018 | 33,184 | 1885.80 | 385 | 343 | 464 | 51 |
| 2/1/2018 | 33,447 | 1886.00 | 327 | 191 | 277 | 46 |
| 2/2/2018 | 33,670 | 1886.20 | 311 | 189 | 232 | 46 |
| 2/3/2018 | 33,823 | 1886.30 | 279 | 187 | 217 | 46 |
| 2/4/2018 | 33,991 | 1886.40 | 269 | 185 | 203 | 46 |
| 2/5/2018 | 34,019 | 1886.40 | 199 | 185 | 196 | 46 |
| 2/6/2018 | 34,075 | 1886.50 | 214 | 186 | 192 | 46 |
| 2/7/2018 | 34,075 | 1886.50 | 187 | 186 | 189 | 45 |
| 2/8/2018 | 34,019 | 1886.40 | 167 | 186 | 188 | 45 |
| 2/9/2018 | 33,963 | 1886.40 | 146 | 186 | 186 | 45 |
| 2/10/2018 | 33,879 | 1886.30 | 134 | 185 | 185 | 45 |
| 2/11/2018 | 33,753 | 1886.20 | 125 | 186 | 183 | 45 |
| 2/12/2018 | 33,670 | 1886.20 | 113 | 185 | 182 | 45 |
| 2/13/2018 | 33,530 | 1886.10 | 106 | 185 | 180 | 45 |
| 2/14/2018 | 33,378 | 1886.00 | 101 | 183 | 178 | 45 |
| 2/15/2018 | 33,240 | 1885.90 | 93 | 174 | 171 | 45 |
| 2/16/2018 | 33,102 | 1885.80 | 90 | 166 | 163 | 45 |
| 2/17/2018 | 32,978 | 1885.70 | 87 | 160 | 157 | 45 |
| 2/18/2018 | 32,882 | 1885.60 | 92 | 150 | 149 | 45 |
| 2/19/2018 | 32,745 | 1885.50 | 75 | 148 | 146 | 45 |
| 2/20/2018 | 32,621 | 1885.40 | 64 | 148 | 145 | 45 |
| 2/21/2018 | 32,485 | 1885.30 | 83 | 147 | 145 | 45 |
| 2/22/2018 | 32,363 | 1885.20 | 78 | 147 | 145 | 45 |
| 2/23/2018 | 32,214 | 1885.10 | 62 | 147 | 144 | 45 |
| 2/24/2018 | 32,092 | 1885.00 | 71 | 145 | 143 | 45 |
| 2/25/2018 | 31,944 | 1884.90 | 67 | 143 | 141 | 45 |
| 2/26/2018 | 31,863 | 1884.90 | 98 | 143 | 146 | 45 |
| 2/27/2018 | 31,729 | 1884.80 | 64 | 143 | 142 | 45 |
| 2/28/2018 | 31,662 | 1884.70 | 102 | 143 | 143 | 45 |
| 3/1/2018 | 32,458 | 1885.30 | 529 | 146 | 368 | 45 |
| 3/2/2018 | 33,102 | 1885.80 | 519 | 147 | 372 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/3/2018 | 33,378 | 1886.00 | 324 | 177 | 293 | 45 |
| 3/4/2018 | 33,530 | 1886.10 | 273 | 196 | 309 | 45 |
| 3/5/2018 | 33,600 | 1886.10 | 231 | 195 | 280 | 45 |
| 3/6/2018 | 33,670 | 1886.20 | 227 | 195 | 267 | 45 |
| 3/7/2018 | 33,823 | 1886.30 | 267 | 195 | 271 | 45 |
| 3/8/2018 | 34,287 | 1886.60 | 430 | 194 | 329 | 45 |
| 3/9/2018 | 34,612 | 1886.90 | 362 | 194 | 294 | 45 |
| 3/10/2018 | 34,826 | 1887.00 | 308 | 194 | 271 | 45 |
| 3/11/2018 | 34,997 | 1887.10 | 281 | 194 | 253 | 45 |
| 3/12/2018 | 35,184 | 1887.30 | 288 | 194 | 240 | 45 |
| 3/13/2018 | 36,479 | 1888.10 | 831 | 196 | 323 | 45 |
| 3/14/2018 | 37,717 | 1889.00 | 866 | 196 | 383 | 45 |
| 3/15/2018 | 39,026 | 1889.80 | 856 | 200 | 410 | 45 |
| 3/16/2018 | 40,567 | 1890.80 | 1,027 | 203 | 582 | 45 |
| 3/17/2018 | 41,517 | 1891.40 | 708 | 203 | 495 | 45 |
| 3/18/2018 | 42,142 | 1891.80 | 534 | 203 | 423 | 45 |
| 3/19/2018 | 42,626 | 1892.10 | 447 | 203 | 378 | 45 |
| 3/20/2018 | 43,113 | 1892.40 | 441 | 203 | 356 | 45 |
| 3/21/2018 | 44,610 | 1893.30 | 945 | 204 | 413 | 45 |
| 3/22/2018 | 52,195 | 1897.70 | 4,191 | 213 | 729 | 45 |
| 3/23/2018 | 55,631 | 1899.60 | 2,037 | 215 | 594 | 45 |
| 3/24/2018 | 58,064 | 1900.90 | 1,517 | 223 | 589 | 45 |
| 3/25/2018 | 59,741 | 1901.70 | 1,122 | 226 | 529 | 45 |
| 3/26/2018 | 60,992 | 1902.40 | 884 | 227 | 488 | 45 |
| 3/27/2018 | 61,921 | 1902.80 | 725 | 228 | 444 | 45 |
| 3/28/2018 | 62,758 | 1903.30 | 651 | 229 | 411 | 44 |
| 3/29/2018 | 63,502 | 1903.60 | 610 | 226 | 377 | 44 |
| 3/30/2018 | 64,167 | 1904.00 | 566 | 220 | 345 | 44 |
| 3/31/2018 | 64,735 | 1904.20 | 510 | 212 | 311 | 44 |
| 4/1/2018 | 65,163 | 1904.50 | 465 | 258 | 340 | 45 |
| 4/2/2018 | 65,491 | 1904.60 | 434 | 260 | 339 | 45 |
| 4/3/2018 | 65,962 | 1904.80 | 497 | 260 | 327 | 45 |
| 4/4/2018 | 66,146 | 1904.90 | 342 | 259 | 316 | 45 |
| 4/5/2018 | 66,475 | 1905.10 | 413 | 260 | 316 | 45 |
| 4/6/2018 | 68,447 | 1906.00 | 4,387 | 3,469 | 3,869 | 33 |
| 4/7/2018 | 67,366 | 1905.50 | 5,863 | 6,806 | 8,307 | 9 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/8/2018 | 69,433 | 1906.50 | 2,386 | 1,336 | 2,026 | 45 |
| 4/9/2018 | 70,804 | 1907.20 | 1,414 | 769 | 1,016 | 45 |
| 4/10/2018 | 71,935 | 1907.70 | 1,029 | 508 | 756 | 45 |
| 4/11/2018 | 72,406 | 1907.90 | 865 | 634 | 739 | 45 |
| 4/12/2018 | 72,707 | 1908.10 | 777 | 702 | 932 | 44 |
| 4/13/2018 | 73,290 | 1908.30 | 610 | 341 | 494 | 45 |
| 4/14/2018 | 73,550 | 1908.50 | 530 | 458 | 542 | 56 |
| 4/15/2018 | 73,724 | 1908.50 | 557 | 548 | 616 | 81 |
| 4/16/2018 | 73,941 | 1908.60 | 588 | 534 | 619 | 81 |
| 4/17/2018 | 74,226 | 1908.80 | 493 | 419 | 508 | 80 |
| 4/18/2018 | 74,512 | 1908.90 | 450 | 337 | 393 | 80 |
| 4/19/2018 | 74,732 | 1909.00 | 413 | 333 | 374 | 80 |
| 4/20/2018 | 74,887 | 1909.10 | 397 | 312 | 339 | 80 |
| 4/21/2018 | 75,019 | 1909.10 | 368 | 306 | 319 | 80 |
| 4/22/2018 | 75,108 | 1909.20 | 343 | 306 | 308 | 80 |
| 4/23/2018 | 75,130 | 1909.20 | 312 | 306 | 298 | 80 |
| 4/24/2018 | 75,108 | 1909.20 | 292 | 306 | 290 | 80 |
| 4/25/2018 | 75,063 | 1909.10 | 274 | 305 | 282 | 80 |
| 4/26/2018 | 74,975 | 1909.10 | 249 | 305 | 276 | 80 |
| 4/27/2018 | 74,887 | 1909.10 | 246 | 304 | 272 | 80 |
| 4/28/2018 | 74,798 | 1909.00 | 249 | 304 | 271 | 80 |
| 4/29/2018 | 74,666 | 1909.00 | 234 | 303 | 273 | 80 |
| 4/30/2018 | 74,556 | 1908.90 | 230 | 304 | 261 | 80 |
| 5/1/2018 | 74,402 | 1908.80 | 221 | 303 | 256 | 80 |
| 5/2/2018 | 74,226 | 1908.80 | 199 | 304 | 251 | 80 |
| 5/3/2018 | 74,029 | 1908.70 | 179 | 293 | 239 | 80 |
| 5/4/2018 | 73,832 | 1908.60 | 166 | 283 | 226 | 80 |
| 5/5/2018 | 73,637 | 1908.50 | 162 | 280 | 221 | 80 |
| 5/6/2018 | 73,398 | 1908.40 | 138 | 275 | 216 | 80 |
| 5/7/2018 | 73,182 | 1908.30 | 142 | 271 | 210 | 80 |
| 5/8/2018 | 72,966 | 1908.20 | 130 | 260 | 202 | 80 |
| 5/9/2018 | 72,750 | 1908.10 | 113 | 246 | 188 | 80 |
| 5/10/2018 | 72,513 | 1908.00 | 121 | 241 | 184 | 80 |
| 5/11/2018 | 72,277 | 1907.90 | 97 | 240 | 183 | 80 |
| 5/12/2018 | 72,063 | 1907.80 | 102 | 230 | 178 | 80 |
| 5/13/2018 | 71,828 | 1907.60 | 89 | 213 | 167 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/14/2018 | 71,635 | 1907.60 | 89 | 203 | 157 | 80 |
| 5/15/2018 | 71,400 | 1907.40 | 84 | 225 | 164 | 80 |
| 5/16/2018 | 71,144 | 1907.30 | 101 | 250 | 191 | 79 |
| 5/17/2018 | 70,889 | 1907.20 | 101 | 241 | 184 | 79 |
| 5/18/2018 | 70,635 | 1907.10 | 94 | 235 | 180 | 80 |
| 5/19/2018 | 70,360 | 1907.00 | 76 | 231 | 174 | 81 |
| 5/20/2018 | 70,085 | 1906.80 | 68 | 230 | 176 | 81 |
| 5/21/2018 | 69,791 | 1906.70 | 74 | 227 | 173 | 81 |
| 5/22/2018 | 69,538 | 1906.60 | 74 | 222 | 171 | 81 |
| 5/23/2018 | 69,285 | 1906.40 | 70 | 210 | 163 | 81 |
| 5/24/2018 | 69,054 | 1906.30 | 64 | 202 | 156 | 81 |
| 5/25/2018 | 68,844 | 1906.20 | 79 | 198 | 156 | 81 |
| 5/26/2018 | 68,656 | 1906.10 | 89 | 194 | 154 | 80 |
| 5/27/2018 | 68,447 | 1906.00 | 77 | 193 | 152 | 80 |
| 5/28/2018 | 68,217 | 1905.90 | 58 | 187 | 149 | 80 |
| 5/29/2018 | 67,926 | 1905.80 | 49 | 208 | 155 | 80 |
| 5/30/2018 | 67,555 | 1905.60 | 35 | 224 | 172 | 80 |
| 5/31/2018 | 67,225 | 1905.50 | 23 | 225 | 173 | 81 |
| 6/1/2018 | 66,917 | 1905.30 | 33 | 213 | 166 | 81 |
| 6/2/2018 | 66,610 | 1905.20 | 38 | 209 | 157 | 80 |
| 6/3/2018 | 66,304 | 1905.00 | 27 | 205 | 156 | 80 |
| 6/4/2018 | 65,999 | 1904.90 | 14 | 197 | 146 | 80 |
| 6/5/2018 | 65,695 | 1904.70 | 20 | 186 | 140 | 80 |
| 6/6/2018 | 65,432 | 1904.60 | 17 | 180 | 131 | 80 |
| 6/7/2018 | 65,128 | 1904.40 | 7 | 179 | 129 | 80 |
| 6/8/2018 | 64,846 | 1904.30 | 10 | 172 | 123 | 80 |
| 6/9/2018 | 64,544 | 1904.20 | 1 | 168 | 115 | 80 |
| 6/10/2018 | 64,283 | 1904.00 | 6 | 163 | 110 | 81 |
| 6/11/2018 | 64,043 | 1903.90 | 15 | 161 | 104 | 80 |
| 6/12/2018 | 63,804 | 1903.80 | 14 | 159 | 101 | 80 |
| 6/13/2018 | 63,566 | 1903.70 | 4 | 149 | 90 | 80 |
| 6/14/2018 | 63,347 | 1903.60 | 3 | 141 | 77 | 80 |
| 6/15/2018 | 63,129 | 1903.50 | 0 | 137 | 71 | 81 |
| 6/16/2018 | 62,911 | 1903.30 | -4 | 128 | 62 | 81 |
| 6/17/2018 | 62,714 | 1903.20 | -3 | 120 | 51 | 81 |
| 6/18/2018 | 62,537 | 1903.20 | 1 | 118 | 48 | 80 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/19/2018 | 62,359 | 1903.10 | 29 | 121 | 49 | 80 |
| 6/20/2018 | 62,140 | 1903.00 | -5 | 121 | 48 | 80 |
| 6/21/2018 | 61,941 | 1902.90 | 5 | 112 | 45 | 80 |
| 6/22/2018 | 61,803 | 1902.80 | 8 | 100 | 33 | 80 |
| 6/23/2018 | 61,644 | 1902.70 | 6 | 96 | 28 | 80 |
| 6/24/2018 | 61,447 | 1902.60 | 0 | 94 | 23 | 80 |
| 6/25/2018 | 61,288 | 1902.50 | -3 | 94 | 20 | 81 |
| 6/26/2018 | 61,130 | 1902.40 | 0 | 93 | 20 | 81 |
| 6/27/2018 | 60,933 | 1902.30 | -4 | 95 | 19 | 81 |
| 6/28/2018 | 60,737 | 1902.20 | -11 | 96 | 22 | 81 |
| 6/29/2018 | 60,580 | 1902.20 | -6 | 94 | 21 | 81 |
| 6/30/2018 | 60,384 | 1902.10 | -6 | 93 | 18 | 80 |
| 7/1/2018 | 60,208 | 1902.00 | -6 | 93 | 17 | 80 |
| 7/2/2018 | 60,033 | 1901.90 | -6 | 94 | 17 | 80 |
| 7/3/2018 | 59,838 | 1901.80 | -13 | 94 | 16 | 80 |
| 7/4/2018 | 59,644 | 1901.70 | -13 | 95 | 16 | 79 |
| 7/5/2018 | 59,450 | 1901.60 | -12 | 92 | 16 | 79 |
| 7/6/2018 | 59,295 | 1901.50 | -7 | 87 | 13 | 79 |
| 7/7/2018 | 59,120 | 1901.40 | -8 | 90 | 12 | 79 |
| 7/8/2018 | 58,947 | 1901.30 | -14 | 93 | 16 | 79 |
| 7/9/2018 | 58,735 | 1901.20 | -19 | 92 | 15 | 79 |
| 7/10/2018 | 58,542 | 1901.10 | -13 | 92 | 15 | 79 |
| 7/11/2018 | 58,370 | 1901.00 | -11 | 91 | 15 | 79 |
| 7/12/2018 | 58,178 | 1900.90 | -15 | 90 | 14 | 79 |
| 7/13/2018 | 58,006 | 1900.80 | -10 | 88 | 12 | 80 |
| 7/14/2018 | 57,835 | 1900.70 | -10 | 87 | 11 | 80 |
| 7/15/2018 | 57,645 | 1900.60 | -18 | 90 | 11 | 80 |
| 7/16/2018 | 57,454 | 1900.50 | -14 | 94 | 12 | 80 |
| 7/17/2018 | 57,245 | 1900.40 | -21 | 93 | 13 | 80 |
| 7/18/2018 | 57,036 | 1900.30 | -20 | 93 | 13 | 80 |
| 7/19/2018 | 56,846 | 1900.20 | -16 | 92 | 12 | 80 |
| 7/20/2018 | 56,637 | 1900.10 | -22 | 94 | 13 | 80 |
| 7/21/2018 | 56,430 | 1900.00 | -19 | 93 | 13 | 80 |
| 7/22/2018 | 56,222 | 1899.90 | -16 | 94 | 13 | 80 |
| 7/23/2018 | 56,016 | 1899.80 | -17 | 93 | 12 | 80 |
| 7/24/2018 | 55,810 | 1899.70 | -22 | 92 | 11 | 80 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/25/2018 | 55,604 | 1899.60 | -24 | 95 | 11 | 80 |
| 7/26/2018 | 55,380 | 1899.40 | -21 | 94 | 15 | 80 |
| 7/27/2018 | 55,156 | 1899.30 | -26 | 93 | 12 | 80 |
| 7/28/2018 | 54,952 | 1899.20 | -17 | 93 | 13 | 80 |
| 7/29/2018 | 54,748 | 1899.10 | -24 | 93 | 12 | 80 |
| 7/30/2018 | 54,527 | 1899.00 | -29 | 93 | 13 | 80 |
| 7/31/2018 | 54,306 | 1898.90 | -21 | 93 | 12 | 80 |
| 8/1/2018 | 54,104 | 1898.80 | -23 | 93 | 12 | 80 |
| 8/2/2018 | 53,884 | 1898.60 | -22 | 93 | 12 | 80 |
| 8/3/2018 | 53,681 | 1898.50 | -25 | 92 | 11 | 81 |
| 8/4/2018 | 53,461 | 1898.40 | -28 | 92 | 10 | 81 |
| 8/5/2018 | 53,242 | 1898.30 | -22 | 91 | 10 | 81 |
| 8/6/2018 | 53,023 | 1898.20 | -26 | 94 | 11 | 81 |
| 8/7/2018 | 52,823 | 1898.00 | -17 | 95 | 15 | 81 |
| 8/8/2018 | 52,623 | 1897.90 | -7 | 94 | 13 | 81 |
| 8/9/2018 | 52,442 | 1897.80 | -8 | 94 | 13 | 81 |
| 8/10/2018 | 52,244 | 1897.70 | -15 | 94 | 12 | 81 |
| 8/11/2018 | 52,045 | 1897.60 | -16 | 94 | 12 | 81 |
| 8/12/2018 | 51,846 | 1897.50 | -11 | 94 | 12 | 81 |
| 8/13/2018 | 51,647 | 1897.40 | -12 | 94 | 12 | 81 |
| 8/14/2018 | 51,449 | 1897.30 | -11 | 94 | 12 | 81 |
| 8/15/2018 | 51,251 | 1897.20 | -15 | 94 | 13 | 81 |
| 8/16/2018 | 51,054 | 1897.10 | -10 | 94 | 12 | 81 |
| 8/17/2018 | 50,858 | 1897.00 | -13 | 93 | 11 | 81 |
| 8/18/2018 | 50,680 | 1896.90 | -9 | 92 | 11 | 81 |
| 8/19/2018 | 50,484 | 1896.70 | -12 | 92 | 11 | 81 |
| 8/20/2018 | 50,289 | 1896.60 | -9 | 91 | 11 | 81 |
| 8/21/2018 | 50,129 | 1896.50 | -7 | 91 | 11 | 81 |
| 8/22/2018 | 49,933 | 1896.40 | -5 | 90 | 11 | 81 |
| 8/23/2018 | 49,756 | 1896.30 | -12 | 88 | 9 | 80 |
| 8/24/2018 | 49,579 | 1896.20 | -6 | 89 | 10 | 80 |
| 8/25/2018 | 49,385 | 1896.10 | -7 | 91 | 10 | 80 |
| 8/26/2018 | 49,192 | 1896.00 | -7 | 94 | 13 | 81 |
| 8/27/2018 | 48,999 | 1895.90 | -9 | 93 | 13 | 81 |
| 8/28/2018 | 48,842 | 1895.80 | -5 | 91 | 10 | 81 |
| 8/29/2018 | 48,650 | 1895.70 | -8 | 91 | 11 | 81 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/30/2018 | 48,458 | 1895.60 | -11 | 91 | 11 | 80 |
| 8/31/2018 | 48,283 | 1895.50 | -6 | 91 | 10 | 80 |
| 9/1/2018 | 48,092 | 1895.40 | -5 | 91 | 10 | 80 |
| 9/2/2018 | 47,901 | 1895.30 | -8 | 91 | 10 | 80 |
| 9/3/2018 | 47,710 | 1895.20 | -12 | 91 | 10 | 80 |
| 9/4/2018 | 47,538 | 1895.10 | -6 | 90 | 10 | 80 |
| 9/5/2018 | 47,331 | 1894.90 | -13 | 90 | 10 | 80 |
| 9/6/2018 | 47,142 | 1894.80 | -12 | 89 | 10 | 80 |
| 9/7/2018 | 46,937 | 1894.70 | -12 | 92 | 10 | 80 |
| 9/8/2018 | 46,766 | 1894.60 | -7 | 94 | 14 | 80 |
| 9/9/2018 | 46,560 | 1894.50 | -11 | 93 | 12 | 80 |
| 9/10/2018 | 46,372 | 1894.40 | -15 | 92 | 12 | 80 |
| 9/11/2018 | 46,134 | 1894.20 | -14 | 92 | 12 | 80 |
| 9/12/2018 | 45,947 | 1894.10 | -45 | 92 | 12 | 81 |
| 9/13/2018 | 45,760 | 1894.00 | -17 | 91 | 12 | 81 |
| 9/14/2018 | 45,557 | 1893.90 | -14 | 91 | 12 | 80 |
| 9/15/2018 | 45,338 | 1893.80 | -17 | 96 | 12 | 81 |
| 9/16/2018 | 45,152 | 1893.60 | -12 | 103 | 13 | 90 |
| 9/17/2018 | 44,933 | 1893.50 | -12 | 103 | 13 | 90 |
| 9/18/2018 | 44,731 | 1893.40 | -9 | 103 | 12 | 90 |
| 9/19/2018 | 44,513 | 1893.30 | -10 | 106 | 12 | 90 |
| 9/20/2018 | 44,329 | 1893.20 | -5 | 106 | 12 | 90 |
| 9/21/2018 | 44,113 | 1893.00 | -10 | 106 | 13 | 90 |
| 9/22/2018 | 43,897 | 1892.90 | -11 | 106 | 12 | 90 |
| 9/23/2018 | 43,682 | 1892.80 | -6 | 106 | 11 | 90 |
| 9/24/2018 | 43,484 | 1892.60 | -9 | 106 | 11 | 90 |
| 9/25/2018 | 43,269 | 1892.50 | -6 | 106 | 11 | 90 |
| 9/26/2018 | 43,071 | 1892.40 | -8 | 106 | 11 | 90 |
| 9/27/2018 | 42,874 | 1892.30 | -5 | 106 | 11 | 90 |
| 9/28/2018 | 42,662 | 1892.10 | -8 | 106 | 11 | 90 |
| 9/29/2018 | 42,450 | 1892.00 | -6 | 107 | 11 | 90 |
| 9/30/2018 | 42,271 | 1891.90 | 4 | 107 | 13 | 90 |
| 10/1/2018 | 42,108 | 1891.80 | 15 | 102 | 12 | 90 |
| 10/2/2018 | 42,010 | 1891.70 | 56 | 104 | 18 | 90 |
| 10/3/2018 | 41,929 | 1891.70 | 62 | 106 | 22 | 90 |
| 10/4/2018 | 41,767 | 1891.60 | 25 | 106 | 20 | 90 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/5/2018 | 41,573 | 1891.50 | 11 | 115 | 20 | 90 |
| 10/6/2018 | 41,348 | 1891.30 | 5 | 121 | 31 | 90 |
| 10/7/2018 | 41,124 | 1891.20 | 3 | 119 | 29 | 90 |
| 10/8/2018 | 40,916 | 1891.00 | 8 | 120 | 29 | 91 |
| 10/9/2018 | 40,694 | 1890.90 | 6 | 122 | 30 | 90 |
| 10/10/2018 | 40,488 | 1890.80 | -18 | 122 | 31 | 90 |
| 10/11/2018 | 40,269 | 1890.60 | -4 | 116 | 29 | 91 |
| 10/12/2018 | 40,034 | 1890.50 | -3 | 115 | 30 | 90 |
| 10/13/2018 | 39,800 | 1890.30 | -4 | 117 | 33 | 90 |
| 10/14/2018 | 39,567 | 1890.20 | -3 | 116 | 32 | 90 |
| 10/15/2018 | 39,303 | 1890.00 | -24 | 117 | 33 | 90 |
| 10/16/2018 | 39,134 | 1889.90 | -2 | 94 | 38 | 75 |
| 10/17/2018 | 38,980 | 1889.80 | -8 | 71 | 38 | 45 |
| 10/18/2018 | 38,842 | 1889.70 | -6 | 71 | 36 | 45 |
| 10/19/2018 | 38,735 | 1889.60 | -5 | 71 | 37 | 45 |
| 10/20/2018 | 38,551 | 1889.50 | -4 | 72 | 37 | 45 |
| 10/21/2018 | 38,398 | 1889.40 | -4 | 72 | 37 | 45 |
| 10/22/2018 | 38,261 | 1889.30 | -6 | 73 | 37 | 45 |
| 10/23/2018 | 38,110 | 1889.20 | -3 | 73 | 37 | 46 |
| 10/24/2018 | 37,958 | 1889.10 | -2 | 75 | 33 | 47 |
| 10/25/2018 | 37,837 | 1889.00 | 3 | 76 | 34 | 47 |
| 10/26/2018 | 37,682 | 1888.90 | 3 | 77 | 34 | 47 |
| 10/27/2018 | 37,540 | 1888.90 | 5 | 78 | 32 | 47 |
| 10/28/2018 | 37,398 | 1888.80 | 1 | 80 | 35 | 47 |
| 10/29/2018 | 37,227 | 1888.60 | 2 | 80 | 34 | 47 |
| 10/30/2018 | 37,056 | 1888.50 | 8 | 91 | 34 | 48 |
| 10/31/2018 | 36,901 | 1888.40 | 15 | 110 | 49 | 54 |
| 11/1/2018 | 36,701 | 1888.30 | 18 | 112 | 38 | 65 |
| 11/2/2018 | 36,502 | 1888.20 | 14 | 111 | 35 | 68 |
| 11/3/2018 | 36,304 | 1888.00 | 10 | 112 | 35 | 68 |
| 11/4/2018 | 36,107 | 1887.90 | 9 | 112 | 34 | 68 |
| 11/5/2018 | 35,896 | 1887.70 | 11 | 117 | 35 | 68 |
| 11/6/2018 | 35,704 | 1887.60 | -24 | 124 | 42 | 78 |
| 11/7/2018 | 35,472 | 1887.50 | -2 | 113 | 42 | 73 |
| 11/8/2018 | 35,256 | 1887.30 | -5 | 113 | 43 | 76 |
| 11/9/2018 | 35,040 | 1887.20 | -1 | 114 | 37 | 83 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/10/2018 | 34,840 | 1887.00 | 0 | 113 | 34 | 85 |
| 11/11/2018 | 34,641 | 1886.90 | 1 | 112 | 34 | 85 |
| 11/12/2018 | 34,414 | 1886.70 | 2 | 112 | 34 | 85 |
| 11/13/2018 | 34,202 | 1886.60 | 2 | 112 | 35 | 84 |
| 11/14/2018 | 33,991 | 1886.40 | 5 | 112 | 35 | 84 |
| 11/15/2018 | 33,795 | 1886.30 | 5 | 112 | 35 | 84 |
| 11/16/2018 | 33,600 | 1886.10 | 4 | 112 | 34 | 84 |
| 11/17/2018 | 33,392 | 1886.00 | 4 | 112 | 33 | 84 |
| 11/18/2018 | 33,184 | 1885.80 | 4 | 113 | 34 | 84 |
| 11/19/2018 | 32,978 | 1885.70 | 4 | 114 | 39 | 81 |
| 11/20/2018 | 32,772 | 1885.50 | 4 | 108 | 38 | 77 |
| 11/21/2018 | 32,662 | 1885.40 | 38 | 104 | 40 | 73 |
| 11/22/2018 | 32,662 | 1885.40 | 100 | 103 | 45 | 73 |
| 11/23/2018 | 33,350 | 1885.90 | 439 | 102 | 80 | 73 |
| 11/24/2018 | 33,809 | 1886.30 | 386 | 115 | 100 | 71 |
| 11/25/2018 | 33,670 | 1886.20 | 87 | 154 | 88 | 70 |
| 11/26/2018 | 33,392 | 1886.00 | 44 | 190 | 135 | 71 |
| 11/27/2018 | 33,309 | 1885.90 | 142 | 201 | 175 | 71 |
| 11/28/2018 | 33,253 | 1885.90 | 181 | 203 | 181 | 70 |
| 11/29/2018 | 34,926 | 1887.10 | 1,082 | 216 | 319 | 71 |
| 11/30/2018 | 35,098 | 1887.20 | 333 | 219 | 208 | 71 |
| 12/1/2018 | 35,098 | 1887.20 | 222 | 218 | 200 | 71 |
| 12/2/2018 | 34,954 | 1887.10 | 141 | 217 | 187 | 71 |
| 12/3/2018 | 34,698 | 1886.90 | 88 | 216 | 176 | 71 |
| 12/4/2018 | 34,428 | 1886.70 | 69 | 216 | 171 | 71 |
| 12/5/2018 | 34,146 | 1886.50 | 62 | 215 | 169 | 71 |
| 12/6/2018 | 33,837 | 1886.30 | 52 | 215 | 167 | 71 |
| 12/7/2018 | 33,517 | 1886.10 | 45 | 215 | 165 | 71 |
| 12/8/2018 | 33,212 | 1885.80 | 48 | 208 | 161 | 71 |
| 12/9/2018 | 32,950 | 1885.70 | 44 | 189 | 148 | 71 |
| 12/10/2018 | 32,704 | 1885.50 | 20 | 176 | 134 | 70 |
| 12/11/2018 | 32,471 | 1885.30 | 42 | 174 | 130 | 71 |
| 12/12/2018 | 32,186 | 1885.10 | 39 | 174 | 129 | 71 |
| 12/13/2018 | 31,944 | 1884.90 | 45 | 173 | 128 | 71 |
| 12/14/2018 | 31,729 | 1884.80 | 60 | 175 | 130 | 71 |
| 12/15/2018 | 31,542 | 1884.60 | 75 | 174 | 132 | 71 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/16/2018 | 33,725 | 1886.20 | 1,171 | 176 | 263 | 71 |
| 12/17/2018 | 35,299 | 1887.30 | 1,149 | 178 | 280 | 71 |
| 12/18/2018 | 35,777 | 1887.70 | 444 | 198 | 198 | 71 |
| 12/19/2018 | 36,127 | 1887.90 | 420 | 216 | 264 | 70 |
| 12/20/2018 | 36,273 | 1888.00 | 290 | 220 | 202 | 71 |
| 12/21/2018 | 36,642 | 1888.20 | 416 | 230 | 240 | 70 |
| 12/22/2018 | 36,731 | 1888.30 | 270 | 228 | 197 | 71 |
| 12/23/2018 | 36,746 | 1888.30 | 232 | 229 | 186 | 71 |
| 12/24/2018 | 37,883 | 1889.10 | 760 | 231 | 338 | 71 |
| 12/25/2018 | 38,888 | 1889.70 | 801 | 232 | 353 | 71 |
| 12/26/2018 | 39,165 | 1889.90 | 386 | 232 | 237 | 71 |
| 12/27/2018 | 39,273 | 1890.00 | 274 | 232 | 201 | 71 |
| 12/28/2018 | 39,242 | 1890.00 | 209 | 230 | 188 | 71 |
| 12/29/2018 | 39,134 | 1889.90 | 173 | 228 | 180 | 71 |
| 12/30/2018 | 39,011 | 1889.80 | 148 | 225 | 173 | 71 |
| 12/31/2018 | 38,857 | 1889.70 | 133 | 223 | 168 | 71 |
| 1/1/2019 | 38,674 | 1889.60 | 111 | 222 | 166 | 71 |
| 1/2/2019 | 38,460 | 1889.50 | 105 | 221 | 164 | 71 |
| 1/3/2019 | 38,261 | 1889.30 | 100 | 221 | 162 | 71 |
| 1/4/2019 | 38,049 | 1889.20 | 94 | 216 | 159 | 71 |
| 1/5/2019 | 38,155 | 1889.30 | 244 | 209 | 171 | 71 |
| 1/6/2019 | 40,238 | 1890.60 | 1,231 | 212 | 464 | 70 |
| 1/7/2019 | 41,374 | 1891.30 | 879 | 214 | 429 | 70 |
| 1/8/2019 | 42,918 | 1892.30 | 963 | 216 | 347 | 70 |
| 1/9/2019 | 50,976 | 1897.00 | 4,382 | 228 | 728 | 70 |
| 1/10/2019 | 54,577 | 1899.00 | 2,235 | 227 | 565 | 70 |
| 1/11/2019 | 56,340 | 1900.00 | 1,180 | 229 | 387 | 70 |
| 1/12/2019 | 57,245 | 1900.40 | 1,002 | 543 | 597 | 70 |
| 1/13/2019 | 57,227 | 1900.40 | 716 | 753 | 866 | 70 |
| 1/14/2019 | 57,075 | 1900.30 | 550 | 662 | 781 | 70 |
| 1/15/2019 | 57,132 | 1900.40 | 606 | 605 | 688 | 70 |
| 1/16/2019 | 62,898 | 1903.30 | 4,183 | 1,911 | 1,690 | 69 |
| 1/17/2019 | 62,001 | 1902.90 | 8,364 | 8,797 | 10,266 | 0 |
| 1/18/2019 | 60,033 | 1901.90 | 3,355 | 4,533 | 5,878 | 34 |
| 1/19/2019 | 60,072 | 1901.90 | 3,324 | 3,489 | 4,295 | 46 |
| 1/20/2019 | 61,842 | 1902.80 | 5,161 | 4,592 | 5,780 | 35 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/21/2019 | 60,130 | 1901.90 | 3,933 | 4,916 | 6,441 | 39 |
| 1/22/2019 | 58,966 | 1901.30 | 2,161 | 2,868 | 3,582 | 59 |
| 1/23/2019 | 58,331 | 1901.00 | 1,519 | 1,935 | 2,248 | 62 |
| 1/24/2019 | 57,930 | 1900.80 | 1,171 | 1,434 | 1,772 | 48 |
| 1/25/2019 | 57,645 | 1900.60 | 916 | 1,120 | 1,476 | 58 |
| 1/26/2019 | 57,378 | 1900.50 | 740 | 911 | 1,238 | 81 |
| 1/27/2019 | 57,227 | 1900.40 | 631 | 747 | 1,056 | 81 |
| 1/28/2019 | 57,113 | 1900.40 | 566 | 653 | 926 | 81 |
| 1/29/2019 | 56,962 | 1900.30 | 526 | 637 | 824 | 127 |
| 1/30/2019 | 56,848 | 1900.20 | 488 | 583 | 720 | 155 |
| 1/31/2019 | 56,735 | 1900.20 | 450 | 526 | 593 | 132 |
| 2/1/2019 | 57,695 | 1900.70 | 474 | 491 | 521 | 83 |
| 2/2/2019 | 58,031 | 1900.90 | 1,664 | 1,065 | 1,022 | 83 |
| 2/3/2019 | 58,368 | 1901.00 | 1,415 | 1,473 | 1,489 | 82 |
| 2/4/2019 | 58,446 | 1901.10 | 1,958 | 1,881 | 1,841 | 83 |
| 2/5/2019 | 57,930 | 1900.80 | 1,269 | 1,606 | 1,665 | 86 |
| 2/6/2019 | 57,549 | 1900.60 | 951 | 1,186 | 1,305 | 86 |
| 2/7/2019 | 57,359 | 1900.50 | 803 | 953 | 1,070 | 84 |
| 2/8/2019 | 57,264 | 1900.40 | 714 | 803 | 905 | 82 |
| 2/9/2019 | 57,702 | 1900.70 | 970 | 836 | 1,017 | 83 |
| 2/10/2019 | 57,473 | 1900.60 | 938 | 1,057 | 1,246 | 83 |
| 2/11/2019 | 57,264 | 1900.40 | 700 | 852 | 1,025 | 82 |
| 2/12/2019 | 57,378 | 1900.50 | 764 | 748 | 902 | 84 |
| 2/13/2019 | 61,585 | 1902.70 | 4,639 | 2,846 | 3,537 | 84 |
| 2/14/2019 | 64,147 | 1904.00 | 12,640 | 11,958 | 13,387 | 1 |
| 2/15/2019 | 61,190 | 1902.50 | 5,433 | 7,146 | 9,185 | 1 |
| 2/16/2019 | 59,857 | 1901.80 | 3,056 | 3,891 | 5,137 | 43 |
| 2/17/2019 | 59,043 | 1901.40 | 2,211 | 2,744 | 3,406 | 46 |
| 2/18/2019 | 58,504 | 1901.10 | 1,691 | 2,068 | 2,317 | 47 |
| 2/19/2019 | 58,197 | 1900.90 | 1,394 | 1,617 | 1,822 | 46 |
| 2/20/2019 | 58,045 | 1900.90 | 1,243 | 1,396 | 1,572 | 46 |
| 2/21/2019 | 57,854 | 1900.80 | 1,091 | 1,246 | 1,397 | 46 |
| 2/22/2019 | 57,664 | 1900.70 | 929 | 1,073 | 1,237 | 46 |
| 2/23/2019 | 57,530 | 1900.60 | 823 | 940 | 1,075 | 66 |
| 2/24/2019 | 57,530 | 1900.60 | 828 | 873 | 980 | 79 |
| 2/25/2019 | 65,041 | 1904.40 | 5,884 | 2,846 | 3,632 | 63 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/26/2019 | 71,166 | 1907.30 | 20,419 | 18,769 | 19,130 | 1 |
| 2/27/2019 | 65,675 | 1904.70 | 23,398 | 26,638 | 27,437 | 1 |
| 2/28/2019 | 61,981 | 1902.90 | 6,512 | 8,666 | 10,897 | 1 |
| 3/1/2019 | 60,325 | 1902.00 | 3,614 | 4,623 | 6,553 | 29 |
| 3/2/2019 | 60,150 | 1901.90 | 3,329 | 3,635 | 5,041 | 45 |
| 3/3/2019 | 59,994 | 1901.90 | 3,042 | 3,321 | 4,362 | 45 |
| 3/4/2019 | 59,557 | 1901.60 | 2,715 | 3,088 | 3,890 | 45 |
| 3/5/2019 | 59,411 | 1901.60 | 2,428 | 2,674 | 3,171 | 45 |
| 3/6/2019 | 61,269 | 1902.50 | 4,713 | 4,067 | 4,620 | 45 |
| 3/7/2019 | 60,345 | 1902.00 | 3,629 | 4,290 | 5,046 | 45 |
| 3/8/2019 | 59,566 | 1901.70 | 2,670 | 3,253 | 3,866 | 46 |
| 3/9/2019 | 59,256 | 1901.50 | 2,306 | 2,651 | 3,037 | 46 |
| 3/10/2019 | 58,927 | 1901.30 | 2,106 | 2,429 | 2,725 | 46 |
| 3/11/2019 | 58,581 | 1901.10 | 1,721 | 2,053 | 2,219 | 46 |
| 3/12/2019 | 58,327 | 1901.00 | 1,494 | 1,767 | 1,909 | 46 |
| 3/13/2019 | 58,083 | 1900.90 | 1,267 | 1,518 | 1,708 | 45 |
| 3/14/2019 | 57,892 | 1900.80 | 1,120 | 1,332 | 1,524 | 45 |
| 3/15/2019 | 57,778 | 1900.70 | 1,023 | 1,207 | 1,361 | 44 |
| 3/16/2019 | 57,721 | 1900.70 | 982 | 1,132 | 1,242 | 44 |
| 3/17/2019 | 57,683 | 1900.70 | 972 | 1,104 | 1,188 | 45 |
| 3/18/2019 | 57,703 | 1900.70 | 991 | 1,108 | 1,228 | 47 |
| 3/19/2019 | 57,726 | 1900.70 | 999 | 1,118 | 1,264 | 46 |
| 3/20/2019 | 58,282 | 1901.00 | 1,458 | 1,289 | 1,478 | 45 |
| 3/21/2019 | 58,170 | 1900.90 | 1,461 | 1,579 | 1,636 | 46 |
| 3/22/2019 | 58,653 | 1901.20 | 1,643 | 1,546 | 1,616 | 46 |
| 3/23/2019 | 58,522 | 1901.10 | 1,877 | 2,020 | 1,942 | 45 |
| 3/24/2019 | 58,218 | 1901.00 | 1,428 | 1,686 | 1,722 | 45 |
| 3/25/2019 | 59,280 | 1901.50 | 2,268 | 1,887 | 1,892 | 45 |
| 3/26/2019 | 58,878 | 1901.30 | 2,109 | 2,417 | 2,374 | 44 |
| 3/27/2019 | 60,729 | 1902.20 | 4,509 | 3,576 | 3,824 | 44 |
| 3/28/2019 | 60,281 | 1902.00 | 3,322 | 3,774 | 4,419 | 44 |
| 3/29/2019 | 59,506 | 1901.60 | 2,669 | 3,199 | 3,698 | 44 |
| 3/30/2019 | 58,987 | 1901.40 | 2,121 | 2,526 | 2,797 | 44 |
| 3/31/2019 | 58,624 | 1901.20 | 1,789 | 2,085 | 2,252 | 44 |
| 4/1/2019 | 58,512 | 1901.10 | 1,652 | 1,822 | 1,959 | 44 |
| 4/2/2019 | 59,170 | 1901.40 | 1,745 | 1,499 | 1,733 | 44 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/3/2019 | 59,523 | 1901.60 | 1,568 | 1,460 | 1,590 | 44 |
| 4/4/2019 | 59,448 | 1901.60 | 1,372 | 1,511 | 1,605 | 44 |
| 4/5/2019 | 59,801 | 1901.80 | 1,609 | 1,533 | 1,640 | 44 |
| 4/6/2019 | 60,840 | 1902.30 | 1,625 | 1,128 | 1,425 | 44 |
| 4/7/2019 | 62,547 | 1903.20 | 1,659 | 808 | 1,045 | 44 |
| 4/8/2019 | 65,626 | 1904.70 | 2,387 | 922 | 1,297 | 44 |
| 4/9/2019 | 69,406 | 1906.50 | 2,854 | 841 | 1,258 | 44 |
| 4/10/2019 | 71,292 | 1907.40 | 1,970 | 1,014 | 1,317 | 45 |
| 4/11/2019 | 72,515 | 1908.00 | 1,623 | 1,030 | 1,289 | 45 |
| 4/12/2019 | 73,273 | 1908.30 | 1,387 | 1,048 | 1,266 | 45 |
| 4/13/2019 | 73,667 | 1908.50 | 1,251 | 1,099 | 1,279 | 45 |
| 4/14/2019 | 73,671 | 1908.50 | 1,160 | 1,259 | 1,356 | 45 |
| 4/15/2019 | 73,414 | 1908.40 | 1,128 | 1,339 | 1,416 | 73 |
| 4/16/2019 | 73,439 | 1908.40 | 1,084 | 1,135 | 1,288 | 90 |
| 4/17/2019 | 73,856 | 1908.60 | 967 | 727 | 877 | 90 |
| 4/18/2019 | 74,539 | 1908.90 | 871 | 533 | 597 | 90 |
| 4/19/2019 | 75,072 | 1909.10 | 898 | 666 | 731 | 90 |
| 4/20/2019 | 75,297 | 1909.20 | 880 | 791 | 816 | 90 |
| 4/21/2019 | 75,322 | 1909.30 | 800 | 839 | 826 | 90 |
| 4/22/2019 | 75,304 | 1909.20 | 738 | 795 | 855 | 90 |
| 4/23/2019 | 75,285 | 1909.20 | 720 | 784 | 798 | 90 |
| 4/24/2019 | 75,311 | 1909.30 | 753 | 799 | 801 | 90 |
| 4/25/2019 | 75,347 | 1909.30 | 731 | 748 | 791 | 90 |
| 4/26/2019 | 75,295 | 1909.20 | 669 | 736 | 718 | 90 |
| 4/27/2019 | 75,321 | 1909.30 | 627 | 660 | 698 | 90 |
| 4/28/2019 | 75,302 | 1909.20 | 601 | 625 | 627 | 90 |
| 4/29/2019 | 75,283 | 1909.20 | 576 | 612 | 618 | 90 |
| 4/30/2019 | 75,242 | 1909.20 | 526 | 572 | 586 | 91 |
| 5/1/2019 | 75,290 | 1909.20 | 461 | 466 | 461 | 91 |
| 5/2/2019 | 75,271 | 1909.20 | 418 | 454 | 413 | 91 |
| 5/3/2019 | 75,319 | 1909.30 | 402 | 420 | 383 | 91 |
| 5/4/2019 | 75,345 | 1909.30 | 387 | 402 | 342 | 91 |
| 5/5/2019 | 75,326 | 1909.30 | 371 | 427 | 362 | 91 |
| 5/6/2019 | 75,330 | 1909.30 | 364 | 398 | 334 | 91 |
| 5/7/2019 | 75,377 | 1909.30 | 366 | 376 | 294 | 91 |
| 5/8/2019 | 75,359 | 1909.30 | 362 | 403 | 308 | 91 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/9/2019 | 75,318 | 1909.30 | 348 | 406 | 324 | 91 |
| 5/10/2019 | 75,277 | 1909.20 | 319 | 382 | 288 | 90 |
| 5/11/2019 | 75,192 | 1909.20 | 297 | 379 | 276 | 90 |
| 5/12/2019 | 75,062 | 1909.10 | 279 | 381 | 275 | 90 |
| 5/13/2019 | 74,931 | 1909.10 | 318 | 379 | 272 | 90 |
| 5/14/2019 | 74,820 | 1909.00 | 263 | 380 | 251 | 110 |
| 5/15/2019 | 74,953 | 1909.10 | 512 | 494 | 272 | 130 |
| 5/16/2019 | 74,776 | 1909.00 | 1,485 | 1,715 | 1,507 | 130 |
| 5/17/2019 | 74,512 | 1908.90 | 1,195 | 1,407 | 1,448 | 129 |
| 5/18/2019 | 74,776 | 1909.00 | 1,103 | 1,047 | 1,048 | 130 |
| 5/19/2019 | 75,041 | 1909.10 | 1,447 | 1,381 | 1,361 | 130 |
| 5/20/2019 | 74,887 | 1909.10 | 1,022 | 1,168 | 1,157 | 130 |
| 5/21/2019 | 74,931 | 1909.10 | 1,154 | 1,217 | 1,273 | 131 |
| 5/22/2019 | 74,931 | 1909.10 | 1,147 | 1,214 | 1,362 | 130 |
| 5/23/2019 | 75,063 | 1909.10 | 910 | 877 | 960 | 130 |
| 5/24/2019 | 75,108 | 1909.20 | 796 | 823 | 863 | 130 |
| 5/25/2019 | 75,285 | 1909.20 | 736 | 674 | 737 | 130 |
| 5/26/2019 | 75,285 | 1909.20 | 651 | 675 | 677 | 130 |
| 5/27/2019 | 75,241 | 1909.20 | 572 | 636 | 653 | 130 |
| 5/28/2019 | 75,285 | 1909.20 | 517 | 530 | 542 | 131 |
| 5/29/2019 | 75,374 | 1909.30 | 530 | 531 | 482 | 135 |
| 5/30/2019 | 75,440 | 1909.30 | 637 | 657 | 642 | 131 |
| 5/31/2019 | 75,285 | 1909.20 | 603 | 695 | 693 | 131 |
| 6/1/2019 | 75,263 | 1909.20 | 480 | 525 | 501 | 131 |
| 6/2/2019 | 75,241 | 1909.20 | 484 | 540 | 454 | 131 |
| 6/3/2019 | 75,087 | 1909.20 | 422 | 541 | 521 | 131 |
| 6/4/2019 | 75,155 | 1909.20 | 373 | 376 | 312 | 130 |
| 6/5/2019 | 75,268 | 1909.20 | 329 | 316 | 191 | 130 |
| 6/6/2019 | 75,203 | 1909.20 | 301 | 355 | 231 | 130 |
| 6/7/2019 | 75,250 | 1909.20 | 270 | 306 | 195 | 130 |
| 6/8/2019 | 75,340 | 1909.30 | 259 | 260 | 160 | 130 |
| 6/9/2019 | 75,364 | 1909.30 | 246 | 259 | 151 | 130 |
| 6/10/2019 | 75,389 | 1909.30 | 232 | 270 | 160 | 130 |
| 6/11/2019 | 75,391 | 1909.30 | 221 | 262 | 152 | 129 |
| 6/12/2019 | 75,415 | 1909.30 | 210 | 242 | 139 | 129 |
| 6/13/2019 | 75,394 | 1909.30 | 190 | 219 | 130 | 129 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/14/2019 | 75,396 | 1909.30 | 186 | 223 | 121 | 129 |
| 6/15/2019 | 75,421 | 1909.30 | 180 | 204 | 110 | 129 |
| 6/16/2019 | 75,400 | 1909.30 | 178 | 226 | 111 | 130 |
| 6/17/2019 | 75,336 | 1909.30 | 168 | 241 | 134 | 129 |
| 6/18/2019 | 75,293 | 1909.20 | 155 | 207 | 109 | 129 |
| 6/19/2019 | 75,251 | 1909.20 | 140 | 188 | 91 | 129 |
| 6/20/2019 | 75,187 | 1909.20 | 126 | 180 | 80 | 129 |
| 6/21/2019 | 75,122 | 1909.20 | 153 | 185 | 82 | 130 |
| 6/22/2019 | 75,014 | 1909.10 | 134 | 189 | 84 | 130 |
| 6/23/2019 | 74,905 | 1909.10 | 110 | 187 | 84 | 129 |
| 6/24/2019 | 74,819 | 1909.00 | 98 | 178 | 78 | 129 |
| 6/25/2019 | 74,710 | 1909.00 | 98 | 173 | 70 | 129 |
| 6/26/2019 | 74,602 | 1908.90 | 106 | 160 | 64 | 129 |
| 6/27/2019 | 74,494 | 1908.90 | 108 | 160 | 59 | 130 |
| 6/28/2019 | 74,408 | 1908.80 | 108 | 165 | 64 | 129 |
| 6/29/2019 | 74,300 | 1908.80 | 108 | 165 | 63 | 130 |
| 6/30/2019 | 74,170 | 1908.70 | 102 | 165 | 62 | 129 |
| 7/1/2019 | 74,041 | 1908.70 | 94 | 170 | 63 | 130 |
| 7/2/2019 | 73,911 | 1908.60 | 94 | 169 | 64 | 130 |
| 7/3/2019 | 73,761 | 1908.50 | 94 | 165 | 61 | 130 |
| 7/4/2019 | 73,676 | 1908.50 | 91 | 165 | 60 | 130 |
| 7/5/2019 | 73,504 | 1908.40 | 85 | 165 | 59 | 130 |
| 7/6/2019 | 73,311 | 1908.30 | 79 | 175 | 62 | 130 |
| 7/7/2019 | 73,118 | 1908.30 | 74 | 179 | 67 | 130 |
| 7/8/2019 | 72,926 | 1908.20 | 74 | 179 | 66 | 130 |
| 7/9/2019 | 72,734 | 1908.10 | 75 | 177 | 66 | 130 |
| 7/10/2019 | 72,564 | 1908.00 | 74 | 177 | 65 | 130 |
| 7/11/2019 | 72,351 | 1907.90 | 74 | 179 | 65 | 129 |
| 7/12/2019 | 72,160 | 1907.80 | 67 | 180 | 66 | 130 |
| 7/13/2019 | 71,948 | 1907.70 | 64 | 180 | 65 | 130 |
| 7/14/2019 | 71,714 | 1907.60 | 57 | 180 | 63 | 130 |
| 7/15/2019 | 71,502 | 1907.50 | 56 | 178 | 64 | 130 |
| 7/16/2019 | 71,248 | 1907.40 | 58 | 181 | 63 | 130 |
| 7/17/2019 | 71,037 | 1907.30 | 57 | 183 | 65 | 130 |
| 7/18/2019 | 70,806 | 1907.20 | 66 | 183 | 62 | 130 |
| 7/19/2019 | 70,617 | 1907.10 | 88 | 183 | 62 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/20/2019 | 70,323 | 1906.90 | 35 | 183 | 62 | 130 |
| 7/21/2019 | 70,071 | 1906.80 | 56 | 183 | 62 | 130 |
| 7/22/2019 | 69,821 | 1906.70 | 57 | 183 | 61 | 130 |
| 7/23/2019 | 69,570 | 1906.60 | 37 | 182 | 61 | 130 |
| 7/24/2019 | 69,319 | 1906.50 | 39 | 181 | 60 | 130 |
| 7/25/2019 | 69,047 | 1906.30 | 42 | 181 | 59 | 130 |
| 7/26/2019 | 68,735 | 1906.20 | 30 | 183 | 60 | 130 |
| 7/27/2019 | 68,465 | 1906.10 | 37 | 186 | 62 | 130 |
| 7/28/2019 | 68,196 | 1905.90 | 34 | 185 | 62 | 130 |
| 7/29/2019 | 67,885 | 1905.80 | 81 | 185 | 61 | 131 |
| 7/30/2019 | 67,595 | 1905.60 | 27 | 184 | 61 | 131 |
| 7/31/2019 | 67,366 | 1905.50 | 28 | 184 | 60 | 130 |
| 8/1/2019 | 67,033 | 1905.40 | 22 | 184 | 59 | 131 |
| 8/2/2019 | 66,744 | 1905.20 | 25 | 178 | 56 | 131 |
| 8/3/2019 | 66,455 | 1905.10 | 23 | 175 | 51 | 130 |
| 8/4/2019 | 66,167 | 1904.90 | 23 | 174 | 52 | 130 |
| 8/5/2019 | 65,900 | 1904.80 | 27 | 174 | 51 | 130 |
| 8/6/2019 | 65,634 | 1904.70 | 22 | 174 | 51 | 130 |
| 8/7/2019 | 65,327 | 1904.50 | 16 | 169 | 48 | 130 |
| 8/8/2019 | 65,041 | 1904.40 | 12 | 167 | 46 | 131 |
| 8/9/2019 | 64,755 | 1904.30 | 22 | 167 | 47 | 131 |
| 8/10/2019 | 64,491 | 1904.10 | 20 | 166 | 46 | 132 |
| 8/11/2019 | 64,207 | 1904.00 | 23 | 166 | 46 | 131 |
| 8/12/2019 | 63,945 | 1903.90 | 21 | 163 | 45 | 130 |
| 8/13/2019 | 63,663 | 1903.70 | 25 | 163 | 44 | 130 |
| 8/14/2019 | 63,421 | 1903.60 | 18 | 162 | 41 | 131 |
| 8/15/2019 | 63,139 | 1903.50 | 18 | 162 | 42 | 130 |
| 8/16/2019 | 62,838 | 1903.30 | 18 | 163 | 41 | 130 |
| 8/17/2019 | 62,578 | 1903.20 | 15 | 160 | 40 | 130 |
| 8/18/2019 | 62,299 | 1903.00 | 12 | 157 | 36 | 130 |
| 8/19/2019 | 62,021 | 1902.90 | 1 | 157 | 36 | 130 |
| 8/20/2019 | 61,743 | 1902.80 | 13 | 157 | 34 | 130 |
| 8/21/2019 | 61,486 | 1902.60 | 11 | 156 | 34 | 130 |
| 8/22/2019 | 61,209 | 1902.50 | 18 | 154 | 33 | 130 |
| 8/23/2019 | 60,933 | 1902.30 | 12 | 153 | 29 | 131 |
| 8/24/2019 | 60,658 | 1902.20 | 12 | 156 | 30 | 131 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/25/2019 | 60,384 | 1902.10 | 11 | 157 | 30 | 131 |
| 8/26/2019 | 60,111 | 1901.90 | 13 | 157 | 30 | 131 |
| 8/27/2019 | 59,835 | 1901.80 | 7 | 152 | 27 | 131 |
| 8/28/2019 | 59,538 | 1901.60 | -1 | 153 | 24 | 131 |
| 8/29/2019 | 59,241 | 1901.50 | -5 | 155 | 28 | 131 |
| 8/30/2019 | 58,964 | 1901.30 | 0 | 155 | 28 | 131 |
| 8/31/2019 | 58,688 | 1901.20 | -3 | 155 | 28 | 131 |
| 9/1/2019 | 58,375 | 1901.00 | -3 | 154 | 27 | 131 |
| 9/2/2019 | 58,101 | 1900.90 | -4 | 154 | 27 | 131 |
| 9/3/2019 | 57,809 | 1900.70 | -4 | 155 | 27 | 131 |
| 9/4/2019 | 57,537 | 1900.60 | 1 | 154 | 26 | 131 |
| 9/5/2019 | 57,227 | 1900.40 | -7 | 155 | 26 | 131 |
| 9/6/2019 | 56,957 | 1900.30 | -6 | 154 | 26 | 130 |
| 9/7/2019 | 56,649 | 1900.10 | -12 | 155 | 27 | 131 |
| 9/8/2019 | 56,361 | 1900.00 | -9 | 155 | 27 | 131 |
| 9/9/2019 | 56,056 | 1899.80 | -1 | 176 | 32 | 138 |
| 9/10/2019 | 55,714 | 1899.60 | 1 | 181 | 30 | 151 |
| 9/11/2019 | 55,411 | 1899.50 | 6 | 181 | 28 | 153 |
| 9/12/2019 | 55,109 | 1899.30 | 7 | 174 | 37 | 137 |
| 9/13/2019 | 54,826 | 1899.10 | 1 | 162 | 33 | 131 |
| 9/14/2019 | 54,526 | 1899.00 | -1 | 158 | 25 | 132 |
| 9/15/2019 | 54,246 | 1898.80 | -6 | 159 | 25 | 132 |
| 9/16/2019 | 53,984 | 1898.70 | 11 | 160 | 26 | 134 |
| 9/17/2019 | 53,722 | 1898.50 | 9 | 159 | 24 | 134 |
| 9/18/2019 | 53,461 | 1898.40 | 15 | 160 | 25 | 134 |
| 9/19/2019 | 53,219 | 1898.30 | 11 | 159 | 26 | 134 |
| 9/20/2019 | 52,941 | 1898.10 | 7 | 160 | 26 | 133 |
| 9/21/2019 | 52,682 | 1898.00 | 7 | 160 | 27 | 132 |
| 9/22/2019 | 52,406 | 1897.80 | -1 | 160 | 25 | 132 |
| 9/23/2019 | 52,132 | 1897.70 | 0 | 158 | 25 | 132 |
| 9/24/2019 | 51,838 | 1897.50 | 2 | 158 | 25 | 132 |
| 9/25/2019 | 51,582 | 1897.40 | 0 | 158 | 24 | 132 |
| 9/26/2019 | 51,329 | 1897.20 | -7 | 146 | 40 | 110 |
| 9/27/2019 | 51,059 | 1897.10 | -14 | 133 | 46 | 91 |
| 9/28/2019 | 50,825 | 1896.90 | -20 | 125 | 41 | 91 |
| 9/29/2019 | 50,628 | 1896.80 | 4 | 121 | 38 | 92 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/30/2019 | 50,395 | 1896.70 | -6 | 120 | 37 | 91 |
| 10/1/2019 | 50,198 | 1896.60 | 21 | 121 | 38 | 90 |
| 10/2/2019 | 49,983 | 1896.50 | 12 | 120 | 37 | 90 |
| 10/3/2019 | 49,769 | 1896.30 | 13 | 121 | 37 | 90 |
| 10/4/2019 | 49,556 | 1896.20 | 14 | 121 | 37 | 90 |
| 10/5/2019 | 49,360 | 1896.10 | 21 | 120 | 36 | 89 |
| 10/6/2019 | 49,183 | 1896.00 | 29 | 118 | 34 | 88 |
| 10/7/2019 | 49,006 | 1895.90 | 29 | 118 | 32 | 90 |
| 10/8/2019 | 48,778 | 1895.80 | 3 | 119 | 30 | 92 |
| 10/9/2019 | 48,567 | 1895.60 | 13 | 119 | 30 | 92 |
| 10/10/2019 | 48,356 | 1895.50 | 18 | 124 | 32 | 92 |
| 10/11/2019 | 48,145 | 1895.40 | 20 | 126 | 37 | 92 |
| 10/12/2019 | 47,918 | 1895.30 | 11 | 126 | 38 | 91 |
| 10/13/2019 | 47,709 | 1895.20 | 20 | 126 | 39 | 91 |
| 10/14/2019 | 47,500 | 1895.00 | 21 | 126 | 38 | 91 |
| 10/15/2019 | 47,310 | 1894.90 | 29 | 125 | 39 | 90 |
| 10/16/2019 | 47,103 | 1894.80 | 16 | 120 | 41 | 84 |
| 10/17/2019 | 46,882 | 1894.70 | 11 | 123 | 46 | 80 |
| 10/18/2019 | 46,729 | 1894.60 | 37 | 115 | 42 | 80 |
| 10/19/2019 | 46,541 | 1894.50 | 16 | 110 | 34 | 80 |
| 10/20/2019 | 46,371 | 1894.40 | 25 | 111 | 34 | 80 |
| 10/21/2019 | 46,219 | 1894.30 | 34 | 111 | 34 | 80 |
| 10/22/2019 | 46,100 | 1894.20 | 39 | 99 | 34 | 71 |
| 10/23/2019 | 45,948 | 1894.10 | 29 | 106 | 38 | 65 |
| 10/24/2019 | 45,746 | 1894.00 | 10 | 112 | 33 | 80 |
| 10/25/2019 | 45,578 | 1893.90 | 27 | 112 | 33 | 81 |
| 10/26/2019 | 45,399 | 1893.80 | 21 | 111 | 34 | 80 |
| 10/27/2019 | 45,194 | 1893.70 | 9 | 112 | 34 | 80 |
| 10/28/2019 | 45,010 | 1893.60 | 19 | 111 | 34 | 80 |
| 10/29/2019 | 44,843 | 1893.50 | 27 | 111 | 35 | 80 |
| 10/30/2019 | 44,659 | 1893.30 | 18 | 111 | 35 | 80 |
| 10/31/2019 | 44,493 | 1893.20 | 27 | 111 | 35 | 80 |
| 11/1/2019 | 44,311 | 1893.10 | 19 | 111 | 35 | 80 |
| 11/2/2019 | 44,146 | 1893.00 | 27 | 111 | 35 | 80 |
| 11/3/2019 | 43,981 | 1892.90 | 28 | 111 | 36 | 79 |
| 11/4/2019 | 43,833 | 1892.80 | 37 | 111 | 36 | 79 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/5/2019 | 43,653 | 1892.70 | 20 | 111 | 35 | 80 |
| 11/6/2019 | 43,506 | 1892.70 | 37 | 111 | 35 | 80 |
| 11/7/2019 | 43,326 | 1892.50 | 20 | 111 | 34 | 79 |
| 11/8/2019 | 43,162 | 1892.40 | 28 | 111 | 35 | 79 |
| 11/9/2019 | 43,015 | 1892.30 | 37 | 111 | 35 | 80 |
| 11/10/2019 | 42,836 | 1892.20 | 21 | 112 | 34 | 80 |
| 11/11/2019 | 42,658 | 1892.10 | 24 | 114 | 36 | 80 |
| 11/12/2019 | 42,367 | 1891.90 | 36 | 183 | 42 | 112 |
| 11/13/2019 | 42,014 | 1891.70 | 41 | 219 | 47 | 167 |
| 11/14/2019 | 41,645 | 1891.50 | 30 | 216 | 41 | 169 |
| 11/15/2019 | 41,405 | 1891.30 | 52 | 172 | 41 | 151 |
| 11/16/2019 | 41,215 | 1891.20 | 23 | 119 | 46 | 80 |
| 11/17/2019 | 41,040 | 1891.10 | 31 | 119 | 44 | 81 |
| 11/18/2019 | 40,866 | 1891.00 | 31 | 119 | 44 | 81 |
| 11/19/2019 | 40,551 | 1890.80 | -2 | 157 | 44 | 92 |
| 11/20/2019 | 40,238 | 1890.60 | 56 | 214 | 45 | 164 |
| 11/21/2019 | 39,893 | 1890.40 | 39 | 213 | 42 | 168 |
| 11/22/2019 | 39,551 | 1890.20 | 39 | 211 | 43 | 167 |
| 11/23/2019 | 39,211 | 1889.90 | 34 | 205 | 40 | 166 |
| 11/24/2019 | 38,857 | 1889.70 | 26 | 204 | 39 | 165 |
| 11/25/2019 | 38,505 | 1889.50 | 26 | 204 | 38 | 165 |
| 11/26/2019 | 38,398 | 1889.40 | 160 | 214 | 45 | 166 |
| 11/27/2019 | 38,216 | 1889.30 | 131 | 223 | 57 | 166 |
| 11/28/2019 | 37,913 | 1889.10 | 79 | 232 | 46 | 166 |
| 11/29/2019 | 37,551 | 1888.90 | 54 | 236 | 53 | 166 |
| 11/30/2019 | 37,252 | 1888.70 | 91 | 242 | 65 | 167 |
| 12/1/2019 | 37,401 | 1888.80 | 327 | 252 | 121 | 167 |
| 12/2/2019 | 37,446 | 1888.80 | 277 | 254 | 129 | 167 |
| 12/3/2019 | 37,327 | 1888.70 | 199 | 259 | 131 | 167 |
| 12/4/2019 | 37,013 | 1888.50 | 107 | 265 | 133 | 167 |
| 12/5/2019 | 36,672 | 1888.30 | 100 | 272 | 135 | 167 |
| 12/6/2019 | 36,672 | 1888.30 | 275 | 275 | 140 | 167 |
| 12/7/2019 | 41,677 | 1891.50 | 2,820 | 297 | 428 | 167 |
| 12/8/2019 | 43,981 | 1892.90 | 1,468 | 307 | 381 | 166 |
| 12/9/2019 | 44,361 | 1893.20 | 498 | 307 | 219 | 166 |
| 12/10/2019 | 44,377 | 1893.20 | 313 | 304 | 187 | 166 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/11/2019 | 44,444 | 1893.20 | 353 | 319 | 195 | 166 |
| 12/12/2019 | 44,859 | 1893.50 | 556 | 346 | 254 | 196 |
| 12/13/2019 | 45,294 | 1893.70 | 569 | 350 | 229 | 217 |
| 12/14/2019 | 45,948 | 1894.10 | 681 | 351 | 280 | 218 |
| 12/15/2019 | 46,033 | 1894.20 | 393 | 350 | 204 | 218 |
| 12/16/2019 | 45,914 | 1894.10 | 291 | 350 | 183 | 219 |
| 12/17/2019 | 45,679 | 1894.00 | 230 | 349 | 205 | 190 |
| 12/18/2019 | 45,511 | 1893.90 | 246 | 331 | 260 | 130 |
| 12/19/2019 | 45,461 | 1893.80 | 291 | 316 | 251 | 130 |
| 12/20/2019 | 45,578 | 1893.90 | 362 | 303 | 255 | 129 |
| 12/21/2019 | 45,511 | 1893.90 | 262 | 296 | 229 | 130 |
| 12/22/2019 | 45,662 | 1893.90 | 369 | 293 | 238 | 130 |
| 12/23/2019 | 45,713 | 1894.00 | 314 | 289 | 232 | 130 |
| 12/24/2019 | 45,629 | 1893.90 | 243 | 286 | 214 | 130 |
| 12/25/2019 | 45,545 | 1893.90 | 240 | 282 | 212 | 129 |
| 12/26/2019 | 45,377 | 1893.80 | 193 | 278 | 198 | 129 |
| 12/27/2019 | 45,160 | 1893.60 | 162 | 271 | 190 | 130 |
| 12/28/2019 | 44,943 | 1893.50 | 159 | 268 | 185 | 129 |
| 12/29/2019 | 44,826 | 1893.40 | 211 | 270 | 193 | 129 |
| 12/30/2019 | 44,709 | 1893.40 | 211 | 270 | 200 | 130 |
| 12/31/2019 | 44,510 | 1893.30 | 168 | 269 | 189 | 130 |
| 1/1/2020 | 44,295 | 1893.10 | 160 | 268 | 186 | 129 |
| 1/2/2020 | 44,097 | 1893.00 | 169 | 268 | 184 | 129 |
| 1/3/2020 | 43,850 | 1892.90 | 143 | 267 | 181 | 129 |
| 1/4/2020 | 43,620 | 1892.70 | 151 | 267 | 184 | 129 |
| 1/5/2020 | 43,358 | 1892.60 | 134 | 267 | 180 | 129 |
| 1/6/2020 | 43,113 | 1892.40 | 130 | 254 | 177 | 115 |
| 1/7/2020 | 42,950 | 1892.30 | 159 | 241 | 175 | 85 |
| 1/8/2020 | 42,836 | 1892.20 | 179 | 236 | 170 | 80 |
| 1/9/2020 | 42,804 | 1892.20 | 214 | 231 | 173 | 80 |
| 1/10/2020 | 42,690 | 1892.10 | 180 | 237 | 175 | 80 |
| 1/11/2020 | 42,545 | 1892.10 | 165 | 238 | 177 | 80 |
| 1/12/2020 | 42,432 | 1892.00 | 181 | 238 | 175 | 81 |
| 1/13/2020 | 42,367 | 1891.90 | 205 | 238 | 173 | 92 |
| 1/14/2020 | 42,367 | 1891.90 | 239 | 239 | 223 | 100 |
| 1/15/2020 | 42,319 | 1891.90 | 217 | 241 | 199 | 100 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/16/2020 | 43,097 | 1892.40 | 640 | 247 | 309 | 106 |
| 1/17/2020 | 43,522 | 1892.70 | 466 | 251 | 349 | 91 |
| 1/18/2020 | 43,719 | 1892.80 | 349 | 250 | 312 | 79 |
| 1/19/2020 | 43,899 | 1892.90 | 335 | 244 | 275 | 79 |
| 1/20/2020 | 44,179 | 1893.10 | 383 | 242 | 269 | 79 |
| 1/21/2020 | 45,377 | 1893.80 | 846 | 242 | 370 | 79 |
| 1/22/2020 | 46,422 | 1894.40 | 794 | 267 | 394 | 99 |
| 1/23/2020 | 46,899 | 1894.70 | 556 | 315 | 313 | 161 |
| 1/24/2020 | 47,377 | 1895.00 | 579 | 338 | 271 | 205 |
| 1/25/2020 | 48,551 | 1895.60 | 932 | 340 | 324 | 211 |
| 1/26/2020 | 52,898 | 1898.10 | 2,538 | 347 | 800 | 209 |
| 1/27/2020 | 54,853 | 1899.20 | 1,333 | 348 | 504 | 209 |
| 1/28/2020 | 56,190 | 1899.90 | 1,023 | 348 | 430 | 209 |
| 1/29/2020 | 56,792 | 1900.20 | 812 | 509 | 484 | 217 |
| 1/30/2020 | 56,829 | 1900.20 | 665 | 645 | 624 | 224 |
| 1/31/2020 | 56,735 | 1900.20 | 578 | 626 | 596 | 220 |
| 2/1/2020 | 56,603 | 1900.10 | 499 | 565 | 507 | 220 |
| 2/2/2020 | 56,472 | 1900.00 | 436 | 502 | 417 | 220 |
| 2/3/2020 | 56,322 | 1900.00 | 364 | 440 | 330 | 226 |
| 2/4/2020 | 56,247 | 1899.90 | 338 | 376 | 263 | 218 |
| 2/5/2020 | 56,190 | 1899.90 | 280 | 308 | 294 | 125 |
| 2/6/2020 | 56,491 | 1900.00 | 410 | 259 | 272 | 80 |
| 2/7/2020 | 56,303 | 1899.90 | 166 | 261 | 228 | 94 |
| 2/8/2020 | 56,303 | 1899.90 | 277 | 277 | 209 | 126 |
| 2/9/2020 | 56,303 | 1899.90 | 266 | 266 | 183 | 140 |
| 2/10/2020 | 56,184 | 1899.90 | 196 | 256 | 174 | 140 |
| 2/11/2020 | 56,066 | 1899.80 | 194 | 254 | 177 | 125 |
| 2/12/2020 | 55,947 | 1899.80 | 187 | 247 | 181 | 73 |
| 2/13/2020 | 55,891 | 1899.70 | 209 | 237 | 187 | 46 |
| 2/14/2020 | 55,873 | 1899.70 | 225 | 234 | 182 | 45 |
| 2/15/2020 | 55,817 | 1899.70 | 206 | 234 | 181 | 45 |
| 2/16/2020 | 55,780 | 1899.70 | 209 | 228 | 178 | 45 |
| 2/17/2020 | 55,705 | 1899.60 | 186 | 224 | 173 | 45 |
| 2/18/2020 | 55,668 | 1899.60 | 205 | 224 | 172 | 45 |
| 2/19/2020 | 55,575 | 1899.50 | 177 | 224 | 171 | 45 |
| 2/20/2020 | 55,482 | 1899.50 | 173 | 220 | 169 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/21/2020 | 55,426 | 1899.50 | 179 | 208 | 162 | 45 |
| 2/22/2020 | 55,352 | 1899.40 | 162 | 200 | 156 | 36 |
| 2/23/2020 | 55,296 | 1899.40 | 158 | 186 | 168 | 22 |
| 2/24/2020 | 55,296 | 1899.40 | 146 | 146 | 157 | 20 |
| 2/25/2020 | 55,278 | 1899.40 | 134 | 143 | 150 | 19 |
| 2/26/2020 | 55,296 | 1899.40 | 146 | 137 | 148 | 19 |
| 2/27/2020 | 55,333 | 1899.40 | 148 | 129 | 142 | 18 |
| 2/28/2020 | 55,352 | 1899.40 | 132 | 123 | 136 | 18 |
| 2/29/2020 | 55,370 | 1899.40 | 131 | 121 | 134 | 17 |
| 3/1/2020 | 55,389 | 1899.40 | 131 | 121 | 134 | 17 |
| 3/2/2020 | 55,370 | 1899.40 | 112 | 121 | 133 | 17 |
| 3/3/2020 | 55,315 | 1899.40 | 133 | 161 | 146 | 16 |
| 3/4/2020 | 55,185 | 1899.30 | 128 | 194 | 174 | 26 |
| 3/5/2020 | 55,056 | 1899.30 | 128 | 193 | 155 | 32 |
| 3/6/2020 | 54,926 | 1899.20 | 128 | 193 | 145 | 46 |
| 3/7/2020 | 54,816 | 1899.10 | 135 | 191 | 145 | 46 |
| 3/8/2020 | 54,705 | 1899.10 | 127 | 183 | 141 | 45 |
| 3/9/2020 | 54,613 | 1899.00 | 128 | 174 | 135 | 45 |
| 3/10/2020 | 54,503 | 1899.00 | 113 | 168 | 131 | 45 |
| 3/11/2020 | 54,411 | 1898.90 | 121 | 167 | 128 | 45 |
| 3/12/2020 | 54,301 | 1898.90 | 109 | 165 | 127 | 45 |
| 3/13/2020 | 54,192 | 1898.80 | 107 | 162 | 123 | 45 |
| 3/14/2020 | 54,155 | 1898.80 | 144 | 163 | 127 | 45 |
| 3/15/2020 | 54,192 | 1898.80 | 182 | 163 | 135 | 45 |
| 3/16/2020 | 54,137 | 1898.80 | 142 | 170 | 131 | 48 |
| 3/17/2020 | 54,119 | 1898.80 | 164 | 174 | 131 | 55 |
| 3/18/2020 | 54,155 | 1898.80 | 192 | 174 | 135 | 55 |
| 3/19/2020 | 54,135 | 1898.80 | 168 | 178 | 136 | 55 |
| 3/20/2020 | 54,064 | 1898.70 | 149 | 185 | 138 | 55 |
| 3/21/2020 | 53,991 | 1898.70 | 152 | 188 | 139 | 55 |
| 3/22/2020 | 53,918 | 1898.60 | 158 | 194 | 142 | 55 |
| 3/23/2020 | 53,826 | 1898.60 | 148 | 195 | 142 | 55 |
| 3/24/2020 | 53,826 | 1898.60 | 195 | 195 | 145 | 55 |
| 3/25/2020 | 53,790 | 1898.60 | 177 | 196 | 151 | 55 |
| 3/26/2020 | 53,717 | 1898.50 | 158 | 195 | 146 | 55 |
| 3/27/2020 | 53,589 | 1898.50 | 148 | 212 | 148 | 57 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/28/2020 | 53,479 | 1898.40 | 159 | 214 | 149 | 64 |
| 3/29/2020 | 53,406 | 1898.40 | 174 | 211 | 146 | 64 |
| 3/30/2020 | 53,406 | 1898.40 | 204 | 204 | 150 | 56 |
| 3/31/2020 | 53,516 | 1898.40 | 251 | 196 | 152 | 52 |
| 4/1/2020 | 53,662 | 1898.50 | 267 | 193 | 154 | 52 |
| 4/2/2020 | 53,698 | 1898.50 | 229 | 211 | 155 | 52 |
| 4/3/2020 | 53,625 | 1898.50 | 189 | 226 | 162 | 61 |
| 4/4/2020 | 53,735 | 1898.50 | 286 | 231 | 178 | 64 |
| 4/5/2020 | 54,338 | 1898.90 | 532 | 228 | 255 | 64 |
| 4/6/2020 | 54,632 | 1899.00 | 383 | 235 | 257 | 64 |
| 4/7/2020 | 54,742 | 1899.10 | 297 | 241 | 255 | 57 |
| 4/8/2020 | 54,760 | 1899.10 | 254 | 245 | 252 | 53 |
| 4/9/2020 | 54,816 | 1899.10 | 272 | 245 | 253 | 48 |
| 4/10/2020 | 54,853 | 1899.20 | 262 | 243 | 245 | 46 |
| 4/11/2020 | 54,889 | 1899.20 | 261 | 243 | 238 | 45 |
| 4/12/2020 | 55,000 | 1899.20 | 299 | 243 | 232 | 45 |
| 4/13/2020 | 55,111 | 1899.30 | 300 | 244 | 229 | 46 |
| 4/14/2020 | 55,074 | 1899.30 | 227 | 246 | 232 | 46 |
| 4/15/2020 | 55,000 | 1899.20 | 206 | 243 | 235 | 38 |
| 4/16/2020 | 54,939 | 1899.20 | 208 | 239 | 228 | 34 |
| 4/17/2020 | 54,834 | 1899.10 | 184 | 237 | 218 | 34 |
| 4/18/2020 | 54,760 | 1899.10 | 196 | 233 | 208 | 34 |
| 4/19/2020 | 54,668 | 1899.10 | 181 | 228 | 194 | 34 |
| 4/20/2020 | 54,613 | 1899.00 | 194 | 221 | 188 | 34 |
| 4/21/2020 | 54,521 | 1899.00 | 159 | 205 | 179 | 34 |
| 4/22/2020 | 54,485 | 1899.00 | 168 | 186 | 164 | 34 |
| 4/23/2020 | 54,393 | 1898.90 | 131 | 177 | 157 | 34 |
| 4/24/2020 | 54,338 | 1898.90 | 139 | 167 | 150 | 35 |
| 4/25/2020 | 54,265 | 1898.80 | 138 | 175 | 147 | 39 |
| 4/26/2020 | 54,173 | 1898.80 | 128 | 174 | 142 | 50 |
| 4/27/2020 | 54,082 | 1898.70 | 128 | 174 | 137 | 54 |
| 4/28/2020 | 53,991 | 1898.70 | 126 | 172 | 132 | 60 |
| 4/29/2020 | 53,881 | 1898.60 | 112 | 167 | 127 | 60 |
| 4/30/2020 | 53,790 | 1898.60 | 113 | 159 | 127 | 53 |
| 5/1/2020 | 53,698 | 1898.50 | 108 | 154 | 125 | 50 |
| 5/2/2020 | 53,589 | 1898.50 | 95 | 151 | 122 | 50 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/3/2020 | 53,497 | 1898.40 | 101 | 147 | 121 | 50 |
| 5/4/2020 | 53,388 | 1898.40 | 91 | 146 | 118 | 50 |
| 5/5/2020 | 53,260 | 1898.30 | 83 | 148 | 117 | 50 |
| 5/6/2020 | 53,133 | 1898.20 | 87 | 151 | 118 | 50 |
| 5/7/2020 | 53,024 | 1898.20 | 97 | 152 | 117 | 50 |
| 5/8/2020 | 52,879 | 1898.10 | 80 | 153 | 117 | 50 |
| 5/9/2020 | 52,771 | 1898.00 | 100 | 154 | 116 | 50 |
| 5/10/2020 | 52,575 | 1897.90 | 57 | 156 | 116 | 50 |
| 5/11/2020 | 52,464 | 1897.80 | 102 | 158 | 119 | 51 |
| 5/12/2020 | 52,410 | 1897.80 | 124 | 151 | 132 | 40 |
| 5/13/2020 | 52,357 | 1897.80 | 121 | 148 | 132 | 36 |
| 5/14/2020 | 52,285 | 1897.70 | 117 | 154 | 136 | 36 |
| 5/15/2020 | 52,195 | 1897.70 | 103 | 148 | 130 | 35 |
| 5/16/2020 | 52,105 | 1897.70 | 121 | 167 | 135 | 35 |
| 5/17/2020 | 52,123 | 1897.70 | 187 | 178 | 159 | 35 |
| 5/18/2020 | 53,061 | 1898.20 | 648 | 175 | 272 | 29 |
| 5/19/2020 | 53,576 | 1898.50 | 471 | 211 | 289 | 25 |
| 5/20/2020 | 53,753 | 1898.60 | 331 | 242 | 271 | 25 |
| 5/21/2020 | 53,698 | 1898.50 | 233 | 261 | 288 | 25 |
| 5/22/2020 | 53,497 | 1898.40 | 179 | 281 | 305 | 24 |
| 5/23/2020 | 53,279 | 1898.30 | 174 | 284 | 303 | 24 |
| 5/24/2020 | 52,988 | 1898.10 | 140 | 287 | 300 | 24 |
| 5/25/2020 | 52,699 | 1898.00 | 142 | 288 | 299 | 24 |
| 5/26/2020 | 52,357 | 1897.80 | 128 | 300 | 293 | 32 |
| 5/27/2020 | 52,015 | 1897.60 | 125 | 297 | 281 | 46 |
| 5/28/2020 | 51,673 | 1897.40 | 95 | 267 | 251 | 45 |
| 5/29/2020 | 51,369 | 1897.20 | 96 | 249 | 221 | 45 |
| 5/30/2020 | 51,119 | 1897.10 | 113 | 239 | 196 | 45 |
| 5/31/2020 | 50,887 | 1897.00 | 114 | 230 | 184 | 45 |
| 6/1/2020 | 50,710 | 1896.90 | 123 | 213 | 168 | 45 |
| 6/2/2020 | 50,515 | 1896.80 | 97 | 195 | 154 | 45 |
| 6/3/2020 | 50,356 | 1896.70 | 98 | 178 | 145 | 45 |
| 6/4/2020 | 50,197 | 1896.60 | 83 | 163 | 129 | 50 |
| 6/5/2020 | 50,002 | 1896.50 | 61 | 159 | 114 | 59 |
| 6/6/2020 | 49,844 | 1896.40 | 71 | 151 | 107 | 61 |
| 6/7/2020 | 49,632 | 1896.30 | 40 | 147 | 101 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/8/2020 | 49,492 | 1896.20 | 76 | 146 | 100 | 61 |
| 6/9/2020 | 49,352 | 1896.10 | 73 | 144 | 97 | 61 |
| 6/10/2020 | 49,212 | 1896.00 | 67 | 137 | 91 | 61 |
| 6/11/2020 | 49,090 | 1895.90 | 70 | 132 | 85 | 60 |
| 6/12/2020 | 48,933 | 1895.90 | 44 | 123 | 78 | 60 |
| 6/13/2020 | 48,829 | 1895.80 | 58 | 110 | 72 | 57 |
| 6/14/2020 | 48,724 | 1895.70 | 50 | 103 | 70 | 50 |
| 6/15/2020 | 48,655 | 1895.70 | 63 | 98 | 65 | 50 |
| 6/16/2020 | 48,534 | 1895.60 | 32 | 93 | 59 | 50 |
| 6/17/2020 | 48,463 | 1895.60 | 56 | 91 | 55 | 50 |
| 6/18/2020 | 48,412 | 1895.60 | 63 | 89 | 52 | 50 |
| 6/19/2020 | 48,325 | 1895.50 | 42 | 86 | 48 | 50 |
| 6/20/2020 | 48,221 | 1895.40 | 35 | 87 | 35 | 62 |
| 6/21/2020 | 48,134 | 1895.40 | 46 | 90 | 28 | 71 |
| 6/22/2020 | 48,030 | 1895.30 | 37 | 89 | 28 | 70 |
| 6/23/2020 | 47,927 | 1895.30 | 35 | 87 | 24 | 70 |
| 6/24/2020 | 47,841 | 1895.20 | 42 | 86 | 20 | 70 |
| 6/25/2020 | 47,720 | 1895.20 | 26 | 87 | 20 | 70 |
| 6/26/2020 | 47,600 | 1895.10 | 26 | 86 | 21 | 70 |
| 6/27/2020 | 47,497 | 1895.00 | 35 | 87 | 18 | 70 |
| 6/28/2020 | 47,325 | 1894.90 | 1 | 87 | 20 | 70 |
| 6/29/2020 | 47,172 | 1894.80 | 9 | 87 | 23 | 71 |
| 6/30/2020 | 47,052 | 1894.80 | 27 | 87 | 19 | 71 |
| 7/1/2020 | 46,933 | 1894.70 | 27 | 87 | 17 | 71 |
| 7/2/2020 | 46,797 | 1894.60 | 18 | 87 | 19 | 71 |
| 7/3/2020 | 46,643 | 1894.50 | 9 | 86 | 18 | 71 |
| 7/4/2020 | 46,507 | 1894.40 | 17 | 86 | 18 | 71 |
| 7/5/2020 | 46,354 | 1894.40 | 9 | 86 | 17 | 71 |
| 7/6/2020 | 46,202 | 1894.30 | 8 | 85 | 17 | 70 |
| 7/7/2020 | 46,066 | 1894.20 | 18 | 86 | 17 | 70 |
| 7/8/2020 | 45,914 | 1894.10 | 12 | 89 | 17 | 70 |
| 7/9/2020 | 45,763 | 1894.00 | 13 | 89 | 17 | 70 |
| 7/10/2020 | 45,629 | 1893.90 | 21 | 89 | 18 | 70 |
| 7/11/2020 | 45,461 | 1893.80 | 4 | 89 | 18 | 70 |
| 7/12/2020 | 45,311 | 1893.70 | 13 | 88 | 17 | 70 |
| 7/13/2020 | 45,177 | 1893.70 | 22 | 89 | 18 | 70 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/14/2020 | 45,026 | 1893.60 | 13 | 89 | 16 | 70 |
| 7/15/2020 | 44,859 | 1893.50 | 3 | 87 | 20 | 67 |
| 7/16/2020 | 44,709 | 1893.40 | 9 | 85 | 23 | 65 |
| 7/17/2020 | 44,560 | 1893.30 | 7 | 83 | 18 | 65 |
| 7/18/2020 | 44,411 | 1893.20 | 7 | 83 | 17 | 65 |
| 7/19/2020 | 44,262 | 1893.10 | 7 | 83 | 16 | 65 |
| 7/20/2020 | 44,130 | 1893.00 | 16 | 82 | 15 | 65 |
| 7/21/2020 | 43,965 | 1892.90 | -1 | 82 | 15 | 65 |
| 7/22/2020 | 43,784 | 1892.80 | -9 | 83 | 15 | 65 |
| 7/23/2020 | 43,637 | 1892.70 | 8 | 82 | 15 | 65 |
| 7/24/2020 | 43,457 | 1892.60 | -2 | 89 | 17 | 65 |
| 7/25/2020 | 43,293 | 1892.50 | 10 | 92 | 19 | 72 |
| 7/26/2020 | 43,097 | 1892.40 | -6 | 92 | 13 | 75 |
| 7/27/2020 | 42,918 | 1892.30 | 2 | 92 | 13 | 76 |
| 7/28/2020 | 42,739 | 1892.20 | 2 | 92 | 13 | 76 |
| 7/29/2020 | 42,545 | 1892.10 | -3 | 95 | 12 | 76 |
| 7/30/2020 | 42,367 | 1891.90 | 5 | 94 | 16 | 76 |
| 7/31/2020 | 42,174 | 1891.80 | -4 | 93 | 14 | 76 |
| 8/1/2020 | 41,982 | 1891.70 | -4 | 93 | 14 | 76 |
| 8/2/2020 | 41,805 | 1891.60 | 5 | 94 | 15 | 76 |
| 8/3/2020 | 41,613 | 1891.50 | -3 | 94 | 16 | 75 |
| 8/4/2020 | 41,421 | 1891.40 | -2 | 95 | 15 | 75 |
| 8/5/2020 | 41,230 | 1891.20 | 0 | 96 | 15 | 75 |
| 8/6/2020 | 41,056 | 1891.10 | 10 | 98 | 16 | 76 |
| 8/7/2020 | 40,866 | 1891.00 | 2 | 98 | 17 | 76 |
| 8/8/2020 | 40,677 | 1890.90 | 2 | 98 | 16 | 75 |
| 8/9/2020 | 40,504 | 1890.80 | 10 | 98 | 16 | 75 |
| 8/10/2020 | 40,301 | 1890.70 | -6 | 97 | 17 | 75 |
| 8/11/2020 | 40,112 | 1890.50 | 1 | 96 | 14 | 75 |
| 8/12/2020 | 39,917 | 1890.40 | -4 | 95 | 15 | 75 |
| 8/13/2020 | 39,722 | 1890.30 | -4 | 94 | 15 | 75 |
| 8/14/2020 | 39,536 | 1890.20 | 1 | 95 | 15 | 75 |
| 8/15/2020 | 39,350 | 1890.00 | 2 | 95 | 15 | 75 |
| 8/16/2020 | 39,165 | 1889.90 | 3 | 96 | 15 | 75 |
| 8/17/2020 | 38,985 | 1889.80 | 6 | 97 | 16 | 75 |
| 8/18/2020 | 38,835 | 1889.70 | 22 | 97 | 17 | 76 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/19/2020 | 38,674 | 1889.60 | 14 | 96 | 16 | 76 |
| 8/20/2020 | 38,475 | 1889.50 | -8 | 92 | 19 | 75 |
| 8/21/2020 | 38,322 | 1889.40 | 13 | 90 | 18 | 75 |
| 8/22/2020 | 38,155 | 1889.30 | 6 | 91 | 19 | 75 |
| 8/23/2020 | 37,973 | 1889.10 | -1 | 91 | 19 | 75 |
| 8/24/2020 | 37,807 | 1889.00 | 5 | 89 | 21 | 72 |
| 8/25/2020 | 37,641 | 1888.90 | 1 | 84 | 16 | 65 |
| 8/26/2020 | 37,476 | 1888.80 | 0 | 84 | 17 | 65 |
| 8/27/2020 | 37,327 | 1888.70 | 8 | 84 | 17 | 65 |
| 8/28/2020 | 37,177 | 1888.60 | 8 | 84 | 18 | 65 |
| 8/29/2020 | 37,028 | 1888.50 | 8 | 84 | 19 | 65 |
| 8/30/2020 | 36,835 | 1888.40 | -15 | 83 | 20 | 65 |
| 8/31/2020 | 36,701 | 1888.30 | 16 | 83 | 19 | 65 |
| 9/1/2020 | 36,538 | 1888.20 | 0 | 82 | 19 | 65 |
| 9/2/2020 | 36,406 | 1888.10 | 15 | 82 | 17 | 65 |
| 9/3/2020 | 36,244 | 1888.00 | 1 | 82 | 17 | 65 |
| 9/4/2020 | 36,097 | 1887.90 | 9 | 83 | 16 | 65 |
| 9/5/2020 | 35,937 | 1887.80 | 2 | 84 | 20 | 65 |
| 9/6/2020 | 35,762 | 1887.60 | -7 | 81 | 21 | 65 |
| 9/7/2020 | 35,617 | 1887.60 | 6 | 79 | 20 | 65 |
| 9/8/2020 | 35,458 | 1887.40 | -3 | 77 | 15 | 65 |
| 9/9/2020 | 35,299 | 1887.30 | -4 | 76 | 19 | 64 |
| 9/10/2020 | 35,175 | 1887.20 | 11 | 74 | 17 | 60 |
| 9/11/2020 | 35,040 | 1887.20 | 4 | 71 | 20 | 60 |
| 9/12/2020 | 34,912 | 1887.10 | 5 | 70 | 18 | 57 |
| 9/13/2020 | 34,769 | 1887.00 | -2 | 70 | 17 | 54 |
| 9/14/2020 | 34,641 | 1886.90 | 0 | 65 | 16 | 53 |
| 9/15/2020 | 34,499 | 1886.80 | -8 | 64 | 11 | 50 |
| 9/16/2020 | 34,400 | 1886.70 | 18 | 68 | 16 | 51 |
| 9/17/2020 | 34,216 | 1886.60 | -26 | 67 | 18 | 51 |
| 9/18/2020 | 34,075 | 1886.50 | -5 | 65 | 15 | 50 |
| 9/19/2020 | 33,960 | 1886.40 | 7 | 65 | 16 | 51 |
| 9/20/2020 | 33,840 | 1886.30 | 4 | 65 | 16 | 50 |
| 9/21/2020 | 33,720 | 1886.20 | 4 | 65 | 14 | 50 |
| 9/22/2020 | 33,590 | 1886.10 | -1 | 65 | 14 | 50 |
| 9/23/2020 | 33,470 | 1886.00 | 4 | 65 | 15 | 50 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/24/2020 | 33,350 | 1885.90 | 5 | 65 | 15 | 50 |
| 9/25/2020 | 33,212 | 1885.80 | 6 | 76 | 16 | 54 |
| 9/26/2020 | 33,060 | 1885.70 | 5 | 81 | 15 | 65 |
| 9/27/2020 | 32,923 | 1885.60 | 12 | 81 | 15 | 65 |
| 9/28/2020 | 32,731 | 1885.50 | -15 | 82 | 14 | 65 |
| 9/29/2020 | 32,581 | 1885.40 | 5 | 81 | 15 | 65 |
| 9/30/2020 | 32,390 | 1885.20 | 3 | 99 | 15 | 72 |
| 10/1/2020 | 32,173 | 1885.10 | 5 | 114 | 22 | 89 |
| 10/2/2020 | 31,957 | 1884.90 | 6 | 114 | 22 | 90 |
| 10/3/2020 | 31,742 | 1884.80 | 7 | 115 | 21 | 91 |
| 10/4/2020 | 31,528 | 1884.60 | 9 | 117 | 21 | 91 |
| 10/5/2020 | 31,302 | 1884.40 | 5 | 120 | 25 | 90 |
| 10/6/2020 | 31,050 | 1884.20 | -8 | 119 | 26 | 90 |
| 10/7/2020 | 30,826 | 1884.10 | 6 | 119 | 26 | 90 |
| 10/8/2020 | 30,604 | 1883.90 | 7 | 119 | 26 | 90 |
| 10/9/2020 | 30,395 | 1883.70 | 14 | 119 | 27 | 90 |
| 10/10/2020 | 30,187 | 1883.60 | 15 | 119 | 28 | 90 |
| 10/11/2020 | 29,968 | 1883.40 | 8 | 119 | 30 | 89 |
| 10/12/2020 | 29,762 | 1883.30 | 14 | 118 | 27 | 90 |
| 10/13/2020 | 29,545 | 1883.10 | 9 | 119 | 24 | 90 |
| 10/14/2020 | 29,328 | 1882.90 | 13 | 122 | 25 | 89 |
| 10/15/2020 | 29,088 | 1882.70 | 15 | 136 | 39 | 89 |
| 10/16/2020 | 28,886 | 1882.60 | 14 | 116 | 52 | 65 |
| 10/17/2020 | 28,698 | 1882.40 | 5 | 99 | 46 | 55 |
| 10/18/2020 | 28,524 | 1882.30 | 7 | 95 | 43 | 55 |
| 10/19/2020 | 28,350 | 1882.10 | 3 | 91 | 40 | 55 |
| 10/20/2020 | 28,189 | 1882.00 | 8 | 89 | 38 | 55 |
| 10/21/2020 | 28,029 | 1881.90 | 9 | 90 | 38 | 55 |
| 10/22/2020 | 27,858 | 1881.70 | 5 | 92 | 38 | 55 |
| 10/23/2020 | 27,687 | 1881.60 | 5 | 91 | 38 | 56 |
| 10/24/2020 | 27,529 | 1881.50 | 11 | 91 | 38 | 56 |
| 10/25/2020 | 27,372 | 1881.30 | 11 | 90 | 38 | 56 |
| 10/26/2020 | 27,179 | 1881.20 | -6 | 91 | 37 | 56 |
| 10/27/2020 | 27,011 | 1881.00 | 8 | 92 | 37 | 56 |
| 10/28/2020 | 26,856 | 1880.90 | 14 | 92 | 37 | 56 |
| 10/29/2020 | 26,701 | 1880.80 | 26 | 104 | 37 | 56 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/30/2020 | 26,548 | 1880.60 | 14 | 92 | 37 | 56 |
| 10/31/2020 | 26,371 | 1880.50 | 3 | 92 | 37 | 56 |
| 11/1/2020 | 26,230 | 1880.40 | 20 | 91 | 42 | 48 |
| 11/2/2020 | 26,090 | 1880.30 | 11 | 81 | 40 | 46 |
| 11/3/2020 | 25,950 | 1880.10 | 9 | 79 | 38 | 46 |
| 11/4/2020 | 25,811 | 1880.00 | 9 | 79 | 38 | 46 |
| 11/5/2020 | 25,684 | 1879.90 | 15 | 79 | 38 | 45 |
| 11/6/2020 | 25,545 | 1879.80 | 10 | 79 | 38 | 45 |
| 11/7/2020 | 25,396 | 1879.70 | 4 | 79 | 38 | 46 |
| 11/8/2020 | 25,248 | 1879.50 | 5 | 79 | 36 | 46 |
| 11/9/2020 | 25,123 | 1879.40 | 17 | 79 | 24 | 46 |
| 11/10/2020 | 24,988 | 1879.30 | 12 | 80 | 26 | 46 |
| 11/11/2020 | 24,864 | 1879.20 | 18 | 81 | 38 | 46 |
| 11/12/2020 | 24,740 | 1879.10 | 20 | 83 | 39 | 46 |
| 11/13/2020 | 24,718 | 1879.10 | 73 | 85 | 39 | 46 |
| 11/14/2020 | 24,628 | 1879.00 | 39 | 84 | 39 | 46 |
| 11/15/2020 | 24,539 | 1878.90 | 39 | 84 | 39 | 46 |
| 11/16/2020 | 24,439 | 1878.80 | 33 | 83 | 38 | 46 |
| 11/17/2020 | 24,384 | 1878.80 | 56 | 84 | 45 | 46 |
| 11/18/2020 | 24,395 | 1878.80 | 91 | 85 | 51 | 46 |
| 11/19/2020 | 24,384 | 1878.80 | 79 | 84 | 45 | 46 |
| 11/20/2020 | 24,273 | 1878.70 | 40 | 96 | 43 | 46 |
| 11/21/2020 | 24,141 | 1878.50 | 42 | 109 | 55 | 46 |
| 11/22/2020 | 23,999 | 1878.40 | 37 | 108 | 55 | 46 |
| 11/23/2020 | 23,869 | 1878.30 | 40 | 106 | 55 | 46 |
| 11/24/2020 | 23,717 | 1878.20 | 28 | 104 | 52 | 46 |
| 11/25/2020 | 23,598 | 1878.00 | 46 | 106 | 52 | 46 |
| 11/26/2020 | 23,448 | 1877.90 | 29 | 105 | 51 | 46 |
| 11/27/2020 | 23,319 | 1877.80 | 35 | 100 | 48 | 46 |
| 11/28/2020 | 23,117 | 1877.60 | -7 | 95 | 43 | 46 |
| 11/29/2020 | 23,012 | 1877.50 | 15 | 68 | 40 | 46 |
| 11/30/2020 | 22,812 | 1877.30 | 24 | 124 | 52 | 46 |
| 12/1/2020 | 22,520 | 1877.00 | 30 | 177 | 119 | 46 |
| 12/2/2020 | 22,241 | 1876.80 | 29 | 169 | 123 | 46 |
| 12/3/2020 | 21,966 | 1876.50 | 28 | 167 | 119 | 46 |
| 12/4/2020 | 21,703 | 1876.20 | 34 | 167 | 118 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/5/2020 | 21,432 | 1876.00 | 30 | 167 | 118 | 46 |
| 12/6/2020 | 21,154 | 1875.70 | 26 | 166 | 118 | 46 |
| 12/7/2020 | 20,899 | 1875.40 | 38 | 167 | 117 | 46 |
| 12/8/2020 | 20,637 | 1875.10 | 35 | 167 | 117 | 46 |
| 12/9/2020 | 20,368 | 1874.90 | 31 | 167 | 116 | 46 |
| 12/10/2020 | 20,120 | 1874.60 | 41 | 166 | 116 | 46 |
| 12/11/2020 | 19,867 | 1874.30 | 38 | 166 | 118 | 46 |
| 12/12/2020 | 19,617 | 1874.10 | 40 | 166 | 120 | 46 |
| 12/13/2020 | 19,488 | 1873.90 | 102 | 167 | 131 | 46 |
| 12/14/2020 | 19,451 | 1873.90 | 148 | 166 | 134 | 46 |
| 12/15/2020 | 19,286 | 1873.70 | 83 | 166 | 130 | 46 |
| 12/16/2020 | 19,195 | 1873.60 | 121 | 166 | 128 | 46 |
| 12/17/2020 | 19,096 | 1873.50 | 108 | 158 | 129 | 46 |
| 12/18/2020 | 18,998 | 1873.40 | 119 | 169 | 125 | 46 |
| 12/19/2020 | 18,900 | 1873.30 | 119 | 169 | 123 | 46 |
| 12/20/2020 | 18,704 | 1873.10 | 64 | 162 | 120 | 46 |
| 12/21/2020 | 18,466 | 1872.80 | 41 | 161 | 119 | 46 |
| 12/22/2020 | 18,258 | 1872.50 | 56 | 161 | 117 | 46 |
| 12/23/2020 | 18,026 | 1872.30 | 43 | 160 | 116 | 46 |
| 12/24/2020 | 17,788 | 1872.00 | 39 | 159 | 116 | 46 |
| 12/25/2020 | 17,644 | 1871.80 | 87 | 160 | 127 | 46 |
| 12/26/2020 | 17,966 | 1872.20 | 323 | 161 | 178 | 46 |
| 12/27/2020 | 17,949 | 1872.20 | 151 | 160 | 143 | 46 |
| 12/28/2020 | 17,830 | 1872.00 | 99 | 159 | 128 | 46 |
| 12/29/2020 | 17,644 | 1871.80 | 64 | 158 | 123 | 46 |
| 12/30/2020 | 17,477 | 1871.60 | 71 | 156 | 120 | 45 |
| 12/31/2020 | 17,292 | 1871.40 | 63 | 156 | 123 | 45 |
| 1/1/2021 | 17,124 | 1871.20 | 70 | 154 | 120 | 45 |
| 1/2/2021 | 16,953 | 1871.00 | 67 | 153 | 120 | 45 |
| 1/3/2021 | 16,784 | 1870.80 | 67 | 152 | 120 | 44 |
| 1/4/2021 | 17,856 | 1872.10 | 696 | 156 | 250 | 44 |
| 1/5/2021 | 18,458 | 1872.80 | 461 | 157 | 190 | 45 |
| 1/6/2021 | 18,669 | 1873.00 | 269 | 162 | 150 | 45 |
| 1/7/2021 | 18,891 | 1873.30 | 269 | 157 | 151 | 45 |
| 1/8/2021 | 19,236 | 1873.60 | 332 | 158 | 172 | 45 |
| 1/9/2021 | 19,414 | 1873.80 | 247 | 157 | 156 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/10/2021 | 19,451 | 1873.90 | 175 | 156 | 144 | 45 |
| 1/11/2021 | 19,442 | 1873.90 | 151 | 156 | 138 | 45 |
| 1/12/2021 | 19,497 | 1873.90 | 184 | 156 | 138 | 45 |
| 1/13/2021 | 20,262 | 1874.80 | 568 | 182 | 280 | 45 |
| 1/14/2021 | 20,531 | 1875.00 | 337 | 202 | 277 | 44 |
| 1/15/2021 | 20,579 | 1875.10 | 237 | 213 | 207 | 44 |
| 1/16/2021 | 20,540 | 1875.00 | 187 | 207 | 184 | 44 |
| 1/17/2021 | 20,444 | 1874.90 | 158 | 206 | 179 | 45 |
| 1/18/2021 | 20,329 | 1874.80 | 147 | 205 | 174 | 45 |
| 1/19/2021 | 20,139 | 1874.60 | 110 | 206 | 171 | 45 |
| 1/20/2021 | 19,942 | 1874.40 | 106 | 205 | 169 | 45 |
| 1/21/2021 | 19,756 | 1874.20 | 110 | 204 | 167 | 45 |
| 1/22/2021 | 19,552 | 1874.00 | 97 | 199 | 164 | 46 |
| 1/23/2021 | 19,350 | 1873.80 | 88 | 190 | 157 | 46 |
| 1/24/2021 | 19,168 | 1873.60 | 88 | 180 | 150 | 46 |
| 1/25/2021 | 18,998 | 1873.40 | 85 | 171 | 144 | 46 |
| 1/26/2021 | 18,935 | 1873.30 | 131 | 163 | 141 | 47 |
| 1/27/2021 | 18,998 | 1873.40 | 187 | 156 | 154 | 47 |
| 1/28/2021 | 19,515 | 1874.00 | 420 | 159 | 204 | 47 |
| 1/29/2021 | 19,886 | 1874.40 | 346 | 159 | 204 | 47 |
| 1/30/2021 | 19,989 | 1874.50 | 211 | 159 | 204 | 47 |
| 1/31/2021 | 20,329 | 1874.80 | 331 | 159 | 204 | 47 |
| 2/1/2021 | 21,472 | 1876.00 | 771 | 195 | 324 | 46 |
| 2/2/2021 | 24,875 | 1879.20 | 1,918 | 202 | 681 | 46 |
| 2/3/2021 | 26,043 | 1880.20 | 794 | 205 | 425 | 46 |
| 2/4/2021 | 26,642 | 1880.70 | 507 | 205 | 348 | 46 |
| 2/5/2021 | 27,011 | 1881.00 | 391 | 205 | 287 | 47 |
| 2/6/2021 | 27,336 | 1881.30 | 369 | 205 | 246 | 47 |
| 2/7/2021 | 27,565 | 1881.50 | 322 | 206 | 230 | 47 |
| 2/8/2021 | 27,833 | 1881.70 | 341 | 206 | 217 | 47 |
| 2/9/2021 | 27,995 | 1881.90 | 287 | 205 | 206 | 46 |
| 2/10/2021 | 28,160 | 1882.00 | 289 | 206 | 199 | 46 |
| 2/11/2021 | 28,264 | 1882.10 | 260 | 207 | 220 | 46 |
| 2/12/2021 | 29,968 | 1883.40 | 1,067 | 208 | 298 | 46 |
| 2/13/2021 | 31,011 | 1884.20 | 732 | 206 | 318 | 47 |
| 2/14/2021 | 31,769 | 1884.80 | 589 | 207 | 288 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/15/2021 | 33,019 | 1885.70 | 838 | 208 | 304 | 47 |
| 2/16/2021 | 33,795 | 1886.30 | 600 | 209 | 277 | 47 |
| 2/17/2021 | 34,301 | 1886.60 | 464 | 210 | 253 | 47 |
| 2/18/2021 | 34,671 | 1886.90 | 396 | 210 | 237 | 47 |
| 2/19/2021 | 35,083 | 1887.20 | 419 | 211 | 247 | 47 |
| 2/20/2021 | 35,588 | 1887.50 | 467 | 213 | 298 | 47 |
| 2/21/2021 | 35,951 | 1887.80 | 396 | 212 | 280 | 47 |
| 2/22/2021 | 36,259 | 1888.00 | 367 | 212 | 261 | 47 |
| 2/23/2021 | 36,479 | 1888.10 | 323 | 212 | 245 | 47 |
| 2/24/2021 | 36,627 | 1888.20 | 286 | 211 | 231 | 47 |
| 2/25/2021 | 36,716 | 1888.30 | 255 | 210 | 220 | 47 |
| 2/26/2021 | 36,734 | 1888.30 | 219 | 210 | 213 | 47 |
| 2/27/2021 | 36,753 | 1888.30 | 219 | 210 | 207 | 47 |
| 2/28/2021 | 36,772 | 1888.30 | 219 | 210 | 201 | 47 |
| 3/1/2021 | 36,805 | 1888.40 | 229 | 212 | 197 | 47 |
| 3/2/2021 | 36,790 | 1888.30 | 206 | 213 | 195 | 47 |
| 3/3/2021 | 36,790 | 1888.30 | 213 | 213 | 192 | 47 |
| 3/4/2021 | 36,790 | 1888.30 | 213 | 213 | 185 | 47 |
| 3/5/2021 | 36,790 | 1888.30 | 213 | 213 | 186 | 47 |
| 3/6/2021 | 36,790 | 1888.30 | 213 | 213 | 195 | 47 |
| 3/7/2021 | 36,790 | 1888.30 | 213 | 213 | 189 | 47 |
| 3/8/2021 | 36,894 | 1888.40 | 252 | 200 | 188 | 47 |
| 3/9/2021 | 36,909 | 1888.40 | 207 | 200 | 198 | 47 |
| 3/10/2021 | 37,192 | 1888.60 | 353 | 210 | 246 | 47 |
| 3/11/2021 | 37,536 | 1888.80 | 384 | 210 | 254 | 47 |
| 3/12/2021 | 37,822 | 1889.00 | 354 | 210 | 243 | 47 |
| 3/13/2021 | 38,094 | 1889.20 | 334 | 197 | 239 | 47 |
| 3/14/2021 | 38,368 | 1889.40 | 347 | 209 | 254 | 47 |
| 3/15/2021 | 39,103 | 1889.90 | 572 | 202 | 327 | 47 |
| 3/16/2021 | 39,714 | 1890.30 | 528 | 220 | 292 | 47 |
| 3/17/2021 | 40,325 | 1890.70 | 528 | 220 | 269 | 47 |
| 3/18/2021 | 40,936 | 1891.10 | 528 | 220 | 287 | 47 |
| 3/19/2021 | 41,547 | 1891.40 | 528 | 220 | 309 | 47 |
| 3/20/2021 | 42,158 | 1891.80 | 528 | 220 | 278 | 47 |
| 3/21/2021 | 42,626 | 1892.10 | 447 | 211 | 255 | 47 |
| 3/22/2021 | 42,966 | 1892.30 | 382 | 210 | 244 | 47 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/23/2021 | 43,080 | 1892.40 | 392 | 334 | 324 | 50 |
| 3/24/2021 | 43,195 | 1892.50 | 363 | 306 | 334 | 57 |
| 3/25/2021 | 43,408 | 1892.60 | 323 | 216 | 228 | 46 |
| 3/26/2021 | 43,604 | 1892.70 | 314 | 215 | 216 | 45 |
| 3/27/2021 | 43,735 | 1892.80 | 281 | 215 | 209 | 45 |
| 3/28/2021 | 43,866 | 1892.90 | 281 | 214 | 205 | 45 |
| 3/29/2021 | 44,014 | 1893.00 | 289 | 214 | 202 | 45 |
| 3/30/2021 | 44,130 | 1893.00 | 261 | 203 | 199 | 45 |
| 3/31/2021 | 44,113 | 1893.00 | 249 | 257 | 215 | 53 |
| 4/1/2021 | 44,047 | 1893.00 | 248 | 282 | 246 | 57 |
| 4/2/2021 | 43,998 | 1892.90 | 242 | 267 | 242 | 58 |
| 4/3/2021 | 43,883 | 1892.90 | 210 | 268 | 241 | 58 |
| 4/4/2021 | 43,768 | 1892.80 | 210 | 268 | 240 | 57 |
| 4/5/2021 | 43,653 | 1892.70 | 225 | 282 | 238 | 57 |
| 4/6/2021 | 43,588 | 1892.70 | 244 | 277 | 234 | 57 |
| 4/7/2021 | 43,430 | 1892.60 | 194 | 273 | 222 | 57 |
| 4/8/2021 | 43,273 | 1892.50 | 194 | 273 | 207 | 63 |
| 4/9/2021 | 43,116 | 1892.40 | 188 | 267 | 196 | 66 |
| 4/10/2021 | 42,972 | 1892.30 | 163 | 235 | 191 | 66 |
| 4/11/2021 | 42,824 | 1892.20 | 171 | 245 | 189 | 67 |
| 4/12/2021 | 42,674 | 1892.10 | 162 | 238 | 178 | 73 |
| 4/13/2021 | 42,528 | 1892.00 | 159 | 232 | 170 | 75 |
| 4/14/2021 | 42,335 | 1891.90 | 117 | 214 | 165 | 75 |
| 4/15/2021 | 42,190 | 1891.80 | 147 | 220 | 166 | 69 |
| 4/16/2021 | 42,046 | 1891.70 | 136 | 209 | 158 | 66 |
| 4/17/2021 | 41,870 | 1891.60 | 117 | 206 | 152 | 66 |
| 4/18/2021 | 41,725 | 1891.50 | 125 | 198 | 153 | 60 |
| 4/19/2021 | 41,565 | 1891.40 | 108 | 189 | 149 | 54 |
| 4/20/2021 | 41,437 | 1891.40 | 120 | 185 | 146 | 54 |
| 4/21/2021 | 41,326 | 1891.30 | 106 | 163 | 142 | 52 |
| 4/22/2021 | 41,246 | 1891.20 | 124 | 164 | 138 | 45 |
| 4/23/2021 | 41,119 | 1891.20 | 98 | 162 | 135 | 39 |
| 4/24/2021 | 41,008 | 1891.10 | 105 | 161 | 132 | 39 |
| 4/25/2021 | 40,961 | 1891.10 | 137 | 161 | 137 | 39 |
| 4/26/2021 | 40,866 | 1891.00 | 116 | 164 | 135 | 42 |
| 4/27/2021 | 40,724 | 1890.90 | 100 | 171 | 133 | 49 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/28/2021 | 40,583 | 1890.80 | 104 | 175 | 129 | 55 |
| 4/29/2021 | 40,410 | 1890.70 | 88 | 175 | 127 | 55 |
| 4/30/2021 | 40,230 | 1890.60 | 83 | 174 | 124 | 55 |
| 5/1/2021 | 40,050 | 1890.50 | 82 | 172 | 121 | 55 |
| 5/2/2021 | 39,871 | 1890.40 | 81 | 171 | 119 | 55 |
| 5/3/2021 | 39,691 | 1890.30 | 81 | 171 | 118 | 55 |
| 5/4/2021 | 39,505 | 1890.10 | 77 | 171 | 117 | 55 |
| 5/5/2021 | 39,334 | 1890.00 | 78 | 164 | 126 | 35 |
| 5/6/2021 | 39,195 | 1889.90 | 82 | 152 | 117 | 35 |
| 5/7/2021 | 38,980 | 1889.80 | 49 | 157 | 119 | 38 |
| 5/8/2021 | 38,812 | 1889.70 | 71 | 156 | 121 | 37 |
| 5/9/2021 | 38,612 | 1889.60 | 52 | 152 | 117 | 34 |
| 5/10/2021 | 38,429 | 1889.40 | 59 | 151 | 116 | 34 |
| 5/11/2021 | 38,246 | 1889.30 | 58 | 150 | 114 | 35 |
| 5/12/2021 | 38,049 | 1889.20 | 50 | 150 | 113 | 35 |
| 5/13/2021 | 37,852 | 1889.10 | 51 | 150 | 113 | 35 |
| 5/14/2021 | 37,672 | 1888.90 | 58 | 150 | 112 | 35 |
| 5/15/2021 | 37,446 | 1888.80 | 40 | 153 | 114 | 37 |
| 5/16/2021 | 37,237 | 1888.70 | 45 | 150 | 112 | 39 |
| 5/17/2021 | 37,058 | 1888.50 | 50 | 140 | 104 | 39 |
| 5/18/2021 | 36,879 | 1888.40 | 36 | 127 | 90 | 39 |
| 5/19/2021 | 36,716 | 1888.30 | 37 | 119 | 82 | 39 |
| 5/20/2021 | 36,538 | 1888.20 | 25 | 115 | 77 | 40 |
| 5/21/2021 | 36,406 | 1888.10 | 46 | 113 | 73 | 41 |
| 5/22/2021 | 36,273 | 1888.00 | 42 | 109 | 76 | 35 |
| 5/23/2021 | 36,171 | 1887.90 | 49 | 101 | 72 | 32 |
| 5/24/2021 | 36,054 | 1887.80 | 36 | 95 | 67 | 32 |
| 5/25/2021 | 35,981 | 1887.80 | 42 | 79 | 60 | 30 |
| 5/26/2021 | 35,908 | 1887.70 | 31 | 68 | 46 | 28 |
| 5/27/2021 | 35,878 | 1887.70 | 52 | 66 | 43 | 28 |
| 5/28/2021 | 35,806 | 1887.70 | 30 | 66 | 41 | 30 |
| 5/29/2021 | 35,733 | 1887.60 | 29 | 66 | 39 | 30 |
| 5/30/2021 | 35,660 | 1887.60 | 30 | 67 | 38 | 30 |
| 5/31/2021 | 35,588 | 1887.50 | 28 | 64 | 38 | 30 |
| 6/1/2021 | 35,515 | 1887.50 | 25 | 62 | 35 | 30 |
| 6/2/2021 | 35,414 | 1887.40 | 10 | 61 | 33 | 31 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/3/2021 | 35,313 | 1887.30 | 17 | 68 | 33 | 32 |
| 6/4/2021 | 35,227 | 1887.30 | 26 | 70 | 34 | 39 |
| 6/5/2021 | 35,126 | 1887.20 | 18 | 69 | 31 | 40 |
| 6/6/2021 | 35,012 | 1887.10 | 8 | 66 | 33 | 37 |
| 6/7/2021 | 34,912 | 1887.10 | 10 | 61 | 34 | 32 |
| 6/8/2021 | 34,826 | 1887.00 | 12 | 55 | 30 | 30 |
| 6/9/2021 | 34,740 | 1886.90 | 7 | 50 | 26 | 30 |
| 6/10/2021 | 34,683 | 1886.90 | 25 | 53 | 20 | 31 |
| 6/11/2021 | 34,612 | 1886.90 | 22 | 58 | 18 | 40 |
| 6/12/2021 | 34,556 | 1886.80 | 29 | 57 | 17 | 40 |
| 6/13/2021 | 34,513 | 1886.80 | 36 | 57 | 16 | 40 |
| 6/14/2021 | 34,428 | 1886.70 | 14 | 57 | 16 | 40 |
| 6/15/2021 | 34,329 | 1886.70 | 13 | 63 | 17 | 41 |
| 6/16/2021 | 34,230 | 1886.60 | 20 | 70 | 21 | 47 |
| 6/17/2021 | 34,118 | 1886.50 | 15 | 72 | 22 | 50 |
| 6/18/2021 | 33,991 | 1886.40 | 12 | 76 | 21 | 51 |
| 6/19/2021 | 33,865 | 1886.30 | 15 | 79 | 22 | 55 |
| 6/20/2021 | 33,725 | 1886.20 | 7 | 77 | 24 | 52 |
| 6/21/2021 | 33,600 | 1886.10 | 7 | 71 | 21 | 50 |
| 6/22/2021 | 33,490 | 1886.10 | 8 | 64 | 19 | 44 |
| 6/23/2021 | 33,380 | 1886.00 | 5 | 60 | 16 | 42 |
| 6/24/2021 | 33,275 | 1885.90 | 6 | 59 | 13 | 43 |
| 6/25/2021 | 33,157 | 1885.80 | -1 | 58 | 12 | 43 |
| 6/26/2021 | 33,060 | 1885.70 | 10 | 58 | 11 | 43 |
| 6/27/2021 | 32,950 | 1885.70 | 3 | 58 | 11 | 43 |
| 6/28/2021 | 32,854 | 1885.60 | 3 | 52 | 18 | 35 |
| 6/29/2021 | 32,786 | 1885.50 | 12 | 47 | 16 | 30 |
| 6/30/2021 | 32,690 | 1885.50 | -2 | 47 | 14 | 31 |
| 7/1/2021 | 32,594 | 1885.40 | -1 | 47 | 15 | 30 |
| 7/2/2021 | 32,499 | 1885.30 | -1 | 48 | 15 | 30 |
| 7/3/2021 | 32,417 | 1885.30 | 6 | 47 | 15 | 30 |
| 7/4/2021 | 32,308 | 1885.20 | -8 | 47 | 14 | 30 |
| 7/5/2021 | 32,214 | 1885.10 | -2 | 46 | 14 | 30 |
| 7/6/2021 | 32,105 | 1885.00 | -9 | 46 | 14 | 30 |
| 7/7/2021 | 31,984 | 1884.90 | -16 | 46 | 14 | 30 |
| 7/8/2021 | 31,890 | 1884.90 | -2 | 46 | 14 | 30 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/9/2021 | 31,796 | 1884.80 | -2 | 46 | 14 | 30 |
| 7/10/2021 | 31,689 | 1884.70 | -8 | 46 | 14 | 30 |
| 7/11/2021 | 31,582 | 1884.70 | -8 | 46 | 14 | 30 |
| 7/12/2021 | 31,475 | 1884.60 | -4 | 50 | 14 | 30 |
| 7/13/2021 | 31,355 | 1884.50 | -8 | 53 | 14 | 30 |
| 7/14/2021 | 31,222 | 1884.40 | -10 | 57 | 15 | 32 |
| 7/15/2021 | 31,077 | 1884.30 | -13 | 60 | 16 | 41 |
| 7/16/2021 | 30,945 | 1884.20 | -6 | 60 | 17 | 40 |
| 7/17/2021 | 30,800 | 1884.10 | -13 | 60 | 18 | 39 |
| 7/18/2021 | 30,682 | 1884.00 | -1 | 58 | 17 | 39 |
| 7/19/2021 | 30,564 | 1883.90 | -3 | 57 | 15 | 39 |
| 7/20/2021 | 30,434 | 1883.80 | -10 | 56 | 15 | 39 |
| 7/21/2021 | 30,291 | 1883.70 | -16 | 56 | 14 | 39 |
| 7/22/2021 | 30,174 | 1883.60 | -4 | 55 | 13 | 39 |
| 7/23/2021 | 30,045 | 1883.50 | -11 | 54 | 14 | 39 |
| 7/24/2021 | 29,916 | 1883.40 | -11 | 54 | 13 | 39 |
| 7/25/2021 | 29,788 | 1883.30 | -11 | 54 | 13 | 39 |
| 7/26/2021 | 29,659 | 1883.20 | -13 | 52 | 20 | 33 |
| 7/27/2021 | 29,557 | 1883.10 | -2 | 50 | 21 | 29 |
| 7/28/2021 | 29,442 | 1883.00 | -8 | 49 | 19 | 29 |
| 7/29/2021 | 29,341 | 1882.90 | -1 | 50 | 18 | 29 |
| 7/30/2021 | 29,214 | 1882.80 | -14 | 50 | 18 | 29 |
| 7/31/2021 | 29,100 | 1882.70 | -7 | 50 | 17 | 29 |
| 8/1/2021 | 28,974 | 1882.60 | -11 | 53 | 18 | 30 |
| 8/2/2021 | 28,849 | 1882.50 | -8 | 55 | 14 | 36 |
| 8/3/2021 | 28,723 | 1882.40 | -7 | 56 | 14 | 36 |
| 8/4/2021 | 28,611 | 1882.30 | -1 | 55 | 13 | 36 |
| 8/5/2021 | 28,474 | 1882.20 | -13 | 56 | 13 | 36 |
| 8/6/2021 | 28,363 | 1882.10 | 0 | 57 | 14 | 36 |
| 8/7/2021 | 28,251 | 1882.10 | 1 | 57 | 15 | 35 |
| 8/8/2021 | 28,135 | 1882.00 | -3 | 56 | 16 | 34 |
| 8/9/2021 | 28,023 | 1881.90 | -5 | 51 | 16 | 26 |
| 8/10/2021 | 27,910 | 1881.80 | -5 | 52 | 14 | 29 |
| 8/11/2021 | 27,798 | 1881.70 | -5 | 51 | 16 | 30 |
| 8/12/2021 | 27,685 | 1881.60 | -6 | 50 | 15 | 30 |
| 8/13/2021 | 27,573 | 1881.50 | -3 | 54 | 14 | 35 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/14/2021 | 27,460 | 1881.40 | 2 | 59 | 14 | 35 |
| 8/15/2021 | 27,348 | 1881.30 | -1 | 56 | 17 | 35 |
| 8/16/2021 | 27,239 | 1881.20 | -2 | 53 | 13 | 35 |
| 8/17/2021 | 27,119 | 1881.10 | -8 | 52 | 10 | 35 |
| 8/18/2021 | 27,011 | 1881.00 | -2 | 52 | 10 | 35 |
| 8/19/2021 | 26,880 | 1880.90 | -16 | 50 | 15 | 32 |
| 8/20/2021 | 26,763 | 1880.80 | -11 | 48 | 14 | 30 |
| 8/21/2021 | 26,646 | 1880.70 | -11 | 48 | 13 | 30 |
| 8/22/2021 | 26,529 | 1880.60 | -11 | 47 | 14 | 30 |
| 8/23/2021 | 26,412 | 1880.50 | -11 | 48 | 14 | 30 |
| 8/24/2021 | 26,394 | 1880.50 | 39 | 48 | 14 | 30 |
| 8/25/2021 | 26,289 | 1880.40 | -5 | 48 | 13 | 30 |
| 8/26/2021 | 26,195 | 1880.40 | 1 | 48 | 12 | 30 |
| 8/27/2021 | 26,090 | 1880.30 | -5 | 48 | 12 | 30 |
| 8/28/2021 | 25,985 | 1880.20 | -5 | 48 | 13 | 30 |
| 8/29/2021 | 25,892 | 1880.10 | 1 | 48 | 13 | 30 |
| 8/30/2021 | 25,787 | 1880.00 | -5 | 48 | 13 | 30 |
| 8/31/2021 | 25,684 | 1879.90 | -5 | 47 | 13 | 30 |
| 9/1/2021 | 25,568 | 1879.80 | -11 | 48 | 13 | 30 |
| 9/2/2021 | 25,472 | 1879.70 | -1 | 48 | 14 | 30 |
| 9/3/2021 | 25,373 | 1879.60 | -2 | 48 | 14 | 30 |
| 9/4/2021 | 25,271 | 1879.60 | -4 | 48 | 14 | 30 |
| 9/5/2021 | 25,169 | 1879.50 | -4 | 48 | 14 | 30 |
| 9/6/2021 | 25,078 | 1879.40 | 2 | 48 | 14 | 30 |
| 9/7/2021 | 24,976 | 1879.30 | -1 | 51 | 14 | 30 |
| 9/8/2021 | 24,864 | 1879.20 | -1 | 56 | 18 | 33 |
| 9/9/2021 | 24,673 | 1879.00 | 0 | 56 | 14 | 35 |
| 9/10/2021 | 24,584 | 1878.90 | 11 | 56 | 15 | 35 |
| 9/11/2021 | 24,473 | 1878.80 | -1 | 55 | 14 | 35 |
| 9/12/2021 | 24,362 | 1878.70 | -1 | 55 | 14 | 35 |
| 9/13/2021 | 24,262 | 1878.60 | 5 | 55 | 14 | 35 |
| 9/14/2021 | 24,152 | 1878.60 | -1 | 54 | 18 | 32 |
| 9/15/2021 | 24,043 | 1878.50 | -2 | 53 | 20 | 30 |
| 9/16/2021 | 23,958 | 1878.40 | 7 | 50 | 17 | 30 |
| 9/17/2021 | 23,874 | 1878.30 | 5 | 48 | 13 | 30 |
| 9/18/2021 | 23,789 | 1878.20 | 5 | 48 | 16 | 30 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/19/2021 | 23,704 | 1878.10 | 6 | 49 | 16 | 30 |
| 9/20/2021 | 23,620 | 1878.10 | 6 | 48 | 14 | 30 |
| 9/21/2021 | 23,523 | 1878.00 | -1 | 48 | 14 | 30 |
| 9/22/2021 | 23,437 | 1877.90 | 5 | 48 | 13 | 30 |
| 9/23/2021 | 23,373 | 1877.80 | 16 | 48 | 12 | 30 |
| 9/24/2021 | 23,273 | 1877.70 | -2 | 48 | 12 | 30 |
| 9/25/2021 | 23,174 | 1877.60 | -2 | 48 | 12 | 30 |
| 9/26/2021 | 23,075 | 1877.60 | -3 | 47 | 13 | 30 |
| 9/27/2021 | 22,991 | 1877.50 | 5 | 47 | 14 | 30 |
| 9/28/2021 | 22,885 | 1877.40 | -5 | 48 | 14 | 30 |
| 9/29/2021 | 22,802 | 1877.30 | 5 | 47 | 17 | 30 |
| 9/30/2021 | 22,718 | 1877.20 | 3 | 46 | 13 | 30 |
| 10/1/2021 | 22,635 | 1877.10 | 3 | 46 | 13 | 30 |
| 10/2/2021 | 22,551 | 1877.10 | 4 | 46 | 13 | 30 |
| 10/3/2021 | 22,458 | 1877.00 | 2 | 49 | 15 | 30 |
| 10/4/2021 | 22,365 | 1876.90 | 1 | 48 | 18 | 30 |
| 10/5/2021 | 22,272 | 1876.80 | 0 | 47 | 15 | 30 |
| 10/6/2021 | 22,180 | 1876.70 | 0 | 47 | 21 | 24 |
| 10/7/2021 | 22,098 | 1876.60 | 4 | 46 | 38 | 10 |
| 10/8/2021 | 22,006 | 1876.50 | -2 | 44 | 37 | 10 |
| 10/9/2021 | 21,925 | 1876.40 | 1 | 42 | 35 | 10 |
| 10/10/2021 | 21,838 | 1876.40 | -1 | 43 | 35 | 10 |
| 10/11/2021 | 21,743 | 1876.30 | -4 | 43 | 36 | 10 |
| 10/12/2021 | 21,624 | 1876.10 | -16 | 44 | 36 | 10 |
| 10/13/2021 | 21,542 | 1876.10 | 3 | 45 | 37 | 10 |
| 10/14/2021 | 21,462 | 1876.00 | 4 | 45 | 36 | 10 |
| 10/15/2021 | 21,382 | 1875.90 | 5 | 45 | 36 | 10 |
| 10/16/2021 | 21,305 | 1875.80 | 8 | 46 | 37 | 10 |
| 10/17/2021 | 21,243 | 1875.80 | 16 | 48 | 40 | 10 |
| 10/18/2021 | 21,164 | 1875.70 | 8 | 48 | 41 | 10 |
| 10/19/2021 | 21,134 | 1875.60 | 32 | 47 | 40 | 10 |
| 10/20/2021 | 21,095 | 1875.60 | 27 | 46 | 43 | 10 |
| 10/21/2021 | 21,115 | 1875.60 | 57 | 47 | 41 | 10 |
| 10/22/2021 | 21,854 | 1876.40 | 421 | 48 | 67 | 10 |
| 10/23/2021 | 22,088 | 1876.60 | 173 | 55 | 50 | 10 |
| 10/24/2021 | 37,342 | 1888.70 | 7,778 | 87 | 1,384 | 10 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/25/2021 | 43,375 | 1892.60 | 3,191 | 149 | 797 | 30 |
| 10/26/2021 | 44,993 | 1893.50 | 1,007 | 191 | 331 | 45 |
| 10/27/2021 | 45,864 | 1894.10 | 630 | 191 | 244 | 46 |
| 10/28/2021 | 46,320 | 1894.30 | 422 | 191 | 211 | 45 |
| 10/29/2021 | 46,592 | 1894.50 | 330 | 193 | 191 | 45 |
| 10/30/2021 | 46,729 | 1894.60 | 261 | 193 | 182 | 45 |
| 10/31/2021 | 46,763 | 1894.60 | 211 | 194 | 175 | 45 |
| 11/1/2021 | 47,052 | 1894.80 | 342 | 196 | 183 | 45 |
| 11/2/2021 | 47,600 | 1895.10 | 472 | 196 | 201 | 45 |
| 11/3/2021 | 47,927 | 1895.30 | 359 | 194 | 181 | 45 |
| 11/4/2021 | 48,394 | 1895.50 | 433 | 197 | 241 | 44 |
| 11/5/2021 | 48,776 | 1895.80 | 389 | 197 | 219 | 45 |
| 11/6/2021 | 48,985 | 1895.90 | 302 | 197 | 200 | 45 |
| 11/7/2021 | 49,107 | 1896.00 | 258 | 197 | 188 | 45 |
| 11/8/2021 | 49,439 | 1896.10 | 365 | 197 | 190 | 45 |
| 11/9/2021 | 52,500 | 1897.90 | 1,747 | 203 | 644 | 45 |
| 11/10/2021 | 53,500 | 1898.40 | 704 | 200 | 345 | 45 |
| 11/11/2021 | 54,100 | 1898.70 | 502 | 200 | 261 | 45 |
| 11/12/2021 | 54,600 | 1899.00 | 451 | 199 | 229 | 45 |
| 11/13/2021 | 54,871 | 1899.20 | 335 | 199 | 210 | 45 |
| 11/14/2021 | 55,056 | 1899.30 | 292 | 199 | 198 | 45 |
| 11/15/2021 | 55,148 | 1899.30 | 245 | 198 | 191 | 45 |
| 11/16/2021 | 55,241 | 1899.40 | 245 | 198 | 185 | 45 |
| 11/17/2021 | 55,241 | 1899.40 | 199 | 199 | 182 | 45 |
| 11/18/2021 | 55,310 | 1899.40 | 234 | 199 | 179 | 45 |
| 11/19/2021 | 55,289 | 1899.40 | 189 | 200 | 184 | 45 |
| 11/20/2021 | 55,269 | 1899.40 | 188 | 199 | 179 | 45 |
| 11/21/2021 | 55,230 | 1899.40 | 179 | 199 | 175 | 45 |
| 11/22/2021 | 55,143 | 1899.30 | 156 | 200 | 174 | 45 |
| 11/23/2021 | 55,008 | 1899.20 | 135 | 203 | 175 | 45 |
| 11/24/2021 | 54,892 | 1899.20 | 145 | 203 | 175 | 44 |
| 11/25/2021 | 54,730 | 1899.10 | 118 | 199 | 172 | 43 |
| 11/26/2021 | 54,587 | 1899.00 | 121 | 194 | 164 | 45 |
| 11/27/2021 | 54,471 | 1899.00 | 125 | 184 | 155 | 46 |
| 11/28/2021 | 54,356 | 1898.90 | 112 | 170 | 143 | 46 |
| 11/29/2021 | 54,259 | 1898.80 | 107 | 156 | 130 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/30/2021 | 54,240 | 1898.80 | 143 | 153 | 123 | 46 |
| 12/1/2021 | 54,048 | 1898.70 | 57 | 154 | 125 | 46 |
| 12/2/2021 | 53,954 | 1898.70 | 107 | 154 | 124 | 46 |
| 12/3/2021 | 53,826 | 1898.60 | 89 | 154 | 124 | 46 |
| 12/4/2021 | 53,717 | 1898.50 | 101 | 156 | 124 | 46 |
| 12/5/2021 | 53,570 | 1898.50 | 85 | 158 | 125 | 46 |
| 12/6/2021 | 53,589 | 1898.50 | 168 | 159 | 126 | 46 |
| 12/7/2021 | 54,046 | 1898.70 | 389 | 158 | 126 | 46 |
| 12/8/2021 | 54,503 | 1899.00 | 389 | 158 | 125 | 46 |
| 12/9/2021 | 54,963 | 1899.20 | 391 | 159 | 125 | 46 |
| 12/10/2021 | 55,426 | 1899.50 | 393 | 160 | 126 | 46 |
| 12/11/2021 | 55,891 | 1899.70 | 394 | 160 | 125 | 46 |
| 12/12/2021 | 56,359 | 1900.00 | 400 | 165 | 267 | 46 |
| 12/13/2021 | 56,829 | 1900.20 | 405 | 168 | 393 | 46 |
| 12/14/2021 | 57,227 | 1900.40 | 649 | 449 | 671 | 46 |
| 12/15/2021 | 58,562 | 1901.10 | 1,464 | 791 | 1,224 | 46 |
| 12/16/2021 | 58,504 | 1901.10 | 2,087 | 2,116 | 2,677 | 44 |
| 12/17/2021 | 57,816 | 1900.70 | 1,009 | 1,356 | 1,826 | 44 |
| 12/18/2021 | 57,454 | 1900.50 | 629 | 811 | 1,240 | 44 |
| 12/19/2021 | 57,151 | 1900.40 | 481 | 634 | 940 | 44 |
| 12/20/2021 | 57,018 | 1900.30 | 475 | 541 | 778 | 44 |
| 12/21/2021 | 57,094 | 1900.40 | 531 | 493 | 684 | 44 |
| 12/22/2021 | 58,581 | 1901.10 | 1,856 | 1,107 | 1,351 | 44 |
| 12/23/2021 | 61,249 | 1902.50 | 5,384 | 4,039 | 4,473 | 44 |
| 12/24/2021 | 59,974 | 1901.90 | 3,334 | 3,977 | 4,444 | 44 |
| 12/25/2021 | 60,169 | 1902.00 | 3,544 | 3,446 | 4,353 | 44 |
| 12/26/2021 | 59,082 | 1901.40 | 2,151 | 2,699 | 3,355 | 44 |
| 12/27/2021 | 58,542 | 1901.10 | 1,672 | 1,944 | 2,532 | 45 |
| 12/28/2021 | 58,159 | 1900.90 | 1,233 | 1,426 | 1,985 | 44 |
| 12/29/2021 | 57,930 | 1900.80 | 1,056 | 1,171 | 1,747 | 45 |
| 12/30/2021 | 57,683 | 1900.70 | 849 | 974 | 1,524 | 45 |
| 12/31/2021 | 57,492 | 1900.60 | 725 | 821 | 1,281 | 44 |
| 1/1/2022 | 57,321 | 1900.50 | 631 | 717 | 1,087 | 44 |
| 1/2/2022 | 57,227 | 1900.40 | 599 | 647 | 964 | 44 |
| 1/3/2022 | 58,696 | 1901.20 | 1,467 | 726 | 1,129 | 44 |
| 1/4/2022 | 59,275 | 1901.50 | 2,868 | 2,576 | 2,961 | 44 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/5/2022 | 59,159 | 1901.40 | 2,499 | 2,558 | 2,821 | 45 |
| 1/6/2022 | 58,908 | 1901.30 | 2,286 | 2,412 | 2,658 | 46 |
| 1/7/2022 | 58,869 | 1901.30 | 2,185 | 2,205 | 2,510 | 46 |
| 1/8/2022 | 58,446 | 1901.10 | 1,803 | 2,016 | 2,322 | 46 |
| 1/9/2022 | 58,045 | 1900.90 | 1,364 | 1,567 | 1,898 | 46 |
| 1/10/2022 | 57,778 | 1900.70 | 1,093 | 1,228 | 1,569 | 46 |
| 1/11/2022 | 57,607 | 1900.60 | 940 | 1,026 | 1,360 | 46 |
| 1/12/2022 | 57,492 | 1900.60 | 847 | 905 | 1,231 | 46 |
| 1/13/2022 | 57,378 | 1900.50 | 781 | 838 | 1,140 | 46 |
| 1/14/2022 | 57,302 | 1900.50 | 740 | 778 | 1,061 | 46 |
| 1/15/2022 | 57,208 | 1900.40 | 680 | 728 | 986 | 46 |
| 1/16/2022 | 57,151 | 1900.40 | 655 | 684 | 919 | 46 |
| 1/17/2022 | 57,056 | 1900.30 | 597 | 645 | 859 | 46 |
| 1/18/2022 | 57,113 | 1900.40 | 538 | 607 | 797 | 46 |
| 1/19/2022 | 57,037 | 1900.30 | 532 | 570 | 743 | 46 |
| 1/20/2022 | 56,980 | 1900.30 | 511 | 539 | 694 | 46 |
| 1/21/2022 | 56,943 | 1900.30 | 508 | 527 | 669 | 46 |
| 1/22/2022 | 56,943 | 1900.30 | 508 | 508 | 636 | 46 |
| 1/23/2022 | 56,869 | 1900.20 | 439 | 477 | 585 | 45 |
| 1/24/2022 | 56,834 | 1900.20 | 435 | 452 | 545 | 46 |
| 1/25/2022 | 56,792 | 1900.20 | 413 | 434 | 517 | 46 |
| 1/26/2022 | 56,754 | 1900.20 | 393 | 412 | 483 | 46 |
| 1/27/2022 | 56,697 | 1900.10 | 359 | 387 | 448 | 46 |
| 1/28/2022 | 56,660 | 1900.10 | 344 | 363 | 415 | 46 |
| 1/29/2022 | 56,603 | 1900.10 | 313 | 341 | 386 | 46 |
| 1/30/2022 | 56,566 | 1900.10 | 305 | 324 | 360 | 46 |
| 1/31/2022 | 56,547 | 1900.10 | 300 | 310 | 339 | 46 |
| 2/1/2022 | 56,509 | 1900.00 | 276 | 295 | 322 | 46 |
| 2/2/2022 | 56,472 | 1900.00 | 260 | 279 | 299 | 46 |
| 2/3/2022 | 56,453 | 1900.00 | 257 | 267 | 283 | 46 |
| 2/4/2022 | 56,397 | 1900.00 | 226 | 255 | 261 | 46 |
| 2/5/2022 | 56,359 | 1900.00 | 225 | 244 | 244 | 46 |
| 2/6/2022 | 56,340 | 1900.00 | 228 | 238 | 234 | 46 |
| 2/7/2022 | 56,303 | 1899.90 | 212 | 231 | 225 | 46 |
| 2/8/2022 | 56,303 | 1899.90 | 228 | 228 | 220 | 46 |
| 2/9/2022 | 56,265 | 1899.90 | 209 | 228 | 217 | 46 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/10/2022 | 56,265 | 1899.90 | 228 | 228 | 216 | 46 |
| 2/11/2022 | 56,303 | 1899.90 | 250 | 231 | 219 | 46 |
| 2/12/2022 | 56,303 | 1899.90 | 236 | 236 | 224 | 46 |
| 2/13/2022 | 56,303 | 1899.90 | 235 | 235 | 223 | 46 |
| 2/14/2022 | 56,322 | 1900.00 | 244 | 235 | 221 | 46 |
| 2/15/2022 | 56,284 | 1899.90 | 217 | 236 | 222 | 46 |
| 2/16/2022 | 56,247 | 1899.90 | 212 | 231 | 215 | 46 |
| 2/17/2022 | 56,190 | 1899.90 | 193 | 221 | 204 | 46 |
| 2/18/2022 | 56,153 | 1899.90 | 194 | 213 | 196 | 46 |
| 2/19/2022 | 56,116 | 1899.80 | 189 | 208 | 189 | 46 |
| 2/20/2022 | 56,041 | 1899.80 | 167 | 205 | 184 | 46 |
| 2/21/2022 | 56,003 | 1899.80 | 184 | 203 | 182 | 46 |
| 2/22/2022 | 55,966 | 1899.80 | 183 | 201 | 178 | 46 |
| 2/23/2022 | 55,891 | 1899.70 | 163 | 200 | 177 | 46 |
| 2/24/2022 | 55,798 | 1899.70 | 153 | 200 | 175 | 46 |
| 2/25/2022 | 55,705 | 1899.60 | 152 | 199 | 174 | 46 |
| 2/26/2022 | 55,631 | 1899.60 | 161 | 199 | 174 | 46 |
| 2/27/2022 | 55,500 | 1899.50 | 131 | 196 | 172 | 46 |
| 2/28/2022 | 55,389 | 1899.40 | 139 | 195 | 169 | 45 |
| 3/1/2022 | 55,315 | 1899.40 | 158 | 195 | 168 | 46 |
| 3/2/2022 | 55,222 | 1899.40 | 144 | 191 | 166 | 45 |
| 3/3/2022 | 55,185 | 1899.30 | 168 | 187 | 164 | 45 |
| 3/4/2022 | 55,130 | 1899.30 | 157 | 185 | 160 | 45 |
| 3/5/2022 | 55,000 | 1899.20 | 119 | 185 | 159 | 46 |
| 3/6/2022 | 54,908 | 1899.20 | 137 | 183 | 159 | 46 |
| 3/7/2022 | 54,779 | 1899.10 | 116 | 181 | 156 | 45 |
| 3/8/2022 | 54,650 | 1899.00 | 115 | 180 | 153 | 45 |
| 3/9/2022 | 54,540 | 1899.00 | 125 | 180 | 152 | 45 |
| 3/10/2022 | 54,430 | 1898.90 | 122 | 177 | 149 | 45 |
| 3/11/2022 | 54,283 | 1898.80 | 98 | 172 | 145 | 45 |
| 3/12/2022 | 54,192 | 1898.80 | 121 | 167 | 141 | 45 |
| 3/13/2022 | 54,100 | 1898.70 | 118 | 164 | 138 | 45 |
| 3/14/2022 | 54,009 | 1898.70 | 114 | 160 | 135 | 45 |
| 3/15/2022 | 54,009 | 1898.70 | 160 | 160 | 139 | 45 |
| 3/16/2022 | 53,936 | 1898.70 | 131 | 168 | 137 | 50 |
| 3/17/2022 | 53,826 | 1898.60 | 118 | 174 | 132 | 61 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/18/2022 | 53,717 | 1898.50 | 121 | 176 | 131 | 60 |
| 3/19/2022 | 53,625 | 1898.50 | 132 | 178 | 130 | 65 |
| 3/20/2022 | 53,497 | 1898.40 | 114 | 179 | 128 | 66 |
| 3/21/2022 | 53,379 | 1898.40 | 126 | 185 | 128 | 66 |
| 3/22/2022 | 53,260 | 1898.30 | 130 | 190 | 130 | 66 |
| 3/23/2022 | 53,151 | 1898.20 | 132 | 187 | 125 | 66 |
| 3/24/2022 | 53,061 | 1898.20 | 141 | 187 | 122 | 69 |
| 3/25/2022 | 52,934 | 1898.10 | 122 | 186 | 119 | 71 |
| 3/26/2022 | 52,807 | 1898.00 | 123 | 187 | 118 | 72 |
| 3/27/2022 | 52,735 | 1898.00 | 148 | 185 | 118 | 72 |
| 3/28/2022 | 52,663 | 1898.00 | 147 | 183 | 121 | 72 |
| 3/29/2022 | 52,536 | 1897.90 | 117 | 181 | 118 | 72 |
| 3/30/2022 | 52,428 | 1897.80 | 127 | 182 | 116 | 72 |
| 3/31/2022 | 52,285 | 1897.70 | 109 | 181 | 115 | 72 |
| 4/1/2022 | 52,123 | 1897.70 | 98 | 179 | 120 | 62 |
| 4/2/2022 | 51,979 | 1897.60 | 101 | 173 | 119 | 58 |
| 4/3/2022 | 51,835 | 1897.50 | 97 | 170 | 115 | 58 |
| 4/4/2022 | 51,691 | 1897.40 | 96 | 168 | 112 | 59 |
| 4/5/2022 | 51,548 | 1897.30 | 94 | 167 | 108 | 59 |
| 4/6/2022 | 51,404 | 1897.30 | 90 | 162 | 105 | 58 |
| 4/7/2022 | 51,279 | 1897.20 | 100 | 163 | 105 | 58 |
| 4/8/2022 | 51,083 | 1897.10 | 66 | 165 | 104 | 58 |
| 4/9/2022 | 50,914 | 1897.00 | 82 | 168 | 103 | 58 |
| 4/10/2022 | 50,745 | 1896.90 | 87 | 172 | 101 | 58 |
| 4/11/2022 | 50,675 | 1896.80 | 128 | 164 | 103 | 59 |
| 4/12/2022 | 50,551 | 1896.80 | 101 | 164 | 99 | 59 |
| 4/13/2022 | 50,427 | 1896.70 | 105 | 167 | 101 | 62 |
| 4/14/2022 | 50,941 | 1897.00 | 434 | 175 | 198 | 65 |
| 4/15/2022 | 51,261 | 1897.20 | 328 | 166 | 165 | 60 |
| 4/16/2022 | 52,627 | 1897.90 | 869 | 181 | 295 | 57 |
| 4/17/2022 | 53,043 | 1898.20 | 436 | 226 | 230 | 57 |
| 4/18/2022 | 53,279 | 1898.30 | 376 | 257 | 233 | 57 |
| 4/19/2022 | 53,900 | 1898.60 | 592 | 279 | 344 | 53 |
| 4/20/2022 | 54,797 | 1899.10 | 732 | 280 | 371 | 51 |
| 4/21/2022 | 57,664 | 1900.70 | 1,736 | 291 | 806 | 49 |
| 4/22/2022 | 59,082 | 1901.40 | 1,016 | 301 | 565 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/23/2022 | 59,935 | 1901.80 | 743 | 313 | 457 | 46 |
| 4/24/2022 | 60,541 | 1902.10 | 605 | 300 | 386 | 46 |
| 4/25/2022 | 60,992 | 1902.40 | 545 | 317 | 365 | 46 |
| 4/26/2022 | 61,367 | 1902.60 | 480 | 291 | 326 | 46 |
| 4/27/2022 | 61,644 | 1902.70 | 435 | 296 | 304 | 46 |
| 4/28/2022 | 61,822 | 1902.80 | 372 | 283 | 292 | 46 |
| 4/29/2022 | 61,961 | 1902.90 | 336 | 266 | 266 | 46 |
| 4/30/2022 | 62,060 | 1902.90 | 323 | 273 | 259 | 46 |
| 5/1/2022 | 62,120 | 1902.90 | 302 | 272 | 244 | 52 |
| 5/2/2022 | 62,060 | 1902.90 | 245 | 275 | 245 | 55 |
| 5/3/2022 | 62,080 | 1902.90 | 289 | 279 | 236 | 63 |
| 5/4/2022 | 61,981 | 1902.90 | 233 | 283 | 242 | 60 |
| 5/5/2022 | 61,882 | 1902.80 | 228 | 278 | 237 | 58 |
| 5/6/2022 | 61,783 | 1902.80 | 222 | 272 | 233 | 58 |
| 5/7/2022 | 61,664 | 1902.70 | 213 | 273 | 232 | 53 |
| 5/8/2022 | 61,625 | 1902.70 | 262 | 282 | 229 | 51 |
| 5/9/2022 | 61,506 | 1902.60 | 209 | 269 | 233 | 51 |
| 5/10/2022 | 61,367 | 1902.60 | 205 | 275 | 236 | 54 |
| 5/11/2022 | 61,209 | 1902.50 | 188 | 268 | 216 | 62 |
| 5/12/2022 | 61,091 | 1902.40 | 201 | 261 | 207 | 65 |
| 5/13/2022 | 60,973 | 1902.40 | 189 | 249 | 203 | 59 |
| 5/14/2022 | 60,815 | 1902.30 | 178 | 257 | 196 | 64 |
| 5/15/2022 | 60,619 | 1902.20 | 182 | 281 | 183 | 98 |
| 5/16/2022 | 60,384 | 1902.10 | 153 | 271 | 176 | 97 |
| 5/17/2022 | 60,150 | 1901.90 | 139 | 257 | 163 | 97 |
| 5/18/2022 | 59,916 | 1901.80 | 140 | 257 | 158 | 97 |
| 5/19/2022 | 59,644 | 1901.70 | 110 | 247 | 151 | 97 |
| 5/20/2022 | 59,411 | 1901.60 | 117 | 234 | 139 | 98 |
| 5/21/2022 | 59,178 | 1901.40 | 118 | 235 | 132 | 103 |
| 5/22/2022 | 58,927 | 1901.30 | 110 | 237 | 131 | 105 |
| 5/23/2022 | 58,715 | 1901.20 | 122 | 229 | 123 | 105 |
| 5/24/2022 | 58,504 | 1901.10 | 117 | 224 | 115 | 109 |
| 5/25/2022 | 58,274 | 1901.00 | 101 | 217 | 109 | 111 |
| 5/26/2022 | 58,025 | 1900.80 | 93 | 218 | 104 | 111 |
| 5/27/2022 | 57,759 | 1900.70 | 85 | 219 | 103 | 111 |
| 5/28/2022 | 57,530 | 1900.60 | 103 | 218 | 106 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/29/2022 | 57,302 | 1900.50 | 95 | 210 | 101 | 111 |
| 5/30/2022 | 57,056 | 1900.30 | 79 | 203 | 92 | 111 |
| 5/31/2022 | 56,811 | 1900.20 | 77 | 201 | 90 | 111 |
| 6/1/2022 | 56,603 | 1900.10 | 93 | 198 | 86 | 111 |
| 6/2/2022 | 56,367 | 1900.00 | 78 | 198 | 85 | 112 |
| 6/3/2022 | 56,116 | 1899.80 | 70 | 197 | 83 | 113 |
| 6/4/2022 | 56,003 | 1899.80 | 135 | 192 | 86 | 108 |
| 6/5/2022 | 56,003 | 1899.80 | 184 | 184 | 104 | 101 |
| 6/6/2022 | 55,854 | 1899.70 | 108 | 184 | 95 | 94 |
| 6/7/2022 | 55,649 | 1899.60 | 80 | 184 | 98 | 90 |
| 6/8/2022 | 55,445 | 1899.50 | 77 | 180 | 95 | 90 |
| 6/9/2022 | 55,222 | 1899.40 | 63 | 175 | 89 | 90 |
| 6/10/2022 | 55,019 | 1899.20 | 68 | 170 | 85 | 91 |
| 6/11/2022 | 54,816 | 1899.10 | 67 | 170 | 77 | 99 |
| 6/12/2022 | 54,595 | 1899.00 | 56 | 167 | 75 | 102 |
| 6/13/2022 | 54,375 | 1898.90 | 49 | 160 | 69 | 102 |
| 6/14/2022 | 54,210 | 1898.80 | 69 | 152 | 68 | 97 |
| 6/15/2022 | 54,009 | 1898.70 | 46 | 148 | 65 | 95 |
| 6/16/2022 | 53,808 | 1898.60 | 51 | 153 | 58 | 106 |
| 6/17/2022 | 53,570 | 1898.50 | 35 | 154 | 57 | 111 |
| 6/18/2022 | 53,351 | 1898.30 | 41 | 151 | 55 | 111 |
| 6/19/2022 | 53,170 | 1898.20 | 54 | 146 | 51 | 111 |
| 6/20/2022 | 52,988 | 1898.10 | 50 | 142 | 47 | 110 |
| 6/21/2022 | 52,825 | 1898.00 | 55 | 137 | 42 | 111 |
| 6/22/2022 | 52,645 | 1897.90 | 43 | 134 | 38 | 112 |
| 6/23/2022 | 52,554 | 1897.90 | 89 | 134 | 35 | 115 |
| 6/24/2022 | 52,312 | 1897.80 | 14 | 136 | 34 | 116 |
| 6/25/2022 | 52,069 | 1897.60 | 13 | 135 | 33 | 116 |
| 6/26/2022 | 51,826 | 1897.50 | 10 | 133 | 30 | 116 |
| 6/27/2022 | 51,583 | 1897.40 | 11 | 133 | 26 | 118 |
| 6/28/2022 | 51,351 | 1897.20 | 16 | 134 | 26 | 120 |
| 6/29/2022 | 51,119 | 1897.10 | 17 | 134 | 27 | 120 |
| 6/30/2022 | 50,887 | 1897.00 | 18 | 135 | 27 | 120 |
| 7/1/2022 | 50,657 | 1896.80 | 19 | 135 | 29 | 120 |
| 7/2/2022 | 50,462 | 1896.70 | 35 | 133 | 26 | 120 |
| 7/3/2022 | 50,232 | 1896.60 | 17 | 133 | 26 | 120 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/4/2022 | 50,055 | 1896.50 | 43 | 133 | 26 | 120 |
| 7/5/2022 | 49,844 | 1896.40 | 23 | 130 | 26 | 120 |
| 7/6/2022 | 49,650 | 1896.30 | 31 | 129 | 23 | 119 |
| 7/7/2022 | 49,422 | 1896.10 | 13 | 129 | 22 | 120 |
| 7/8/2022 | 49,194 | 1896.00 | 14 | 129 | 21 | 120 |
| 7/9/2022 | 48,968 | 1895.90 | 14 | 128 | 20 | 120 |
| 7/10/2022 | 48,759 | 1895.80 | 23 | 128 | 20 | 120 |
| 7/11/2022 | 48,534 | 1895.60 | 17 | 131 | 18 | 121 |
| 7/12/2022 | 48,290 | 1895.50 | 9 | 132 | 22 | 120 |
| 7/13/2022 | 48,030 | 1895.30 | 0 | 131 | 20 | 119 |
| 7/14/2022 | 47,806 | 1895.20 | 21 | 134 | 22 | 120 |
| 7/15/2022 | 47,531 | 1895.00 | -1 | 137 | 24 | 120 |
| 7/16/2022 | 47,274 | 1894.90 | 8 | 138 | 27 | 120 |
| 7/17/2022 | 47,018 | 1894.70 | 8 | 137 | 26 | 120 |
| 7/18/2022 | 46,729 | 1894.60 | -9 | 137 | 25 | 120 |
| 7/19/2022 | 46,456 | 1894.40 | -1 | 136 | 24 | 121 |
| 7/20/2022 | 46,185 | 1894.30 | 1 | 138 | 26 | 121 |
| 7/21/2022 | 45,914 | 1894.10 | 0 | 136 | 26 | 120 |
| 7/22/2022 | 45,612 | 1893.90 | -18 | 135 | 24 | 119 |
| 7/23/2022 | 45,377 | 1893.80 | 16 | 134 | 23 | 120 |
| 7/24/2022 | 45,077 | 1893.60 | -17 | 134 | 23 | 120 |
| 7/25/2022 | 44,809 | 1893.40 | -3 | 131 | 21 | 121 |
| 7/26/2022 | 44,560 | 1893.30 | 3 | 129 | 21 | 117 |
| 7/27/2022 | 44,311 | 1893.10 | 3 | 128 | 22 | 115 |
| 7/28/2022 | 44,113 | 1893.00 | -5 | 95 | 22 | 90 |
| 7/29/2022 | 43,981 | 1892.90 | 6 | 73 | 24 | 57 |
| 7/30/2022 | 43,850 | 1892.90 | 2 | 68 | 18 | 55 |
| 7/31/2022 | 43,719 | 1892.80 | 2 | 68 | 17 | 55 |
| 8/1/2022 | 43,604 | 1892.70 | 11 | 69 | 17 | 55 |
| 8/2/2022 | 43,473 | 1892.60 | 3 | 69 | 18 | 55 |
| 8/3/2022 | 43,358 | 1892.60 | 12 | 70 | 17 | 55 |
| 8/4/2022 | 43,211 | 1892.50 | -4 | 71 | 16 | 55 |
| 8/5/2022 | 43,080 | 1892.40 | 7 | 72 | 14 | 55 |
| 8/6/2022 | 42,934 | 1892.30 | -1 | 73 | 18 | 55 |
| 8/7/2022 | 42,788 | 1892.20 | -2 | 72 | 16 | 56 |
| 8/8/2022 | 42,642 | 1892.10 | -2 | 72 | 15 | 56 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/9/2022 | 42,480 | 1892.00 | -10 | 72 | 15 | 56 |
| 8/10/2022 | 42,351 | 1891.90 | 6 | 72 | 15 | 56 |
| 8/11/2022 | 42,190 | 1891.80 | -9 | 72 | 15 | 56 |
| 8/12/2022 | 42,046 | 1891.70 | -1 | 72 | 15 | 56 |
| 8/13/2022 | 41,886 | 1891.60 | -9 | 71 | 15 | 56 |
| 8/14/2022 | 41,741 | 1891.60 | 0 | 73 | 15 | 56 |
| 8/15/2022 | 41,597 | 1891.50 | 4 | 77 | 17 | 56 |
| 8/16/2022 | 41,453 | 1891.40 | 0 | 72 | 15 | 56 |
| 8/17/2022 | 41,326 | 1891.30 | 7 | 71 | 12 | 56 |
| 8/18/2022 | 41,151 | 1891.20 | -14 | 74 | 12 | 55 |
| 8/19/2022 | 40,993 | 1891.10 | -5 | 75 | 13 | 56 |
| 8/20/2022 | 40,850 | 1891.00 | 4 | 76 | 15 | 56 |
| 8/21/2022 | 40,693 | 1890.90 | -7 | 72 | 19 | 50 |
| 8/22/2022 | 40,551 | 1890.80 | -3 | 68 | 19 | 47 |
| 8/23/2022 | 40,426 | 1890.70 | 4 | 67 | 15 | 48 |
| 8/24/2022 | 40,301 | 1890.70 | 9 | 72 | 19 | 47 |
| 8/25/2022 | 40,081 | 1890.50 | 10 | 75 | 19 | 50 |
| 8/26/2022 | 39,925 | 1890.40 | 1 | 80 | 20 | 53 |
| 8/27/2022 | 39,769 | 1890.30 | 2 | 81 | 21 | 54 |
| 8/28/2022 | 39,613 | 1890.20 | 2 | 80 | 19 | 56 |
| 8/29/2022 | 39,474 | 1890.10 | 10 | 80 | 18 | 56 |
| 8/30/2022 | 39,303 | 1890.00 | -4 | 82 | 18 | 56 |
| 8/31/2022 | 39,149 | 1889.90 | 5 | 82 | 19 | 56 |
| 9/1/2022 | 38,980 | 1889.80 | -3 | 82 | 18 | 56 |
| 9/2/2022 | 38,812 | 1889.70 | -4 | 81 | 19 | 51 |
| 9/3/2022 | 38,658 | 1889.60 | 5 | 82 | 18 | 50 |
| 9/4/2022 | 38,505 | 1889.50 | 4 | 81 | 21 | 50 |
| 9/5/2022 | 38,368 | 1889.40 | 7 | 76 | 20 | 55 |
| 9/6/2022 | 38,216 | 1889.30 | -1 | 76 | 16 | 57 |
| 9/7/2022 | 38,049 | 1889.20 | -4 | 80 | 20 | 56 |
| 9/8/2022 | 37,883 | 1889.10 | 1 | 85 | 25 | 55 |
| 9/9/2022 | 37,702 | 1889.00 | -2 | 89 | 24 | 56 |
| 9/10/2022 | 37,521 | 1888.80 | -5 | 86 | 26 | 56 |
| 9/11/2022 | 37,342 | 1888.70 | -6 | 84 | 26 | 56 |
| 9/12/2022 | 37,192 | 1888.60 | 6 | 81 | 30 | 50 |
| 9/13/2022 | 37,013 | 1888.50 | -14 | 76 | 30 | 47 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/14/2022 | 36,864 | 1888.40 | -1 | 74 | 33 | 43 |
| 9/15/2022 | 36,731 | 1888.30 | 4 | 72 | 33 | 42 |
| 9/16/2022 | 36,598 | 1888.20 | 1 | 68 | 37 | 38 |
| 9/17/2022 | 36,487 | 1888.10 | 6 | 62 | 41 | 29 |
| 9/18/2022 | 36,553 | 1888.20 | 91 | 58 | 46 | 25 |
| 9/19/2022 | 36,672 | 1888.30 | 111 | 51 | 47 | 20 |
| 9/20/2022 | 36,642 | 1888.20 | 67 | 82 | 57 | 17 |
| 9/21/2022 | 36,494 | 1888.10 | 32 | 106 | 91 | 17 |
| 9/22/2022 | 36,332 | 1888.00 | 26 | 108 | 96 | 17 |
| 9/23/2022 | 36,185 | 1887.90 | 30 | 104 | 93 | 16 |
| 9/24/2022 | 36,024 | 1887.80 | 20 | 101 | 93 | 15 |
| 9/25/2022 | 35,864 | 1887.70 | 19 | 100 | 90 | 15 |
| 9/26/2022 | 35,689 | 1887.60 | 12 | 100 | 91 | 15 |
| 9/27/2022 | 35,515 | 1887.50 | 13 | 101 | 91 | 15 |
| 9/28/2022 | 35,328 | 1887.40 | 9 | 104 | 92 | 15 |
| 9/29/2022 | 35,083 | 1887.20 | 10 | 104 | 93 | 15 |
| 9/30/2022 | 34,897 | 1887.10 | 12 | 105 | 91 | 17 |
| 10/1/2022 | 34,740 | 1886.90 | 9 | 88 | 108 | 20 |
| 10/2/2022 | 34,612 | 1886.90 | 9 | 73 | 79 | 20 |
| 10/3/2022 | 34,499 | 1886.80 | 15 | 72 | 73 | 17 |
| 10/4/2022 | 34,386 | 1886.70 | 10 | 67 | 65 | 15 |
| 10/5/2022 | 34,273 | 1886.60 | 8 | 65 | 54 | 15 |
| 10/6/2022 | 34,174 | 1886.50 | 15 | 65 | 54 | 15 |
| 10/7/2022 | 34,075 | 1886.50 | 15 | 65 | 54 | 15 |
| 10/8/2022 | 33,935 | 1886.40 | 2 | 73 | 55 | 18 |
| 10/9/2022 | 33,777 | 1886.30 | -2 | 78 | 58 | 25 |
| 10/10/2022 | 33,620 | 1886.10 | 7 | 86 | 59 | 27 |
| 10/11/2022 | 33,449 | 1886.00 | 6 | 92 | 61 | 35 |
| 10/12/2022 | 33,293 | 1885.90 | 13 | 92 | 62 | 35 |
| 10/13/2022 | 33,124 | 1885.80 | 7 | 92 | 62 | 35 |
| 10/14/2022 | 32,941 | 1885.70 | 9 | 101 | 65 | 36 |
| 10/15/2022 | 32,745 | 1885.50 | 9 | 107 | 65 | 45 |
| 10/16/2022 | 32,550 | 1885.40 | 9 | 107 | 65 | 45 |
| 10/17/2022 | 32,355 | 1885.20 | 6 | 104 | 67 | 42 |
| 10/18/2022 | 32,189 | 1885.10 | 18 | 102 | 66 | 41 |
| 10/19/2022 | 31,997 | 1885.00 | 6 | 102 | 67 | 41 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/20/2022 | 31,836 | 1884.80 | 16 | 97 | 67 | 38 |
| 10/21/2022 | 31,675 | 1884.70 | 10 | 91 | 72 | 29 |
| 10/22/2022 | 31,488 | 1884.60 | -7 | 87 | 70 | 25 |
| 10/23/2022 | 31,342 | 1884.50 | 12 | 86 | 69 | 25 |
| 10/24/2022 | 31,183 | 1884.30 | 5 | 85 | 70 | 25 |
| 10/25/2022 | 31,037 | 1884.20 | 13 | 86 | 70 | 25 |
| 10/26/2022 | 30,879 | 1884.10 | 6 | 86 | 70 | 25 |
| 10/27/2022 | 30,734 | 1884.00 | 13 | 86 | 71 | 25 |
| 10/28/2022 | 30,590 | 1883.90 | 13 | 85 | 71 | 25 |
| 10/29/2022 | 30,447 | 1883.80 | 13 | 86 | 71 | 25 |
| 10/30/2022 | 30,291 | 1883.70 | 7 | 85 | 72 | 25 |
| 10/31/2022 | 30,148 | 1883.60 | 14 | 86 | 72 | 24 |
| 11/1/2022 | 30,084 | 1883.50 | 42 | 74 | 81 | 11 |
| 11/2/2022 | 29,980 | 1883.40 | 20 | 72 | 73 | 11 |
| 11/3/2022 | 29,877 | 1883.40 | 19 | 71 | 72 | 11 |
| 11/4/2022 | 29,800 | 1883.30 | 31 | 70 | 72 | 11 |
| 11/5/2022 | 29,736 | 1883.20 | 36 | 69 | 74 | 11 |
| 11/6/2022 | 29,736 | 1883.20 | 69 | 69 | 73 | 11 |
| 11/7/2022 | 29,749 | 1883.30 | 77 | 70 | 84 | 11 |
| 11/8/2022 | 29,903 | 1883.40 | 152 | 74 | 90 | 11 |
| 11/9/2022 | 29,903 | 1883.40 | 82 | 82 | 84 | 11 |
| 11/10/2022 | 29,813 | 1883.30 | 48 | 94 | 91 | 11 |
| 11/11/2022 | 29,711 | 1883.20 | 50 | 102 | 98 | 11 |
| 11/12/2022 | 29,608 | 1883.10 | 53 | 105 | 101 | 11 |
| 11/13/2022 | 29,481 | 1883.00 | 43 | 107 | 102 | 11 |
| 11/14/2022 | 29,328 | 1882.90 | 31 | 108 | 104 | 11 |
| 11/15/2022 | 29,227 | 1882.80 | 50 | 102 | 101 | 11 |
| 11/16/2022 | 29,126 | 1882.80 | 40 | 91 | 93 | 11 |
| 11/17/2022 | 29,037 | 1882.70 | 36 | 80 | 85 | 11 |
| 11/18/2022 | 28,962 | 1882.60 | 37 | 75 | 79 | 11 |
| 11/19/2022 | 28,886 | 1882.60 | 36 | 74 | 76 | 11 |
| 11/20/2022 | 28,811 | 1882.50 | 35 | 73 | 76 | 11 |
| 11/21/2022 | 28,723 | 1882.40 | 29 | 73 | 76 | 11 |
| 11/22/2022 | 28,648 | 1882.40 | 36 | 74 | 76 | 11 |
| 11/23/2022 | 28,561 | 1882.30 | 29 | 73 | 76 | 11 |
| 11/24/2022 | 28,499 | 1882.30 | 43 | 74 | 76 | 11 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/25/2022 | 28,412 | 1882.20 | 30 | 74 | 76 | 11 |
| 11/26/2022 | 28,338 | 1882.10 | 36 | 74 | 76 | 11 |
| 11/27/2022 | 28,264 | 1882.10 | 36 | 74 | 76 | 11 |
| 11/28/2022 | 28,177 | 1882.00 | 30 | 74 | 76 | 11 |
| 11/29/2022 | 27,965 | 1881.80 | -33 | 74 | 76 | 12 |
| 11/30/2022 | 27,864 | 1881.70 | 37 | 88 | 76 | 12 |
| 12/1/2022 | 28,008 | 1881.90 | 220 | 148 | 159 | 12 |
| 12/2/2022 | 27,944 | 1881.80 | 110 | 143 | 142 | 11 |
| 12/3/2022 | 27,806 | 1881.70 | 73 | 143 | 137 | 12 |
| 12/4/2022 | 27,803 | 1881.70 | 128 | 129 | 136 | 12 |
| 12/5/2022 | 28,403 | 1882.20 | 431 | 128 | 225 | 11 |
| 12/6/2022 | 28,562 | 1882.30 | 207 | 127 | 162 | 11 |
| 12/7/2022 | 28,509 | 1882.30 | 121 | 148 | 145 | 11 |
| 12/8/2022 | 28,518 | 1882.30 | 165 | 160 | 165 | 11 |
| 12/9/2022 | 28,590 | 1882.30 | 194 | 158 | 196 | 11 |
| 12/10/2022 | 30,840 | 1884.10 | 1,305 | 171 | 419 | 11 |
| 12/11/2022 | 32,130 | 1885.10 | 828 | 177 | 395 | 11 |
| 12/12/2022 | 32,438 | 1885.30 | 328 | 172 | 230 | 11 |
| 12/13/2022 | 32,530 | 1885.40 | 213 | 167 | 191 | 11 |
| 12/14/2022 | 32,540 | 1885.40 | 172 | 167 | 178 | 11 |
| 12/15/2022 | 32,510 | 1885.30 | 152 | 168 | 171 | 11 |
| 12/16/2022 | 32,438 | 1885.30 | 132 | 168 | 167 | 11 |
| 12/17/2022 | 32,367 | 1885.20 | 132 | 168 | 164 | 11 |
| 12/18/2022 | 32,296 | 1885.20 | 133 | 169 | 162 | 11 |
| 12/19/2022 | 32,184 | 1885.10 | 113 | 169 | 160 | 11 |
| 12/20/2022 | 32,086 | 1885.00 | 111 | 161 | 154 | 11 |
| 12/21/2022 | 32,056 | 1885.00 | 128 | 143 | 140 | 11 |
| 12/22/2022 | 32,174 | 1885.10 | 193 | 134 | 128 | 11 |
| 12/23/2022 | 32,415 | 1885.30 | 253 | 132 | 128 | 11 |
| 12/24/2022 | 32,712 | 1885.50 | 280 | 130 | 126 | 11 |
| 12/25/2022 | 33,010 | 1885.70 | 285 | 134 | 125 | 11 |
| 12/26/2022 | 33,643 | 1886.20 | 473 | 154 | 146 | 11 |
| 12/27/2022 | 46,862 | 1894.70 | 6,873 | 209 | 1,469 | 34 |
| 12/28/2022 | 49,643 | 1896.30 | 1,617 | 215 | 439 | 45 |
| 12/29/2022 | 52,201 | 1897.70 | 1,510 | 220 | 395 | 45 |
| 12/30/2022 | 65,043 | 1904.40 | 9,644 | 3,170 | 4,171 | 34 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/31/2022 | 60,753 | 1902.30 | 7,402 | 9,565 | 9,492 | 10 |
| 1/1/2023 | 58,843 | 1901.30 | 2,832 | 3,796 | 3,482 | 45 |
| 1/2/2023 | 58,051 | 1900.90 | 1,782 | 2,181 | 2,037 | 45 |
| 1/3/2023 | 57,514 | 1900.60 | 1,214 | 1,485 | 1,492 | 45 |
| 1/4/2023 | 59,855 | 1901.80 | 2,639 | 1,459 | 1,660 | 45 |
| 1/5/2023 | 62,430 | 1903.10 | 9,039 | 7,741 | 7,503 | 9 |
| 1/6/2023 | 60,460 | 1902.10 | 4,551 | 5,544 | 5,387 | 22 |
| 1/7/2023 | 61,469 | 1902.60 | 5,267 | 4,758 | 4,672 | 46 |
| 1/8/2023 | 62,465 | 1903.10 | 7,818 | 7,315 | 7,383 | 14 |
| 1/9/2023 | 63,461 | 1903.60 | 9,678 | 9,175 | 9,196 | 2 |
| 1/10/2023 | 62,698 | 1903.20 | 7,477 | 7,861 | 7,847 | 2 |
| 1/11/2023 | 62,858 | 1903.30 | 7,498 | 7,417 | 7,471 | 2 |
| 1/12/2023 | 61,625 | 1902.70 | 7,570 | 8,192 | 8,071 | 2 |
| 1/13/2023 | 62,878 | 1903.30 | 7,250 | 6,618 | 6,147 | 14 |
| 1/14/2023 | 63,945 | 1903.90 | 9,938 | 9,401 | 9,538 | 2 |
| 1/15/2023 | 61,427 | 1902.60 | 6,441 | 7,710 | 8,295 | 2 |
| 1/16/2023 | 59,877 | 1901.80 | 4,006 | 4,787 | 4,631 | 43 |
| 1/17/2023 | 58,927 | 1901.30 | 2,690 | 3,169 | 3,034 | 45 |
| 1/18/2023 | 58,715 | 1901.20 | 2,260 | 2,367 | 2,288 | 45 |
| 1/19/2023 | 58,293 | 1901.00 | 1,985 | 2,198 | 2,120 | 45 |
| 1/20/2023 | 57,892 | 1900.80 | 1,527 | 1,729 | 1,715 | 45 |
| 1/21/2023 | 57,645 | 1900.60 | 1,262 | 1,386 | 1,448 | 45 |
| 1/22/2023 | 57,454 | 1900.50 | 1,082 | 1,178 | 1,269 | 45 |
| 1/23/2023 | 57,321 | 1900.50 | 961 | 1,028 | 1,130 | 45 |
| 1/24/2023 | 57,170 | 1900.40 | 833 | 910 | 1,016 | 45 |
| 1/25/2023 | 57,454 | 1900.50 | 949 | 805 | 907 | 45 |
| 1/26/2023 | 57,359 | 1900.50 | 727 | 775 | 868 | 45 |
| 1/27/2023 | 57,283 | 1900.50 | 689 | 728 | 813 | 45 |
| 1/28/2023 | 57,264 | 1900.40 | 691 | 701 | 776 | 45 |
| 1/29/2023 | 57,208 | 1900.40 | 645 | 673 | 743 | 45 |
| 1/30/2023 | 57,113 | 1900.40 | 580 | 628 | 692 | 44 |
| 1/31/2023 | 57,037 | 1900.30 | 537 | 576 | 626 | 44 |
| 2/1/2023 | 56,980 | 1900.30 | 518 | 547 | 580 | 44 |
| 2/2/2023 | 56,924 | 1900.30 | 494 | 523 | 542 | 45 |
| 2/3/2023 | 56,980 | 1900.30 | 554 | 526 | 547 | 45 |
| 2/4/2023 | 57,037 | 1900.30 | 554 | 525 | 538 | 45 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 2/5/2023 | 57,816 | 1900.70 | 1,232 | 839 | 939 | 45 |
| 2/6/2023 | 57,549 | 1900.60 | 858 | 993 | 1,099 | 45 |
| 2/7/2023 | 57,359 | 1900.50 | 704 | 799 | 902 | 45 |
| 2/8/2023 | 57,227 | 1900.40 | 634 | 701 | 790 | 45 |
| 2/9/2023 | 57,132 | 1900.40 | 595 | 643 | 717 | 45 |
| 2/10/2023 | 57,094 | 1900.40 | 588 | 607 | 667 | 45 |
| 2/11/2023 | 57,056 | 1900.30 | 563 | 582 | 633 | 45 |
| 2/12/2023 | 56,980 | 1900.30 | 515 | 553 | 594 | 45 |
| 2/13/2023 | 56,943 | 1900.30 | 512 | 531 | 556 | 45 |
| 2/14/2023 | 56,886 | 1900.20 | 478 | 507 | 522 | 45 |
| 2/15/2023 | 56,829 | 1900.20 | 454 | 483 | 490 | 45 |
| 2/16/2023 | 56,792 | 1900.20 | 440 | 459 | 458 | 45 |
| 2/17/2023 | 56,773 | 1900.20 | 431 | 441 | 430 | 45 |
| 2/18/2023 | 56,716 | 1900.20 | 394 | 422 | 406 | 45 |
| 2/19/2023 | 56,679 | 1900.10 | 387 | 406 | 382 | 45 |
| 2/20/2023 | 56,660 | 1900.10 | 382 | 392 | 362 | 45 |
| 2/21/2023 | 56,679 | 1900.10 | 405 | 396 | 361 | 45 |
| 2/22/2023 | 56,697 | 1900.10 | 408 | 398 | 364 | 45 |
| 2/23/2023 | 56,754 | 1900.20 | 431 | 403 | 370 | 45 |
| 2/24/2023 | 56,773 | 1900.20 | 452 | 443 | 421 | 45 |
| 2/25/2023 | 56,735 | 1900.20 | 408 | 427 | 403 | 45 |
| 2/26/2023 | 56,924 | 1900.30 | 547 | 452 | 448 | 45 |
| 2/27/2023 | 57,132 | 1900.40 | 660 | 555 | 589 | 45 |
| 2/28/2023 | 57,283 | 1900.50 | 732 | 656 | 744 | 45 |
| 3/1/2023 | 57,170 | 1900.40 | 603 | 661 | 756 | 45 |
| 3/2/2023 | 57,208 | 1900.40 | 646 | 626 | 707 | 45 |
| 3/3/2023 | 57,340 | 1900.50 | 755 | 688 | 772 | 45 |
| 3/4/2023 | 57,511 | 1900.60 | 865 | 778 | 875 | 45 |
| 3/5/2023 | 57,530 | 1900.60 | 871 | 861 | 983 | 45 |
| 3/6/2023 | 57,454 | 1900.50 | 778 | 817 | 948 | 45 |
| 3/7/2023 | 57,454 | 1900.50 | 777 | 777 | 914 | 45 |
| 3/8/2023 | 57,568 | 1900.60 | 904 | 846 | 1,019 | 45 |
| 3/9/2023 | 59,178 | 1901.40 | 1,838 | 1,027 | 1,230 | 45 |
| 3/10/2023 | 60,560 | 1902.20 | 4,966 | 4,269 | 4,496 | 45 |
| 3/11/2023 | 59,586 | 1901.70 | 2,777 | 3,269 | 3,274 | 45 |
| 3/12/2023 | 62,498 | 1903.10 | 5,164 | 3,695 | 3,990 | 40 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/13/2023 | 64,066 | 1903.90 | 9,495 | 8,705 | 9,676 | 2 |
| 3/14/2023 | 67,407 | 1905.50 | 14,895 | 13,211 | 15,903 | 2 |
| 3/15/2023 | 62,718 | 1903.20 | 9,305 | 11,669 | 14,610 | 2 |
| 3/16/2023 | 60,874 | 1902.30 | 4,451 | 5,380 | 6,544 | 27 |
| 3/17/2023 | 59,877 | 1901.80 | 3,191 | 3,694 | 4,368 | 45 |
| 3/18/2023 | 59,392 | 1901.60 | 2,716 | 2,961 | 3,401 | 46 |
| 3/19/2023 | 60,462 | 1902.10 | 3,845 | 3,305 | 3,784 | 45 |
| 3/20/2023 | 60,033 | 1901.90 | 3,521 | 3,738 | 4,345 | 45 |
| 3/21/2023 | 59,877 | 1901.80 | 3,125 | 3,203 | 3,625 | 45 |
| 3/22/2023 | 59,605 | 1901.70 | 3,024 | 3,161 | 3,463 | 45 |
| 3/23/2023 | 59,140 | 1901.40 | 2,477 | 2,712 | 2,982 | 45 |
| 3/24/2023 | 58,735 | 1901.20 | 2,066 | 2,270 | 2,501 | 45 |
| 3/25/2023 | 58,408 | 1901.00 | 1,725 | 1,890 | 2,102 | 45 |
| 3/26/2023 | 58,178 | 1900.90 | 1,484 | 1,600 | 1,793 | 45 |
| 3/27/2023 | 57,987 | 1900.80 | 1,291 | 1,387 | 1,578 | 45 |
| 3/28/2023 | 58,773 | 1901.20 | 2,056 | 1,659 | 1,831 | 45 |
| 3/29/2023 | 58,715 | 1901.20 | 2,052 | 2,081 | 2,350 | 45 |
| 3/30/2023 | 58,446 | 1901.10 | 1,736 | 1,872 | 2,131 | 45 |
| 3/31/2023 | 58,255 | 1901.00 | 1,580 | 1,677 | 1,906 | 45 |
| 4/1/2023 | 58,178 | 1900.90 | 1,523 | 1,562 | 1,772 | 45 |
| 4/2/2023 | 58,159 | 1900.90 | 1,550 | 1,560 | 1,773 | 45 |
| 4/3/2023 | 57,949 | 1900.80 | 1,344 | 1,450 | 1,665 | 45 |
| 4/4/2023 | 57,759 | 1900.70 | 1,167 | 1,263 | 1,497 | 45 |
| 4/5/2023 | 57,645 | 1900.60 | 1,065 | 1,123 | 1,360 | 45 |
| 4/6/2023 | 57,664 | 1900.70 | 1,071 | 1,061 | 1,283 | 45 |
| 4/7/2023 | 58,351 | 1901.00 | 1,798 | 1,452 | 1,632 | 45 |
| 4/8/2023 | 58,217 | 1900.90 | 1,635 | 1,703 | 1,883 | 44 |
| 4/9/2023 | 58,197 | 1900.90 | 1,554 | 1,564 | 1,732 | 44 |
| 4/10/2023 | 58,542 | 1901.10 | 1,892 | 1,718 | 1,821 | 44 |
| 4/11/2023 | 58,927 | 1901.30 | 2,289 | 2,095 | 2,157 | 44 |
| 4/12/2023 | 58,715 | 1901.20 | 2,043 | 2,150 | 2,243 | 44 |
| 4/13/2023 | 58,408 | 1901.00 | 1,685 | 1,840 | 1,956 | 44 |
| 4/14/2023 | 58,178 | 1900.90 | 1,460 | 1,576 | 1,686 | 44 |
| 4/15/2023 | 58,045 | 1900.90 | 1,374 | 1,442 | 1,518 | 78 |
| 4/16/2023 | 58,006 | 1900.80 | 1,365 | 1,384 | 1,439 | 89 |
| 4/17/2023 | 57,968 | 1900.80 | 1,321 | 1,341 | 1,396 | 89 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/18/2023 | 57,816 | 1900.70 | 1,179 | 1,256 | 1,341 | 90 |
| 4/19/2023 | 57,607 | 1900.60 | 978 | 1,083 | 1,194 | 90 |
| 4/20/2023 | 57,454 | 1900.50 | 864 | 940 | 1,058 | 90 |
| 4/21/2023 | 57,530 | 1900.60 | 947 | 909 | 1,002 | 90 |
| 4/22/2023 | 57,778 | 1900.70 | 1,176 | 1,051 | 1,102 | 90 |
| 4/23/2023 | 57,968 | 1900.80 | 1,349 | 1,253 | 1,269 | 90 |
| 4/24/2023 | 57,930 | 1900.80 | 1,303 | 1,323 | 1,340 | 90 |
| 4/25/2023 | 57,835 | 1900.70 | 1,189 | 1,237 | 1,280 | 90 |
| 4/26/2023 | 57,816 | 1900.70 | 1,171 | 1,180 | 1,220 | 90 |
| 4/27/2023 | 57,873 | 1900.80 | 1,227 | 1,198 | 1,223 | 90 |
| 4/28/2023 | 57,930 | 1900.80 | 1,282 | 1,253 | 1,260 | 90 |
| 4/29/2023 | 57,854 | 1900.80 | 1,227 | 1,265 | 1,274 | 90 |
| 4/30/2023 | 57,721 | 1900.70 | 1,114 | 1,182 | 1,208 | 90 |
| 5/1/2023 | 57,645 | 1900.60 | 1,027 | 1,065 | 1,107 | 90 |
| 5/2/2023 | 57,588 | 1900.60 | 1,019 | 1,048 | 1,100 | 90 |
| 5/3/2023 | 57,359 | 1900.50 | 802 | 917 | 991 | 90 |
| 5/4/2023 | 57,454 | 1900.50 | 765 | 717 | 791 | 90 |
| 5/5/2023 | 57,511 | 1900.60 | 700 | 671 | 699 | 90 |
| 5/6/2023 | 57,568 | 1900.60 | 753 | 724 | 779 | 90 |
| 5/7/2023 | 57,492 | 1900.60 | 663 | 702 | 750 | 90 |
| 5/8/2023 | 57,492 | 1900.60 | 673 | 673 | 715 | 90 |
| 5/9/2023 | 57,416 | 1900.50 | 613 | 651 | 692 | 90 |
| 5/10/2023 | 57,378 | 1900.50 | 604 | 623 | 653 | 90 |
| 5/11/2023 | 57,378 | 1900.50 | 609 | 609 | 632 | 90 |
| 5/12/2023 | 57,435 | 1900.50 | 656 | 627 | 639 | 89 |
| 5/13/2023 | 57,492 | 1900.60 | 726 | 697 | 695 | 89 |
| 5/14/2023 | 57,378 | 1900.50 | 728 | 786 | 803 | 89 |
| 5/15/2023 | 57,245 | 1900.40 | 701 | 768 | 778 | 117 |
| 5/16/2023 | 57,132 | 1900.40 | 692 | 749 | 737 | 130 |
| 5/17/2023 | 57,056 | 1900.30 | 683 | 721 | 713 | 130 |
| 5/18/2023 | 57,018 | 1900.30 | 674 | 693 | 678 | 130 |
| 5/19/2023 | 56,962 | 1900.30 | 647 | 676 | 656 | 130 |
| 5/20/2023 | 56,905 | 1900.30 | 620 | 649 | 626 | 130 |
| 5/21/2023 | 56,829 | 1900.20 | 582 | 620 | 589 | 130 |
| 5/22/2023 | 56,735 | 1900.20 | 535 | 583 | 547 | 130 |
| 5/23/2023 | 56,660 | 1900.10 | 510 | 548 | 499 | 130 |

Notes:
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Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/24/2023 | 56,566 | 1900.10 | 471 | 519 | 451 | 130 |
| 5/25/2023 | 56,434 | 1900.00 | 424 | 490 | 414 | 130 |
| 5/26/2023 | 56,359 | 1900.00 | 422 | 460 | 369 | 130 |
| 5/27/2023 | 56,265 | 1899.90 | 387 | 434 | 334 | 130 |
| 5/28/2023 | 56,209 | 1899.90 | 384 | 413 | 304 | 130 |
| 5/29/2023 | 56,116 | 1899.80 | 353 | 401 | 288 | 130 |
| 5/30/2023 | 56,041 | 1899.80 | 353 | 390 | 274 | 130 |
| 5/31/2023 | 55,947 | 1899.80 | 338 | 385 | 266 | 130 |
| 6/1/2023 | 55,929 | 1899.70 | 356 | 365 | 252 | 130 |
| 6/2/2023 | 55,798 | 1899.70 | 278 | 344 | 228 | 131 |
| 6/3/2023 | 55,686 | 1899.60 | 263 | 319 | 202 | 130 |
| 6/4/2023 | 55,575 | 1899.50 | 244 | 301 | 177 | 130 |
| 6/5/2023 | 55,500 | 1899.50 | 249 | 287 | 164 | 130 |
| 6/6/2023 | 55,426 | 1899.50 | 226 | 263 | 165 | 110 |
| 6/7/2023 | 55,370 | 1899.40 | 213 | 241 | 150 | 100 |
| 6/8/2023 | 55,315 | 1899.40 | 196 | 224 | 137 | 100 |
| 6/9/2023 | 55,352 | 1899.40 | 226 | 208 | 120 | 99 |
| 6/10/2023 | 55,333 | 1899.40 | 190 | 199 | 111 | 99 |
| 6/11/2023 | 55,315 | 1899.40 | 187 | 197 | 107 | 100 |
| 6/12/2023 | 55,426 | 1899.50 | 245 | 188 | 101 | 99 |
| 6/13/2023 | 55,408 | 1899.50 | 173 | 182 | 94 | 99 |
| 6/14/2023 | 55,408 | 1899.50 | 179 | 179 | 90 | 100 |
| 6/15/2023 | 55,389 | 1899.40 | 164 | 174 | 85 | 101 |
| 6/16/2023 | 55,315 | 1899.40 | 132 | 169 | 78 | 104 |
| 6/17/2023 | 55,241 | 1899.40 | 129 | 166 | 73 | 106 |
| 6/18/2023 | 55,130 | 1899.30 | 108 | 164 | 70 | 106 |
| 6/19/2023 | 55,037 | 1899.30 | 117 | 164 | 70 | 105 |
| 6/20/2023 | 54,945 | 1899.20 | 109 | 155 | 67 | 106 |
| 6/21/2023 | 54,908 | 1899.20 | 127 | 146 | 57 | 105 |
| 6/22/2023 | 54,853 | 1899.20 | 113 | 141 | 53 | 104 |
| 6/23/2023 | 54,779 | 1899.10 | 101 | 138 | 50 | 105 |
| 6/24/2023 | 54,724 | 1899.10 | 108 | 136 | 47 | 105 |
| 6/25/2023 | 54,668 | 1899.10 | 107 | 135 | 45 | 106 |
| 6/26/2023 | 54,613 | 1899.00 | 107 | 135 | 46 | 105 |
| 6/27/2023 | 54,558 | 1899.00 | 107 | 135 | 45 | 105 |
| 6/28/2023 | 54,485 | 1899.00 | 102 | 139 | 43 | 108 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/29/2023 | 54,411 | 1898.90 | 104 | 141 | 44 | 110 |
| 6/30/2023 | 54,301 | 1898.90 | 83 | 139 | 41 | 111 |
| 7/1/2023 | 54,192 | 1898.80 | 83 | 138 | 39 | 110 |
| 7/2/2023 | 54,064 | 1898.70 | 74 | 138 | 39 | 110 |
| 7/3/2023 | 53,918 | 1898.60 | 64 | 138 | 38 | 110 |
| 7/4/2023 | 53,771 | 1898.60 | 64 | 138 | 38 | 110 |
| 7/5/2023 | 53,625 | 1898.50 | 64 | 138 | 38 | 110 |
| 7/6/2023 | 53,461 | 1898.40 | 55 | 137 | 37 | 110 |
| 7/7/2023 | 53,297 | 1898.30 | 55 | 138 | 37 | 110 |
| 7/8/2023 | 53,115 | 1898.20 | 46 | 137 | 37 | 110 |
| 7/9/2023 | 52,952 | 1898.10 | 55 | 137 | 37 | 110 |
| 7/10/2023 | 52,789 | 1898.00 | 51 | 133 | 41 | 102 |
| 7/11/2023 | 52,645 | 1897.90 | 57 | 130 | 41 | 99 |
| 7/12/2023 | 52,536 | 1897.90 | 76 | 130 | 40 | 99 |
| 7/13/2023 | 52,357 | 1897.80 | 39 | 130 | 39 | 100 |
| 7/14/2023 | 52,213 | 1897.70 | 58 | 130 | 38 | 100 |
| 7/15/2023 | 52,051 | 1897.60 | 49 | 131 | 38 | 100 |
| 7/16/2023 | 51,871 | 1897.50 | 40 | 131 | 38 | 100 |
| 7/17/2023 | 51,691 | 1897.40 | 40 | 131 | 38 | 100 |
| 7/18/2023 | 51,494 | 1897.30 | 34 | 134 | 37 | 103 |
| 7/19/2023 | 51,369 | 1897.20 | 72 | 136 | 38 | 105 |
| 7/20/2023 | 51,172 | 1897.10 | 37 | 136 | 37 | 106 |
| 7/21/2023 | 50,976 | 1897.00 | 35 | 134 | 36 | 106 |
| 7/22/2023 | 50,763 | 1896.90 | 26 | 133 | 34 | 106 |
| 7/23/2023 | 50,568 | 1896.80 | 35 | 133 | 33 | 106 |
| 7/24/2023 | 50,339 | 1896.70 | 18 | 133 | 34 | 105 |
| 7/25/2023 | 50,109 | 1896.50 | 17 | 133 | 34 | 105 |
| 7/26/2023 | 49,914 | 1896.40 | 36 | 134 | 34 | 105 |
| 7/27/2023 | 49,703 | 1896.30 | 25 | 131 | 36 | 101 |
| 7/28/2023 | 49,474 | 1896.20 | 17 | 132 | 35 | 103 |
| 7/29/2023 | 49,247 | 1896.00 | 19 | 134 | 71 | 66 |
| 7/30/2023 | 49,020 | 1895.90 | 19 | 133 | 33 | 105 |
| 7/31/2023 | 48,811 | 1895.80 | 28 | 133 | 33 | 105 |
| 8/1/2023 | 48,568 | 1895.60 | 11 | 134 | 33 | 105 |
| 8/2/2023 | 48,342 | 1895.50 | 20 | 134 | 34 | 105 |
| 8/3/2023 | 48,100 | 1895.40 | 12 | 135 | 31 | 105 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 8/4/2023 | 47,806 | 1895.20 | -10 | 138 | 34 | 105 |
| 8/5/2023 | 47,565 | 1895.10 | 16 | 138 | 35 | 105 |
| 8/6/2023 | 47,342 | 1894.90 | 29 | 141 | 35 | 105 |
| 8/7/2023 | 47,103 | 1894.80 | 21 | 141 | 39 | 105 |
| 8/8/2023 | 46,865 | 1894.70 | 15 | 135 | 34 | 105 |
| 8/9/2023 | 46,626 | 1894.50 | 13 | 133 | 31 | 105 |
| 8/10/2023 | 46,422 | 1894.40 | 27 | 130 | 29 | 105 |
| 8/11/2023 | 46,202 | 1894.30 | 18 | 129 | 28 | 105 |
| 8/12/2023 | 45,982 | 1894.10 | 18 | 129 | 27 | 105 |
| 8/13/2023 | 45,763 | 1894.00 | 19 | 129 | 27 | 105 |
| 8/14/2023 | 45,545 | 1893.90 | 19 | 129 | 27 | 105 |
| 8/15/2023 | 45,377 | 1893.80 | 45 | 129 | 26 | 106 |
| 8/16/2023 | 45,160 | 1893.60 | 20 | 129 | 26 | 105 |
| 8/17/2023 | 44,926 | 1893.50 | 14 | 133 | 25 | 107 |
| 8/18/2023 | 44,693 | 1893.40 | 16 | 134 | 24 | 110 |
| 8/19/2023 | 44,477 | 1893.20 | 24 | 133 | 24 | 110 |
| 8/20/2023 | 44,295 | 1893.10 | 41 | 133 | 25 | 111 |
| 8/21/2023 | 44,080 | 1893.00 | 25 | 133 | 25 | 111 |
| 8/22/2023 | 43,866 | 1892.90 | 22 | 130 | 27 | 107 |
| 8/23/2023 | 43,653 | 1892.70 | 21 | 129 | 26 | 104 |
| 8/24/2023 | 43,457 | 1892.60 | 30 | 129 | 25 | 105 |
| 8/25/2023 | 43,211 | 1892.50 | 4 | 128 | 24 | 106 |
| 8/26/2023 | 42,999 | 1892.30 | 21 | 128 | 24 | 106 |
| 8/27/2023 | 42,771 | 1892.20 | 12 | 127 | 24 | 106 |
| 8/28/2023 | 42,545 | 1892.10 | 11 | 125 | 26 | 102 |
| 8/29/2023 | 42,303 | 1891.90 | -1 | 121 | 27 | 97 |
| 8/30/2023 | 42,094 | 1891.80 | 14 | 119 | 26 | 96 |
| 8/31/2023 | 41,886 | 1891.60 | 12 | 117 | 31 | 92 |
| 9/1/2023 | 41,677 | 1891.50 | 10 | 115 | 31 | 90 |
| 9/2/2023 | 41,429 | 1891.40 | -10 | 115 | 31 | 90 |
| 9/3/2023 | 41,237 | 1891.20 | 19 | 116 | 30 | 90 |
| 9/4/2023 | 41,047 | 1891.10 | 20 | 116 | 29 | 90 |
| 9/5/2023 | 40,866 | 1891.00 | 24 | 115 | 28 | 90 |
| 9/6/2023 | 40,646 | 1890.90 | 12 | 124 | 30 | 93 |
| 9/7/2023 | 40,410 | 1890.70 | 17 | 136 | 30 | 103 |
| 9/8/2023 | 40,159 | 1890.60 | 13 | 139 | 29 | 111 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/9/2023 | 39,909 | 1890.40 | 13 | 139 | 29 | 111 |
| 9/10/2023 | 39,660 | 1890.20 | 13 | 139 | 29 | 110 |
| 9/11/2023 | 39,412 | 1890.10 | 14 | 139 | 29 | 110 |
| 9/12/2023 | 39,165 | 1889.90 | 13 | 138 | 31 | 107 |
| 9/13/2023 | 38,919 | 1889.80 | 11 | 135 | 31 | 104 |
| 9/14/2023 | 38,674 | 1889.60 | 11 | 134 | 29 | 104 |
| 9/15/2023 | 38,429 | 1889.40 | 10 | 134 | 29 | 105 |
| 9/16/2023 | 38,231 | 1889.30 | 14 | 114 | 33 | 89 |
| 9/17/2023 | 38,049 | 1889.20 | 13 | 105 | 27 | 80 |
| 9/18/2023 | 37,852 | 1889.10 | 5 | 104 | 29 | 77 |
| 9/19/2023 | 37,687 | 1888.90 | 21 | 104 | 31 | 75 |
| 9/20/2023 | 37,506 | 1888.80 | 13 | 104 | 31 | 75 |
| 9/21/2023 | 37,297 | 1888.70 | -2 | 103 | 31 | 73 |
| 9/22/2023 | 37,118 | 1888.60 | 9 | 100 | 35 | 69 |
| 9/23/2023 | 36,939 | 1888.40 | 7 | 97 | 33 | 67 |
| 9/24/2023 | 36,775 | 1888.30 | 17 | 100 | 33 | 69 |
| 9/25/2023 | 36,627 | 1888.20 | 27 | 101 | 32 | 74 |
| 9/26/2023 | 36,465 | 1888.10 | 16 | 98 | 37 | 67 |
| 9/27/2023 | 36,317 | 1888.00 | 22 | 96 | 37 | 64 |
| 9/28/2023 | 36,185 | 1887.90 | 30 | 97 | 36 | 64 |
| 9/29/2023 | 36,024 | 1887.80 | 16 | 97 | 39 | 61 |
| 9/30/2023 | 35,893 | 1887.70 | 26 | 92 | 37 | 60 |
| 10/1/2023 | 35,762 | 1887.60 | 25 | 91 | 34 | 60 |
| 10/2/2023 | 35,602 | 1887.50 | 12 | 93 | 35 | 60 |
| 10/3/2023 | 35,472 | 1887.50 | 25 | 91 | 36 | 60 |
| 10/4/2023 | 35,328 | 1887.40 | 14 | 87 | 34 | 56 |
| 10/5/2023 | 35,213 | 1887.30 | 25 | 83 | 34 | 51 |
| 10/6/2023 | 35,083 | 1887.20 | 15 | 80 | 32 | 51 |
| 10/7/2023 | 34,969 | 1887.10 | 22 | 79 | 30 | 51 |
| 10/8/2023 | 34,840 | 1887.00 | 14 | 79 | 30 | 51 |
| 10/9/2023 | 34,740 | 1886.90 | 29 | 79 | 32 | 51 |
| 10/10/2023 | 34,655 | 1886.90 | 36 | 79 | 33 | 50 |
| 10/11/2023 | 34,527 | 1886.80 | 15 | 79 | 33 | 50 |
| 10/12/2023 | 34,414 | 1886.70 | 20 | 77 | 35 | 46 |
| 10/13/2023 | 34,315 | 1886.60 | 26 | 76 | 35 | 43 |
| 10/14/2023 | 34,216 | 1886.60 | 26 | 76 | 35 | 43 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 10/15/2023 | 34,118 | 1886.50 | 25 | 75 | 34 | 43 |
| 10/16/2023 | 34,019 | 1886.40 | 25 | 75 | 34 | 43 |
| 10/17/2023 | 33,935 | 1886.40 | 32 | 74 | 35 | 41 |
| 10/18/2023 | 33,739 | 1886.20 | 31 | 73 | 34 | 40 |
| 10/19/2023 | 33,656 | 1886.20 | 31 | 73 | 34 | 40 |
| 10/20/2023 | 33,544 | 1886.10 | 17 | 73 | 33 | 40 |
| 10/21/2023 | 33,433 | 1886.00 | 17 | 73 | 33 | 41 |
| 10/22/2023 | 33,475 | 1886.00 | 94 | 73 | 39 | 41 |
| 10/23/2023 | 33,447 | 1886.00 | 59 | 73 | 40 | 41 |
| 10/24/2023 | 33,364 | 1886.00 | 31 | 73 | 36 | 41 |
| 10/25/2023 | 33,281 | 1885.90 | 31 | 73 | 36 | 41 |
| 10/26/2023 | 33,198 | 1885.80 | 31 | 73 | 35 | 41 |
| 10/27/2023 | 33,102 | 1885.80 | 27 | 75 | 36 | 41 |
| 10/28/2023 | 33,005 | 1885.70 | 26 | 75 | 41 | 38 |
| 10/29/2023 | 32,909 | 1885.60 | 25 | 74 | 40 | 35 |
| 10/30/2023 | 32,813 | 1885.60 | 26 | 75 | 42 | 35 |
| 10/31/2023 | 32,717 | 1885.50 | 25 | 73 | 42 | 35 |
| 11/1/2023 | 32,608 | 1885.40 | 18 | 73 | 40 | 35 |
| 11/2/2023 | 32,526 | 1885.30 | 30 | 71 | 40 | 35 |
| 11/3/2023 | 32,471 | 1885.30 | 42 | 69 | 37 | 35 |
| 11/4/2023 | 32,390 | 1885.20 | 27 | 68 | 35 | 35 |
| 11/5/2023 | 32,322 | 1885.20 | 33 | 67 | 34 | 35 |
| 11/6/2023 | 32,308 | 1885.20 | 61 | 68 | 37 | 35 |
| 11/7/2023 | 32,322 | 1885.20 | 74 | 67 | 36 | 35 |
| 11/8/2023 | 32,241 | 1885.10 | 29 | 70 | 458 | 35 |
| 11/9/2023 | 32,173 | 1885.10 | 37 | 71 | 38 | 35 |
| 11/10/2023 | 32,105 | 1885.00 | 38 | 72 | 38 | 35 |
| 11/11/2023 | 32,038 | 1885.00 | 38 | 72 | 40 | 35 |
| 11/12/2023 | 31,957 | 1884.90 | 31 | 72 | 40 | 35 |
| 11/13/2023 | 31,890 | 1884.90 | 38 | 72 | 40 | 35 |
| 11/14/2023 | 31,863 | 1884.90 | 59 | 72 | 43 | 35 |
| 11/15/2023 | 31,850 | 1884.80 | 66 | 72 | 42 | 35 |
| 11/16/2023 | 31,796 | 1884.80 | 46 | 73 | 41 | 35 |
| 11/17/2023 | 31,782 | 1884.80 | 67 | 73 | 41 | 35 |
| 11/18/2023 | 32,308 | 1885.20 | 341 | 76 | 63 | 35 |
| 11/19/2023 | 32,758 | 1885.50 | 311 | 84 | 72 | 35 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 11/20/2023 | 32,758 | 1885.50 | 116 | 116 | 80 | 35 |
| 11/21/2023 | 32,649 | 1885.40 | 87 | 142 | 107 | 35 |
| 11/22/2023 | 32,512 | 1885.30 | 88 | 156 | 122 | 35 |
| 11/23/2023 | 32,268 | 1885.20 | 41 | 165 | 135 | 35 |
| 11/24/2023 | 32,051 | 1885.00 | 53 | 162 | 132 | 35 |
| 11/25/2023 | 31,836 | 1884.80 | 51 | 159 | 129 | 35 |
| 11/26/2023 | 31,622 | 1884.70 | 47 | 155 | 127 | 35 |
| 11/27/2023 | 31,462 | 1884.60 | 44 | 125 | 113 | 35 |
| 11/28/2023 | 31,342 | 1884.50 | 39 | 99 | 80 | 35 |
| 11/29/2023 | 31,236 | 1884.40 | 41 | 94 | 78 | 38 |
| 11/30/2023 | 31,183 | 1884.30 | 49 | 76 | 58 | 36 |
| 12/1/2023 | 31,037 | 1884.20 | 39 | 112 | 62 | 35 |
| 12/2/2023 | 30,932 | 1884.20 | 90 | 143 | 112 | 35 |
| 12/3/2023 | 30,945 | 1884.20 | 142 | 136 | 145 | 35 |
| 12/4/2023 | 30,918 | 1884.10 | 120 | 133 | 120 | 35 |
| 12/5/2023 | 30,826 | 1884.10 | 88 | 134 | 109 | 35 |
| 12/6/2023 | 30,774 | 1884.00 | 110 | 136 | 116 | 35 |
| 12/7/2023 | 30,984 | 1884.20 | 242 | 136 | 153 | 35 |
| 12/8/2023 | 31,077 | 1884.30 | 180 | 134 | 135 | 35 |
| 12/9/2023 | 31,011 | 1884.20 | 111 | 144 | 119 | 36 |
| 12/10/2023 | 30,892 | 1884.10 | 108 | 168 | 125 | 0 |
| 12/11/2023 | 30,761 | 1884.00 | 92 | 159 | 126 | 36 |
| 12/12/2023 | 30,617 | 1883.90 | 80 | 152 | 122 | 36 |
| 12/13/2023 | 30,460 | 1883.80 | 69 | 148 | 116 | 36 |
| 12/14/2023 | 30,317 | 1883.70 | 73 | 145 | 114 | 36 |
| 12/15/2023 | 30,161 | 1883.60 | 64 | 143 | 111 | 36 |
| 12/16/2023 | 30,006 | 1883.50 | 63 | 141 | 109 | 36 |
| 12/17/2023 | 29,865 | 1883.30 | 69 | 141 | 111 | 36 |
| 12/18/2023 | 32,159 | 1885.10 | 1,305 | 148 | 268 | 36 |
| 12/19/2023 | 38,231 | 1889.30 | 3,229 | 168 | 632 | 36 |
| 12/20/2023 | 43,129 | 1892.40 | 2,680 | 210 | 728 | 43 |
| 12/21/2023 | 44,960 | 1893.50 | 1,144 | 222 | 342 | 47 |
| 12/22/2023 | 45,780 | 1894.00 | 633 | 219 | 246 | 46 |
| 12/23/2023 | 46,219 | 1894.30 | 441 | 219 | 215 | 46 |
| 12/24/2023 | 46,456 | 1894.40 | 341 | 221 | 201 | 46 |
| 12/25/2023 | 46,592 | 1894.50 | 292 | 224 | 193 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 12/26/2023 | 46,643 | 1894.50 | 251 | 225 | 189 | 46 |
| 12/27/2023 | 47,240 | 1894.90 | 527 | 226 | 209 | 46 |
| 12/28/2023 | 48,100 | 1895.40 | 663 | 229 | 240 | 46 |
| 12/29/2023 | 50,515 | 1896.80 | 1,454 | 236 | 372 | 46 |
| 12/30/2023 | 53,552 | 1898.40 | 1,768 | 237 | 572 | 46 |
| 12/31/2023 | 54,963 | 1899.20 | 948 | 236 | 373 | 46 |
| 1/1/2024 | 55,817 | 1899.70 | 667 | 237 | 296 | 46 |
| 1/2/2024 | 56,886 | 1900.20 | 820 | 281 | 325 | 46 |
| 1/3/2024 | 57,530 | 1900.60 | 1,135 | 810 | 1,068 | 46 |
| 1/4/2024 | 57,321 | 1900.50 | 713 | 818 | 1,110 | 46 |
| 1/5/2024 | 57,113 | 1900.40 | 576 | 681 | 887 | 46 |
| 1/6/2024 | 57,037 | 1900.30 | 564 | 602 | 758 | 46 |
| 1/7/2024 | 56,905 | 1900.30 | 490 | 557 | 703 | 46 |
| 1/8/2024 | 56,773 | 1900.20 | 417 | 484 | 585 | 46 |
| 1/9/2024 | 56,867 | 1900.20 | 518 | 471 | 571 | 46 |
| 1/10/2024 | 57,549 | 1900.60 | 932 | 588 | 893 | 46 |
| 1/11/2024 | 57,530 | 1900.60 | 888 | 898 | 1,354 | 46 |
| 1/12/2024 | 57,227 | 1900.40 | 606 | 759 | 1,092 | 46 |
| 1/13/2024 | 60,697 | 1902.20 | 3,223 | 1,473 | 2,032 | 46 |
| 1/14/2024 | 59,489 | 1901.60 | 3,265 | 3,874 | 4,680 | 46 |
| 1/15/2024 | 58,389 | 1901.00 | 1,567 | 2,121 | 2,581 | 46 |
| 1/16/2024 | 58,504 | 1901.10 | 1,494 | 1,436 | 1,772 | 46 |
| 1/17/2024 | 58,389 | 1901.00 | 1,666 | 1,724 | 2,085 | 46 |
| 1/18/2024 | 57,949 | 1900.80 | 1,161 | 1,383 | 1,703 | 46 |
| 1/19/2024 | 57,911 | 1900.80 | 1,049 | 1,069 | 1,375 | 47 |
| 1/20/2024 | 60,697 | 1902.20 | 4,013 | 2,608 | 2,739 | 47 |
| 1/21/2024 | 61,822 | 1902.80 | 5,213 | 4,645 | 5,168 | 47 |
| 1/22/2024 | 60,756 | 1902.30 | 5,249 | 5,786 | 6,690 | 31 |
| 1/23/2024 | 59,333 | 1901.50 | 2,835 | 3,552 | 4,230 | 47 |
| 1/24/2024 | 59,430 | 1901.60 | 2,735 | 2,686 | 3,269 | 46 |
| 1/25/2024 | 58,831 | 1901.30 | 1,979 | 2,282 | 2,821 | 46 |
| 1/26/2024 | 58,370 | 1901.00 | 1,494 | 1,727 | 2,213 | 46 |
| 1/27/2024 | 58,121 | 1900.90 | 1,263 | 1,389 | 1,826 | 46 |
| 1/28/2024 | 57,892 | 1900.80 | 1,046 | 1,161 | 1,571 | 46 |
| 1/29/2024 | 57,683 | 1900.70 | 865 | 970 | 1,357 | 46 |
| 1/30/2024 | 57,511 | 1900.60 | 751 | 837 | 1,182 | 46 |

Notes:
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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 1/31/2024 | 59,721 | 1901.70 | 2,187 | 1,073 | 1,314 | 46 |
| 2/1/2024 | 59,527 | 1901.60 | 3,109 | 3,207 | 3,712 | 46 |
| 2/2/2024 | 58,927 | 1901.30 | 2,005 | 2,308 | 2,870 | 46 |
| 2/3/2024 | 58,466 | 1901.10 | 1,582 | 1,814 | 2,355 | 46 |
| 2/4/2024 | 59,994 | 1901.90 | 2,662 | 1,891 | 2,574 | 40 |
| 2/5/2024 | 60,541 | 1902.10 | 4,254 | 3,978 | 4,933 | 46 |
| 2/6/2024 | 59,508 | 1901.60 | 3,358 | 3,878 | 4,001 | 46 |
| 2/7/2024 | 58,927 | 1901.30 | 2,927 | 3,220 | 2,929 | 46 |
| 2/8/2024 | 58,427 | 1901.10 | 1,524 | 1,777 | 2,311 | 46 |
| 2/9/2024 | 58,083 | 1900.90 | 1,198 | 1,372 | 1,836 | 46 |
| 2/10/2024 | 57,873 | 1900.80 | 1,005 | 1,111 | 1,546 | 46 |
| 2/11/2024 | 57,702 | 1900.70 | 861 | 948 | 1,349 | 46 |
| 2/12/2024 | 57,607 | 1900.60 | 805 | 853 | 1,221 | 46 |
| 2/13/2024 | 57,511 | 1900.60 | 754 | 802 | 1,130 | 46 |
| 2/14/2024 | 58,140 | 1900.90 | 1,157 | 841 | 1,167 | 46 |
| 2/15/2024 | 58,677 | 1901.20 | 1,947 | 1,676 | 2,163 | 46 |
| 2/16/2024 | 58,351 | 1901.00 | 1,392 | 1,557 | 2,137 | 46 |
| 2/17/2024 | 60,189 | 1902.00 | 2,466 | 1,539 | 2,045 | 46 |
| 2/18/2024 | 61,328 | 1902.50 | 4,186 | 3,611 | 4,658 | 46 |
| 2/19/2024 | 63,341 | 1903.60 | 7,366 | 6,351 | 8,132 | 19 |
| 2/20/2024 | 62,798 | 1903.30 | 7,565 | 7,839 | 10,290 | 2 |
| 2/21/2024 | 60,973 | 1902.40 | 4,572 | 5,493 | 7,557 | 14 |
| 2/22/2024 | 59,857 | 1901.80 | 3,032 | 3,595 | 4,713 | 46 |
| 2/23/2024 | 59,178 | 1901.40 | 2,255 | 2,597 | 3,343 | 46 |
| 2/24/2024 | 58,715 | 1901.20 | 1,791 | 2,024 | 2,569 | 46 |
| 2/25/2024 | 58,427 | 1901.10 | 1,517 | 1,663 | 2,113 | 46 |
| 2/26/2024 | 58,197 | 1900.90 | 1,305 | 1,421 | 1,821 | 46 |
| 2/27/2024 | 57,987 | 1900.80 | 1,112 | 1,218 | 1,604 | 46 |
| 2/28/2024 | 57,835 | 1900.70 | 972 | 1,049 | 1,423 | 46 |
| 2/29/2024 | 58,351 | 1901.00 | 1,351 | 1,091 | 1,495 | 46 |
| 3/1/2024 | 60,013 | 1901.90 | 2,726 | 1,888 | 2,581 | 46 |
| 3/2/2024 | 59,624 | 1901.70 | 2,945 | 3,141 | 4,295 | 46 |
| 3/3/2024 | 59,159 | 1901.40 | 2,137 | 2,372 | 3,213 | 46 |
| 3/4/2024 | 58,869 | 1901.30 | 1,850 | 1,996 | 2,696 | 46 |
| 3/5/2024 | 60,697 | 1902.20 | 3,532 | 2,610 | 3,517 | 46 |
| 3/6/2024 | 61,130 | 1902.40 | 4,867 | 4,649 | 6,489 | 46 |

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WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 3/7/2024 | 60,169 | 1902.00 | 3,344 | 3,828 | 5,111 | 46 |
| 3/8/2024 | 59,547 | 1901.60 | 2,586 | 2,900 | 3,696 | 46 |
| 3/9/2024 | 59,217 | 1901.50 | 2,151 | 2,317 | 3,034 | 46 |
| 3/10/2024 | 59,237 | 1901.50 | 2,150 | 2,141 | 2,696 | 46 |
| 3/11/2024 | 59,159 | 1901.40 | 2,282 | 2,321 | 2,920 | 46 |
| 3/12/2024 | 59,198 | 1901.50 | 2,283 | 2,264 | 2,876 | 46 |
| 3/13/2024 | 58,947 | 1901.30 | 1,993 | 2,120 | 2,684 | 47 |
| 3/14/2024 | 58,696 | 1901.20 | 1,747 | 1,874 | 2,368 | 46 |
| 3/15/2024 | 58,504 | 1901.10 | 1,554 | 1,651 | 2,095 | 46 |
| 3/16/2024 | 58,331 | 1901.00 | 1,409 | 1,496 | 1,892 | 46 |
| 3/17/2024 | 58,274 | 1901.00 | 1,354 | 1,383 | 1,748 | 46 |
| 3/18/2024 | 58,236 | 1901.00 | 1,319 | 1,338 | 1,671 | 46 |
| 3/19/2024 | 58,255 | 1901.00 | 1,346 | 1,336 | 1,642 | 46 |
| 3/20/2024 | 58,197 | 1900.90 | 1,291 | 1,320 | 1,614 | 46 |
| 3/21/2024 | 58,083 | 1900.90 | 1,178 | 1,236 | 1,520 | 46 |
| 3/22/2024 | 58,523 | 1901.10 | 1,430 | 1,208 | 1,508 | 46 |
| 3/23/2024 | 59,062 | 1901.40 | 2,141 | 1,869 | 2,270 | 46 |
| 3/24/2024 | 58,754 | 1901.20 | 1,840 | 1,996 | 2,557 | 44 |
| 3/25/2024 | 58,370 | 1901.00 | 1,418 | 1,612 | 2,067 | 45 |
| 3/26/2024 | 58,140 | 1900.90 | 1,222 | 1,338 | 1,732 | 46 |
| 3/27/2024 | 58,908 | 1901.30 | 1,683 | 1,296 | 1,716 | 46 |
| 3/28/2024 | 59,159 | 1901.40 | 2,414 | 2,287 | 2,808 | 46 |
| 3/29/2024 | 58,831 | 1901.30 | 1,837 | 2,002 | 2,528 | 46 |
| 3/30/2024 | 58,638 | 1901.20 | 1,689 | 1,786 | 2,238 | 46 |
| 3/31/2024 | 58,427 | 1901.10 | 1,484 | 1,591 | 1,993 | 46 |
| 4/1/2024 | 58,197 | 1900.90 | 1,327 | 1,443 | 1,792 | 46 |
| 4/2/2024 | 58,083 | 1900.90 | 1,227 | 1,285 | 1,611 | 46 |
| 4/3/2024 | 58,083 | 1900.90 | 1,249 | 1,249 | 1,543 | 46 |
| 4/4/2024 | 58,121 | 1900.90 | 1,259 | 1,240 | 1,536 | 46 |
| 4/5/2024 | 57,949 | 1900.80 | 1,103 | 1,189 | 1,513 | 46 |
| 4/6/2024 | 57,740 | 1900.70 | 932 | 1,037 | 1,363 | 46 |
| 4/7/2024 | 57,607 | 1900.60 | 834 | 901 | 1,214 | 46 |
| 4/8/2024 | 57,492 | 1900.60 | 769 | 826 | 1,104 | 52 |
| 4/9/2024 | 57,397 | 1900.50 | 727 | 775 | 1,016 | 55 |
| 4/10/2024 | 57,416 | 1900.50 | 763 | 753 | 975 | 48 |
| 4/11/2024 | 57,473 | 1900.60 | 800 | 771 | 982 | 46 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 4/12/2024 | 57,549 | 1900.60 | 852 | 814 | 1,027 | 46 |
| 4/13/2024 | 57,778 | 1900.70 | 1,022 | 907 | 1,168 | 46 |
| 4/14/2024 | 57,740 | 1900.70 | 934 | 953 | 1,246 | 46 |
| 4/15/2024 | 57,607 | 1900.60 | 858 | 925 | 1,185 | 77 |
| 4/16/2024 | 57,530 | 1900.60 | 833 | 871 | 1,095 | 90 |
| 4/17/2024 | 57,492 | 1900.60 | 818 | 837 | 1,037 | 90 |
| 4/18/2024 | 57,492 | 1900.60 | 836 | 836 | 1,022 | 90 |
| 4/19/2024 | 57,492 | 1900.60 | 833 | 833 | 1,012 | 88 |
| 4/20/2024 | 57,454 | 1900.50 | 807 | 826 | 995 | 90 |
| 4/21/2024 | 57,416 | 1900.50 | 818 | 837 | 952 | 90 |
| 4/22/2024 | 57,378 | 1900.50 | 797 | 816 | 921 | 90 |
| 4/23/2024 | 57,359 | 1900.50 | 794 | 803 | 896 | 90 |
| 4/24/2024 | 57,302 | 1900.50 | 753 | 782 | 869 | 90 |
| 4/25/2024 | 57,208 | 1900.40 | 699 | 747 | 826 | 90 |
| 4/26/2024 | 57,151 | 1900.40 | 677 | 705 | 772 | 90 |
| 4/27/2024 | 57,037 | 1900.30 | 611 | 668 | 726 | 89 |
| 4/28/2024 | 56,943 | 1900.30 | 571 | 618 | 654 | 90 |
| 4/29/2024 | 56,848 | 1900.20 | 527 | 575 | 592 | 90 |
| 4/30/2024 | 56,792 | 1900.20 | 512 | 540 | 535 | 90 |
| 5/1/2024 | 56,716 | 1900.20 | 478 | 516 | 490 | 90 |
| 5/2/2024 | 56,754 | 1900.20 | 472 | 453 | 411 | 90 |
| 5/3/2024 | 56,924 | 1900.30 | 457 | 371 | 318 | 90 |
| 5/4/2024 | 57,588 | 1900.60 | 847 | 513 | 540 | 90 |
| 5/5/2024 | 57,435 | 1900.50 | 603 | 680 | 947 | 90 |
| 5/6/2024 | 57,302 | 1900.50 | 519 | 586 | 773 | 90 |
| 5/7/2024 | 57,245 | 1900.40 | 499 | 527 | 650 | 90 |
| 5/8/2024 | 57,151 | 1900.40 | 445 | 493 | 573 | 90 |
| 5/9/2024 | 57,094 | 1900.40 | 424 | 453 | 499 | 90 |
| 5/10/2024 | 57,094 | 1900.40 | 432 | 432 | 444 | 90 |
| 5/11/2024 | 57,094 | 1900.40 | 433 | 433 | 433 | 90 |
| 5/12/2024 | 57,018 | 1900.30 | 422 | 460 | 450 | 90 |
| 5/13/2024 | 56,867 | 1900.20 | 406 | 482 | 502 | 90 |
| 5/14/2024 | 56,773 | 1900.20 | 401 | 449 | 450 | 101 |
| 5/15/2024 | 56,735 | 1900.20 | 387 | 406 | 329 | 130 |
| 5/16/2024 | 56,697 | 1900.10 | 369 | 388 | 296 | 130 |
| 5/17/2024 | 56,622 | 1900.10 | 355 | 393 | 295 | 130 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 5/18/2024 | 56,472 | 1900.00 | 334 | 409 | 311 | 130 |
| 5/19/2024 | 56,228 | 1899.90 | 292 | 415 | 321 | 130 |
| 5/20/2024 | 56,022 | 1899.80 | 285 | 389 | 280 | 130 |
| 5/21/2024 | 55,798 | 1899.70 | 266 | 379 | 262 | 130 |
| 5/22/2024 | 55,519 | 1899.50 | 230 | 371 | 249 | 130 |
| 5/23/2024 | 55,333 | 1899.40 | 266 | 359 | 258 | 112 |
| 5/24/2024 | 55,111 | 1899.30 | 204 | 316 | 266 | 90 |
| 5/25/2024 | 54,945 | 1899.20 | 213 | 297 | 245 | 89 |
| 5/26/2024 | 54,779 | 1899.10 | 218 | 302 | 221 | 90 |
| 5/27/2024 | 54,613 | 1899.00 | 214 | 297 | 223 | 90 |
| 5/28/2024 | 54,430 | 1898.90 | 200 | 292 | 206 | 90 |
| 5/29/2024 | 54,210 | 1898.80 | 180 | 291 | 198 | 94 |
| 5/30/2024 | 54,009 | 1898.70 | 188 | 289 | 194 | 96 |
| 5/31/2024 | 53,790 | 1898.60 | 171 | 282 | 187 | 96 |
| 6/1/2024 | 53,552 | 1898.40 | 157 | 277 | 180 | 96 |
| 6/2/2024 | 53,333 | 1898.30 | 159 | 269 | 175 | 96 |
| 6/3/2024 | 53,133 | 1898.20 | 160 | 260 | 168 | 96 |
| 6/4/2024 | 52,934 | 1898.10 | 146 | 247 | 155 | 96 |
| 6/5/2024 | 52,861 | 1898.10 | 193 | 230 | 139 | 96 |
| 6/6/2024 | 52,681 | 1898.00 | 130 | 221 | 125 | 99 |
| 6/7/2024 | 52,500 | 1897.90 | 126 | 217 | 120 | 100 |
| 6/8/2024 | 52,339 | 1897.80 | 126 | 208 | 112 | 100 |
| 6/9/2024 | 52,141 | 1897.70 | 96 | 195 | 103 | 100 |
| 6/10/2024 | 51,979 | 1897.60 | 102 | 184 | 95 | 100 |
| 6/11/2024 | 51,817 | 1897.50 | 98 | 180 | 86 | 103 |
| 6/12/2024 | 51,655 | 1897.40 | 93 | 174 | 96 | 0 |
| 6/13/2024 | 51,476 | 1897.30 | 79 | 169 | 94 | 0 |
| 6/14/2024 | 51,297 | 1897.20 | 77 | 168 | 76 | 100 |
| 6/15/2024 | 51,101 | 1897.10 | 63 | 162 | 74 | 100 |
| 6/16/2024 | 50,923 | 1897.00 | 69 | 159 | 71 | 100 |
| 6/17/2024 | 50,745 | 1896.90 | 67 | 157 | 69 | 100 |
| 6/18/2024 | 50,586 | 1896.80 | 72 | 152 | 66 | 100 |
| 6/19/2024 | 50,427 | 1896.70 | 67 | 147 | 62 | 100 |
| 6/20/2024 | 50,286 | 1896.60 | 74 | 145 | 59 | 100 |
| 6/21/2024 | 50,126 | 1896.50 | 63 | 144 | 59 | 97 |
| 6/22/2024 | 49,967 | 1896.40 | 63 | 143 | 57 | 99 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 6/23/2024 | 49,808 | 1896.40 | 61 | 141 | 52 | 100 |
| 6/24/2024 | 49,632 | 1896.30 | 51 | 139 | 51 | 100 |
| 6/25/2024 | 49,509 | 1896.20 | 79 | 141 | 47 | 103 |
| 6/26/2024 | 49,334 | 1896.10 | 54 | 142 | 46 | 105 |
| 6/27/2024 | 49,142 | 1896.00 | 37 | 134 | 51 | 93 |
| 6/28/2024 | 49,037 | 1895.90 | 58 | 111 | 50 | 70 |
| 6/29/2024 | 48,915 | 1895.80 | 41 | 102 | 42 | 70 |
| 6/30/2024 | 48,794 | 1895.80 | 38 | 99 | 37 | 70 |
| 7/1/2024 | 48,690 | 1895.70 | 48 | 100 | 37 | 70 |
| 7/2/2024 | 48,603 | 1895.70 | 56 | 100 | 37 | 70 |
| 7/3/2024 | 48,481 | 1895.60 | 38 | 100 | 35 | 70 |
| 7/4/2024 | 48,360 | 1895.50 | 33 | 95 | 36 | 64 |
| 7/5/2024 | 48,256 | 1895.50 | 39 | 92 | 35 | 60 |
| 7/6/2024 | 48,152 | 1895.40 | 36 | 89 | 34 | 60 |
| 7/7/2024 | 48,030 | 1895.30 | 25 | 86 | 32 | 60 |
| 7/8/2024 | 47,927 | 1895.30 | 31 | 84 | 30 | 60 |
| 7/9/2024 | 47,806 | 1895.20 | 22 | 83 | 29 | 60 |
| 7/10/2024 | 47,686 | 1895.10 | 22 | 83 | 29 | 60 |
| 7/11/2024 | 47,582 | 1895.10 | 31 | 83 | 29 | 60 |
| 7/12/2024 | 47,454 | 1895.00 | 18 | 83 | 28 | 60 |
| 7/13/2024 | 47,342 | 1894.90 | 28 | 84 | 28 | 60 |
| 7/14/2024 | 47,206 | 1894.90 | 14 | 82 | 28 | 60 |
| 7/15/2024 | 47,086 | 1894.80 | 21 | 82 | 27 | 60 |
| 7/16/2024 | 46,950 | 1894.70 | 13 | 81 | 26 | 16 |
| 7/17/2024 | 46,814 | 1894.60 | 12 | 81 | 26 | 58 |
| 7/18/2024 | 46,677 | 1894.50 | 12 | 81 | 26 | 58 |
| 7/19/2024 | 46,541 | 1894.50 | 9 | 78 | 31 | 56 |
| 7/20/2024 | 46,405 | 1894.40 | 11 | 79 | 33 | 56 |
| 7/21/2024 | 46,269 | 1894.30 | 13 | 81 | 28 | 56 |
| 7/22/2024 | 46,151 | 1894.20 | 20 | 79 | 26 | 56 |
| 7/23/2024 | 46,016 | 1894.20 | 11 | 79 | 25 | 56 |
| 7/24/2024 | 45,864 | 1894.10 | 8 | 85 | 26 | 57 |
| 7/25/2024 | 45,713 | 1894.00 | 11 | 87 | 27 | 60 |
| 7/26/2024 | 45,561 | 1893.90 | 10 | 86 | 27 | 60 |
| 7/27/2024 | 45,411 | 1893.80 | 8 | 84 | 26 | 60 |
| 7/28/2024 | 45,244 | 1893.70 | 0 | 84 | 25 | 60 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 7/29/2024 | 45,110 | 1893.60 | 15 | 82 | 25 | 60 |
| 7/30/2024 | 44,960 | 1893.50 | 6 | 82 | 23 | 60 |
| 7/31/2024 | 44,826 | 1893.40 | 15 | 82 | 23 | 60 |
| 8/1/2024 | 44,676 | 1893.40 | 8 | 84 | 23 | 60 |
| 8/2/2024 | 44,527 | 1893.30 | 13 | 88 | 24 | 60 |
| 8/3/2024 | 44,361 | 1893.20 | 6 | 90 | 25 | 60 |
| 8/4/2024 | 44,245 | 1893.10 | 38 | 96 | 26 | 60 |
| 8/5/2024 | 44,047 | 1893.00 | 4 | 104 | 29 | 60 |
| 8/6/2024 | 43,833 | 1892.80 | -4 | 104 | 33 | 60 |
| 8/7/2024 | 43,669 | 1892.70 | 12 | 94 | 27 | 60 |
| 8/8/2024 | 43,506 | 1892.70 | 5 | 87 | 23 | 60 |
| 8/9/2024 | 43,342 | 1892.50 | 3 | 86 | 22 | 60 |
| 8/10/2024 | 43,195 | 1892.50 | 10 | 84 | 21 | 60 |
| 8/11/2024 | 43,032 | 1892.40 | 1 | 84 | 21 | 60 |
| 8/12/2024 | 42,869 | 1892.30 | 1 | 83 | 20 | 60 |
| 8/13/2024 | 42,723 | 1892.20 | 8 | 82 | 20 | 59 |
| 8/14/2024 | 42,577 | 1892.10 | 9 | 82 | 20 | 59 |
| 8/15/2024 | 42,415 | 1892.00 | 1 | 82 | 20 | 59 |
| 8/16/2024 | 42,287 | 1891.90 | 18 | 83 | 20 | 60 |
| 8/17/2024 | 42,142 | 1891.80 | 10 | 83 | 19 | 61 |
| 8/18/2024 | 41,998 | 1891.70 | 10 | 83 | 19 | 61 |
| 8/19/2024 | 41,853 | 1891.60 | 10 | 83 | 19 | 61 |
| 8/20/2024 | 41,709 | 1891.50 | 9 | 82 | 20 | 56 |
| 8/21/2024 | 41,581 | 1891.50 | 16 | 80 | 21 | 53 |
| 8/22/2024 | 41,421 | 1891.40 | -1 | 79 | 21 | 53 |
| 8/23/2024 | 41,310 | 1891.30 | 20 | 76 | 21 | 50 |
| 8/24/2024 | 41,215 | 1891.20 | 26 | 74 | 22 | 47 |
| 8/25/2024 | 41,135 | 1891.20 | 32 | 72 | 20 | 47 |
| 8/26/2024 | 41,040 | 1891.10 | 23 | 71 | 19 | 47 |
| 8/27/2024 | 40,929 | 1891.00 | 17 | 73 | 19 | 47 |
| 8/28/2024 | 40,819 | 1891.00 | 19 | 75 | 21 | 47 |
| 8/29/2024 | 40,677 | 1890.90 | 6 | 77 | 20 | 47 |
| 8/30/2024 | 40,536 | 1890.80 | 8 | 79 | 22 | 49 |
| 8/31/2024 | 40,394 | 1890.70 | 11 | 82 | 23 | 53 |
| 9/1/2024 | 40,253 | 1890.60 | 11 | 82 | 23 | 53 |
| 9/2/2024 | 40,097 | 1890.50 | 3 | 82 | 24 | 52 |

Notes:
Pillsbury elevation in PG&E datum using 2015-2016 bathymetry.
WY2006-2023 records reviewed and finalized. WY2024 record preliminary (not yet reviewed and finalized).
Pillsbury inflows calculated using mass balance approach. When evaporation exceeds inflow, calculated inflow is negative.

| Date | Pillsbury storage (AF) | Pillsbury elevation (ft) | Pillsbury inflow (cfs) | E-2: Pillsbury outflow (cfs) | E-11: Eel River below Cape Horn Dam (cfs) | E-16: Diversion to Russian River basin (cfs) |
|---|---|---|---|---|---|---|
| 9/3/2024 | 39,956 | 1890.40 | 11 | 82 | 25 | 51 |
| 9/4/2024 | 39,800 | 1890.30 | 3 | 82 | 22 | 52 |
| 9/5/2024 | 39,567 | 1890.20 | -36 | 82 | 23 | 52 |
| 9/6/2024 | 39,443 | 1890.10 | 16 | 79 | 23 | 51 |
| 9/7/2024 | 39,303 | 1890.00 | 5 | 75 | 24 | 44 |
| 9/8/2024 | 39,165 | 1889.90 | 5 | 75 | 24 | 42 |
| 9/9/2024 | 39,026 | 1889.80 | 8 | 78 | 25 | 42 |
| 9/10/2024 | 38,888 | 1889.70 | 9 | 78 | 26 | 42 |
| 9/11/2024 | 38,735 | 1889.60 | 1 | 79 | 26 | 42 |
| 9/12/2024 | 38,612 | 1889.60 | 17 | 79 | 26 | 42 |
| 9/13/2024 | 38,490 | 1889.50 | 17 | 79 | 26 | 42 |
| 9/14/2024 | 38,338 | 1889.40 | 2 | 79 | 27 | 42 |
| 9/15/2024 | 38,185 | 1889.30 | 2 | 79 | 27 | 41 |
| 9/16/2024 | 38,079 | 1889.20 | 32 | 85 | 28 | 44 |
| 9/17/2024 | 37,928 | 1889.10 | 16 | 92 | 30 | 51 |
| 9/18/2024 | 37,792 | 1889.00 | 24 | 92 | 30 | 51 |
| 9/19/2024 | 37,641 | 1888.90 | 15 | 90 | 30 | 52 |
| 9/20/2024 | 37,506 | 1888.80 | 14 | 82 | 28 | 49 |
| 9/21/2024 | 37,387 | 1888.70 | 17 | 77 | 27 | 47 |
| 9/22/2024 | 37,267 | 1888.70 | 15 | 76 | 28 | 43 |
| 9/23/2024 | 37,147 | 1888.60 | 13 | 74 | 27 | 42 |
| 9/24/2024 | 37,043 | 1888.50 | 17 | 70 | 25 | 42 |
| 9/25/2024 | 36,909 | 1888.40 | 9 | 77 | 26 | 45 |
| 9/26/2024 | 36,760 | 1888.30 | 8 | 83 | 28 | 46 |
| 9/27/2024 | 36,642 | 1888.20 | 22 | 82 | 36 | 39 |
| 9/28/2024 | 36,524 | 1888.20 | 14 | 74 | 32 | 36 |
| 9/29/2024 | 36,406 | 1888.10 | 11 | 71 | 28 | 36 |
| 9/30/2024 | 36,273 | 1888.00 | 21 | 87 | 33 | 38 |

# Appendix B – Wildfire Suppression

## Analysis of Effects of Proposed Scott Dam Decommissioning

### County of Lake

Prepared by:

**SLR International Corporation**

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SLR Project No.: 102.021249.00001

August 22, 2025

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# Acronyms and Abbreviations

| | |
|---|---|
| ac-ft | acre-feet |
| AEP | annual exceedance probability |
| CADSOD | California Division of Dam Safety |
| CDEC | California Data Exchange Center |
| COY | Coyote Lake Mendocino Weather Station |
| cfs | Cubic feet per second |
| DWR | Department of Water Resources |
| ELP | Eel River below Lake Pillsbury Weather Station |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| FFA | flood frequency analysis |
| ft | feet |
| HEC-RAS | Hydrologic Engineering Center's River Analysis System |
| LAT | Large Airtanker |
| LPY | Lake Pillsbury Weather Station |
| MSL | mean seal level |
| NAVD88 | North American Vertical Datum of 1988 |
| NCEI | National Centers for Environmental Information |
| NIDIS | National Integrated Drought Information System |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NOAA | National Oceanic and Atmospheric Administration |
| NMFS | National Marine Fisheries Service |
| NERF | New Eel Russian Facility |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| SCD | Scott Dam Weather Station |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| SPI | Standardized Precipitation Index |
| USACE | United States Army Corps of Engineers |
| USDM | U.S. Drought Monitor |
| USGS | United States Geologic Survey |
| VLAT | Very Large Airtanker |

| | |
|---|---|
| WSE | water surface elevation |
| WY | water year |

# Wildfire Suppression

This Appendix presents SLR Team's evaluation of the potential negative impacts of Scott Dam decommissioning on fire risk, wildfire suppression, and vegetation management and then examines as mitigation options, an alternate reservoir or water storage requirements for areal wildfire suppression in Lake County. The details of our evaluations are presented in the following sections of this Appendix with conclusions provided at the end.

# B1.0 Introduction

The proposed removal of Scott Dam and the subsequent draining of Lake Pillsbury in Lake County, California, presents significant challenges for wildfire suppression and vegetation management in the region. This section presents the preliminary analysis of the potential negative impacts of Scott Dam decommissioning on fire risk, suppression efforts, and vegetation management in the area.

## B1.1 Background on Scott Dam and Lake Pillsbury

Scott Dam, constructed in 1922, created Lake Pillsbury as part of the Potter Valley Project. The dam is located on the Eel River in Lake County, within the Mendocino National Forest. Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts (Friends of the Eel River, 2023).

The proposed decommissioning of Scott Dam is part of a broader initiative to restore the Eel River's ecosystem and improve fish passage. However, this project raises concerns about its impact on wildfire management in an area already prone to significant fire events (California Trout, 2023).

## B1.2 Historical Fire Activity in the Region

The Mendocino National Forest and surrounding areas have a long history of significant wildfires. Some notable fires in recent years include:

a) Mendocino Complex Fire (2018):
   - Burned 459,123 acres
   - Largest wildfire in California history at the time
   - Destroyed 280 structures and led to significant evacuations

b) August Complex Fire (2020):
   - Burned 1,032,648 acres
   - Largest wildfire in California history
   - Destroyed 935 structures and caused significant environmental damage

c) Ranch Fire (2018):
   - Part of the Mendocino Complex Fire
   - Burned 410,203 acres

d) Redwood Valley Complex Fire (2017):
   - Burned 36,523 acres
   - Destroyed over five hundred structures and resulted in several fatalities

These fires demonstrate the region's susceptibility to large-scale wildfire events and underscore the importance of effective fire suppression resources (Cal Fire, 2021).

## B1.3 Current Fire Suppression Resources and Strategies

### B1.3.1 Water Sources for Fire Suppression

Lake Pillsbury currently serves as a critical water source for fire suppression efforts in the region. Its importance is highlighted by the following factors:

a) Proximity to fire-prone areas: Lake Pillsbury is centrally located within the Mendocino National Forest, providing quick access to water for firefighting efforts.

b) Size and capacity: With a surface area of approximately 2,000 acres and a capacity of 80,000 acre-feet, Lake Pillsbury offers a substantial and reliable water source for firefighting (PG&E, 2022).

c) Accessibility for various firefighting methods: The lake is suitable for multiple water collection methods, including:

- Helicopter bucket operations
- Fixed-wing aircraft water scooping (e.g., Canadair CL-415 “Super Scooper”)
- Ground-based drafting for fire engines and water tenders

### B1.3.2 Aerial Firefighting Resources

The region relies heavily on aerial firefighting resources that can access Lake Pillsbury for water replenishment during firefighting operations. Firefighters depend on accessible water sources like Lake Pillsbury. Key aerial assets include:

a) Helicopters:

- Cal-Fire Howard Forest Helitack Base (19.7 miles from Lake Pillsbury)
- Boggs Mountain Helitack Base

b) Fixed-wing aircraft: Cal-Fire Ukiah Air Attack Base (23 miles from Lake Pillsbury)

### B1.3.3 Ground-based Firefighting Resources

Ground-based firefighting efforts also benefit from Lake Pillsbury's presence:

a) Fire engines and water tenders can draft water directly from the lake.

b) The lake serves as a reliable water source for establishing portable water tanks and fill sites near fire locations.

In the case of the fixed wing aircraft, these water carriers rely on a system called “Super Scooper,” whereby an aircraft literally repeatedly scoops water from a lake and drops it on fires.

### B1.3.4 Super Scooper Key Features

**Water Scooping**: The Super Scooper can scoop up to 1,412 gallons of water in approximately 10 seconds from nearby water sources.

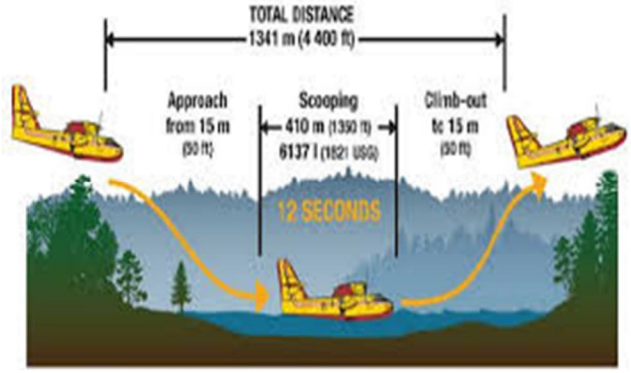
**Firefighting Efficiency**: It can remain on station for up to three hours, typically dropping nine tank loads of water on a fire.

### B1.3.5 Super Scooper Operational Benefits

**Rapid Response:** With 90% of wildfires occurring within 20 miles of a major water source, the Super Scooper can quickly refill and return to the fire, making it highly effective.

**Cost-Effective:** It provides an economical solution for delivering large quantities of water to fires without needing to return to an airport to refill.

For a Canadair CL-415, the most typical “Super Scooper” aircraft in use in California, a lake needs to be at least about 4,400 feet long to safely scoop water.



Remote areas such as the Lake Pillsbury already brings water availability problems when dealing with wildfires in the area. No longer having the lake would increase response times for firefighting efforts to relay water to affected sites.

**Table B-1: Surrounding Water Sources for Super Scooper and Helicopter use**

| | |
|---|---|
| **East Park Reservoir**<br>39.34938019929924, -122.51182382180117<br>▪ Distance from Lake Pillsbury closest edges – 23.6 miles East by Southeast<br>▪ Length of waterways - 3.0 miles long | **Black Butte Lake**<br>39.785852302476016, -122.36262039993922<br>▪ Distance from Lake Pillsbury - 36 miles<br>▪ Length of waterways – 4.4 miles |
| **Stony Gorge Reservoir**<br>39.557787859251185, -122.52688311655206<br>▪ Distance from Lake Pillsbury closest edges twenty-three miles<br>▪ Length of waterways – 4 miles long | **Upper Letts Lake**<br>39.3060884576836, -122.71276408932592<br>▪ Distance from Lake Pillsbury 14.4 miles Southeast<br>▪ Length of waterways – 2400 feet ( too small to scoop) |

**Clear Lake**

39.034929, -122.784312

- Distance from Lake Pillsbury closest edges – 21 miles south
- Length of waterways – 8 miles

**Indian Valley Reservoir**

39.12100732092549, -122.53998770436975

- Distance from Lake Pillsbury closest edges – 25.8 Miles
- Length of waterways – 4.5 miles

**Ruth Lake**

40.342161063068524, -123.41182091332125

- Distance from Lake Pillsbury closest edges 67.5 Miles North of Lake Pillsbury
- Length of waterways 5,279 Feet long

**Lake Mendocino**

39.21632420100953, -123.17323576134581

- Distance from Lake Pillsbury closest edges - 16.5 miles
- Length of waterways - 2.78 Miles

Helicopters can get water from a variety of spots including swimming pools, ponds, and lakes. The limitations are few- Mainly trees, wires, and cross contamination of lakes.

One notable instance of water scooping from Lake Pillsbury occurred during the **August Complex Fire** in 2020. This fire, which became the largest wildfire in California's history, required extensive firefighting efforts, including the use of water from Lake Pillsbury. For over three weeks, 4,000-gallon water tenders were drafting water from Lake Pillsbury 24/7 to support firefighting efforts. Aircraft, including helicopters, frequently scooped water from the lake to make repeated drops on the fire, helping to control its spread and protect nearby communities.

## B1.4 Potential Negative Impacts of Scott Dam Removal on Fire Suppression

The removal of Scott Dam and the subsequent loss of Lake Pillsbury would have several negative impacts on fire suppression efforts in the region as noted below:

**Reduced Water Availability**

a) Loss of a central water source: The removal of Lake Pillsbury would eliminate a significant, centrally located water source for firefighting efforts in the Mendocino National Forest and surrounding areas.

b) Increased water shuttle distances: Firefighting resources would need to travel farther to access alternative water sources, potentially increasing turnaround times and reducing operational efficiency.

c) Limited options for aerial firefighting: The loss of Lake Pillsbury would restrict options for aerial firefighting operations, particularly for large air tankers and "Super Scooper" aircraft that require substantial bodies of water for effective operations.

**Increased Response Times**

a) Extended travel for water resupply: Ground-based and aerial firefighting resources would need to spend more time traveling to and from more distant water sources, potentially allowing fires to grow larger before being contained.

b) Reduced operational efficiency: The increased time spent on water shuttling would decrease the overall efficiency of firefighting efforts, potentially requiring more resources to achieve the same level of fire suppression.

**Limitations on Firefighting Strategies**

a) Reduced flexibility in firefighting tactics: The loss of a large, centrally located water source may limit the range of firefighting strategies available to incident commanders, potentially impacting their ability to respond effectively to dynamic fire conditions.

b) Constraints on large-scale aerial operations: Without Lake Pillsbury, the ability to conduct sustained, large-scale aerial firefighting operations in the immediate area would be significantly hampered.

## B1.5 Vegetation Management Challenges

The removal of Scott Dam and Lake Pillsbury would also present several challenges for vegetation management in the area:

**Initial Vegetation Loss and Soil Exposure**

a) Immediate impact: The draining of Lake Pillsbury would expose approximately 2,000 acres of previously submerged land, leading to initial vegetation loss and soil exposure (PG&E, 2022).

b) Erosion risk: The newly exposed soil would be susceptible to erosion, potentially impacting water quality in the Eel River and its tributaries (American Rivers, 2023).

**Invasive Species Colonization**

a) Opportunistic invasion: The newly exposed reservoir bed could be quickly colonized by invasive plant species before native vegetation has a chance to establish (Tullos et al., 2016).

b) Altered fire regimes: Invasive species may have different burning characteristics compared to native vegetation, potentially altering fire behavior and intensity in the affected areas (Brooks et al., 2004).

**Changes in Fuel Moisture and Fire Risk**

a) Drying of reservoir bed: The exposed reservoir bed would initially dry out, potentially leading to lower fuel moisture content and increased fire risk in those areas until vegetation re-establishes (Ahlstrom et al., 2020).

b) Altered hydrology: The removal of the dam could change local hydrology, potentially affecting groundwater levels and vegetation moisture content in the surrounding area (Foley et al., 2017).

**Long-term Vegetation Shifts**

a) Ecosystem transformation: The area currently occupied by Lake Pillsbury would undergo a significant ecological transformation, transitioning from an aquatic to a terrestrial ecosystem over time (Bellmore et al., 2019).

b) Changes in species composition: The vegetation community in the affected area would likely shift, potentially altering the overall fire risk and behavior in the region (Pettit et al., 2020).

## B1.6 Implications for Fire Risk and Behavior

The removal of Scott Dam and Lake Pillsbury could have several implications for fire risk and behavior in the region:

**Potential Increase in Fire Risk**

a) Reduced moisture availability: The loss of a large body of water could lead to lower relative humidity in the immediate area, potentially creating more favorable conditions for fire ignition and spread (Boisramé et al., 2017).

b) Changes in wind patterns: The absence of Lake Pillsbury could alter local wind patterns, potentially affecting fire behavior and spread in the region (Syphard et al., 2019).

**Altered Fire Behavior**

a) Changes in fuel continuity: As vegetation reestablishes in the former reservoir area, it may create new patterns of fuel continuity, potentially affecting fire spread patterns (Coop et al., 2020).

b) Variation in fuel types: The introduction of new plant species and changes in vegetation structure could lead to different fire intensities and rates of spread compared to the current landscape (Pausas & Keeley, 2021).

## B1.7 Mitigation Strategies and Recommendations

The importance of maintaining water sources for firefighting is well-documented. According to the California Department of Forestry and Fire Protection (CAL FIRE), water availability is crucial for effective fire suppression (CAL FIRE, 2019). To address potential negative impacts of Scott Dam removal on wildfire suppression and vegetation management, several mitigation strategies and recommendations should therefore be considered and adopted. In the December 21, 2023, letter from the Lake Pillsbury Alliance (LPA) entitled “Comments on PG&E's Initial Draft Surrender Application and Conceptual Decommissioning Plan [Letter to Tony Gigliotti, Pacific Gas & Electric Company]” they noted significant water suppression resources were needed to provide community resilience upon removal of Lake Pillsbury. These mitigation strategies and recommendations are provided herein, along with others.

**Enhanced Water Infrastructure**

a) Develop alternative water sources: The LPA suggests creating alternative water supply sources such as ponds, pools, or smaller lakes in case the lake basin supply becomes insufficient for aerial or ground firefighting efforts. This recommendation aligns with best practices in fire management, which emphasize the importance of having multiple water sources available (National Wildfire Coordinating Group, 2021). This will partially compensate for the loss of Lake Pillsbury as a firefighting water source.

b) Improve access to remaining water sources: Enhance Road infrastructure and develop additional drafting points at nearby lakes and rivers to facilitate easier access for firefighting resources.

### B1.7.1 Augmented Firefighting Resources

a) Increase aerial firefighting capacity: Consider establishing additional helitack bases or expanding existing ones to compensate for the loss of Lake Pillsbury as a central water source.

b) Enhance ground-based resources: Increase the number of water tenders and portable water tanks available in the region to support extended water-shuttling operations. The LPA also recommend this mitigation strategy by providing equipment and water to the Lake Pillsbury Fire Protection District (LPFPD) with input from various stakeholders, including Cal Fire, the United States Forest Service, LPFPD, County of Lake, and the Lake Pillsbury communities. This collaborative approach aligns with best practices in community wildfire protection planning (Community Wildfire Planning Center, 2020).

c) Roadway access enhancements. The LPA suggests constructing or modifying roads to provide adequate emergency transport capacity throughout the lake basin, including roads connecting permanent or alternative water sources. Improved access is crucial for effective firefighting, as noted in the National Wildfire Coordinating Group's Wildland Fire Incident Management Field Guide (2021).

### B1.7.2 Proactive Vegetation Management

a) Develop a comprehensive revegetation plan: Create and implement a plan for the exposed reservoir bed that prioritizes native species and fire-resistant vegetation (Shafroth et al., 2016).

b) Implement invasive species control: Establish an aggressive invasive species monitoring and control program to prevent the establishment of fire-prone non-native vegetation (Holl & Aide, 2011).

### B1.7.3 Enhanced Fire Prevention and Detection

a) Expand fire detection systems: Implement additional fire detection technologies, such as cameras and sensors, to compensate for potentially increased fire risk in the area. The LPA recommends implementing brushfire detection and warning systems in the region, including the Mendocino National Forest. Early detection systems can significantly improve response times and effectiveness of firefighting efforts. Research has shown that early detection can reduce the spread and impact of wildfires (Alkhatib, 2014).

b) Increase public education: Enhance community outreach and education programs to promote fire safety and prevention in the affected region.

### B1.7.4 Adaptive Management Approach

a) Monitoring and assessment: Implement a robust monitoring program to track changes in vegetation, fire risk, and ecosystem health following dam removal (Foley et al., 2017).

b) Flexible management strategies: Develop adaptive management plans that can be adjusted based on observed changes in fire risk and ecosystem dynamics over time.

## B1.8 Findings

The proposed removal of Scott Dam and the subsequent loss of Lake Pillsbury present significant challenges for wildfire suppression and vegetation management in Lake County and the surrounding regions. The potential negative impacts include reduced water availability for firefighting, increased turnaround times, limitations on firefighting strategies, and complex vegetation management issues.

These challenges are particularly concerning given the area's history of large-scale wildfires and the increasing fire risk due to climate change. While the dam removal project aims to improve river ecosystem health and fish passage, it is crucial to carefully consider and mitigate the potential increased fire risks and suppression challenges.

Implementing a comprehensive set of mitigation strategies, including enhanced water infrastructure, augmented firefighting resources, proactive vegetation management, and adaptive management approaches, will be essential to maintaining effective fire suppression capabilities and managing vegetation-related fire risks in the post-dam removal landscape.

Further research and detailed planning will be necessary to fully understand and address the long-term implications of this project on wildfire management in the region. Collaboration between fire management agencies, ecological researchers, and local communities will be crucial in developing effective strategies to navigate these challenges and ensure the continued protection of the area's natural resources and human communities from wildfire threats.

# B2.0 Reservoir Requirements for Aerial Wildfire Suppression

## B2.1 Introduction

The decommissioning of Scott Dam and the potential elimination of Lake Pillsbury in Lake County, California, would result in the need to establish new water sources for aerial firefighting operations, correlating with rotary winged aircraft.

Criteria for reservoir size and area are determined by the capabilities of the helicopters that are being utilized for aerial firefighting operations.

Wildfire air resources are categorized and typed to ensure effective and efficient response during wildfires. This typing system helps determine the type of aircraft, crew, and equipment needed for specific fire situations.

## B2.2 Helicopter Water Capacities

The largest and most capable helicopters used in aerial firefighting are classified as Type 1 or heavy helicopters. These heavy-lift helicopters can carry more than 3,000 gallons of water or fire retardant. These are primarily the Boeing CH-47 Chinook (also known as CH-47D or Chinook. This enormous capacity makes the Chinook ideal for tackling large, complex wildfires. With a maximum external load capacity of up to 25,000 lbs., a top speed of around 132 mph, and a range of approximately 244 miles. The Sikorsky S-70 Firehawk is also a type-1 helicopter capable of holding 1,000 gallons of water in the belly of the aircraft making it a very reliable resource when fighting rural wildfires (Lockheed Martin, 2025).

Type 2 helicopters are very effective initial attack resources. These helicopters can support firefighters on the ground by delivering up to 300 gallons of water to the fire line in a bucket or tank.

The need for aerial water drops is based on the duration, size, and values threatened during the lifecycle of those events. This pattern is most pronounced for Type 1 helicopters (74%), multiengine scoopers (74%), Large Airtankers (LATs; 61%), and Very Large Airtankers (VLATs; 65%). LATs and VLATs are used to drop fire retardant or water on wildfires. LATs generally carry 2,000 to 4,000 gallons of retardant or water, while VLATs can carry over 8,000 gallons (U.S. Department of Agriculture 2020).

Helicopters will take water from many places including small ponds and portable water tanks with easy access. The fixed winged super scoopers primarily use large bodies of water for aerial firefighting operations. Specifications for a reservoir to be constructed for wildfire suppression, should be designed with aircraft capabilities and limitations, and the anticipated suppression needs for the region.

The Canadair CL-415 (Super Scooper) scoops at 80 knots and is on the water for approximately 10 to 12 seconds. The pickup water source should be a minimum of 1 mile long, depending on environmental conditions. The scooping path does not have to be straight, as the aircraft is maneuverable while scooping. (U.S. Department of Agriculture, 2016) Super Scoopers have limitations when it pertains to aerial firefighting. The planes need a long enough body of water to safely maneuver and scoop up over 1,000 gallons of water to drop on strategic parts of a wildfire. Super scoopers have been used most recently on the Palisades fire in Los Angeles (2025) but have also been used throughout the country, including the 2018 and 2020 fires within the Lake County.

## B2.3 Reservoir Volume

Usage and capacity planning involves analyzing factors such as water volume needs, turnover rates, anticipated scales of operation, environmental regulations, and seasonal variables. When designing ponds or reservoirs for firefighting, capacity versus utilization rates and future demand growth must be balanced. Larger dip ponds must be able to supply 100,000 gallons or more to support extensive aerial operations, while smaller ponds should provide 10,000-30,000 gallons for limited use.

Most guidelines recommend sizing capacity to hold the maximum water volume required for at least 4 hours of aerial firefighting usage, providing a safety buffer in case refilling is delayed. (BTL Liners, 2025) A reservoir or a dip pond would have to be of significant size to support a large-scale aerial helicopter operation during a wildfire. A reservoir or dip pond would also have to utilize its own water source to be able to be used throughout the year by localized natural watershed.

### B2.3.1 Reservoir Dimensions

The reservoir should have sufficient surface area to allow safe helicopter dipping maneuvers. As a rule, safety circles should be at a minimum 1.5 times the rotor diameter. In addition, separation between safety circles should be at a minimum one rotor blade(s) diameter. (National Wildfire Coordinating Group, 2019). Dimensions of the reservoir matter due to operational safety of the types and size of the helicopter utilizing the body of water. Clearance of rotary wing would play a significant role in determining dimensions for a reservoir.

The size of a pond or reservoir required to supply water for aerial wildfire operations depends on its **capacity** in gallons, which is determined by its dimensions (surface area and average depth). Below is a breakdown of the minimum size required for two scenarios: supporting **extensive aerial wildfire operations** (100,000+ gallons) and **limited use** (10,000–30,000 gallons).

***General Formula for Water Volume***

The volume of a pond or reservoir can be estimated using the formula:

**Volume (gallons) = Surface Area (square feet) × Average Depth (feet) × 7.48**

Where:

- 7.48 is the number of gallons in one cubic foot.
- Surface area is typically measured in square feet, and depth is in feet.

***Minimum Size for 100,000+ Gallons (Extensive Use)***

To support extensive aerial wildfire operations requiring **100,000 gallons or more**, the minimum size can be calculated as follows:

a) **Assumptions**:

- A small pond typically has an average depth of **6 feet** for practicality and ease of construction.
- Surface area is calculated based on the required capacity.

b) **Calculation**:
- Rearrange the formula:

**Surface Area = Volume / (Depth × 7.48)**

**Surface Area = 100,000 / (6 × 7.48) ≈ 2,230 square feet**

c) **Dimensions**:
- A pond with a surface area of **2,230 square feet** would be approximately:

**47 feet × 47 feet** (roughly square), or

**30 feet × 75 feet** (rectangular).

d) **Depth Considerations**:
- If the depth is increased to **10 feet**, the required surface area decreases:

**Surface Area = 100,000 / (10 × 7.48) ≈ 1,337 square feet**

  - **Dimensions: 37 feet × 37 feet** (square), or **25 feet × 55 feet** (rectangular).

***Minimum Size for 10,000–30,000 Gallons (Limited Use)***

For limited aerial wildfire operations requiring 10,000–30,000 gallons, the pond or reservoir size can be significantly smaller:

a) **Assumptions**:
- Average depth remains **6 feet**.

b) **Calculations**:
- For **10,000 gallons**:

**Surface Area = 10,000 / (6 × 7.48) ≈ 223 square feet**

  - Dimensions: 15 feet × 15 feet (square), or 10 feet × 23 feet (rectangular).
- For **30,000 gallons**:

**Surface Area = 30,000 / (6 × 7.48) ≈ 669 square feet**

  - Dimensions: **26 feet × 26 feet** (square), or **15 feet × 45 feet** (rectangular).

c) **Depth Considerations**:
- Increasing depth to **10 feet** reduces surface area further:
  - For **10,000 gallons**: **Surface Area ≈ 134 square feet** (12 feet × 12 feet).
  - For **30,000 gallons**: **Surface Area ≈ 401 square feet** (20 feet × 20 feet).

## B2.4 Practical Considerations

1. **Shape of the Pond**:
   - Ponds do not need to be perfect rectangles or squares. Irregular shapes are common, but the total surface area and depth should meet the volume requirement.

2. **Access for Aerial Operations**:
   - The pond must be large enough for helicopters or planes to safely scoop water. A minimum width of **50 feet** is often recommended for efficient helicopter bucket operations, even for smaller ponds.

3. **Water Replenishment**:
   - The pond or reservoir should have a reliable water source (e.g., rainfall, streams, or wells) to ensure it can be refilled during extended wildfire operations.

4. **Depth and Safety**:
   - Shallow ponds with a depth of less than **6 feet** may evaporate quickly, especially during hot and dry wildfire conditions, reducing their long-term usability.

**Table B-2: General Aeiral Firefighting Water Source Requirements based upon Operational Need**

| Operational Need | Required Volume (Gallons) | Average Depth (feet) | Minimum Surface Area (square feet) | Approximate Dimensions (feet x feet) |
|---|---|---|---|---|
| Extensive Use | 100,000+ | 6 | 2,230 | 47 × 47 or 30 × 75 |
| Limited Use (Low Volume) | 10,000 | 6 | 223 | 15 × 15 or 10 × 23 |
| Limited Use (High Volume) | 30,000 | 6 | 669 | 26 × 26 or 15 × 45 |

## B2.5 Constructing a Dedicated Reservoir vs. Utilizing Distant Water Sources

### B2.5.1 Constructing a Dedicated Reservoir

*Pros*:

1. Reduced response/turnaround time for aerial firefighting aircraft.
2. Higher amount of water air drops due to closer proximity of water source.
3. Beneficial placement of constructed reservoir.
4. Lower cost for fuel due to proximity of water to the fire.

*Cons*:

1. High cost for new infrastructure and the maintenance that comes with it.
2. Environmental permits/hurdles needed to be obtained to build a reservoir of this size.
3. Potential for ecosystems to be disrupted from introducing another water source.

### B2.5.2 Utilizing Distant Water Sources

*Pros*:

1. Low cost, no new infrastructure being introduced.
2. Low impact of the environment.

*Cons*:

1. Longer response times/turnaround time to travel to a nearby water source.
2. Drought conditions for the area may impact water availability near the incident.
3. Increase fuel consumption for aircraft due to longer turnaround times.
4. Longer flight times for aircraft would create more air traffic.

## B2.6 Findings

Reservoir dimensions directly correlate with the firefighting equipment being used and the capabilities that they can produce. A body of water needs to be able to support an operational period of aerial firefighting in abundance when dealing with any large-scale wildfire. To supply 100,000 gallons or more, a pond or reservoir with a surface area of 2,230 square feet (e.g., 47 feet × 47 feet) and an average depth of 6 feet is sufficient. For 10,000–30,000 gallons, much smaller ponds of about 223–669 square feet can meet the need, if they are deep enough and accessible for aerial operations. Additionally, the water source would need to be supplied by a river to replenish the water supply that was taken for aerial water drops for firefighting. A 24-hour period of aerial firefighting can use up to 80,000 to 100,000 gallons of water depending on wildfire conditions and the length of time from when the fire started to end. Maintaining existing or constructing a new reservoir or body of water to support firefighting aircraft with their needs is essential to wildland firefighting within the Lake County.

# B3.0 Conclusions

## B3.1 Fire Risk, Wildfire Suppression Efforts and Vegetation Management Aspects

The proposed removal of Scott Dam and the subsequent loss of Lake Pillsbury present significant challenges for wildfire suppression and vegetation management in Lake County and the surrounding regions. The potential negative impacts include reduced water availability for firefighting, increased turnaround times, limitations on firefighting strategies, and complex vegetation management issues.

These challenges are particularly concerning given the area's history of large-scale wildfires and the increasing fire risk due to climate change. While the dam removal project aims to improve river ecosystem health and fish passage, it is crucial to carefully consider and mitigate the potential increased fire risks and suppression challenges.

Implementing a comprehensive set of mitigation strategies, including enhanced water infrastructure, augmented firefighting resources, proactive vegetation management, and adaptive management approaches, will be essential to maintaining effective fire suppression capabilities and managing vegetation-related fire risks in the post-dam removal landscape.

Further research and detailed planning will be necessary to fully understand and address the long-term implications of this project on wildfire management in the region. Collaboration between fire management agencies, ecological researchers, and local communities will be crucial in developing effective strategies to navigate these challenges and ensure the continued protection of the area's natural resources and human communities from wildfire threats.

## B3.2 Potential Alternate Water Storage or Reservoir Requirements for Fire Suppression

SLR Team examined the reservoir requirements for aerial wildfire suppression in Lake County, California, focusing on the potential impact of decommissioning Scott Dam and Lake Pillsbury. The loss of these water sources requires the establishment of alternative reservoirs to support aerial firefighting operations, particularly for rotary-wing aircraft like the Type 1 Boeing CH-47 Chinook and Sikorsky S-70 Firehawk, which are critical resources for combating large-scale wildfires.

Reservoir design must consider the water capacity, accessibility, and operational safety for aerial firefighting. For extensive operations, a reservoir capable of holding at least 100,000 gallons is essential, requiring a surface area of approximately 2,230 square feet at an average depth of 6 feet. Smaller reservoirs, supporting limited operations, should hold 10,000–30,000 gallons with surface areas ranging from 223 to 669 square feet. Properly designed reservoirs must also account for environmental factors, safety clearances for helicopter dipping, and year-round water replenishment from natural watersheds.

Constructing local reservoirs offers several advantages, including reduced turnaround times, increased water availability, and lower aircraft fuel costs. However, challenges such as high construction expenses, environmental permitting, and potential ecosystem disruption must be addressed. Alternatively, relying on distant water sources minimizes infrastructure costs but increases response times, fuel consumption, and air traffic congestion.

The conclusion emphasizes the necessity of maintaining or constructing reservoirs to ensure effective wildfire suppression. A 24-hour aerial firefighting operation may require up to 100,000

gallons of water, underscoring the importance of proximity and capacity for sustained operations. Balancing reservoir design with environmental and logistical considerations is crucial for enhancing County of Lake\’s wildfire readiness.

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# Appendix C – Fish Passage Scenario Evaluation

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

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# Acronyms and Abbreviations

| | |
|---|---|
| CDFG | California Department of Fish and Game |
| CDFW | California Department of Fish and Wildlife |
| CESA | California Endangered Species Act |
| DO | Dissolved Oxygen |
| DSOD | California Division of Dam Safety |
| DWR | Department of Water Resources |
| ESA | Endangered Species Act |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| NERF | New Eel-Russian Facility |
| NHD | National Hydrology Dataset |
| NMFS | National Marine Fisheries Service |
| NOAA | National Oceanic and Atmospheric Administration |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| RPA | Reasonable and Prudent Alternative |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| STD | Standard Deviation |
| USGS | United States Geological Survey |
| VAFS | Van Arsdale Fish Station |
| VTN | VTN Oregon, Inc. |
| WSE | water surface elevation |
| WY | water year |

# C1.0 Introduction

SLR International Corporation (SLR) was contracted by the County of Lake to conduct an analysis of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam. This Appendix outlines potential fish pathways and restoration opportunities, and evaluates conceptual infrastructure alternatives. Both the County of Lake and PG&E have identified the need for fish migration to the upper reaches of the Eel river (upstream of Scott Dam) as a benefit to the local and down stream communities as part of the Scott Dam Decommissioning project.

Based on our impact evaluation we compared a total of seven scenarios, including existing conditions and the PG&E's proposal. Seven alternate scenarios were added to address more appropriate mitigation alternatives for Scott Dam decommissioning. The scenarios include:

1. Existing Operational Conditions Scenario covers operational conditions during the WY[1] period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft (PG&E Datum)[2] and operation of the needle valve to provide water supply downstream).
2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Surrender Application dated January 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and construction of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

The results of this study are meant to provide the following:

1. Provide a high-level overview of the current infrastructure, resident fish communities, and current barriers to fish passage;

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

2. Explore potential fish pathway opportunities within the seven (7) mitigation scenarios identified above for Scott Dam Decommissioning; and
3. Based on the high level analysis presented here for fish pathway options, offer alternate options for fish pathways for consideration by the County of Lake and PG&E as part of final site restoration.

## C1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with 2,275 acres of surface area and 31 miles of shoreline (Stillwater, 2021) (PG&E, 2023). The Lake's main flow contributor is the Eel River's upper watershed[3], located in Lake County, California. Scott Dam, which forms Lake Pillsbury, is a complete barrier to upstream fish migration, preventing access to high value habitat for federally Endangered Species Act (ESA)-listed anadromous[4] salmonids including Chinook Salmon (*Oncorhynchus tshawytscha),* Coho Salmon *(O. kisutch)* and Steelhead Trout (*O. mykiss)*.

Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam primarily provides water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley, which has been offline since July 2021 after PG&E discovered a transformer did not meet current operating standards. The Scott Dam has also historically provided water for irrigation to the Potter Valley Irrigation District (PVID) and to meet the minimum instream flow requirements in the Eel River and East Fork Russian River (Stillwater, 2021) (PG&E, 2025). Lake Pillsbury also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID are diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pilsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the Russian River. Cape Horn Dam has a fish ladder allowing fish species to migrate upstream until encountering Scott Dam.

Scott Dam currently operates with an approximate maximum capacity of 56,000 acre-feet at Lake Pillsbury (PG&E, 2025). The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs). Water can be released from the dam through two structures: a needle valve[5] located at the base of the dam and the spillway[6]. The flow capacity of the needle valve is approximately 400 cubic feet per second (cfs) but can operate at varying flows for each Water Surface Elevation (WSE). The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[7]) controls. The spillway consists of multiple sliding and radial gates that are typically fully open between November 1 and April 1 and closed the remainder of the year. Flow through the gated spillway will only occur when the WSE within Lake Pilsbury

[3] An area of land drained by a river, river system, or other body of water.

[4] Fish that are born in freshwater, migrate to the ocean to grow and mature, and then return to freshwater to spawn.

[5] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[6] A structure used to provide controlled release of water downstream of a dam or levee.

[7] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

exceeds the spillway invert elevation of 1,900 feet. The top of the spillway gates are all at elevation 1,910 feet. Slide gate #13 and one of the five (5) radial gates area SCADA controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained fully open during the whole year.

As part of the PG&E's license surrender, a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E and the Eel-Russian Project Authority (ERPA) as non-project use of project land. On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023). ERPA will be responsible for the construction, maintenance and operation of NERF after the completion of the surrender requirements to be included in a regulatory order to be issues by FERC.

## C1.2 Existing Climate and Hydrology

Lake Pillsbury has several watercourses contributing to water storage availability. These tributaries historically routed directly into the Eel River prior to the construction of Scott Dam. A majority of the tributaries to Lake Pillsbury are ephemeral[8] or intermittent[9] streams with the Eel River, Habematolel Creek, and Rice Fork identified as entirely perennial[10] by the United States Geological Survey's (USGS's) National Hydrography Dataset (NHD). Other notable watercourses with large perennial reaches and short stretches that are intermittent, specifically as they reach Lake Pillsbury include Smokehouse Creek, Habematolel Creek, Salt Spring Creek, and Salmon Creek.

The hydrology of the Eel River for the Scott Dam and Cape Horn Dam system is monitored using a network of gaging stations that track water storage and flow. There are two main gages used to monitor Scott Dam, a PG&E gage that reports average daily water storage within Lake Pillsbury and a PG&E gage that reports average daily river flows downstream of Scott Dam. The Eel River watershed experiences a Mediterranean climate, typical of California's northern coast. It is characterized by intense floods in late fall and winter, limited snowmelt and spring runoff (which may decline due to climate change), minimal summer flows, and early fall rainfall that signals the start of the high-flow winter season. Peak impaired outflows from Scott Dam have been reported to be between 0.1 and 45,300 where flows are attenuated by the dam and impoundment during the wet and dry season. The wet season is currently defined between December and April and the dry season falls between May and November. Unimpaired flows post Scott Dam removal are projected to exceed 1,500 cfs during the wet months and drop below 17 cfs during the dry season. The area is also susceptible to extreme drought and flood conditions as well as climate change. Additional details on watershed hydrology are presented in SLR's Draft Impact Analysis and Adaptation Strategies Report Appendix A - Draft Hydrology and Hydraulics Scenario Evaluation.

[8] This stream flows only briefly after rainfall events. It is usually dry most of the year and only carries water for a short duration following precipitation. Ephemeral streams do not have a continuous source of water and are highly dependent on immediate weather conditions.

[9] This type of stream flows only during certain times of the year, typically when there is enough rainfall or snowmelt to sustain flow. It may have water for weeks or months at a time, but it dries up during periods of low precipitation.

[10] A perennial stream is one that flows continuously throughout the year, having a consistent and reliable source of water, such as groundwater, springs, or sustained rainfall.

## C1.3 Aquatic Habitat and Fish Populations

SLR reviewed the existing information regarding the aquatic habitat and fish populations as relates to the Eel River and Lake Pillsbury to document the fish population of concern and understand the avenues needed for fish pathway evaluations. We present our evaluation in the following sections.

### C.1.3.1 Target Fish Species

Target fish species in the Eel River watershed identified by Cal Trout and the California Department of Fish and Wildlife (CDFW, Formerly California Department of Fish and Game) include Steelhead Trout (*Oncorhynchus mykiss*), Chinook Salmon (*Oncorhynchus tshawytscha*), Coho Salmon (*Oncorhynchus kisutch),* and the Pacific Lamprey (*Entosphenus tridentatus*) (CDFW, 2024).

### C.1.3.2 Eel River Downstream of Scott Dam

The Eel River, extending from Scott Dam to Outlet Creek (approximately 31 miles downstream of Cape Horn Dam), flows through a confined valley with limited floodplain development. An overview of the Eel River is presented in Figure C1-1 . Riparian11 vegetation is generally restricted to narrow corridors, typically no wider than 40 feet, along the channel margins. Below the confluence12 with the Middle Fork Eel River, the channel features extensive, nearly continuous sediment bars. Under low-flow conditions, the river typically presents as a single-thread, sinuous channel, though it may exhibit multi-thread characteristics where it bifurcates13 around mid-channel bars. Sparse riparian vegetation along the low-flow channel and sediment bars suggests a dynamic fluvial environment14 with frequent flood events that inhibit vegetation establishment. Further downstream, from the vicinity of the Van Duzen River confluence to the estuary15, the Eel River transitions into a multi-threaded, anastomosing16 channel system, with flow paths diverging around vegetated islands.

The reach immediately downstream of Scott Dam supports high-quality spawning habitat for anadromous fish, largely due to the substantial gravel supply from Soda Creek, located approximately 1.1 miles downstream of the dam (SEC, 1998). Two major tributaries, Tomki Creek (3.8 miles below Cape Horn Dam) and Outlet Creek (31 miles downstream) provide extensive spawning and rearing habitat for anadromous salmonids (SEC, 1998). In addition, several smaller, unregulated tributaries to the Eel River, including Soda Creek, Benmore Creek, Bucknell Creek, Trout Creek, Thomas Creek, and Garcia Creek, offer important habitat for fish and other aquatic species.

[11] Land and environment along the edges of rivers, streams, or other bodies of flowing water. These areas are important because they support unique plants and animals, help prevent erosion, and improve water quality.

[12] The place where two or more rivers or streams meet and flow together.

[13] Channel splits into two separate branches.

[14] A river or stream system that is constantly changing over time due to the movement of water, sediment, and the shaping of the landscape.

[15] A coastal area where freshwater from rivers and streams mixes with saltwater from the ocean.

[16] A type of river or stream system made up of multiple interconnected channels that split and rejoin around stable islands or bars.

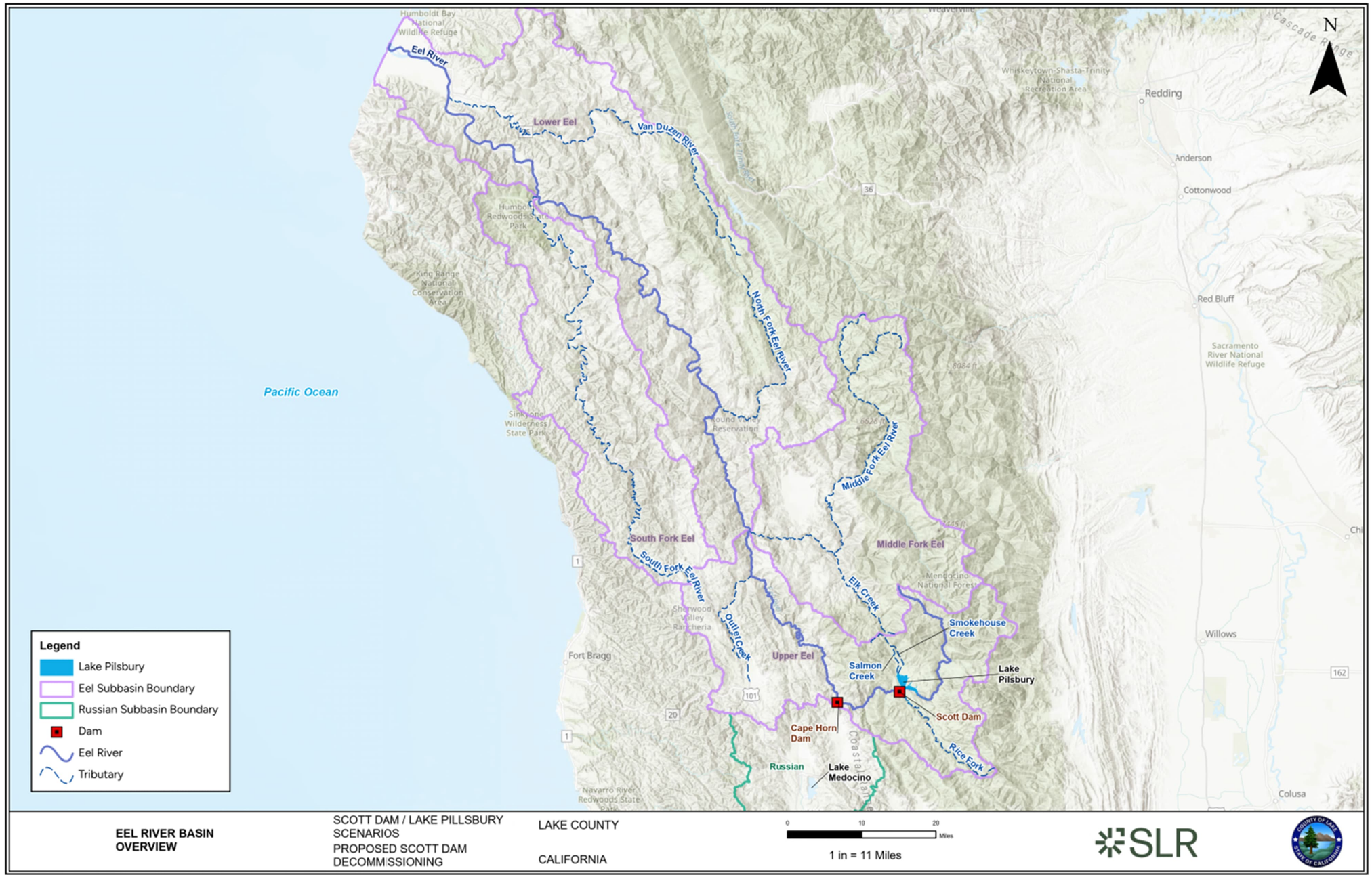


**Figure C1-1: Eel River Basin Overview**

The Eel River downstream of Scott Dam, including the lower river and estuary, provides important aquatic habitat for numerous special-status species. This includes several anadromous salmonids listed as threatened under the ESA, such as Chinook salmon (*Oncorhynchus tshawytscha*), coho salmon (*O. kisutch*), and steelhead (*O. mykiss*), as well as the threatened Pacific eulachon (*Thaleichthys pacificus*) and the endangered tidewater goby (*Eucyclogobius newberryi*). Species proposed for ESA listing, such as the northwestern pond turtle (*Actinemys marmorata marmorata*) and the longfin smelt (*Spirinchus thaleichthys*), also inhabit this area.

Other notable species include those listed under the California Endangered Species Act (CESA), like the endangered summer steelhead (*O. mykiss irideus*), the candidate species white sturgeon (*Acipenser transmontanus*), and species of special concern such as the green sturgeon (Acipenser medirostris), Pacific lamprey (*Entosphenus tridentatus and Lampetra spp.),* foothill yellow-legged frog (*Rana boylii, Northwest/North Coast clade*), coastal tailed frog (*Ascaphus truei*), southern torrent salamander (*Rhyacotriton variegatus*), California floater (*Anodonta californiensis*), western pearlshell (*Margaritifera falcata*), and the northern red-legged frog (*Rana aurora*). (CDFG, 2010), (CDFW, 2024a), (iNaturalist, 2024).

The river also supports a variety of non-listed native freshwater fish, including sculpin (*Cottus spp.),* stickleback (*Gasterosteus spp.),* and sucker (*Catostomus spp.).* Additionally, occasional runs of pink salmon (*O. gorbuscha*) and chum salmon (*O. keta*) have been documented (Yoshiyama and Moyle, 2010).

### C.1.3.3 Lake Pillsbury

Aquatic habitat within Lake Pillsbury is primarily composed of shallow shoreline and flat areas lacking vegetative cover, along with deeper pelagic zones[17]. Seasonal fluctuations in reservoir elevation typically inhibit the establishment of extensive riparian vegetation or aquatic macrophyte (vascular plant) communities. Limited riparian/wetland vegetation is present at tributary inlets and along the northern shore (Gravelly Valley), which becomes submerged at peak reservoir levels. The littoral zone—characterized by shallow, light-penetrating waters with potential for macrophyte growth—offers minimal structural cover for aquatic fauna.

Thermal stratification[18] during spring through fall separates the reservoir into a large, epilimnetic[19] layer of warm water, which supports aquatic habitat for warm-water fishes including Largemouth bass (*Micropterus salmoides*), Green sunfish (*Lepomis cyanellus*) and Bluegill (*Lepomis macrochirus)*, along with various amphibians, aquatic reptiles, and macroinvertebrates. Native species such as Rainbow trout (*Oncorhynchus mykiss*) and Sacramento sucker (*Catostomus occidentalis*) are found in the relatively limited volume of colder, deeper water in the metalimnion[20] and hypolimnion[21] layers of the reservoir (CDFW, 2016).

[17] The open water area of a lake that is not close to the bottom or the shore that provides different habitats supporting various stages of a fish's life and influence their behavior, feeding, and survival.

[18] Temperatures in a lake vary in layers based on depth and season.

[19] The top layer of warm water in a thermally stratified lake.

[20] The colder middle thermally stratified layer just below the epilimnetic layer.

[21] The bottom and coldest layer of a thermally stratified lake below the metalimnion layer.

### C.1.3.4 Upper Eel River

Upstream of Lake Pillsbury, the Eel River and Rice Fork currently offer habitat for the foothill yellow-legged frog, northwestern pond turtle, and a variety of other non-listed native fish, amphibians, aquatic reptiles, and macroinvertebrates. Historically, before construction of Scott Dam in 1921, the Upper Eel watershed supported extensive spawning and rearing habitat for Steelhead and Chinook. Anadromous fish species are blocked from accessing this habitat by Scott Dam and the Lake Pillsbury reservoir (See Section C1.4).

### C.1.3.5 Van Arsdale Fish Station (VAFS)

Since 1909, upstream and downstream fish passage for migratory species has been provided at Cape Horn Dam via the Van Arsdale pool-and-weir fish ladder, approximately 12 miles downstream of Scott Dam/Lake Pillsbury. This ladder has undergone several upgrades to enhance fish passage, with modifications made in 1915, 1962, 1987, and most recently in 2020. Additionally, experimental improvements targeting Pacific lamprey passage were implemented between 2014 and 2016. A summary of is outlined in Figure C1-2 and Figure C1-3 summarize fish passage counts at the Van Arsdale Fisheries Station (VAFS).

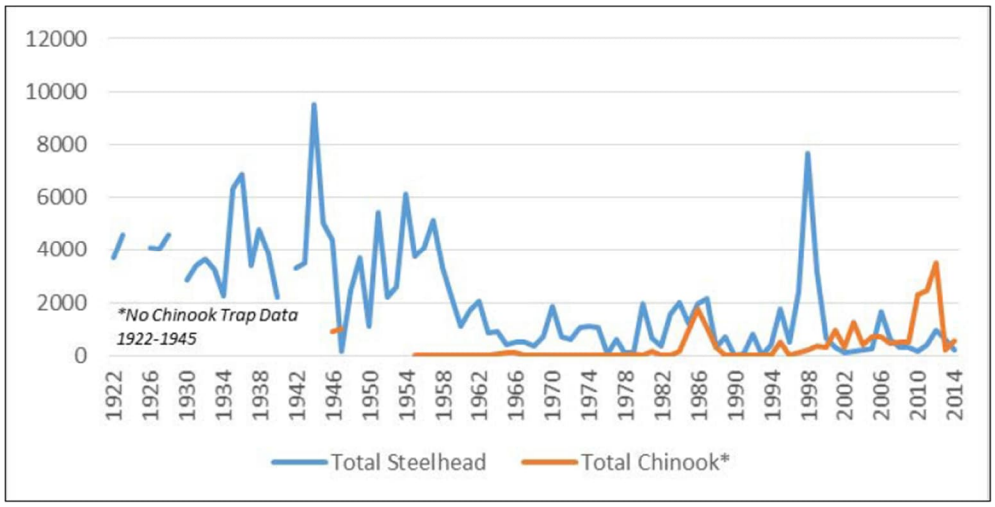


**Figure C1-2: Figure A-4 from McMillen Jacobs Potter Valley Project Capital Modifications Feasibility Study Report, 2018: Total Adult Steelhead and Chinook Trapped at Van Arsdale Fisheries Station; 1922-2014 (from CDFW counts).**

| Steelhead | | | |
|---|---|---|---|
| Season | Number Wild | Number Hatchery | Total Count |
| 1990/1991 | 19 | 12 | 31 |
| 1991/1992 | 26 | 34 | 60 |
| 1992/1993 | 52 | 771 | 823 |
| 1993/1994 | 23 | 11 | 34 |
| 1994/1995 | 116 | 318 | 434 |
| 1995/1996 | 158 | 1585 | 1743 |
| 1996/1997 | 104 | 407 | 511 |
| 1997/1998 | 175 | 2218 | 2393 |
| 1998/1999 | 355 | 7324 | 7679 |
| 1999/2000 | 189 | 2961 | 3150 |
| 2000/2001 | 250 | 391 | 641 |
| 2001/2002 | 226 | 82 | 308 |
| 2002/2003 | 99 | 3 | 102 |
| 2003/2004 | 149 | 0 | 149 |
| 2004/2005 | 234 | 0 | 234 |
| 2005/2006 | 184 | 69 | 253 |
| 2006/2007 | 492 | 1143 | 1635 |
| 2007/2008 | 423 | 199 | 622 |
| 2008/2009 | 305 | 10 | 315 |
| 2009/2010 | 324 | 0 | 324 |
| 2010/2011 | 166 | 0 | 166 |
| 2011/2012 | 423 | 0 | 423 |
| 2012/2013 | 934 | 1 | 935 |
| 2013/2014 | 609 | 0 | 609 |
| 2014/2015 | 215 | 0 | 215 |
| 10 year mean (2005-2015): | | | 550 |
| 10 year STDEV: | | | 448 |

| Chinook | | | |
|---|---|---|---|
| Season | Number Wild | Number Hatchery | Total Count |
| 1990/1991 | na | na | 0 |
| 1991/1992 | na | na | 5 |
| 1992/1993 | na | na | 4 |
| 1993/1994 | na | na | 1 |
| 1994/1995 | na | na | 21 |
| 1995/1996 | 525 | 0 | 525 |
| 1996/1997 | 26 | 0 | 26 |
| 1997/1998 | 106 | 1 | 107 |
| 1998/1999 | 141 | 59 | 200 |
| 1999/2000 | 232 | 107 | 339 |
| 2000/2001 | 223 | 80 | 303 |
| 2001/2002 | 641 | 314 | 955 |
| 2002/2003 | 268 | 61 | 329 |
| 2003/2004 | 997 | 236 | 1233 |
| 2004/2005 | 309 | 83 | 392 |
| 2005/2006 | 620 | 105 | 725 |
| 2006/2007 | 697 | 2 | 699 |
| 2007/2008 | 478 | 0 | 478 |
| 2008/2009 | 496 | 0 | 496 |
| 2009/2010 | 518 | 1 | 519 |
| 2010/2011 | 2314 | 0 | 2314 |
| 2011/2012 | 2436 | 0 | 2436 |
| 2012/2013 | 3471 | 0 | 3471 |
| 2013/2014 | 215 | 0 | 215 |
| 2014/2015 | 583 | 0 | 583 |
| 10 year mean (2005-2015): | | | 1194 |
| 10 year STDEV: | | | 1117 |

**Figure C1-3: Table A-1 from McMillen Jacobs Potter Valley Project Capital Modifications Feasibility Study Report, 2018.: Number of Upstream Migrating Adult Steelhead Oncorhynchus mykiss and Chinook O. tshawytscha Trapped annually at Van Arsdale Fisheries Station, 1990-2015 (Source: CDFW).**

## C1.4 Scott Dam as a Fish Barrier

The construction of Scott Dam in 1921 created a 130-foot-high barrier that prevents upstream fish migration. Currently, there are no facilities at the dam to allow fish passage. Estimates of the blocked river habitat and potential returning fish numbers vary widely. A 1982 VTN study identified 35.7 miles of primary spawning habitat above the dam, estimating possible returns of 1,250 Chinook salmon and 1,499 Steelhead (VTN, 1982). National Marine Fisheries Services (NMFS) (2002) referenced this and suggested that historically, 2,000 to 4,000 fall-run Chinook and winter-run steelhead may have spawned upstream. Other studies estimate 100–150 miles of blocked habitat, while Cooper et al. (2020) projected even greater totals—up to 288 miles for steelhead spawning. PG&E's 2018 relicensing efforts included aerial and field surveys to assess habitat, barriers, and temperature conditions, which identified a minimum of 50 miles of spawning and rearing habitat upstream of Scott Dam (PG&E, 2025).

There are currently no downstream fish passage facilities at Scott Dam. However, potential downstream fish passage can occur during spill events when water flows over the dam crest. When the reservoir is not spilling, water is released through the low-level outlet (needle valve). Fish movement into the low-level outlet is expected to be minimal, as studies at other reservoirs have documented very low fish densities at the depths where low-level outlets are located (Placer County Water Agency 2011; Nevada Irrigation District and PG&E 2011).

# C2.0 Fish Passage Objectives and Design Criteria

## C2.1 Objective

The functional objective when considering fish passage is to facilitate upstream and downstream fish movement at the existing Scott Dam to restore historical migration routes for anadromous fish in the Eel River watershed. Other objectives include maintaining the currently established fish habitats at Lake Pillsbury, as possible and where a supplemental water body is proposed as part of the mitigation scenarios.

## C2.2 Fish Passage Design Criteria

A wide range of design criteria were considered by SLR when evaluating fish passage alternatives, including but not limited to:

- Ability to provide volitional passage and accommodate biological design criteria for target fish species;
- Accommodating in-stream flow requirements during critical fish migration windows;
- Complexity and cost of implementation;
- Environmental impact during and after construction; and
- Long term maintenance and operations.

### C.2.2.1 Biological Design Criteria

The NOAA Fisheries West Coast Region *Anadromous Salmonid Passage Design Manual* (NMFS, 2022b) establishes engineering criteria for fishways and associated fish passage in the state of California. These guidelines are intended to ensure that selected fish passage facilities function effectively within project-specific contexts. The manual outlines quantitative design parameters, such as minimum water depth, maximum water velocity, and energy dissipation factors for fish ladders, as well as mesh size, porosity, and approach velocity requirements for fish screens.

Additional biological design criteria for the specific target fish species are summarized in Table C2-1:, below.

**Table C2-1: Biological Design Criteria for Target Fish Species (Modified from McMillen Jacobs, 2018)**

| Criteria / Target Species | Units | Value | Comments |
|---|---|---|---|
| | | | |
| **Fish Size** | | | |
| Steelhead | mm | 340 - 1050 | Fork length |
| Coho Salmon (Winter Run) | mm | 341 - 1050 | Fork length |
| Chinook Salmon (Fall Run) | mm | 342 - 1050 | Fork length |
| Sea Run Cutthroat | mm | 400 | Fork length |
| Pacific Lamprey | mm | 152 - 910 | Total length |
| | | | |
| **Average Fish Weight (Adult)** | | | |

| Criteria / Target Species | Units | Value | Comments |
|---|---|---|---|
| Steelhead | lb | 4-5 | |
| Coho Salmon (Winter Run) | lb | 4-6 | |
| Chinook Salmon (Fall Run) | lb | 18-22 | |
| Sea Run Cutthroat | lb | 1.5 | |
| Pacific Lamprey | lb | 1.0 | |
| | | | |
| **Swimming Capabilities (Sustained/Burst)** | | **Adults** | |
| Steelhead | fps | 3.0 / 20.3 | Jones et al. 1974 / Bell 1991 |
| Coho Salmon (Winter Run) | fps | 4.0 / 21.0 | Bell 1991 |
| Chinook Salmon (Fall Run) | fps | 5.0 / 14.0 | Geist et al. 2003 / Hunter and Mayor 1986 |
| Sea Run Cutthroat | fps | 3.0 / 14.0 | Bell 1991 |
| Pacific Lamprey | fps | 1.3 / 2.7 | Moursund et al. 2003 |
| | | | |
| **Migration Timing** | | | |
| Steelhead | mo. | Dec-Apr | Higgins 2010 |
| Coho Salmon (Winter Run) | mo. | Nov-Jan | Shapovalov 1954 |
| Chinook Salmon (Fall Run) | mo. | Oct-Dec | Higgins 2010 |
| Sea Run Cutthroat | mo. | Dec-Apr | Johnson and Jones 2002 |
| Pacific Lamprey | mo. | Variable | Luzier et al. 2001 |
| | | | |
| **Numbers of Fish at VAFS** | | | |
| Steelhead - mean annual (STD) | total | 550 (448) | 2005-2015 mean counts |
| Coho - mean annual (STD) | total | N/A | |
| Chinook - mean annual (STD) | total | 1194 (1117) | |
| Pacific Lamprey - mean annual | total | N/A | |

### C.2.2.2 Instream Flows during Fish Migration Periods

Target fish species in the Eel River typically migrate during the wet season, from November to May. Fall run Chinook salmon migrate upstream from November through January and downstream migration occurs primarily April through May. Steelhead migrate upstream January through May and downstream primarily February through May (Table C2-2:).

**Table C2-2: Adult Migration and Smolt Outmigration for Chinook Salmon and Steelhead from the Potential Ecosystem and Fisheries Responses to Project Alternatives Technical Memorandum (Stillwater Sciences et al., 2021)**

| Species | Life Stage | Month | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| Chinook Salmon Fall-run | Adult Migration | | | | | | | | | | | | |
| | Smolt Outmigration | | | | | | | | | | | | |
| Steelhead (Summer-run and Winter-run) | Adult Migration (Winter-run) | | | | | | | | | | | | |
| | Adult Migration (Summer-run) | | | | | | | | | | | | |
| | Smolt Outmigration | | | | | | | | | | | | |
| | Kelt (Adult) Outmigration | | | | | | | | | | | | |

Per VTN 1982, SEC 1998, CDFW, unpublished VAFS count data, Beak 1986 and Moyle et al. Per VTN (1982), SEC (1998), CDFW, unpublished VAFS count data, Beak (1986) and Moyle et al. (2017).

Minimum streamflow requirements have been established in the Eel River below Lake Pillsbury and Van Arsdale Reservoir to protect Chinook salmon and Steelhead populations, and in the East Branch Russian River to support a managed rainbow trout fishery upstream of Lake Mendocino. Before 1979, the year-round minimum flow below Van Arsdale Reservoir was just 2 cubic feet per second (cfs). Since then, more natural flow patterns have been adopted. The NMFS's 2002 Reasonable and Prudent Alternative (RPA) updated flow requirements, aiming to replicate the natural hydrograph, supporting key life stages of salmonids, including fall and winter migration, spring emigration, and summer rearing. From December to March, minimum flows from Cape Horn Dam range between a floor of 100 cfs and a cap of 140 cfs, though the minimum can drop to 25 cfs during periods of exceptionally low inflow to Lake Pillsbury (NMFS, 2002).

# C3.0 Scenario Descriptions and Evaluation

This section presents the range of alternatives considered by SLR in the Impact Analysis and Adaptation Strategies evaluation for potential modifications to the Scott Dam facilities. These alternatives span from partial dam removal and traditional fish passage methods to full decommissioning of Scott Dam (as proposed by PG&E). The following sections provide a summary of the proposed scenarios as applicable to fish migration and the SLR's findings.

## C3.1 Current Operational Conditions Scenario (WY 2019-Present)

### C.3.1.1 General Description

This option involves keeping Scott Dam in its current configuration without adding any fish passage facilities. Based on observed operational data, the maximum operational WSE for Lake Pillsbury was reduced to the spillway invert elevation of 1,900 feet with a corresponding water storage volume of 56,322 ac-ft as depicted in Figure C3-1. In 2023, following concurrence from the California Division of Dam Safety (DSOD), PG&E had determined that the spillway gates were to continuously remain open due to seismic risk concerns. Under current operational conditions, the spillway passively discharges whenever the WSE at Scott Dam is above the spillway invert. Outflow through the needle valve is currently maintained.

### C.3.1.2 Evaluation Summary

#### C.3.1.2.1 Proposed Work for Fish Pathway Improvements

No work proposed under this summary other than outstanding and routine maintenance and operations.

#### C.3.1.2.2 Fish Passage

Under this scenario the dam would continue to be a barrier and block access to upstream habitat for target fish species. However resident fish populations in Lake Pillsbury, including Largemouth bass (*Micropterus salmoides*), Green sunfish (*Lepomis cyanellus*), Bluegill (*Lepomis macrochirus)*, Rainbow trout (*Oncorhynchus mykiss*) and Sacramento sucker (*Catostomus occidentalis*), would be maintained.

#### C.3.1.2.3 Instream Flows

Instream flows downstream of Scott Dam during the fish migration period would remain unchanged. A statistical summary of the observed wet season climate conditions (WYs 2019-2024) corresponding to the fish migration period is presented in Table C3-1. Median and average flows in this scenario meet the minimum in-stream flow requirements, however the minimum average daily flow of 67 cfs drops below the recommended flows of 100 cfs (NMFS, 2002).

**Table C3-1: Existing Operational Conditions – Statistics for Observed Storage within Lake Pillsbury and Observed Outflow Downstream of Scott Dam during the Wet Season Fish Migration (WY 2019-2024)**

| Statistic | Wet Season (December – April) | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 16,800 | 1870.9 | 67 |
| Median | 53,200 | 1898.4 | 207 |
| Average | 45,439 | 1893.8 | 768 |
| Maximum | 67,300 | 1905.6 | 13,200 |

#### C.3.1.2.4 Operation and Maintenance

Continued operation and maintenance with minimal capital investment. If the seismic conditions change additional capital improvement may be needed

#### C.3.1.2.5 Ecosystem

No major changes; current barriers to fish migration remain. Fish habitats within the Lake Pillsbury will be sustained.

#### C.3.1.2.6 Advantages

- Existing biological communities and habitat conditions would remain unchanged.

#### C.3.1.2.7 Disadvantages

- Scott Dam would remain a complete barrier to fish passage, preventing access to upstream habitat in the upper watershed.

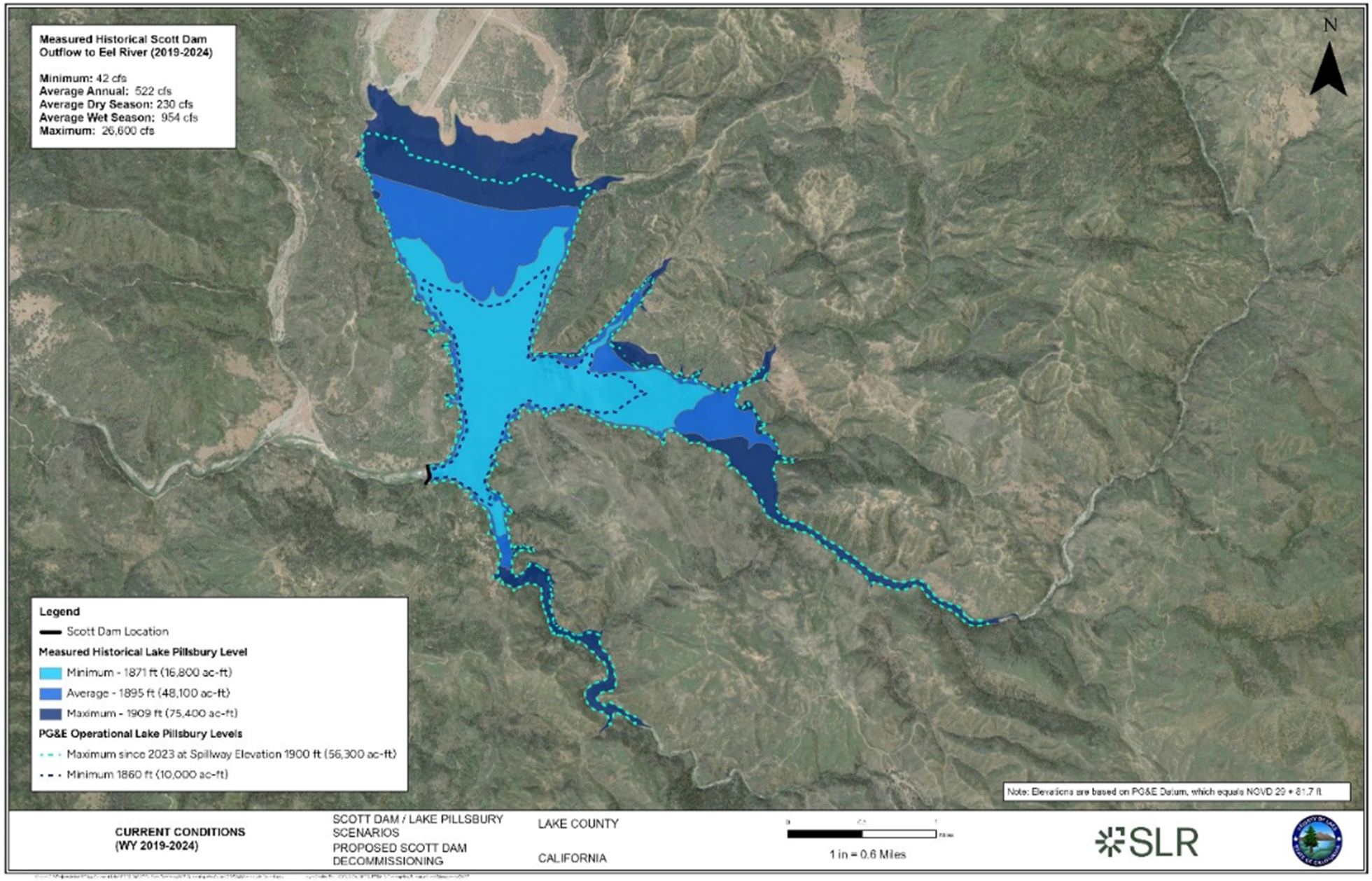


**Figure C3-1: Existing Operational Conditions (WY 2019-2024), with measured minimum average, maximum, and operational Lake Pillsbury WSE levels (Figure A7-3 from Appendix A).**

## C3.2 Scenario A1 – PG&E Proposed Removal

### C.3.2.1 General Description

Under this scenario, PG&E proposes the full decommissioning of Scott Dam. A plan view depiction of the Scott Dam and Lake Pillsbury area with projected response to Scenario A1 conditions are presented in Figure C3-2 and Figure C3-3. The Eel River and tributaries are assumed to return to their pre-dam condition as represented in the 1922 topographic and bathymetric mapping compiled by PG&E (2016).

The proposed approach is divided into three phases per PG&E's current proposed plan (PG&E, 2025), including two low-flow seasons and one high flow season:

a. Low flow season one (June 1 – October 31) - initial dewatering and removal of the upper portion of the dam;

b. High flow season one (November 1 – May 31) - initial sediment flushing; and

c. Low flow season two (June 1 – October 31) - final dam removal.

As proposed by PG&E, the sediments that have accumulated within the impoundment will be managed passively. PG&E's current proposed action under the Draft Application for Surrender of License (PG&E, 2025) includes the decommissioning and removal of Scott Dam, draining Lake Pillsbury and removing associated recreational facilities and features (e.g., campgrounds, day-use facilities, recreation access roads and trails, a kiosk, and boat ramps) located on U.S. Forest Service and PG&E lands. PG&E proposed a Rapid Dam Removal approach estimated to take 2 years. This approach would result in the flushing of a large amount of sediment (approximately 12 million cubic yards) downstream of the Eel River (PG&E, 2025). PG&E anticipates temporary alteration of the Eel River channel and/or floodplain morphology from sediment deposition after deconstruction of the dam until subsequent high-flow events can resuspend the sediment and transport it farther downstream, until the river channel adapts to its natural condition. Other activities proposed by PG&E as part of decommissioning are documented in the Draft Application for Surrender of License (PG&E, 2025). PG&E may propose additional responses at locations where natural fluvial processes are not mobilizing sediment to the volumes anticipated. These locations may have continued issues with bank instability and channel incision that may adversely affect habitat and fish passage.

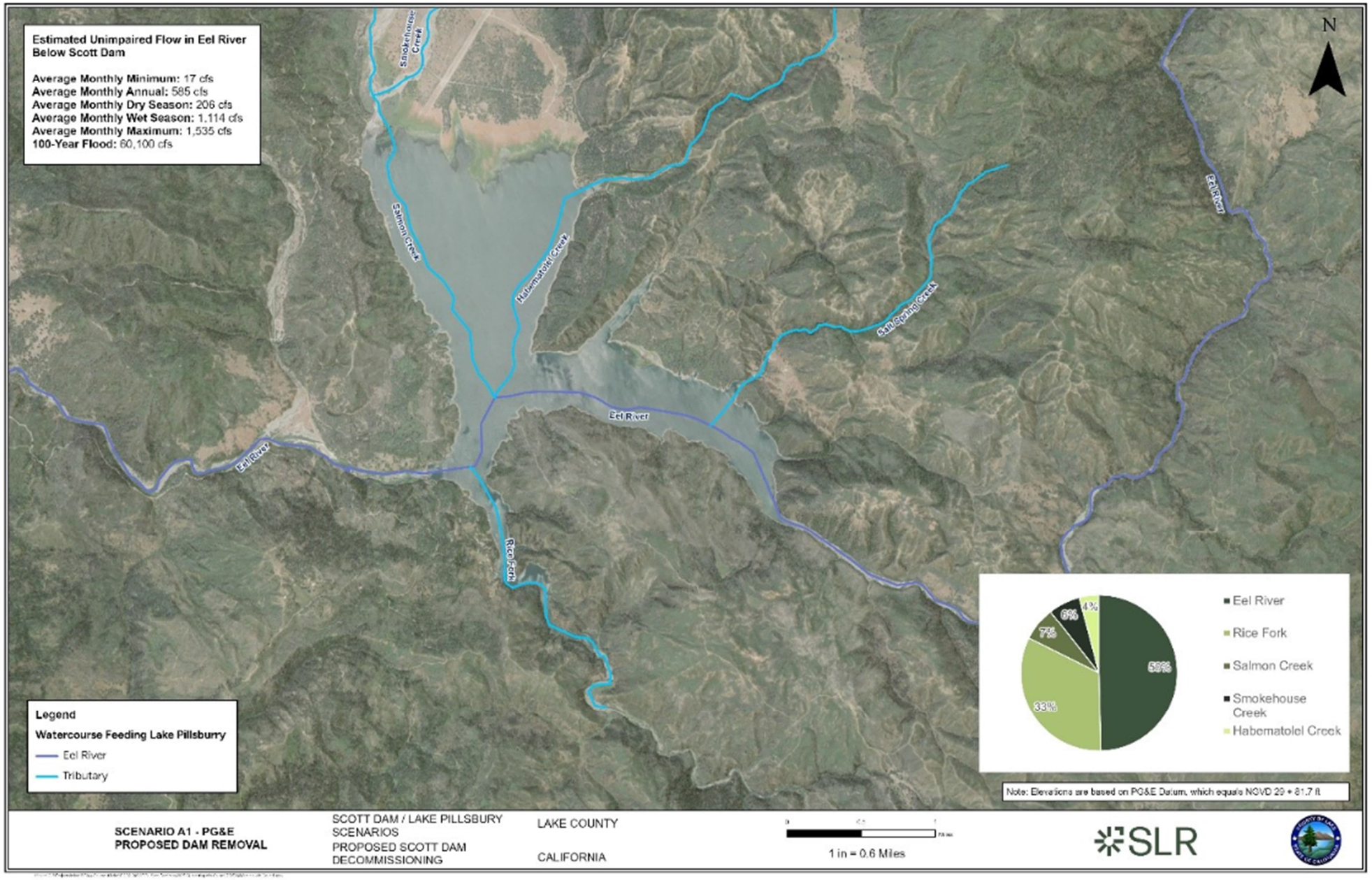


**Figure C3-2: Scenarios A1 and A2 PG&E proposed Scott Dam removal, showing Eel River and tributary watercourses originally feeding Lake Pillsbury (Figure A7-10 from Appendix A).**

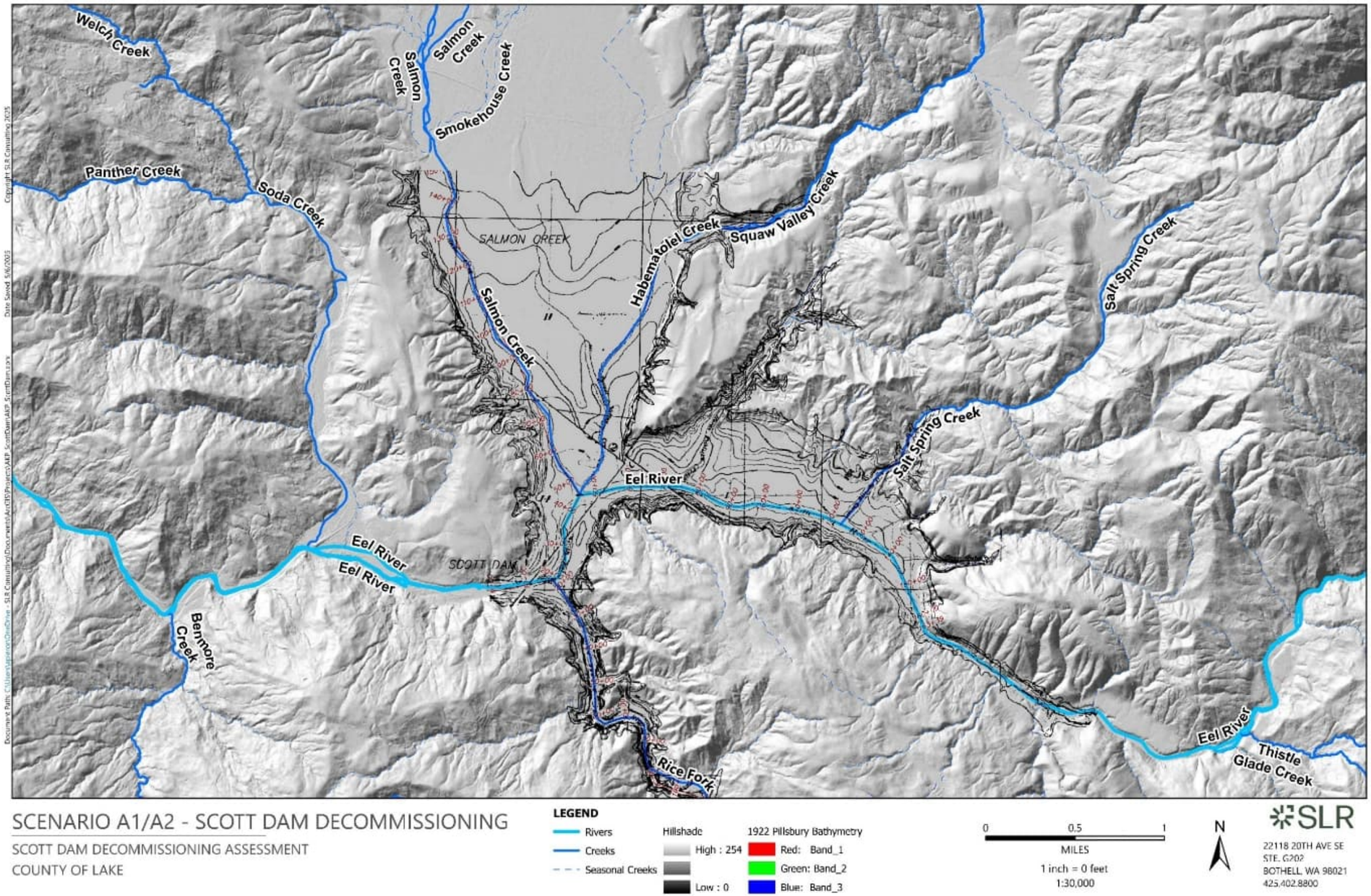


**Figure C3-3: Scenarios A1 and A2 Projected Response Post Dam Removal (Figure A7-11 from Appendix A).**

### C.3.2.2 Evaluation Summary

#### C.3.2.2.1 Proposed Work for Fish Pathway Improvements

Rapid removal of Scott Dam and associated structures per PG&E plans. No additional work is proposed.

#### C.3.2.2.2 Fish Passage

This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. Existing warmwater aquatic habitat in the reservoir would be eliminated.

#### C.3.2.2.3 Instream Flows

Instream flows in the Eel River for Scenario A1 during the wet season fish migration period are summarized in Table C3-2. The average monthly wet season flows range from 791 cfs in April to 1,535 cfs in February, representing an increase in flow during the critical fish migration period and an improvement compared to the existing operating conditions. Instream flows during the wet season remain well above the recommended floor of 100 cfs.

**Table C3-2: Dam Removal Scenario A1 Wet Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 791 |
| Median | 1,237 |
| Average | 1,216 |
| Maximum | 1,535 |

#### C.3.2.2.4 Channel Restoration and Sediment Management

PG&E has proposed a passive sediment management approach with no detailed channel or floodplain restoration plan submitted to date. The plan pledges to develop and implement a Sediment/Channel Monitoring Response and Restoration Plan, which would monitor tributary mouths and critical riffles downstream of Scott Dam for sediment accumulation and the former reservoir footprint for channel downcutting and erosion issues. Field crews and/or equipment would be dispatched to remove/reposition sediment, as required after dam removal to facilitate fish passage through the Eel River, the dam footprint and the former reservoir. Without active management, channels could become deeply incised, resulting in the loss of functional floodplains, degradation of riparian habitat, disruption of natural pool–riffle sequences, and reduction of available spawning habitat.

#### C.3.2.2.5 Ecosystem

The primary environmental impact of the rapid dam removal alternative would be the release of 12 million cubic yards of accumulated sediment from behind Scott Dam. During the initial 1-2

days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species.

The majority of clay and silt sediments are expected to be transported to the coast over time, however extremely high suspended sediment load in the days following the sediment release is expected to result in lethal effects on aquatic species inhabiting the mainstem Eel River. Significant volumes of coarser sand are expected to deposit in habitat features from Scott Dam to approximately the Middle Fork Eel River confluence. Sand deposition would diminish aquatic habitat by burying pools and coarse riffle habitat until the river can flush accumulated sediment and establish a new geomorphic equilibrium[22]. Flushing of accumulated sediment is expected to take a number of years and depends on local hydrology and storm magnitude in the season following removal. Flushing events may also pose risk to aquatic species depending on the wash load, or the fine sediment particles carried in suspension by the Eel River.

An additional impact is the elimination of warmwater aquatic habitat in Lake Pillsbury and the potential release of non-native fish species from the reservoir into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River and will require PG&E to implement a Construction Non-native Invasive Aquatic Species Management Plan to minimize the spread of non-native species.

#### C.3.2.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.2.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.

[22] The river is not gaining or losing material overall—it's stable, even though small changes may still happen.

## C3.3 Scenario A2 – Scenario A1 with Additional Mitigation

### C.3.3.1 General Description

Under this scenario, PG&E proposes the full decommissioning of Scott Dam and the County of Lake proposes additional mitigation efforts implemented to address their concerns and those of the local community. Where Scenario A1 proposed passive sediment removal options, this scenario presents active solutions to mitigate downstream sediment movement. Practices such as planting native vegetation, restoring the channels, creating a restored wetland area, and transporting removed sediment off-site would help maintain the recreational and economic benefits for the County of Lake, and improve the environmental impacts when compared to Scenario A1. This option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage and would have higher ecosystem benefits to the community.

Figure C3-2 and Figure C3-3 will similarly apply when predicting the changes to the Eel River system upstream of Scott Dam upon scenario completion. Figure C3-4 shows an example of the results of sediment management and revegetation efforts in the Elwha Dam reservoir over a 6-year period. The floodplain has been recolonized and stabilized by native plant species.

**Figure C3-4: Elwha River reservoir soon after the dam removal (top) and six years later (bottom), after the implementation of a comprehensive revegetation strategy (photo credit: Gussman, 2018)**

## C.3.3.2 Evaluation Summary

### C.3.3.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures with additional revegetation and other ecosystem enhancements in the reservoir footprint and sediment management.

### C.3.3.2.2 Fish Passage

Similar to Scenario A1, this option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. Existing warmwater aquatic habitat in the reservoir would be eliminated.

### C.3.3.2.3 Instream Flows

Instream flows in the Eel River for Scenario A2 would be the same as Scenario A1, as summarized in Table C3-2. The average monthly wet season flows would range from 791 cfs in April to 1,535 cfs in February, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions.

### C.3.3.2.4 Channel Restoration and Sediment Management

This option calls for the excavation and re-establishment of the river channel through the reservoir reach. Excavated sediment removed to reconstruct the channel would be relocated to the overbank areas or removed off-site. The new river channel and overbank areas would then be stabilized through structural and vegetative techniques. Revegetation of the overbank floodplain in the former reservoir through an extensive planting plan would expedite site stabilization, reduce the volume of sediment transported downstream, and help mitigate some of the environmental impacts identified in Scenario A1 (Section C3.2). With the channel restoration and sediment management measures, Scenario A2 would have a beneficial effect on habitat for fish and aquatic habitat.

### C.3.3.2.5 Ecosystem

Despite sediment management and mitigative measures, the short-term environmental impacts resulting from the initial sediment flush are expected to be similar to Scenario A1. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species. However, the long-term volume of mobilized sediment is expected to be less than Scenario A1 due to sediment management within the reservoir and revegetation efforts that will stabilize expose soils.

This scenario may also result in the release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile

salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River.

#### C.3.3.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.3.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Long-term issues with sediment transport and stability due to altered landscape conditions.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.

## C3.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

### C.3.4.1 General Description

This scenario is similar to the current operational conditions described in Section C3.1, with the addition of a fish ladder, as previously evaluated by Mead and Hunt (2018). The hydrologic conditions will be similar to the current operational conditions as well. The primary objective of this scenario was to explore the feasibility of implementing a fish passage system that would enhance aquatic connectivity while maintaining operational integrity of the existing infrastructure. To implement this, a seismic retrofitting of the dam would also be required.

From an operational standpoint, the fish passage system would function in alignment with standard industry practices to allow passage for the different species at the required times of the year. This would also require coordination with water delivery schedules, and other environmental flow requirements. Due to the existing seismic concerns, retrofitting the dam requires extensive engineering and costly initial construction measures.

### C.3.4.2 Evaluation Summary

#### C.3.4.2.1 Proposed Work for Fish Pathway Improvements

The Mead and Hunt modified conventional fishway (2018) consists of a pool and weir fishway on the southern abutment of the dam to provide volitional fish passage over Scott Dam. The majority of pools would be located downstream of the dam, and the exit pools upstream of the dam, with the two sections connected by a 48-inch diameter tunnel bored through the dam (Figure C3-5). Boring a tunnel through the face of the concrete dam would introduce additional seismic concerns.

#### C.3.4.2.2 Fish Passage and Instream Flows

The fish ladder could be designed to operate over the range of flows observed under current operational conditions, as described in Section C3.1. However, facilitating volitional fish passage at the dam through the fishway would be challenging, considering the range of reservoir fluctuations. To address the fluctuation in reservoir levels, the fishway exit must operate over the full range of water surface elevations. This would be achieved through controllable gated openings through the fishway exit perimeter wall that can be opened or closed as lake levels rise and fall. The exit pool nearest to the lake level can be opened to allow access to migrating salmonids. This highly complex, mechanical fishway would increase the capital and maintenance costs of the system.

The design also relies on the upstream fish ladder for downstream fish passage, which is not a conventional or demonstrated approach. The downstream migration system lacks effective attraction velocities at the downstream passage entry to attract out-migrants to the ladder. It also introduces the risk of predation on out-migrants by adults migrating upstream through the same system.

Mead and Hunt (2018) ultimately concluded that the proposed fish ladder system would be difficult to construct, complex to manage, expensive, and potentially ineffective, primarily due to challenges in attracting out-migrating fish to the ladder and increased predation risks during migration.

#### C.3.4.2.3 Ecosystem

This scenario is expected to have relatively minimal environmental impact during construction compared to other dam removal options. However, excavation and blasting would be required on the left (south) abutment of the dam. The fishway would provide some improvement to fish migration post-construction, but the ultimate effectiveness is uncertain.

#### C.3.4.2.4 Advantages

- Existing biological communities and habitat conditions would remain unchanged.
- Potential increase in resident/anadromous fish production.
- Would provide upstream fish passage over the dam.

#### C.3.4.2.5 Disadvantages

- Uncertain fish passage effectiveness, particularly for downstream migration.

- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level would need to be maintained within a set range to support operation.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.
- Would require extensive excavation and construction activities on the left abutment of the dam.
- Tunnel through concrete dam face may introduce additional seismic concerns.

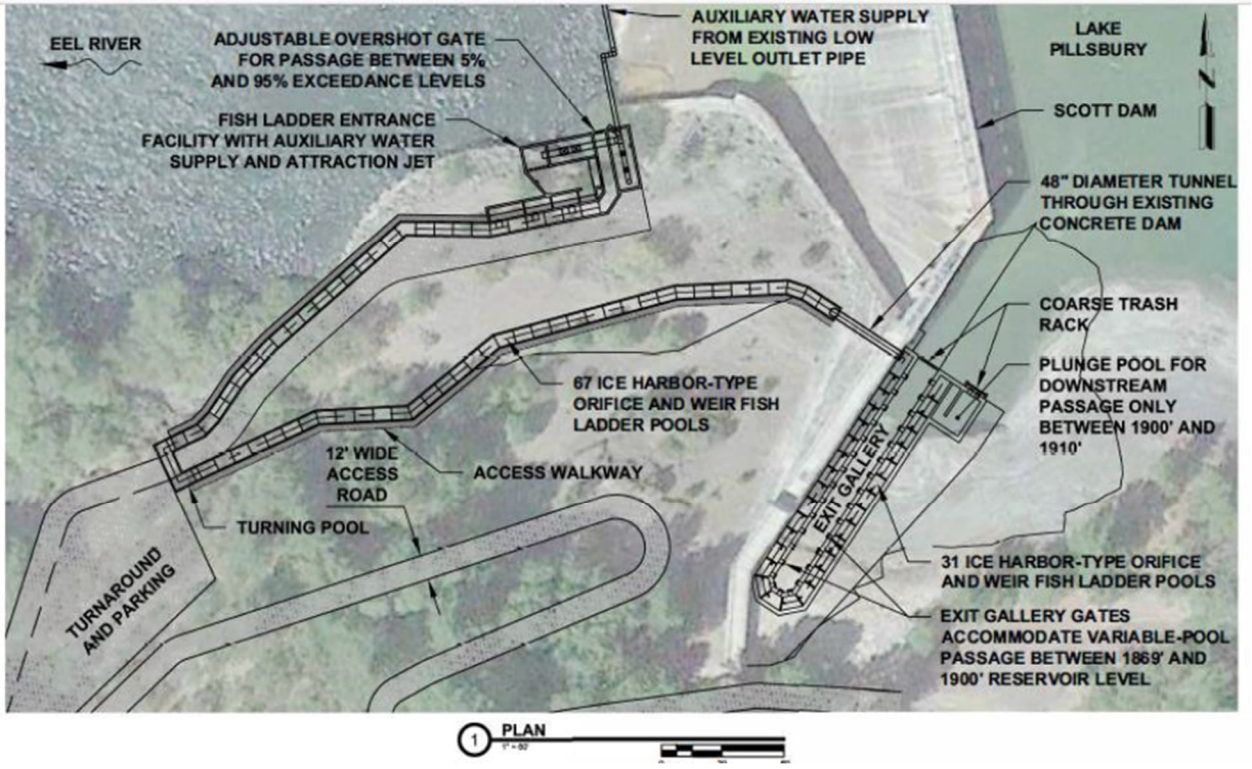


**Figure C3-5: Mead & Hunt Modified Conventional Fishway (2018)**

## C3.5 Scenario C – Lowered Scott Dam with Fish Ladder

### C.3.5.1 General Description

This scenario involves the lowering of the Scott Dam spillway invert down to a new proposed spillway elevation of 1,875 feet (PG&E datum) and the installation of a fish passage system, similar to the Mead and Hunt fishway described in Scenario B. Modifications to the spillway structure would be required to facilitate safe downstream fish passage over the dam and into the tailrace. The primary goal of this scenario would be to provide effective volitional fish passage without completely removing the dam and releasing sediment stored in the reservoir. Lowering the spillway invert elevation to 1,875 feet would create a reservoir of approximately 40 percent of the current storage volume at WSE 1,900 feet. Potentially, the existing needle valve structure may be modified or replaced to provide a controlled low-level outlet that can maintain

minimum ecological outflows (NMFS, 2002) to the Eel River under low Lake Pillsbury WSEs and allow for managed sediment flushing from Scott Dam as needed.

Without the requirement to have a larger water storage at the current maximum operational WSE of 1,900 feet, the spillway invert may be further reduced in height by 25 feet below the existing spillway invert, which would result in a new lake footprint shown in Figure C3-6. The smaller reservoir footprint would resemble Lake Pillsbury during drought conditions, as shown in Figure C37. At that reduced maximum operational WSE, the stability of the concrete gravity dam structure would improve for seismic considerations. The material removed from the upper portion of the existing dam may be potentially reused to reinforce the dam abutment area (where previous slides have occurred) by placing the concrete material on both or either of the downstream and upstream sides of the lowered dam. Further evaluation would be required to understand the effects on the adjacent land area and the need to maintain certain slopes along the abutments. This option provides continued benefits of water supply, fire suppression, recreation, and ecosystem benefits as compared to the full decommissioning.

This alternative would significantly alter project operations. Reduction of water storage in Lake Pillsbury would transition the project to a run-of-river operation, ending typical water releases and diversions from stored supplies. Despite the operational changes, the installation of an upstream fishway - along with downstream fish passage over the lowered spillway - would enhance fish migration conditions at the site.

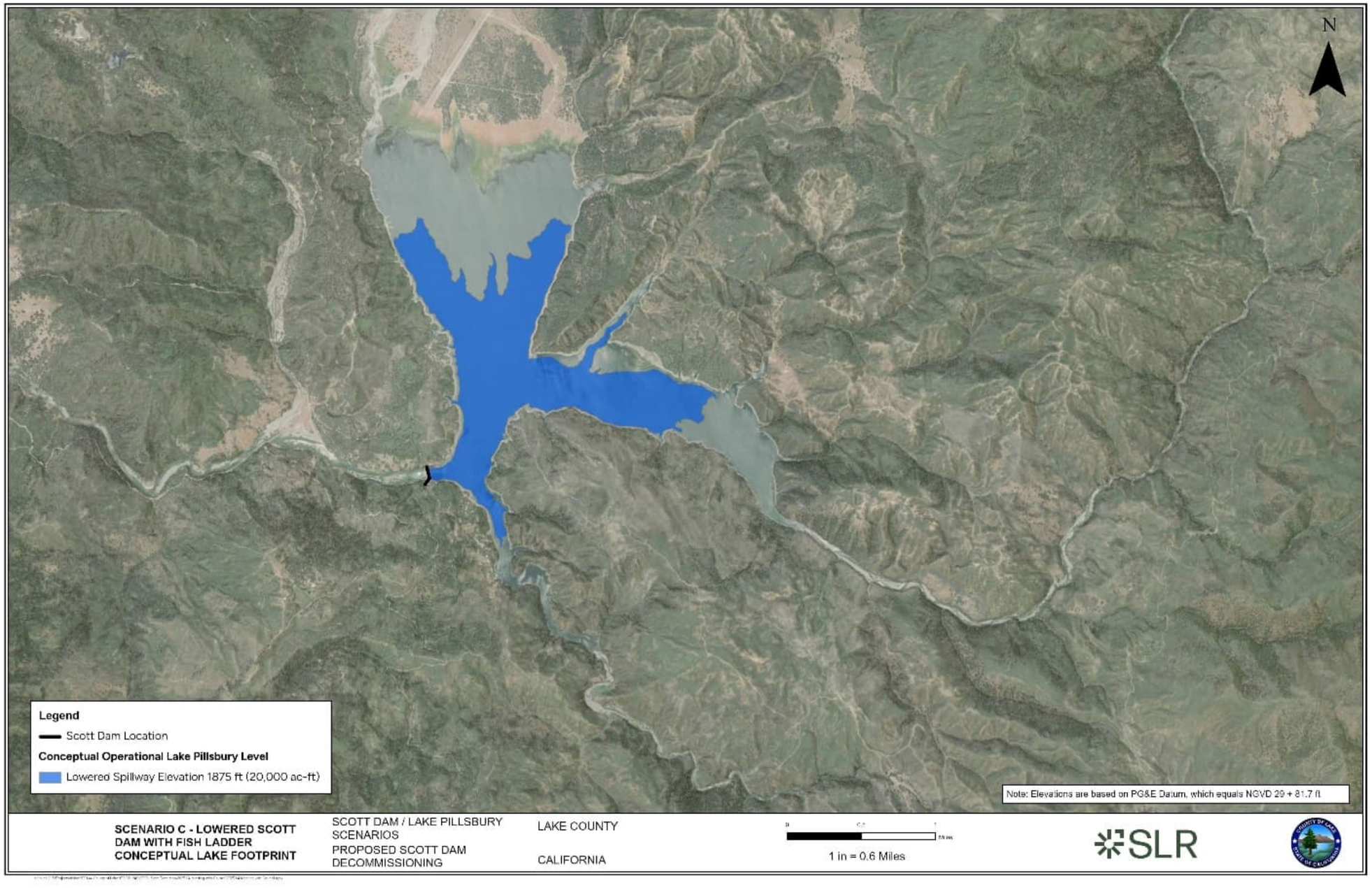


**Figure C3-6: Scenario C lowered Scott Dam with Fish Ladder: Conceptual Lake Pillsbury Footprint (Figure A7-16 from Appendix A).**

## C.3.5.2 Evaluation Summary

### C.3.5.2.1 Proposed Work for Fish Pathway Improvements

At Scott Dam, the existing spillway would be lowered to reduce the hydraulic drop, facilitating direct downstream passage of juvenile fish over the spillway and allowing for a lower height fishway structure. Reservoir operations would transition to run-of-river mode, allowing natural river flows to pass over the spillway or through the fishway. A lower fish ladder, similar to the Mead and Hunt modified conventional fishway (2018) consisting of a pool and weir fishway on the southern abutment of the dam would provide volitional fish passage. The majority of pools would be located downstream of the dam, and the exit pools upstream of the dam, with the two sections connected by a 48-inch diameter tunnel bored through the dam (Figure C3-5).

### C.3.5.2.2 Fish Passage and Instream Flows

The lowered fish ladder could be designed to operate over the range of wet season flows modelled in the hydrological assessment for the lowered dam scenario, summarized in Table C3-3. However, like Scenario B, the fishway exit must be designed to operate over the full range of reservoir elevations. This would be achieved through the same controllable gate mechanism at the fishway exit. The exit pool closest to the lake level would be opened to allow access to migrating salmonids. The lowered dam allows operation over a narrower range of water levels, but still requires a complex, mechanical fishway that would increase the capital and maintenance costs of the system. Downstream migration of juvenile fish would be possible over the crest of the lowered spillway.

**Table C3-3: Lowered Scott Dam with Fish Ladder – Statistics for Projected Storage within Lake Pillsbury and Projected Outflow Downstream of Scott Dam during the Wet Season (WY 1957-2024)**

| Statistic | Wet Season (December – April) | | |
|---|---|---|---|
| | Average Daily Storage (acre-ft) | PG&E Elevation (feet) | Average Daily Outflow (cfs) |
| Minimum | 5,420 | 1850.7 | 20 |
| Average | 18,760 | 1872.7 | 1,116 |
| Maximum | 28,460 | 1882.2 | 47,470 |

### C.3.5.2.3 Ecosystem

This scenario is expected to have relatively minimal environmental impact during construction compared to other dam removal options. However, excavation and blasting would be required on the left (south) abutment of the dam. Post-construction, the fishway would ultimately provide improvements to fish migration compared to current operational conditions.

### C.3.5.2.4 Advantages

- Would provide upstream and downstream fish passage.
- Potential increase in resident/anadromous fish production.
- Restoration of historical ecosystem function and dynamics.

- Potential improvement to water quality and temperature due to reduction of reservoir.

#### C.3.5.2.5 Disadvantages

- Existing biological communities and habitat conditions will be impacted in the short term from the change in lake footprint and depth.
- Requires extensive mechanical systems at the fishway exit to accommodate the reservoir fluctuation or the reservoir level has to be maintained within a set range to support operation.
- May not be fully effective during summer periods due to change in water temperature due to the surface draw of the fishway and lower level release to the river.
- Dam height and resulting length of the fishway may limit fish passage for specific fish species or during warmer water periods.

**Figure C3-7: Channel migration and downcutting and reduced reservoir footprint area at the head of Lake Pillsbury during the 2014 drought. Lowering the crest of Scott Dam will result in a permanently smaller reservoir footprint (Stillwater, 2021).**

## C3.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

### C.3.6.1 General Description

This scenario involves the full decommissioning and removal of Scott Dam, including the outflow structures, valve house, and associated infrastructure, consistent with the Rapid Dam Removal approach outlined in Scenario A1. A new 3,100-foot long earthen dam would be established in the Salmon/Smokehouse and Habematolel Creek watershed areas to maintain a smaller reservoir. Salmon Creek, Smokehouse Creek, and Habematolel Creek would be the primary tributaries flowing into the reduced reservoir. A conceptual rendering of the Salmon Creek reservoir, superimposed on imagery from the Klamath River upstream of the Iron Gate Dam

removal is provided in Figure C3-9**Error! Reference source not found.**, as an analog for visual comparison. The concrete material generated from the Scott Dam removal would be reused as a portion of construction materials for this new construction, with additional engineering to minimize seepage aspects. In addition to the reuse of material from Scott Dam removal, residual sediments within the Lake Pillsbury footprint can also be reused for the new impoundment construction if suitable. Low-permeability clay/silt sediments can be used in the core of the dam, and gravelly sediments can be used in the embankment/slopes of the new dam.

This option will support continued use of recreation facilities, ecosystem benefits and fire suppression. Similarly to Scenarios A1 and A2, this scenario would eliminate the existing fish barrier at Scott Dam and return the Eel River and Rice Fork to a free-flowing river system with unimpeded upstream and downstream fish passage. However, the construction of a new dam would prevent fish passage to Habematolel, Salmon and Smokehouse Creeks. This would result in 86% of the total flows toward Scott Dam being unimpeded, with the remaining 14% of flows blocked by the proposed earthen dam (Figure C3-8).

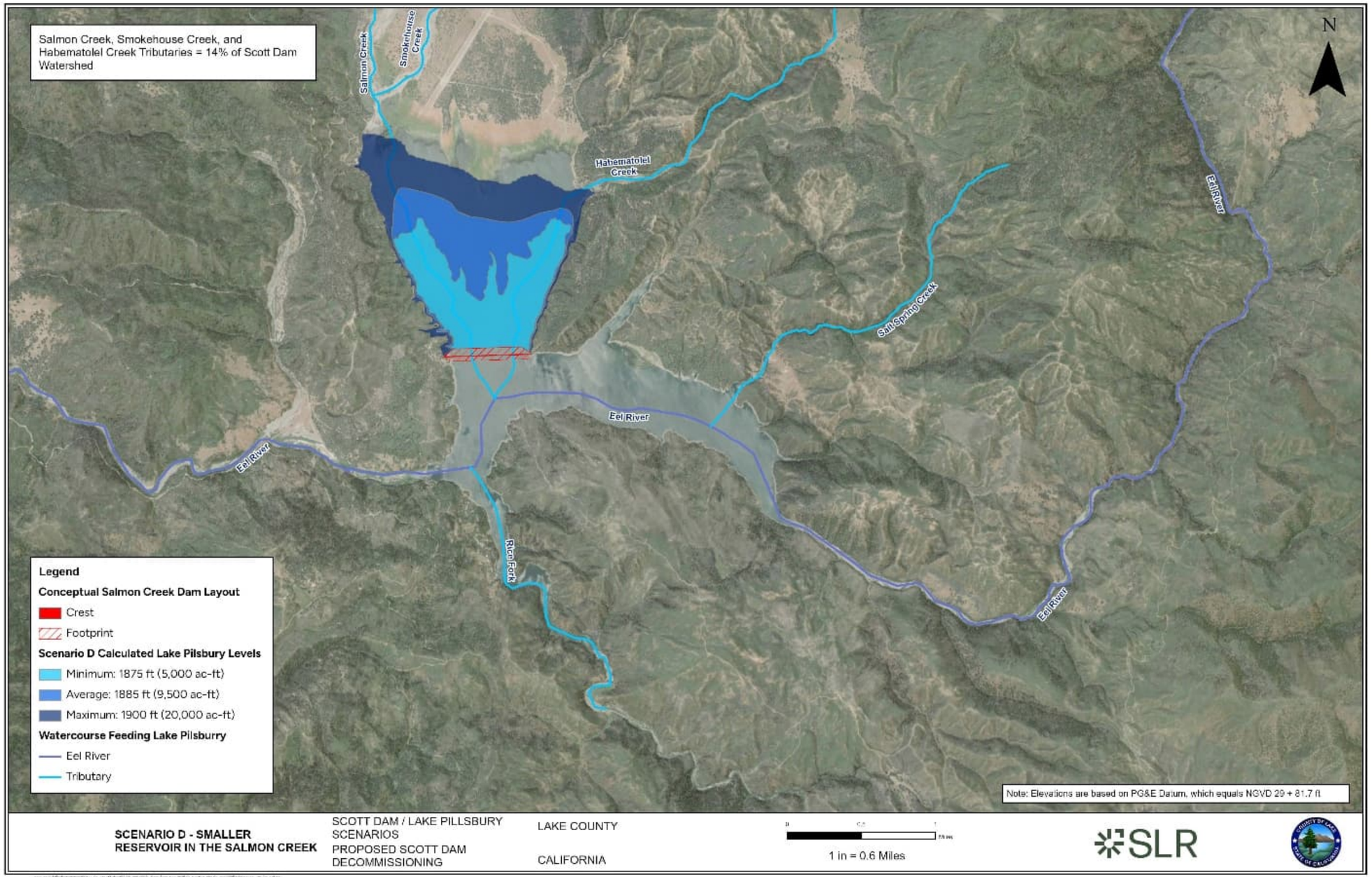


**Figure C3-8: Scenario D smaller reservoir in the Salmon Creek watershed: projected minimum, average, and maximum Lake Pillsbury WSE levels (Figure A7-19 from Appendix A).**

**Figure C3-9: Conceptual Salmon Creek Reservoir mock-up, using Klamath River imagery as an analog (Photo credit: *Shane Anderson/Swiftwater Films, 2024)*.**

## C.3.6.2 Evaluation Summary

### C.3.6.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures would allow fish passage in the upper Eel River. This scenario considers construction of a new earthen dam to retain flows from the Salmon/Smokehouse and Habematolel Creek watershed, no fish passage is proposed for the new earthen dam.

### C.3.6.2.2 Fish Passage

Similar to Scenario A1, this option would eliminate the Scott Dam as an existing fish barrier and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage through the former Lake Pillsbury would occur during the fall season after the final dam removal. No fish passage is proposed for the new earthen dam, nevertheless two thirds of the watershed will allow natural fish passage due to the removal of the Scott Dam.

### C.3.6.2.3 Instream Flows

Instream flows in the Eel River for Scenario D during the wet season fish migration period are summarized in Table C3-4. The average monthly wet season flows range from 1,327 cfs in February to 684 cfs in April, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions. The 14% reduction in contributing drainage from the blocked tributaries is not expected to negatively impact fish passage in the mainstem of the Eel River. Unimpaired flows during the migratory wet season remain well above the recommended floor of 100 cfs.

**Table C3-4: Scenario D Wet Season Monthly Average Statistical Unimpaired Flows within the Eel River Immediately Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 684 |
| Median | 1,070 |
| Average | 1,051 |
| Maximum | 1,327 |

#### C.3.6.2.4 Channel Restoration and Sediment Management

At this stage it is assumed that channel restoration and sediment management in the Lake Pillsbury reservoir footprint for Scenario D will mostly be consistent with PG&E's Final Draft Surrender Application (PG&E, 2025). PG&E will develop a passive Sediment/Channel Monitoring Response and Restoration Plan - no detailed channel or floodplain restoration plans have been submitted to date. However, extensive sediment management is expected to be required in the footprint of the proposed Salmon Creek dam to provide a stable foundation capable of supporting the dams weight and the reservoir's water pressure. Once the reservoir is drained, impounded sediment will be excavated and removed into the overbank or an offsite location. Depending on the existing sediments encountered on site, imported material may be required to prevent settlement, ensure uniform bearing capacity and minimize seepage and erosion.

#### C.3.6.2.5 Ecosystem

The primary environmental impact of the rapid dam removal alternative would be the release of 12 million cubic yards of accumulated sediment from behind Scott Dam. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species.

The majority of clay and silt sediments are expected to be transported to the coast over time, however extremely high suspended sediment load in the days following the sediment release is expected to result in lethal effects on aquatic species inhabiting the mainstem Eel River. Significant volumes of coarser sand are expected to deposit in habitat features from Scott Dam to approximately the Middle Fork Eel River confluence. Sand deposition would diminish aquatic habitat by burying pools and coarse riffle habitat until the river can flush accumulated sediment and establish a new geomorphic equilibrium. Flushing of accumulated sediment is expected to take a number of years and depends on local hydrology and storm magnitude in the season following removal.

An additional impact is the potential release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River downstream.

In addition to impacts resulting from the removal of Scott Dam, Scenario D involves the construction of a 3,100 ft long earthen dam in the Salmon Creek watershed. Construction of the dam is anticipated to have adverse effects on resident fish populations; however, it is noted that removal of Scott Dam, the draining of Lake Pillsbury and the associated impacts to the resident fish population will preclude any new dam construction.

#### C.3.6.2.6 Advantages

- A portion of the existing biological communities and habitat conditions would remain unchanged or undergo short term changes while the new dam is being constructed.
- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat within the Eel River and Rice Fork.
- Restores historic river ecosystem functions and natural hydrologic dynamics within the Eel River and Rice Fork.
- Over time, parts of the Eel River ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.6.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal.
- Requires extensive engineering and construction of new dam.

## C3.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

### C.3.7.1 General Description

Scenario E presents an enhanced version of proposed restoration efforts as part of Scenario A2, but with specific mitigation added to enhance fish passage while maintaining fire suppression and ecosystem benefits. Following the Scott Dam removal, active sediment removal from the bed of Lake Pillsbury along the restored Eel River alignment would be required to construct the river channels. Restoration would include selected stabilization and plantings. Once the sediments have been stabilized and the river systems have reached quasi-equilibrium[23], the off-channel ponds would be constructed.

[23] Quasi-equilibrium in this case refers to the ability of a river to maintain its dimension, pattern and profile without either aggrading or degrading over time.

Two potential pond locations have been identified as shown in Figure C3-10 based upon the 1922 historic pre-dam topography and bathymetry (McBain Associates, 2018). The 1922 topography and bathymetry was georeferenced to the Lake County hillshade to predict how the site will look after restoration efforts post dam removal. The pond locations were identified based on the drainage and assumed floodplain adjacent to the watercourses which would maximize function and water supply. Pond A would be located near the confluence of Eel River and the Salmon and Habematolel Creeks with an approximate footprint for pond creation area of 57.5 acres. Pond B would be further upstream the Eel River with an approximate footprint area for pond creation of 41.4 acres. Table C3-5 summarizes the potential footprints and volumes of Ponds A and B assuming a 10-foot normal depth. The larger pond near the confluence would provide a more reliable water supply as the Habematolel Creek would contribute the entirety of its flow to the pond. The Salmon Creek and Eel River would contribute flow passively from lateral flow through the banks and the percent contribution would be dependent upon the design. The conceptual design would function similarly to the example provided in **Error! Reference source not found.**, where the banks would be developed for high permeability and support a sustainable and resilient system.

**Table C3-5: Potential Pond Footprint and Storage Volume of Ponds A and B under Scenario E**

| Pond | Assumed Normal Depth (ft) | Potential Pond Footprint (sq ft) [ac] | Potential Storage Volume (cu ft) [ac-ft] | Potential Storage Volume {Million gallons} |
|---|---|---|---|---|
| A | 10 ft | 2,505,000 [57.5] | 21,510,000 [494] | 160 |
| B | 10 ft | 1,803,000 [41.4] | 16,088,000 [369] | 120 |

## C.3.7.2 Evaluation Summary

### C.3.7.2.1 Proposed Work for Fish Pathway Improvements

Complete removal of Scott Dam and associated structures with additional active sediment management, channel restoration, off-channel pond excavation and revegetation in the reservoir footprint.

### C.3.7.2.2 Fish Passage

Similar to Scenarios A1, A2 and D, this scenario would eliminate the existing fish barrier at Scott Dam and return the Eel River to a free-flowing river system with unimpeded upstream and downstream fish passage. The ponds could serve as spawning grounds and shelter habitat for fish and other aquatic species, and the free-flowing Eel River could provide easy transportation for fish through the watershed, from ponds to streams. The combination of preserving still-water habitats and allowing for stream-based habitats makes this scenario favorable for long term ecological function.

It is anticipated that biological design criteria for target fish species would be met and full upstream fish passage in the Eel River through the former Lake Pillsbury would occur during the fall season after the final dam removal.

### C.3.7.2.3 Instream Flows

Under this scenario, it is assumed that Pond A will be fed by all inflow from the Habematolel Creek and up to 10% of the Salmon and Eel River during normal wet and dry seasons. Pond B

assumes a contribution of an additional 10% of flows from the Eel River, meaning both ponds will take up to 10% of the Eel River flows each. This scenario will reduce the total flow contribution from the Eel River downstream of Scott Dam by up to 14% when compared to Scenarios A1 and A2 during normal wet and dry seasons.

Average monthly instream flows in the Eel River for Scenario E during the wet season fish migration period are summarized in Table C3-6. The average monthly wet season flows range from 1,324 cfs in February to 682 cfs in April, representing an increase in flow during the critical fish migration period and an improvement compared to the current operating conditions. The 14% reduction in flows resulting from the pond construction is not expected to negatively impact fish passage in the mainstem of the Eel River. Unimpaired flows during the migratory wet season remain well above the recommended floor of 100 cfs.

**Table C3-6: Dam Removal Scenario E Wet Season Monthly Average Statistical Unimpaired Flows Downstream of Scott Dam**

| Statistic | Wet Season (December through April) |
|---|---|
| | Average Monthly Flow (cfs) |
| Minimum | 682 |
| Median | 1,067 |
| Average | 1,048 |
| Maximum | 1,324 |

#### C.3.7.2.4 Channel Restoration and Sediment Management

This option calls for extensive sediment management and the excavation and re-establishment of the river channel through the reservoir reach. Excavated sediment removed to reconstruct the channel would be relocated to the overbank areas or removed off-site. The new river channel and overbank areas would then be stabilized through structural and vegetative techniques. Revegetation of the overbank floodplain in the former reservoir through an extensive planting plan would expedite site stabilization, reduce the volume of sediment transported downstream, and help mitigate some of the environmental impacts identified in Scenario A1. Once vegetation in the floodplain has established and the sediments have been stabilized, the off-channel ponds would be constructed. With the channel restoration, sediment management and off channel pond construction, Scenario E would have a beneficial effect on habitat for fish and aquatic habitat.

#### C.3.7.2.5 Ecosystem

Despite sediment management and mitigative measures, the short-term environmental impacts resulting from the initial sediment flush are expected to be similar to Scenario A1. During the initial 1-2 days of flushing of sediment from behind the dam, water quality parameters including temperature, dissolved oxygen (DO), and suspended sediment are expected to be negatively impacted and will have significant short-term adverse effects on fish and aquatic species. However, the long-term volume of mobilized sediment is expected to be less than Scenario A1 due to sediment management within the reservoir and revegetation efforts that will stabilize expose soils.

This scenario may also result in the release of non-native fish species from Lake Pillsbury into the Eel River when the dam is removed. Several of the non-native species in the reservoir including Sacramento pikeminnow and largemouth bass are known predators of native juvenile salmonids. Release of these fish is expected to have an adverse effect on aquatic species in the Eel River.

After the removal of Scott Dam and associated sediment flushing, environmental impacts resulting from construction of the off-channel ponds in the former reservoir are anticipated to be minor, assuming best management practices can be followed. Implementation of the ponds would have beneficial effects on critical habitat for target fish species and other aquatic organisms, including amphibians, benthic invertebrates and algae.

#### C.3.7.2.6 Advantages

- Enhances the potential for increased production of both resident and anadromous fish by restoring access to upstream habitat. This enhancement is increased from scenario A1 by adding ponds to support spawning and shelter for fish and aquatic species.
- Restores historic river ecosystem functions and natural hydrologic dynamics.
- Rapid re-establishment of vegetation in the former reservoir.
- Over time, parts of the ecosystem may revert to pre-dam conditions.
- Removal of the reservoir would allow for a more natural water temperature regime, benefiting cold-water species.
- Potential for long-term improvements in overall water quality.
- Could lead to reduced regulatory burdens and permitting obligations, including those under FERC.

#### C.3.7.2.7 Disadvantages

- Risk of Sacramento pikeminnow expanding downstream, potentially threatening native fish populations.
- Short term water quality impacts due to sediment plume.
- Need for long-term ecosystem monitoring and potential mitigation measures post-removal. This would be of a lesser degree with active sediment management and restoration efforts than scenario A1.

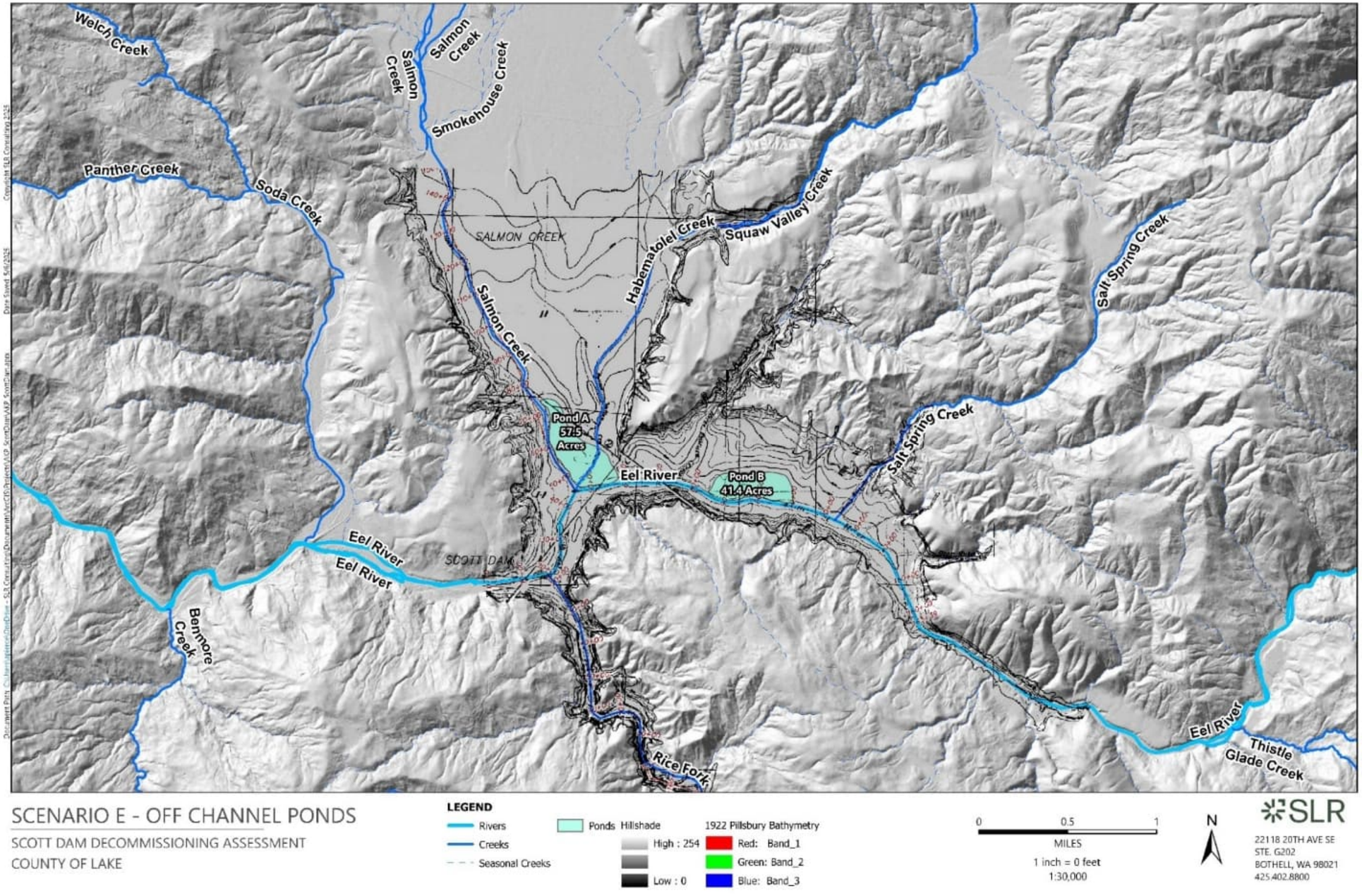


**Figure C3-10: Figure A7-24 from Appendix A: Scenario E - Dam Removal with Off-Channel Pond Restoration.**

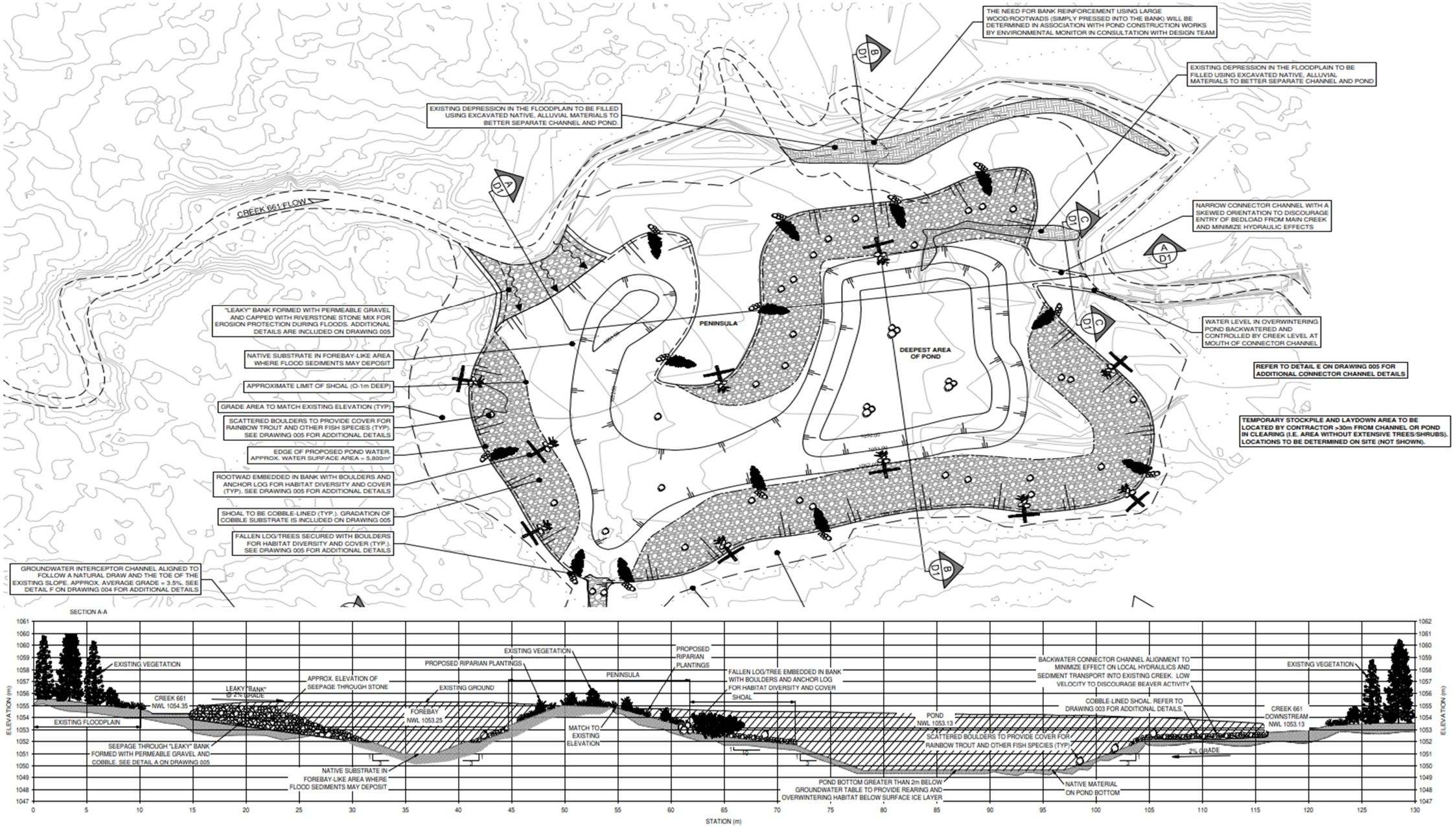


**Figure C3-11: Example design of an off-channel pond from a fish habitat offsetting project in central British Columbia, Canada (Palmer/SLR, 2023)**

# C4.0 Conclusions

The purpose of this fish passage evaluation completed by SLR is to provide feasibility-level technical information to support the County of Lake in its decision-making process and to fulfill the evaluation of mitigation strategies required under the Department of Water Resources (DWR) grant. Specifically, this Appendix outlines how potential decommissioning may affect fish passage in the Eel River system and identifies potential restoration and alternate mitigation opportunities within the Lake Pillsbury area for different scenarios. A summary of all scenarios is presented in Table C4-1.

This Appendix presents the initial qualitative analyses conducted to assess the feasibility of these alternatives, with a focus on fish passage in the Eel River. It is not intended to serve as an exhaustive or comprehensive evaluation of all possible project options but rather as an early-stage informational resource.

This analysis does not represent a final decision or commitment for any of the proposed scenarios. The options presented are based on conceptual designs and preliminary assessments of potential hydrologic and physical changes to the river systems.

The results present here provides an initial step in a broader, ongoing process. Further studies, analyses, and synthesis will be required, ultimately informing decisions by the County of Lake and other stakeholders.

**Table C4-1: Summary of Fish Passage Evaluation Criteria for Current and Proposed Scenarios**

| Scenario | Proposed Work | Fish Passage | Instream Flows | Ecosystem |
|---|---|---|---|---|
| Current Operational Conditions (WY 2019-2024) | Maintain current operational conditions. | No | Unchanged | No major impacts; current fish barrier remains |
| Scenario A1 – PG&E Proposed Removal | Rapid removal of Scott Dam and associated structures. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flows during fish migratory period. | Significant short term sediment release from reservoir;<br>Long term sediment impacts;<br>Release of non-native species from Lake Pillsbury. |
| Scenario A2 - Scenario A1 with Additional Mitigation | Rapid removal of Scott Dam and associated structures. Sediment management and revegetation in the reservoir footprint. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flows in Eel River during fish migratory period. | Significant short term sediment release from reservoir;<br>Long term sediment impacts partly mitigated by sediment management;<br>Release of non-native species from Lake Pillsbury. |
| Scenario B - Maintain the Current Operations of Scott Dam but Retrofit the Dam with Fish Ladder | Fish ladder on southern dam abutment. | Upstream passage through fish ladder. Uncertain downstream passage. | Mechanical gates at fishway exit to accommodate reservoir fluctuations. | Minor |
| Scenario C - Lowered Scott | Lowered dam and fish ladder on | Upstream passage through | Mechanical gates at fishway | Minor |

| Scenario | Proposed Work | Fish Passage | Instream Flows | Ecosystem |
|---|---|---|---|---|
| Dam with Fish Ladder | southern dam abutment. | fish ladder. Downstream passage over lowered spillway. | exit to accommodate reservoir fluctuations. | |
| Scenario D - Reduced Reservoir within Salmon Creek as Main Tributary | Rapid removal of Scott Dam and associated structures. Construction of a new earthen dam in the Salmon Creek watershed. | Unimpeded upstream and downstream fish migration through restored Eel River. | Improved. Increased flow in Eel River during fish migratory period | Significant short term sediment release from reservoir;<br>Long term sediment impacts;<br>Release of non-native species from Lake Pillsbury; short term impacts due to Salmon Creek dam construction. |
| Scenario E – Dam Removal with Off-Channel Pond Restoration | Rapid removal of Scott Dam and associated structures with additional sediment management, off-channel pond excavation and revegetation in the reservoir footprint. | Unimpeded upstream and downstream fish migration through restored Eel River. Additional fish and aquatic habitat provided in off-channel ponds. | Improved. Increased flow in Eel River during fish migratory period | Significant short term sediment release from reservoir;<br>Long term sediment impacts partly mitigated by sediment management and revegetation;<br>Release of non-native species from Lake Pillsbury. |

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# Appendix D – Climate Vulnerability

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

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SLR Project No.: 102.021249.00001

August 22, 2025

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# Appended Exhibits

Exhibit D-1 Primary and Secondary Climate Stressors

# Acronyms and Abbreviations

| | |
|---|---|
| APG | Adaptation Planning Guide |
| ARPA | American Rescue Plan Act |
| GHG | Greenhouse Gas |
| ERPA | Eel-Russian Project Authority |
| ICARP | Integrated Climate Adaptation and Resiliency Program |
| IPCC | Intergovernmental Panel on Climate Change |
| NERF | New Eel-Russian Facility |
| NIDIS | National Integrated Drought Information System |
| NOAA | National Oceanic and Atmospheric Administration’s |
| PG&E | Pacific Gas and Electric Company’s |
| RCPs | Representative Concentration Pathways |
| SPI | Standardized Precipitation Index |

# D1.0 Introduction

Pacific Gas and Electric (PG&E) proposes to decommission the Potter Valley Hydroelectric Project (PVP; Federal Energy Regulatory Commission [FERC] Proceeding No. 77). On July 25, 2025, PG&E submitted a Final Application for Surrender of License and Application for Non-Project Use of Project Lands to the FERC. PG&E's overall project goal is the decommissioning of the PVP as the hydropower generation is no longer economically viable. Scott Dam, which forms Lake Pillsbury in the Eel River Watershed, is one of two dams of the PVP.

The proposed PVP surrender, removal of Scott Dam and the subsequent draining of Lake Pillsbury present complex challenges for climate resilience in Lake County, California. While dam decommissioning is intended to support ecological restoration and fish passage, it also raises concerns about water supply reliability, wildfire risk, and community vulnerability under future climate conditions.

SLR International Corporation (SLR) conducted an evaluation of PG&E Final Application for Surrender of License to assess its alignment with the Lake County Climate Vulnerability Analysis and conducted a gap analysis to provide PG&E with suggestions on how to improve their climate vulnerability assessment (PG&E, 2025; County of Lake, 2025b). Additionally, in response to PG&E's proposed Scott Dam removal and as part of a broader technical evaluation, SLR developed seven (7) mitigation scenarios to explore alternative approaches to dam removal and restoration that reflect a broader range of operational, ecological, and economic outcomes for the benefit of the county and the local stakeholders.

The climate vulnerability analysis presented in this appendix evaluates both PG&E's Proposed Action and the developed alternatives for how well they respond to the climate stressors identified in the Lake County Climate Vulnerability Analysis (County of Lake, 2025b). These stressors include both primary drivers such as temperature and precipitation changes, and secondary impacts such as drought, wildfire, flooding, and public health hazards.

SLR's analysis uses the Historical Operational Conditions (WY 1922–2019) as the baseline scenario, representing long-term operations of Scott Dam prior to recent seismic restrictions. This baseline provides a reference point for evaluating how climate stressors have historically impacted Lake Pillsbury and the surrounding region.

The seven mitigation scenarios are:

1. Existing Operational Conditions Scenario covers current operational conditions during the WY period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft and operation of the needle valve to provide water supply downstream).
2. Scenario A1 follows PG&E's proposal per the PG&E Surrender Application dated January 2025, for the complete removal of Scott Dam.
3. Scenario A2 follows PG&E's proposed removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggestion for additional mitigation (including active sediment management).

4. Scenario B assuming Scott Dam to remain as operated currently (i.e. Spillway gates open at elevation 1,900 ft [PG&E datum[1]] and operation of the needle valve to provide water supply downstream) and retrofitted with a fish passage structure.
5. Scenario C assumes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device, and the inclusion of a fish ladder structure.
6. Scenario D assumes Scott Dam is removed and a new earthen dam is placed to restrict flows from only the Salmon and Habematolel Creek watersheds for a smaller reservoir footprint. The spillway is assumed to be at an elevation of 1,885 ft.
7. Scenario E assumes Scott Dam is removed and one or two small off-channel ponds are built along the Eel River mainstem allowing for water storage and natural fish passage.

SLR's evaluation summarizes how Scott Dam and Lake Pillsbury have historically buffered climate vulnerabilities and assesses how each mitigation scenario is projected to respond to future climate stressors, using the historic baseline as a reference. These findings are intended to support Lake County's decision-making and guide collaborative planning efforts to strengthen long-term climate resilience in the region.

## D1.1 Background on Scott Dam and Lake Pillsbury

Lake Pillsbury is a man-made reservoir located in Lake County, California, within the Mendocino National Forest. It was created in the early 1920s with the construction of Scott Dam, as part of the Potter Valley Hydroelectric Project. This project was designed to divert water from the Eel River to generate electricity in Potter Valley and support agricultural use in the Russian River watershed.

Over time, the lake and dam also became important for recreation and regional water supply. However, due to safety concerns and changing priorities, there are ongoing efforts to transition the system away from hydroelectric power generation and toward locally managed water uses that reflect evolving environmental, community, and regulatory goals. These efforts are occurring alongside increasing climate-related challenges, such as more variable precipitation, reduced snowpack, and heightened wildfire risk, which add complexity to long-term water and infrastructure planning.

In 2023, PG&E published an Initial Draft Surrender Application and Conceptual Decommissioning Plan for the PVP, which includes the planned decommissioning of Scott Dam. The proposed decommissioning is part of a broader initiative to restore the Eel River's ecosystem and improve fish passage (California Trout, 2023). However, this project raises concerns for vulnerable populations in the area and the impact it may have on future climate resiliency. While environmental and tribal organizations largely support the removal as a step toward ecological restoration and salmon recovery, local governments and residents have voiced concerns about the potential loss of water storage, increased wildfire risk, and reduced water reliability for agriculture and rural communities (County of Lake, 2025b; LaFever, 2025). These discussions unfolded in the context of intensifying climate stressors—such as prolonged

[1] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is 78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1,821.2 feet (NAVD88 datum).

drought, reduced snowpack, and more frequent wildfires—which heighten the need for coordinated adaptive water management strategies.

## D1.2 Appendix Organization

This Appendix is organized into several sections and organized to follow the approach taken to analyze the impacts of decommissioning Scott Dam on the County of Lake regarding climate vulnerability.

**D1.0 Introduction:** This section provides an overview of the appendix's purpose and scope and a brief background on the project.

**D2.0 Climate Vulnerability:** This section provides an overview of the climate adaptation resources and planning frameworks available in California. These resources were also used to assess PG&E's Proposed Action.

**D3.0 Historical Climatology:** This section outlines the historical changes in average surface temperature from 1922 to 2019 and important historical events and policies that occurred during this timeframe.

**D4.0 Lake County Climate Vulnerability Analysis:** This section summarizes the County of Lake's Climate Vulnerability Analysis released in April 2025 and how the climate stressors will be affected by the removal of Lake Pillsbury.

**D5.0 Cumulative Effects Identified by PG&E:** This section summarizes Volume II, Section 2.2.5 "Cumulative Effects" of the Final Application for Surrender of License by PG&E and evaluates the completeness of this analysis.

**D6.0 Baseline Scenario - Historical Operational Conditions from 1922-2019:** This section summarizes current conditions and the baseline scenario used to compare the mitigation scenarios.

**D7.0 Mitigation Scenarios – Climate Vulnerability Comparison:** This section compares each proposed mitigation scenario against the baseline with respect to climate vulnerability and adaptation capacity.

**D8.0 Conclusions:** This section summarizes the key findings and insights derived from the analysis.

**D9.0 References:** This section provides a list of sources cited in the report.

# D2.0 Climate Vulnerabiity

The following section provides an overview of the climate adaptation resources and planning frameworks available in California. These tools are used by local governments, including Lake County, to assess climate vulnerability and inform long-term resilience strategies. The summaries below offer context for the climate stressors and planning approaches referenced throughout this document. A comparison of these resources to PG&E's Proposed Action is provided in Section D5.5.

## D2.1 Climate Vulnerability Over

Climate change is defined by the Intergovernmental Panel on Climate Change (IPPC, 2023) as "a change in the state of the climate that can be identified (e.g., by using statistical tests) by changes in the mean and/or the variability of its properties and that persists for an extended period, typically decades or longer." Climate change may result from natural processes (e.g. volcanic activity) or anthropogenic influence such as greenhouse gas (GHG) emissions and land use changes (IPPC, 2023).

Recent observations show that climate change has led to increased frequency and severity of natural disasters, global warming, sea level rise, and loss of polar ice. Human activities have raised global surface temperatures by approximately 2 degrees Fahrenheit (1.1 degrees Celsius) above pre-industrial or 1850-1900 levels (IPCC, 2023).

In response, communities across California have adopted climate adaptation planning to identify vulnerabilities and prepare for future conditions. These efforts include climate action plans, GHG reduction strategies, hazard mitigation planning, and integrated water resource management, etc. (California Governor's Office of Emergency Services [Cal OES], 2020).

According to the U.S. Climate Resilience Toolkit, climate vulnerability is defined as the predisposition of assets to be adversely affected by hazards, which encompasses exposure, sensitivity, potential impacts and adaptive capacity (U.S. Climate Resilience Toolkit, n.d.).

California provides a robust suite of tools and programs to support climate-resilient decision making. These resources include Cal-Adapt, the Integrated Climate Adaptation and Resiliency Program (ICARP), and the California Adaptation Planning Guide (APG) (Cal-Adapt, 2025; ICARP, 2025; Cal OES, 2020).

## D2.2 California Adaptation Planning Guide (APG)

The California APG is a resource developed by the Governor's Office of Emergency Services (Cal OES) to help local governments to incorporate best practices and current science into their adaptation efforts. The APG outlines a four-phase planning process:

- **Phase 1 - Define, Explore, and Initiate**: Scoping the project, identifying climate effects, assets, and stakeholders.
- **Phase 2: Assess Vulnerability**: Evaluating exposure, sensitivity, and adaptive capacity of populations, infrastructure, and ecosystems. Where:
    - **Exposure Reduction**: The extent to which a scenario reduces the likelihood or severity of climate-related hazards (e.g., flooding, drought, extreme heat, wildfire).
    - **Sensitivity Reduction**: The degree to which a scenario reduces the vulnerability of people, ecosystems, and infrastructure to those hazards.

    - **Adaptive Capacity**: The ability of a system to adjust to climate impacts, respond to emergencies, and recover from disruptions through operational flexibility, infrastructure, or ecological resilience.
- **Phase 3: Define Adaptation Framework and Strategies**: Developing strategies to address identified vulnerabilities.
- **Phase 4: Implement, Monitor, Evaluate, and Adjust**: Executing the plan and refining it over time through feedback and learning.

The APG emphasizes inclusive community engagement throughout all phases and encourages cross sector collaboration to build resilience (Cal OES, 2020).

## D2.3 Integrated Climate Adaptation and Resiliency Program (ICARP)

ICARP is a California statewide initiative housed within the Governor's Office of Planning and Research. Its mission is to advance equitable, integrated climate adaptation and resilience solutions across local, regional, and state efforts. ICARP provides:

- **Policy and Programmatic Alignment**: Coordinating adaptation strategies across all levels of government.
- **Climate Services**: Providing technical assistance, decision-support tools (like Cal-Adapt and the Adaptation Clearinghouse), and guidance for local and regional adaptation planning.
- **Actionable Climate Science**: Supporting climate assessments and research to inform policy.
- **Direct Investments**: Administering climate resilience grants and aligning funding with federal programs

ICARP's Vulnerable Communities Platform helps visualize climate vulnerability across California and supports equity-focused planning (ICARP, 2025).

## D2.4 Cal-Adapt Climate Modeling

Cal-Adapt is California's primary platform for accessing localized climate projections. Developed by the California Energy Commission and UC Berkeley's Geospatial Innovation Facility, Cal-Adapt provides downscaled data that helps communities understand how climate change may affect their region. This tool is especially valuable for Lake County as it evaluates the long-term implications of PG&E's proposed decommissioning of Scott Dam and the resulting changes to Lake Pillsbury (Cal-Adapt, 2025).

Cal-Adapt uses Representative Concentration Pathways (RCPs) developed by the IPCC to model future climate conditions:

- RCP 4.5: A moderate-emissions scenario where global emissions peak around 2040.
- RCP 8.5: A high-emissions scenario with continued growth throughout the century.

Projections are provided for two future periods:

- Mid-century (2035–2064)
- End-of-century (2070–2099)

Table 1 summarizes projected changes in Lake County's annual average temperature and precipitation compared to the historical climate baseline (1961-1990) (Cal-Adapt, 2025).

**Table 1: Projected Long-Term (30-year) Annual Average Changes from the Historical Baseline**

| Climate Variable | Mid Century RCP 4.5 (2035–2064) | Mid Century RCP 8.5 (2035–2064) | End of Century RCP 4.5 (2070–2099) | End of Century RCP 8.5 (2070–2099) |
|---|---|---|---|---|
| Max Temperature (Fahrenheit) | +3.8° | +4.7° | +5.0° | +8.0° |
| Min Temperature (Fahrenheit) | +3.3° | +4.3° | +4.4° | +7.5° |
| Precipitation (inch/day) | +0.005 | +0.007 | +0.009 | +0.015 |

These projections suggest that Lake County will experience significantly higher temperatures, more extreme heat days, and greater variability in precipitation. Warmer winters are expected to reduce snowpack and shift runoff timing—factors that directly affect water availability and storage.

# D3.0 Historical Climatology

The National Oceanic and Atmospheric Administration's (NOAA) Global Average Surface Temperature Map was used to contextualize climate variations from 1922-2019 at Scott Dam. NOAA used the 1901-2000 global average temperature as their ±0°F benchmark (NOAA, n.d.). The County of Lake's Climate Vulnerability Study and Cal-Adapt both use 1961-1990 as their historical benchmark for average temperature (County of Lake, 2025b; Cal-Adapt, 2025).

Between the years 1922 and 1961, the global change in average surface temperature went from -0.41°F in 1922 to +0.14°F in 1961, indicating a slight warming trend and the beginnings of climate change (NOAA, 2025). At this time, the greenhouse effect was known, but evidence for a global warming trend had not yet been proven or widely studied. At this time, California suffered from droughts, floods, heat waves, and fires (Library of Congress, 1999), but not as frequently or intensely as seen today.

From 1961 to 1990, the global change in average surface temperature went from +0.14°F in 1961 to +0.81°F in 1990, showing a consistent warming trend. By this time, climate change was well-known and studied, and the scientific community agreed on climate change's existence and future impacts on both global and local scales. More severe weather events took place during this time compared to previous years, including the most severe drought-of-record (at the time) from 1976-1977, and several wildfires. California started to introduce new policies intended to prevent and mitigate fire damage, but experienced difficulties implementing these policies and managing burns successfully (Laney, 2017). California's growing population and urbanization during this time required better flood and drought management, which is still a problem in California today (Barbour, 2002).

From 1990 to 2019, the global change in average surface temperature went from +0.81°F to +1.78°F, showing a consistent and increasing warming trend. The severity of weather events and temperature extremes continued to grow at an accelerating rate. California experienced its two most devastating droughts in living memory, from 2012-2016 and 2020-2022, which were brought on by a persistent lack of rainfall and warmer temperatures. California also experienced dozens of severe wildfires, larger, more dangerous and harder to extinguish than previous fires. The rapid succession in which these events took place shows the present-day impacts of climate change. In response to these events, California passed ten significant climate change laws between 2002 and 2018, and additional regulations (UC Berkeley Law, n.d.). Lake County experienced some of the most devastating fires in California State history, which burned through 70% of Lake County (County of Lake, 2023, see also Appendix B). The 2012-2016 and 2020-2022 droughts, 2006 and 2017 floods also affected Lake County, stressing the water supplies, forests, habitats, and communities (County of Lake, 2025b).

# D4.0 Lake County Climate Vulnerability Analysis

Lake County has experienced a series of severe climate-related disasters over the past century, including wildfires that have burned through approximately 70% of the county, prolonged droughts that have stressed water supplies, and storms that have caused flooding, power outages, and community isolation. These events have resulted in loss of life, destruction of homes, reduced tourism, and economic hardship (County of Lake, 2025b).

In response to these impacts, and in alignment with California's climate planning mandates, the County initiated its Climate Vulnerability Analysis in 2023. This effort was part of a broader update to its General Plan and Safety Element and was supported by the American Rescue Plan Act (ARPA) funding and an Adaptation Planning Grant (County of Lake, June 2023).

The analysis was developed in alignment with the APG (see Section D2.3). It incorporates data from Cal-Adapt and other state supported climate tools to assess future climate scenarios and hazards exposure. The County's approach also aligns with principles from the ICARP, particularly in its emphasis on equity, cross sector planning, and resilience building across natural built systems.

In the Lake County Climate Vulnerability Analysis, the County defines a climate hazard as "an event or physical condition that has the potential to cause fatalities, injuries, physical damage, or other types of harm or loss." The vulnerability assessment examined a wide range of systems and populations

The most significant climate hazards identified in the analysis include:

- Wildfires, which pose the greatest threat to health, safety, and infrastructure
- Severe weather, including atmospheric rivers and winter storms that disrupt transportation and utilities
- Flooding, which is increasing in frequency and severity due to changing precipitation patterns
- Drought, which is intensifying because of rising temperatures and reduced snowpack

The most vulnerable assets include:

- Energy and communication infrastructure
- Residential structures
- Transportation systems
- Agricultural operations

To support the assessment of drought and flood conditions, the County utilized data from the National Integrated Drought Information System (NIDIS), which applies hydrologic models and historical climate records to classify wet and dry periods. A key tool used in this analysis is the Standardized Precipitation Index (SPI), which quantifies precipitation anomalies by expressing them in terms of standard deviations from the historical mean. Negative SPI values indicate below-average precipitation (drought), while positive values suggest above-average precipitation (wet conditions). SPI classifications range from D0 (abnormally dry) to D4 (exceptional drought), and W0 (abnormally wet) to W4 (exceptional wetness). This framework helped the County identify and contextualize historic and projected climate stressors, particularly those related to hydrologic extremes. Further SPI data and visualizations are provided in Appendix A.

## D4.1 Climate Stressors

To better understand the underlying drivers of these vulnerabilities, Lake County's Climate Vulnerability Analysis (2025b) identified a range of climate stressors that interact with local vulnerabilities and compound risks to both natural and human systems. These stressors are categorized as primary[2] and secondary[3]. Lake Pillsbury plays a critical role in buffering many of these stressors due to its water storage, seasonal flow regulation, and support for fire suppression, agriculture, and public health. More detailed information on these climate stressors and their impact on Lake County is provided in Exhibit E-1 (appended).

### D4.1.1 Primary Climate Stressors

Primary climate stressors are the direct physical changes in climate that drive cascading environmental and social impacts. In Lake County, the most significant primary stressors include rising temperatures and shifting precipitation patterns (County of Lake, 2025b). These changes influence water availability, wildfire risk, ecosystem health, and infrastructure resilience. Understanding how these stressors are projected to evolve is essential for evaluating the vulnerability of Lake Pillsbury and surrounding communities, especially in the context of dam removal and long-term climate adaptation planning.

#### D4.1.1.1 Changes in Temperature

Lake County is projected to experience significant increases in annual average temperatures through the end of the century. According to Cal-Adapt, as referenced in Table 1, maximum temperatures may rise to 8˚F under high emissions scenarios, showing a consistent upward trend in countywide annual average temperatures through the end of the century. These increases are expected to:

- Intensify extreme heat events
- Exacerbate drought conditions and wildfire risk
- Place additional stress on ecosystems, agriculture, and public health
- Contribute to greater variability in seasonal and year-to-year climate patterns

Lake Pilsbury acts as a thermal buffer, moderating local temperature extremes. Its presence provides cooling relief for people and wildlife during heat waves. Removal of the lake may increase exposure to extreme temperatures in the surrounding area.

#### D4.1.1.2 Changes in Precipitation Patterns

According to projections from Cal-Adapt, referenced in Table 1, Lake County is expected to experience fewer rainy days but more intense storm events, with greater year-to-year variability in precipitation totals. While annual precipitation may increase, it is expected to be concentrated in shorter, more intense periods. These changes heighten the risk of:

- Flooding and erosion

[2] Primary climate stressors are direct effects of the rising global temperatures due to climate change (County of Lake, 2025b)

[3] Secondary climate stressors are "events or physical conditions that can cause harm to people, property, and natural systems." (County of Lake, 2025b)

- Landslides and debris flow
- Water supply instability, especially during dry years
- Stress on stormwater infrastructure and natural systems

Lake Pillsbury provides flood storage and seasonal flow regulation. Without it, downstream areas may face increased flood risk during high-intensity storms and reduced water availability during dry periods.

### D4.1.2 Secondary Climate Stressors

Secondary climate stressors are the indirect consequences of primary climate changes, such as rising temperatures and altered precipitation patterns. These stressors emerge through interactions with ecological, social, and economic systems, and often compound existing vulnerabilities. In Lake County, secondary stressors include drought, flooding, wildfire, extreme temperatures, public health hazards, and disruptions to agriculture and infrastructure (County of Lake, 2025b). Lake Pillsbury plays a critical role in buffering many of these stressors. Understanding how these stressors manifest and intensify under future climate scenarios is essential for evaluating the long-term resilience of the region and the implications of dam removal.

#### D4.1.2.1 Agriculture, Forestry Pests and Disease

Tree mortality increased by approximately 333% from 2019 and 2024, driven by warmer temperatures and favorable conditions for pests and diseases. This trend is expected to worsen as climate change extends growing seasons and reduces winter die-off. Historically, pest-related damage surged during the 2012–2016 drought, affecting both crops and forests (County of Lake, 2025b).

Environmental consequences include declining agricultural productivity, forest dieback, and habitat loss, which in turn increase wildfire fuel loads and reduce ecosystem resilience. Vulnerable populations, particularly farmers, rural communities, and low-income households—may face income loss, food insecurity, and mental health stress.

Lake Pillsbury supports irrigation and agricultural resilience. Its removal could hinder pest management and disease control efforts, compounding risks for both natural systems and human livelihoods.

#### D4.1.2.2 Drought

Lake County has experienced multiple droughts, notably in 1928-1934, 1976-1977, 2012-2016 and 2020-2022. The 1976-1977 drought was previously considered the most severe, but recent droughts have been intensified by persistent warming and reduced snowpack. In 2021, the U.S. Drought Monitor National Integrated Drought Information System (NIDIS) classification ranged between extreme and exceptional in Lake County (NIDIS, 2025). See section A.3.2.2 (in Appendix A) for more information on the classification system and how droughts have been classified in the past.

Future droughts are expected to be more frequent, longer in duration, and harder to recover from. These conditions threaten water supply reliability, agricultural productivity, and habitat integrity. Vulnerable populations such as tribal communities, low-income households, and those dependent on agriculture face heightened risks.

Lake Pillsbury has historically acted as a drought buffer. Its removal would eliminate this capacity and increase vulnerability across multiple sectors.

#### D4.1.2.3 Extreme Temperatures

Heat events have become more frequent and intense in Lake County. Notable spikes occurred in June 2019 (record-breaking temperatures), July 2021 (100°F+ temperatures), September 2022 (prolonged heat wave), July 2023 (100°F+ temperatures), and July 2024, which was the hottest month ever recorded in California. Projections show extreme heat days increasing from 4 days to 12 by 2030, 20 days by midcentury, and 42 days by late century. Warm nights are expected to increase from 5 to 77 days annually by late century (County of Lake, 2025b).

These conditions stress ecosystems, reduce agricultural yields, and increase energy demand. Elderly individuals, outdoor workers, and those with chronic illnesses are at higher risk of heat-related illness.

Lake Pillsbury acts as a thermal buffer and provides cooling relief for people and wildlife. Without the lake, surrounding areas may experience more intense heat stress and reduced climate resilience.

#### D4.1.2.4 Flooding

Flooding has caused significant damage in Lake County. Floods on record occurred in 1937, 1982-1983, 1995, 1998, 2006, 2014, and 2017. The 1998 flood resulted in more than 60 homes being inundated and caused approximately $5 million in damages. The 2006 flood led to a federal disaster declaration, the 2014 flood forced a state of emergency, and the 2017 flood event forced evacuations and nearly $7 million in road damages. Climate change is expected to increase precipitation intensity, leading to more frequent and severe flooding (County of Lake, 2025b).

In addition to these impacts, floodplain boundaries are projected to expand in extent, and flood flows are expected to occur with greater frequency. These changes will increase the risk to infrastructure, homes, and natural systems, particularly in low-lying and downstream areas.

Flooding can damage infrastructure, erode landscapes, contaminate water supplies, and disrupt transportation. Residents in flood prone areas, especially those with limited mobility or financial resources, may face displacement and reduced access to services.

Lake Pillsbury provides flood storage and seasonal flow regulation. Its removal would heighten downstream flood risks during high-intensity storms and rapid snowmelt.

#### D4.1.2.5 Human Health Hazards

Lake County has faced public health threats from both infectious diseases and environmental hazards. Notable examples include the emergence of the West Nile virus (identified in 2004) and the COVID-19 pandemic, which resulted in 13,860 cases and 192 deaths from COVID-19 as of February 2024. Harmful algal blooms, especially in Clear Lake, are becoming more frequent due to warming and nutrient loading (County of Lake Health Services, n.d.).

Climate change is expected to increase the risk of vector-borne diseases such as West Nile virus and Lyme disease, as well as waterborne illnesses. These hazards strain emergency services and disproportionally affect children, the elderly, and immunocompromised individuals.

Lake Pillsbury supports water supply for health emergencies. Its loss could reduce water quality and increase disease risk, especially during droughts or contamination events.

#### D4.1.2.6 Landslides and Debris Flow

Landslides occurred in Lake County between 1993-1999 and again in 2017. These events were often triggered by intense rainfall following wildfires (County of Lake, 2025b). Future landslides are expected to become more frequent due to overlapping stressors such as drought followed by heavy precipitation.

Landslides can damage infrastructure and property, disrupt transportation, and degrade water quality. Residents in steep terrain or near burn scars may face displacement, injury, and isolation during emergencies.

Lake Pillsbury helps stabilize upstream sediment. Dam removal may increase erosion and slope instability, threatening both natural systems and human safety.

#### D4.1.2.7 Severe Weather

Lake County has experienced damaging winter storms and atmospheric river events. These include the 1986 winter storms, 1998 El Nino floods, the 2005-06 winter storms which caused $5 million in damage, the 2017 winter storms which created two separate federal disaster declarations, the 2019 atmospheric river events, and 2022-23 winter storms (County of Lake, 2025b). More extreme precipitation events are expected in the future with greater year-to-year variability.

Severe weather can cause flash flooding, power outages, and transportation disruptions. Rural and isolated communities may lack access to emergency services and recovery resources.

Lake Pillsbury buffers runoff and mitigates stormwater impacts. Without it, severe weather may lead to greater infrastructure strain and reduced community resilience.

#### D4.1.2.8 Wildfire and Smoke

Since 2015, Lake County has endured ten significant wildfires. These include the Rocky, Jerusalem, and Valley Fires in 2015, the Clayton Fire in 2016, the Sulphur Fire in 2017, the Pawnee and Mendocino Complex Fire in 2018, the August Complex Fire in 2020, the Cache Fire in 2021, and Boyles Fire in 2024. Projections show an estimated 35% increase in acres burned by midcentury and 50% increase in acres burned by late century (County of Lake, 2025b).

Wildfires destroy habitat, degrade air and water quality, and increase erosion. Smoke exposure poses serious health risks, especially for individuals with respiratory conditions, children, and low-income households.

Lake Pillsbury provides water for aerial firefighting and supports vegetation moisture retention. Its removal would reduce fire suppression capacity and increase smoke-related health risks.

# D5.0 Cumulative Effects Identified by PG&E

In Volume 2, Section 2.2.5 “Cumulative Effects” of the Final Application for Surrender of License, PG&E evaluates the cumulative effects of the proposed Scott Dam decommissioning and restoration plans. PG&E defines cumulative effect as “the effect on the environment that results from the incremental effect of the action when added to other past, present, and reasonably foreseeable future actions.” (PG&E, 2025)

## D5.1 Water Supply

The cumulative effect of the removal of Scott Dam and climate change would reduce the reliability of the water supply. PG&E has stated that unavoidable adverse effects on water temperatures and flow regimes in the Eel River and East Branch Russian River are expected, particularly during spring and summer, due to the loss of cold-water storage. This warming will contribute to vegetation loss in high-burned areas and increasing sediment loading in waterways. In conjunction with more frequent and sever wildfires, could result in the loss of habitat for sensitive species and cultural and Tribal resources.

The proposed decommissioning will increase fish passage to the upstream Eel River of Lake Pillsbury, potentially enhancing fish population resiliency and restoring access to over 100 miles of historic habitat. PG&E and the Eel-Russian Project Authority (ERPA)[4] also propose the construction of the New Eel-Russian Facility (NERF) to maintain diversion flows to the Russian River watershed following dam removal. However, there are not enough studies to support that the NERF will be a viable long-term substitute for Scott Dam’s storage and flow regulation functions. Historically, Scott Dam was constructed because the Cape Horn Dam and associated diversion infrastructure failed to reliably divert sufficient water to the Russian River, particularly during drought conditions.

Moreover, PG&E’s hydrologic and hydraulic analysis of unimpaired flows was developed using impaired flow data and did not incorporate precipitation data. This methodological limitation calls into question the validity of their assumptions regarding future water supply reliability. Without incorporating precipitation variability and climate projections, the analysis cannot accurately reflect the full range of potential outcomes—water supply could be better or worse than estimated, depending on future climate conditions.

PG&E acknowledges that the combination of climate change and the decommissioning of the Potter Valley Project would have adverse effects on the water supply and hydrology in the East Branch Russian River, which would be partially mitigated by the construction of the NERF. However, PG&E does not evaluate the potential adverse effects on the water supply, hydrology, or climate resilience for Lake County, despite Lake Pillsbury’s critical role in regional water storage and drought mitigation. This omission represents a significant gap in the cumulative effects analysis.

Additionally, PG&E does not present quantitative modeling of short- or long-term water supply impacts for Lake County, nor does it evaluate the potential loss of Lake Pillsbury as a drought buffer or its role in regional water security. No analysis is provided to estimate future water availability, seasonal variability, or groundwater recharge impacts under climate change scenarios.

[4] ERPA is a joint powers authority formed by a joint exercise of powers agreement between the County of Sonoma, Sonoma County Water Agency (Sonoma Water) and the Mendocino County Inland Water and Power Commission.

## D5.2 Storm Frequency and Intensity

PG&E acknowledges that climate change is expected to increase the frequency and intensity of storms. These changes could increase the magnitude of peak flows for flood events while decreasing their duration. In conjunction with the proposed removal of Scott Dam, PG&E predicts the restoration of unimpaired hydrology could lead to increased sediment mobilization, channel bar and bank deposits, erosion, and changes in riparian vegetation. PG&E concludes that these are natural geomorphic processes and that the cumulative effects of climate change and decommissioning are expected to be neutral in terms of geomorphic function.

However, PG&E does not assess how future high flows may affect the longitudinal profile of the Eel River and its tributaries, sediment supply and capacity, or the resilience of existing infrastructure. Nor does it evaluate the time required for the river to reach a new sediment transport balance, which could take decades to resolve.

While the processes described—such as sediment redistribution and channel adjustment—are indeed natural in a general geomorphic sense, PG&E does not demonstrate whether these processes are consistent with the historical geomorphic conditions of the Eel River prior to the construction of Scott Dam. The assumption of geomorphic neutrality lacks supporting evidence, as PG&E does not provide quantitative sediment modeling, historical channel morphology data, or reference conditions to validate this claim.

To ensure that restoration efforts support long-term ecological and geomorphic resilience, PG&E should evaluate the pre-dam geomorphic state of the Eel River and use that baseline to guide restoration targets. Without this context, the conclusion that geomorphic effects will be neutral is overly generalized and unsupported and may overlook potential long-term instability or unintended consequences in sediment dynamics, channel form, and floodplain function.

## D5.3 Droughts

PG&E acknowledges that climate change will exacerbate droughts, reduce water levels, and adversely affect aquatic habitat quality and recreation opportunities. As a cumulative effect, this will negatively impact recreation value, species and habitat integrity, and local economies. PG&E also notes that socioeconomic impacts may include effects on population stability, housing, recreation-based income, and economic opportunity, which could be further worsened by increased wildfire risk.

While PG&E states that with the dam removal will result in an increase in watershed resiliency, this term is not clearly defined. It is unclear whether PG&E is referring to hydrologic resilience to drought, ecological recovery, or some other system function. Given that Lake Pillsbury currently provides critical water storage capacity, it is not evident how its removal would improve drought resilience without alternative water management strategies. No quantitative analysis or modeling is provided to support this claim or to assess whether such resiliency would offset the project’s adverse effects.

## D5.4 Carbon Emissions

Using conservative estimates, implementation of the proposed Potter Valley Project decommissioning would emit a total of approximately 10,564.52 metric tons of carbon dioxide (PG&E, 2025). However, PG&E does not provide a comparison to the baseline emissions under existing conditions, nor does it evaluate the long-term carbon sequestration potential of restored ecosystems.

## D5.5 Comparison to Lake County's Climate Vulnerability Analysis

PG&E's assessment of climate vulnerability, as presented in its Final Application for Surrender of License, is notably less comprehensive than the Lake County Climate Vulnerability Analysis (2023). While PG&E identifies several climate-related stressors, its evaluation lacks the structure, depth, and geographic specificity necessary to support informed decision-making for Lake County.

Lake County's analysis follows the California APG, which outlines a four-step framework: exposure, sensitivity, adaptive capacity, and vulnerability. The County integrates Cal-Adapt climate modeling, identifies vulnerable populations and critical infrastructure, and evaluates cumulative and compounding risks across environmental, economic, and public health domains.

In contrast, PG&E's assessment:

- Does not follow a standardized vulnerability assessment framework.
- Omits key geographic areas, including Lake County, from its hydrologic and socioeconomic analysis.
- Fails to incorporate Cal-Adapt or other state-supported climate modeling tools.
- Provides limited evaluation of social vulnerability and equity impacts.
- Focuses primarily on fish passage and geomorphic restoration, without assessing broader resilience strategies.

To strengthen its climate vulnerability assessment, PG&E should:

1. Adopt a structured framework such as the APG to evaluate exposure, sensitivity, and adaptive capacity.
2. Use Cal-Adapt and other state-supported tools to model future climate conditions and stressors.
3. Include Lake County in its hydrologic and socioeconomic impact analysis, given Lake Pillsbury's role in regional water storage and drought mitigation.
4. Evaluate impacts on vulnerable populations, including Tribal communities, low-income residents, and those dependent on agriculture or recreation.
5. Quantify and compare greenhouse gas emissions under existing and proposed conditions, including long-term carbon sequestration potential.
6. Assess recovery timelines and resilience for geomorphic and ecological systems.
7. Explore alternative water storage or conservation strategies to offset the loss of reservoir capacity.
8. Engage with local stakeholders and Tribal communities to co-develop adaptation and resilience strategies.

Without these additions, PG&E's climate vulnerability assessment remains incomplete and insufficient to address the full scope of risks associated with dam removal and climate change in Lake County.

# D6.0 Baseline Scenario: Historical Operational Conditions from 1922-2019

From 1922 to 2019 the Scott Dam operations evolved in response to changing climate conditions and regulatory requirements. During this period, County of Lake experienced increasingly variable temperature and precipitation patterns, which influenced water availability, runoff timing, and seasonal extremes. The presence of Scott Dam and Lake Pillsbury provided a measure of control over these fluctuations, helping to stabilize water supply and mitigate hydrologic volatility (Cal-Adapt, 2025; County of Lake, 2023). This section contains additional information about how the past climate conditions affected Lake Pillsbury and Scott Dam, adding on to the past and present climate stressors identified by the Lake County Vulnerability Analysis (2023).

Operational practices supported regional resilience by regulating seasonal flows, maintaining minimum downstream releases for agriculture, and preserving water availability for fire suppression and recreation. These functions became more critical as climate variability intensified and as environmental regulations began to emphasize water reliability and public safety. Historically, these operations contributed to climate resilience by buffering drought conditions, mitigating flood risks, and supporting fire suppression efforts.

However, while Scott Dam buffered climate stressors by regulating water supply and mitigating hydrologic extremes, it also introduced long-term vulnerabilities through ecological tradeoffs. Sedimentation within the reservoir gradually reduced its storage capacity, weakening its ability to absorb flood waters and retain drought reserves. The absence of fish passage disrupted upstream aquatic ecosystems, limiting biodiversity and reducing the resilience of native species to climate-induced stressors such as warming temperatures and altered flow regimes. At the same time, cold-water releases from the dam have historically supported downstream aquatic habitats (Inland Water and Power Commission, 2025), particularly during warmer months when elevated stream temperatures can threaten fish populations. This dual role—providing thermal refuge downstream while constraining upstream connectivity—illustrates the complex interplay between engineered infrastructure and ecological resilience[5]. These factors must be considered together when evaluating the dam's contribution to long-term climate adaptation and vulnerability reduction.

## D6.1 Scott Dam's Response to Historical Climate Stressors

Scott Dam's operational history from 1922 to 2019 reflects evolving responses to climate stressors, particularly drought and flood conditions. While early operations prioritized hydroelectric generation, later decades saw a shift toward more climate-responsive management, informed by seasonal hydrology and community needs (PG&E, 2025).

A long-term series of outflow and storage data from 1922 to 2022 (Figure A7-2, Appendix A) illustrates how Scott Dam buffered hydrological extremes. The graph shows seasonal fluctuations in reservoir storage and outflow, with notable operational adjustments in 1957, 1986, and 2005. These changes likely reflect evolving regulatory requirements, infrastructure upgrades, and updated bathymetric surveys of Lake Pillsbury's storage potential.

[5] SLR did not identify specific studies for the Eel River, though these effects have been documented for the South Fork McKenzie River upstream and downstream of Cougar Reservoir in Oregon. https://www.usgs.gov/water-science-school/science/temperature-and-water

The figure reveals several key patterns in climate stressor response regarding flooding, drought, and season adaptation. These operational patterns align closely with the primary and secondary climate stressors identified in Lake County's Climate Vulnerability Analysis, including increased drought frequency, flood risk, and temperature extremes. Scott Dam's ability to buffer these stressors has historically reduced exposure and sensitivity while enhancing adaptive capacity—key components of the County's resilience framework. As upstream runoff became less predictable due to climate change, the need for real-time monitoring and flexible operations grew. Scott Dam's historical performance demonstrates how infrastructure can evolve to meet changing climate demands, even when initial design priorities were not climate focused. Its role in buffering both drought and flood extremes underscore its importance in regional climate resilience planning. For additional context and data, see NIDIS drought and flood classifications and SPI trends in Appendix A (Figure A3-6).

### D6.1.1 Flood Response

During wet years, such as the early 1980s and mid-1990s, the dam absorbed large inflows and regulated outflows to prevent downstream flooding. For example, in January 1983, peak outflows reached 15,400 cfs yet did not exceed the projected 2-year flood level, indicating effective attenuation. The dam's ability to store excess runoff and release it gradually helped protect infrastructure and agricultural lands downstream (see more details in Appendix A).

### D6.1.2 Drought Response

Extended periods of low storage, particularly during the 1976–1977, 2012–2016, and 2020–2022 droughts, show how the reservoir retained water to support regional needs. Despite reduced inflows, Scott Dam operations maintained minimum releases for agriculture, fire suppression, and ecological stability. These droughts were brought on by a lack of precipitation and hot temperatures. NIDIS classified them between severe and exceptional (NIDIS, 2025). They were marked by prolonged precipitation deficits and elevated temperatures. See Appendix A, Section A3.2.2 for more information on the classification system and how droughts have been classified in the past. The dam's capacity to buffer these conditions was critical for sustaining water availability (NIDIS, 2025).

### D6.1.3 Seasonal Adaptation

Figure A7-2 (in Appendix A) shows consistent seasonal cycling, with storage peaking in spring and early summer due to snowmelt and precipitation, followed by drawdown in late summer and fall. This pattern reflects adaptive management aligned with hydrologic conditions and community demand.

## D6.2 Scott Dam Observed Vulnerabilities to Climate Stressors

In addition to its buffering capacity, Scott Dam exhibited vulnerabilities to several climate stressors during its operational history, specifically wildfire and smoke, extreme temperatures, human health hazards, landslide and debris flow, and agriculture, pests, and disease (County of Lake, 2023).

### D6.2.1 Wildfire and Smoke

Lake Pillsbury has historically supported aerial firefighting operations, particularly during major wildfire events such as the Mendocino Complex Fire (2018) and August Complex Fire (2020). The reservoir reliably supported suppression efforts throughout its operational history, even

during drought years. In addition to fire suppression, wildfire events produced significant smoke exposure across Lake County, impacting public health and air quality. The reservoir's role in supporting emergency response indirectly contributed to mitigating smoke-related health risks by enabling faster containment of fires.

### D6.2.2 Extreme Temperatures

Lake Pillsbury acts as a thermal buffer, moderating local temperature extremes. During heat waves in July 2021 and July 2024 (the hottest month ever recorded in California), the lake provided cooling relief for residents and wildlife. Throughout its operational history, Scott Dam maintained adequate water levels to support regional needs, including fire suppression and recreation. While evaporation is a natural process in open reservoirs, there is no documented evidence that it significantly reduced available storage or stressed water supply systems.

### D6.2.3 Landslides and Debris Flow

Scott Dam is located adjacent to the Bartlett Springs Fault and is partially built on a known landslide feature, making it geologically vulnerable to seismic activity and slope instability. Historically, the dam helped reduce downstream debris flow risk by regulating peak flows and trapping sediment. However, its interruption of sediment transport contributed to sediment starvation in the Eel River downstream of the dam, which may have led to channel incision and bank erosion. These geomorphic changes can increase susceptibility to slope instability and landslides in steep terrain. According to the California Geological Survey's Landslide Susceptibility Map (Map Sheet 58), the area between Scott Dam and Van Arsdale Reservoir is classified as Class IX and X for deep-seated landslide susceptibility. Class IX indicates moderate to very steep slopes in weak rocks with high likelihood of landslides, while Class X represents mapped active landslides or very steep slopes in weak rocks—the highest level of susceptibility. These classifications highlight the elevated risk of deep-seated landslides in the region, particularly in areas affected by sediment starvation and channel degradation.

### D6.2.4 Human Health Hazards

During drought years, Scott Dam played a key role in maintaining water availability for sanitation, drinking water, and emergency services. Its ability to retain water during extended dry periods helped mitigate risks associated with waterborne diseases and contamination. Additionally, stagnant water in shallow reservoir margins may have contributed to mosquito habitat, increasing the risk of vector-borne diseases such as West Nile virus.

### D6.2.5 Agriculture, Pests, and Disease

Scott Dam provided reliable water supply for agricultural irrigation and forest health, especially during drought years. This support helped reduce pest outbreaks and tree mortality by maintaining vegetation vigor and resilience. During the 2012–2016 drought, pest-related damage increased across Lake County, but areas supported by irrigation from Lake Pillsbury were better buffered. The dam's role in sustaining water availability contributed to regional resistance against climate-driven pest pressures.

# D7.0 Mitigation Scenarios – Climate Vulnerability Comparison

This section compares each proposed mitigation scenario against the baseline (historic conditions) with respect to climate vulnerability and adaptation capacity. The analysis is based on the Lake County Climate Vulnerability Analysis (County of Lake, 2025b) and compares each scenario's ability to respond to both primary stressors, such as changes in temperature and precipitation, and secondary stressors, including drought, wildfire, flooding, and public health hazards. The goal of this comparison is to help clarify the differences in resiliency under future climate conditions. Each scenario is evaluated in terms of its ability to reduce exposure, sensitivity, and enhance adaptive capacity relative to historic operations at Scott Dam as defined in the California APG and in Section D2.2 of this appendix (Cal OES, 2020). For consistency, each scenario is described using the terms 'maintained,' 'reduced,' or 'increased' to indicate how its performance compares to the baseline scenario across the three dimensions with appropriate modifiers such as *slightly*, *moderately*, or *significantly* to describe the degree of change from the baseline condition, summarized in Table 2.

**Table 2: Definition of Terms Used for Comparison of Each Scenario to the Baseline/Historical Condition**

| Term | Exposure Reduction | Sensitivity Reduction | Adaptive Capacity | Overall Resilience |
|---|---|---|---|---|
| Eliminated | No ability to buffer climate stressors; system is fully exposed. | Vulnerability is maximized across systems; no protective functions remain. | All adaptive functions are lost; no operational flexibility or resilience mechanisms. | Climate resilience is eliminated. System cannot respond to or recover from climate stressors. |
| Slightly Lessened / Increased | Minor change in exposure to one or two stressors. | Minor change in vulnerability for select systems. | Some functions are retained, but reliability or flexibility is slightly reduced. | Climate resilience is slightly reduced or improved. System retains most functions but is more vulnerable to stressors. |
| Moderately Lessened / Increased | Noticeable change across multiple stressors. | Vulnerability changes across several systems. | Reduced ability to respond to climate stressors; flexibility or infrastructure is constrained. | Climate resilience is moderately reduced or improved. System can still adapt but with limitations. |
| Significantly Lessened / Increased | Major change in exposure; system is highly exposed. | Vulnerability is substantially different across most systems. | Adaptive functions are largely lost or enhanced; system lacks or gains resilience tools. | Climate resilience is significantly reduced or improved. System is either highly vulnerable or highly robust. |

## D7.1 Current Operational Conditions Scenario (WY 2019-Present)

Following seismic safety concerns in 2019, Scott Dam operations were modified to lower the reservoir elevation by 10 feet, reducing overall storage capacity. Despite this reduction, the dam continues to provide critical water supply, seasonal flow regulation, and emergency support functions (see more information in Appendix A). This scenario reflects the most recent operational configuration and serves as a transitional state between historical operations and future mitigation alternatives.

### D7.1.1 Exposure Reduction

Compared to the baseline, current operations project a **slightly lessened ability to reduce exposure** to climate stressors, though with diminished capacity:

- **Flooding**: The dam continues to regulate inflows, but reduced storage and permanently open spillway gates limit flood attenuation, increasing downstream exposure during high-intensity storms.
- **Drought**: Exposure to drought has increased due to lowered reservoir elevation and reduced reserve capacity, which limits the ability to buffer extended dry periods.
- **Extreme temperatures**: Lake Pillsbury still acts as a thermal buffer, offering cooling relief during heat waves.
- **Landslides and debris flow**: Without sediment management landslides and debris flows are at higher risk after the dam is removed. Risk increases further with extreme events without buffering of the dam.
- **Wildfire**: Water availability is slightly lowered but still provides firefighting support under most conditions.

### D7.1.2 Sensitivity Reduction

Current operations project a **slightly increased overall sensitivity or vulnerability** to climate stressors when compared to the baseline across multiple systems:

- **Fire suppression**: Water remains available for aerial firefighting, but reduced storage volume may limit long-term reliability during extended wildfire seasons.
- **Public health**: Water supply supports sanitation and emergency services, but diminished reserve capacity may reduce reliability during prolonged droughts or emergencies.
- **Agriculture**: Irrigation support continues, though with less flexibility and reliability during dry years due to lower reservoir levels.
- **Flood risk**: Sensitivity is increased due to reduced flood storage and passive discharge, which heighten vulnerability of downstream infrastructure.
- **Evaporation**: While reduced surface area may lessen total evaporation, shallower water may warm more quickly, potentially increasing localized evaporation rates during prolonged heat events.
- **Landslides and Debris Flow**: Landslide risk remains due to known geologic hazards adjacent to the dam and local wetting and drying of steep slopes.

### D7.1.3 Adaptive Capacity

This scenario projects a **moderately lessened** adaptive capacity with some key functions retained but long-term resilience constrained when compared to the baseline:

- **Operational flexibility**: The dam continues to support seasonal flow adjustments through the needle valve, but reduced storage limits the range and duration of response options.
- **Multi-use support**: The system still provides water for agriculture, recreation, fire suppression, and public health, though with less reliability due to reduced reservoir volume and reserve capacity.
- **Monitoring and maintenance**: Ongoing oversight enables adaptive management, but aging infrastructure and seismic constraints limit future adaptability and investment potential.

### D7.1.4 Comparison to Baseline

Relative to the historical baseline (1922–2019), current operations project a **slightly less resilient configuration** across climate vulnerability metrics:

- **Exposure reduction** is **lessened** for flooding and drought due to reduced reservoir elevation and passive spillway discharge, though buffering for extreme temperatures is maintained.
- **Sensitivity reduction** is **slightly lessened,** with increased vulnerability across multiple systems including fire suppression, public health, and agriculture due to reduced reliability.
- **Adaptive capacity** is **moderately lessened,** as operational flexibility is retained but long-term resilience is constrained by aging infrastructure and reduced storage volume.

This scenario reflects a transitional state between historical operations and future alternatives. While it continues to provide essential climate buffering functions, its overall resilience is diminished due to structural and operational limitations.

## D7.2 Scenario A1 – PG&E Proposed Removal

Scenario A1 reflects PG&E's proposed full removal of Scott Dam, as outlined in its July 2025 Application for Surrender of License. This action would eliminate Lake Pillsbury as a managed reservoir, restoring unimpaired flow to the Eel River and enhancing fish passage. While ecologically beneficial in some respects, the removal introduces significant climate vulnerability for Lake County due to the loss of water storage, flow regulation, and emergency response capacity.

### D7.2.1 Exposure Reduction

Scenario A1 exhibits a **significantly lessened exposure buffering**[6] compared to the baseline scenario (1922–2019), due to the complete removal of Lake Pillsbury and Scott Dam.

[6] Exposure buffering refers to the ability of a system, infrastructure, or natural feature to reduce the intensity, frequency, or impact of climate-related stressors—such as drought, flooding, wildfire, or extreme temperatures—on surrounding communities, ecosystems, and assets.

- **Flooding**: The absence of regulated outflows from the reservoir increases downstream flood exposure, particularly during high-intensity precipitation events.
- **Drought**: Without stored water reserves, the system loses its seasonal buffering capacity, resulting in heightened exposure to prolonged dry periods.
- **Extreme Temperatures**: The removal of the lake eliminates a thermal buffer, increasing exposure to heat stress in the surrounding environment.
- **Wildfire**: Water availability for aerial firefighting is eliminated, removing a critical emergency response resource and increasing exposure to wildfire impacts.
- **Landslides and Debris Flow**: The loss of sediment control and peak flow attenuation increases exposure to slope instability and debris movement, especially in steep or recently burned terrain.

### D7.2.2 Sensitivity Reduction

Scenario A1 projects **significantly increased sensitivity** when compared to the baseline:

- **Ecological benefits**: Restores access to over 100 miles of upstream habitat, improving resilience for native fish populations.
- **Public health and safety**: Increased sensitivity due to loss of water for sanitation, emergency services, and cooling during heat waves.
- **Infrastructure**: Greater sensitivity to flood damage, erosion, and sediment mobilization without managed flow control.
- **Wildfire**: Loss of aerial firefighting water source increases wildfire sensitivity.

### D7.2.3 Adaptive Capacity

This scenario **eliminates adaptive capacity** when compared to the baseline:

- **No operational control**: Eliminates ability to manage seasonal flows, respond to droughts, or mitigate flood risks.
- **No storage buffer**: Removes capacity to retain water for agriculture, health emergencies, or fire suppression.
- **Restoration uncertainty**: While ecological recovery is anticipated, PG&E does not provide a clear framework for long-term climate adaptation or resilience-building in Lake County.

### D7.2.4 Comparison to Baseline

Compared to historical operations (1922–2019), Scenario A1:

- **Exposure reduction** is **significantly lessened** across all major stressors.
- **Sensitivity reduction** is **significantly increased**, especially for human systems.
- **Adaptive capacity** is **eliminated**, with no retained operational flexibility or emergency support.

This scenario represents a **significantly lessened overall resilience** compared to the baseline and the lowest resilience among all alternatives, particularly for Lake County's vulnerable

populations and critical infrastructure. While it supports ecological restoration goals, it requires substantial mitigation to offset the loss of climate buffering functions historically provided by Scott Dam.

## D7.3 Scenario A2 – Scenario A1 with Additional Mitigation

Scenario A2 builds on PG&E's proposed full removal of Scott Dam (Scenario A1) by incorporating additional mitigation measures suggested by Lake County. These measures are not guaranteed but could be recommended to reduce the climate vulnerabilities introduced by dam removal. Potential actions include managing impounded sediment, stream and floodplain restoration, infrastructure upgrades, and wildfire risk reduction. While these strategies may improve ecological and geomorphic resilience, the loss of Lake Pillsbury's water storage and operational control continues to pose significant challenges for climate adaptation.

### D7.3.1 Exposure Reduction

Compared to the baseline (1922–2019), Scenario A2 results in **significantly lessened exposure reduction**:

- **Flooding**: Floodplain restoration may absorb runoff but lacks the scale and control of the historical reservoir.
- **Drought:** The absence of stored water reserves increases exposure to prolonged dry periods.
- **Extreme Temperatures:** Loss of the lake removes a thermal buffer, increasing exposure to heat stress.
- **Landslides and debris flow**: Sediment management and restoration efforts will reduce risk of landslides and debris flow resulting from the dam removal. However, without the buffering and flow regulation provided by Lake Pillsbury, exposure to landslide and debris flow hazards is increased during extreme events.
- **Wildfire**: Water availability for aerial firefighting is eliminated, removing a critical emergency response resource and increasing exposure to wildfire impacts. Fuel reduction strategies (e.g., prescribed burns, thinning) may help lower exposure to high-intensity fire events, but do not replace the reservoir's support for suppression operations.

### D7.3.2 Sensitivity Reduction

Scenario A2 exhibits a **moderately lessened sensitivity reduction** compared to the baseline scenario (1922–2019), with ecological improvements offset by increased vulnerability in human systems:

- **Ecosystems:** Restored riparian zones and sediment management may improve habitat stability and connectivity, resulting in slightly lessened sensitivity relative to the baseline.
- **Infrastructure**: Upgrades to river crossings and infrastructure will reduce vulnerability to erosion, sediment transport, and extreme flooding.
- **Public Health & Agriculture**: Sensitivity is moderately increased compared to the baseline due to reduced water reliability for sanitation, irrigation, and emergency services.

- **Landslides and Debris Flow**: Sediment management and restoration efforts will help reduce slope instability and post-fire erosion, which would lessen sensitivity compared to the baseline. However, without upstream flow regulation, vulnerability remains during extreme events

### D7.3.3 Adaptive Capacity

Scenario A2 exhibits a **moderately lessened adaptive capacity** compared to the baseline scenario (1922–2019). While the removal of Scott Dam eliminates operational control and water storage functions, the proposed mitigation measures introduce some compensatory mechanisms that support long-term resilience.

- **Operational Control**: No reservoir or flow regulation mechanisms are retained, limiting flexibility in managing droughts, floods, and wildfire emergencies.
- **Emergency Response**: Water availability for aerial firefighting is eliminated, reducing emergency response capacity. However, wildfire risk reduction strategies—such as prescribed burns and vegetation thinning—may improve preparedness and reduce the need for reactive suppression.
- **Infrastructure Resilience**: Upgrades to culverts, bridges, and roads enhance the system's ability to withstand variable flows and sediment transport, contributing to adaptive capacity in transportation and access.
- **Ecological Stability**: Sediment management and floodplain restoration support geomorphic stability and habitat connectivity, which may improve long-term ecological resilience under changing climate conditions.

These mitigation measures help offset some of the lost adaptive functions and provide a foundation for future climate adaptation planning.

### D7.3.4 Comparison to Baseline

Scenario A2 exhibits a **moderately lessened climate resilience** compared to the baseline scenario (1922–2019).

- **Exposure buffering** is **significantly lessened** due to the elimination of Lake Pillsbury's water storage and flow regulation, which previously mitigated flooding, drought, wildfire, and landslide risks.
- **Sensitivity** is **moderately lessened overall**, with **slight improvements** in ecological systems offset by **increased vulnerability** in infrastructure, public health, and landslide-prone areas.
- **Adaptive capacity** is **moderately lessened**, as operational flexibility is lost, but mitigation measures—such as **sediment management**, **infrastructure upgrades**, and **wildfire risk reduction**—provide partial support for long-term resilience.

## D7.4 Scenario B – Maintain the Current Operations of the Scott Dam but Retrofit the Dam with a Fish Ladder

Scenario B assumes Scott Dam remains in place and continues operating under current conditions (spillway gates open at elevation 1,900 ft and needle valve releases downstream), with the addition of a fish ladder to improve upstream aquatic connectivity. This scenario preserves Lake Pillsbury's water storage and flow regulation functions while addressing

ecological concerns related to fish passage. Compared to the baseline scenario (1922–2019), Scenario B slightly lessens most climate resilience functions.

### D7.4.1 Exposure Reduction

This scenario maintains the **slightly lessened exposure buffering** compared to baseline:

- **Flooding**: Modified flow schedules may enhance downstream flood attenuation, but slightly reduced reservoir capacity limits peak flow absorption during high-intensity storms.
- **Drought**: Seasonal water storage is retained, but reduced volume slightly limits drought buffering during prolonged dry periods.
- **Extreme Temperatures**: The lake continues to provide thermal relief, maintaining exposure buffering for heat stress.
- **Wildfire**: Water remains available for aerial firefighting, though reduced volume may constrain long-duration suppression efforts.
- **Landslides and Debris Flow**: Continued sediment retention and flow regulation help reduce exposure to slope instability and debris movement.

### D7.4.2 Sensitivity Reduction

Scenario B exhibits **slightly lessened sensitivity** compared to the baseline:

- **Ecosystems**: Improved flow and temperature regimes may enhance habitat stability and reduce ecological sensitivity.
- **Infrastructure**: Continued flow regulation and sediment retention reduce stress on downstream infrastructure.
- **Public Health & Agriculture**: Water reliability is generally maintained, though slightly reduced reserve capacity may affect reliability during extended droughts.
- **Landslides and Debris Flow**: Sensitivity remains low due to sediment stabilization and peak flow attenuation.

### D7.4.3 Adaptive Capacity

Scenario B exhibits a **slightly lessened adaptive capacity** compared to the baseline:

- **Operational Control**: Flow regulation is retained, but reduced reservoir volume limits flexibility during extreme events.
- **Emergency Response**: Water storage supports firefighting and drought response, though with slightly reduced reliability.
- **Infrastructure Resilience**: Continued investment in dam safety and sediment management supports long-term adaptability.
- **Ecological Stability**: Operational modifications and fish passage improvements enhance ecological resilience.

### D7.4.4 Comparison to Baseline

Scenario B maintains most climate resilience functions but exhibits **slightly lessened performance** compared to the baseline scenario (1922–2019):

- **Exposure buffering** is slightly lessened due to reduced reservoir volume.
- **Sensitivity** is slightly lessened, particularly for ecosystems and infrastructure.
- **Adaptive capacity** is slightly lessened due to constrained operational flexibility during extreme events.

## D7.5 Scenario C – Lowered Scott Dam with Fish Ladder

Scenario C assumes Scott Dam remains in place but is modified to a reduced height. The spillway elevation would be lowered from 1,900 ft to 1,875 ft (25 feet), and a fish ladder would be added to improve upstream aquatic connectivity. The needle valve would be utilized or modified to achieve low-level flow control and manage releases. This scenario reduces the size and storage capacity of Lake Pillsbury while improving fish passage. It introduces new vulnerabilities due to reduced water availability and flood buffering but retains some climate resilience functions.

### D7.5.1 Exposure Reduction

Exposure reduction is **moderately lessened** compared to the baseline (1922–2019). While the lowered dam retains some flood and drought buffering capacity, the reduced reservoir volume limits its effectiveness during extreme events.

- **Flooding**: Moderately lessened due to reduced peak flow absorption.
- **Drought**: Moderately lessened; seasonal storage is retained but insufficient for prolonged dry periods.
- **Extreme Temperatures**: Slightly lessened; thermal buffering continues but with reduced surface area.
- **Wildfire**: Slightly lessened; water remains available but may be insufficient for extended suppression efforts.
- **Landslides and Debris Flow**: Moderately lessened. Reduced reservoir volume may increase peak flow variability; elevating slope instability risks downstream and within the impoundment.
- **Evaporation**: Slightly lessened. Smaller surface area may reduce total evaporation, but shallower water may warm faster, increasing localized evaporation.
- **Sedimentation**: Moderately lessened. Continued sediment accumulation may reduce reservoir capacity and increase downstream sediment transport during high flows.

### D7.5.2 Sensitivity Reduction

Sensitivity reduction is **moderately lessened** compared to the baseline. While ecological sensitivity improves due to fish passage, human systems face increased vulnerability.

- **Aquatic ecosystems**: Improved due to fish ladder.

- **Public health and agriculture**: Slightly increased sensitivity due to reduced water reliability.
- **Infrastructure**: Moderately increased sensitivity to flood damage and sediment transport.
- **Landslides and Debris Flow:** Moderately increased sensitivity due to reduced reservoir volume and increased peak flow variability, which may elevate downstream slope instability and debris flow potential, even though sediment remains impounded.
- **Evaporation**: Slightly increased sensitivity to water loss during heat waves.
- **Sedimentation**: Moderately increased sensitivity as sediment buildup may reduce operational flexibility and affect water quality within the impoundment.

### D7.5.3 Adaptive Capacity

Adaptive capacity is **moderately lessened**. Operational flexibility is retained through flow control devices, but reduced reservoir volume limits long-term resilience.

- **Operational flexibility**: Flow control devices allow for seasonal and emergency water releases.
- **Multi-use support**: The reservoir continues to serve agriculture, recreation, fire suppression, and public health needs.
- **Ecological adaptation**: Fish passage improves species resilience to climate stressors.
- **Sediment Management**: No new sediment control measures are proposed, limiting long-term adaptability and consistency in capacity.

### D7.5.4 Comparison to Baseline

Compared to historical operations (1922–2019), Scenario C:

- **Exposure reduction** is **moderately lessened** due to reduced reservoir size and buffering capacity.
- **Sensitivity reduction** is **moderately lessened**, with ecological improvements offset by increased vulnerability in human systems.
- **Adaptive capacity** is **moderately lessened**, retaining some flexibility but constrained by reduced volume and lack of sediment management.

Scenario C represents a **moderately lessened** climate resilience scenario. It improves fish habitat while retaining some water management functions but introduces new vulnerabilities that would require careful monitoring and mitigation.

## D7.6 Scenario D – Reduced Reservoir within Salmon Creek as Main Tributary

Scenario D assumes Scott Dam is removed and replaced with a smaller earthen dam that restricts flows from only the Salmon and Habematolel Creek watersheds. The reservoir footprint would be significantly reduced, and the spillway elevation would be set at 1,885 ft. Flows from the Eel River and Rice Fork would no longer be stored and would return to natural, unimpaired conditions. This scenario includes sediment management and control efforts within the

impoundment but limited flow control. Compared to historical operations, this scenario reduces water storage availability by approximately 80% and introduces new vulnerabilities related to drought, flood control, and water reliability, while improving fish passage and ecological connectivity.

### D7.6.1 Exposure Reduction

Compared to historical operations, this scenario **significantly lessened** exposure reduction benefits:

- **Flooding**: The smaller reservoir has limited capacity to buffer high-intensity storm events or rapid snowmelt, increasing downstream flood risk.
- **Drought**: Water storage is reduced by approximately 80%, limiting the ability to provide water during dry periods.
- **Extreme temperatures**: The smaller reservoir offers less thermal buffering than Lake Pillsbury.
- **Wildfire**: Reduced water availability may limit aerial firefighting capacity and increase wildfire vulnerability.
- **Landslides and Debris Flow**: Significantly lessened. Minimal sediment retention and increased peak flows elevate exposure to slope instability and debris movement.
- **Evaporation**: Slightly lessened. Smaller surface area may reduce total evaporation, but vulnerability remains during heat waves.
- **Sedimentation**: Significantly lessened. Loss of upstream sediment control increases downstream sediment loads and erosion risk.

### D7.6.2 Sensitivity Reduction

Sensitivity reduction is **significantly lessened**. While ecological sensitivity improves, human systems face increased vulnerability.

- **Aquatic ecosystems**: Restoring unimpaired flows from the Eel River and Rice Fork improves fish passage and habitat connectivity.
- **Agriculture**: Unimpaired flows may support downstream irrigation, but reliability is lower during droughts.
- **Public health**: Less water is available for sanitation and emergency services, especially during dry years.
- **Infrastructure**: Reduced flood buffering increases sensitivity to damage from high flows.
- **Landslides and Debris Flow**: Significantly increased sensitivity due to lack of sediment buffering and flow regulation.
- **Groundwater**: Lower reservoir volume and surface area may reduce groundwater recharge, potentially affecting the regional water table.
- **Evaporation**: Smaller reservoirs are less vulnerable to evaporation but still require monitoring during heat waves.
- **Sedimentation**: Increased sensitivity for infrastructure and aquatic habitat due to uncontrolled sediment transport.

### D7.6.3 Adaptive Capacity

Adaptive capacity is **significantly lessened.** The scenario lacks operational flexibility and sediment management infrastructure.

- **Operational flexibility:** The proposed configuration includes minimal flow control, limiting responsiveness during extreme events.
- **Water supply:** The reduced reservoir size limits drought buffering and emergency response capacity.
- **Ecological adaptation:** Improved. Restored sediment transport and natural flow regimes support habitat complexity, spawning conditions, and long-term resilience for native fish species.
- **Infrastructure:** No specific upgrades are proposed to offset increased flood or sediment risks.
- **Sediment Management**: No sediment control infrastructure is proposed, reducing geomorphic resilience.

### D7.6.4 Comparison to Baseline

Relative to the 1922–2019 baseline:

- **Exposure reduction** is **significantly lessened** due to reduced reservoir size and loss of flow regulation.
- **Sensitivity reduction** is **significantly lessened**, with ecological gains offset by increased vulnerability in human systems.
- **Adaptive capacity** is **significantly lessened**, with minimal operational control and no sediment management.

Scenario D represents a **significantly lessened climate resilience scenario**, prioritizing ecological restoration but introducing major vulnerabilities for water reliability, flood control, and emergency response.

## D7.7 Scenario E – Dam Removal with Off-Channel Pond Restoration

Scenario E assumes full removal of Scott Dam and the creation of one or more small off-channel ponds along the Eel River mainstem. These ponds are designed to provide limited water storage and support natural fish passage, with specific functionality for aerial firefighting via helicopter access. The scenario also includes sediment management and restoration efforts to reestablish natural geomorphic processes. While this approach supports long-term ecological restoration and wetland habitat creation, it eliminates Lake Pillsbury's reservoir and flow regulation functions, significantly reducing climate resilience for human systems.

### D7.7.1 Exposure Reduction

Exposure reduction is **significantly lessened** due to the loss of large-scale water storage and flow control when compared to the baseline condition.

- **Flooding**: Significantly lessened. Without Lake Pillsbury, floodwaters are no longer stored. Off-channel ponds offer minimal buffering, though restored floodplains may absorb some runoff.

- **Drought**: Significantly lessened. The ponds have very limited storage capacity and cannot provide meaningful drought resilience.
- **Extreme Temperatures**: Slightly lessened. Ponds may offer localized cooling for wildlife but lack the scale to buffer regional heat stress.
- **Wildfire**: Moderately lessened. While overall water availability is reduced, the ponds are specifically designed to support aerial firefighting via helicopters, providing some emergency response capacity.
- **Landslides and Debris Flow**: Significantly lessened. No sediment retention or flow regulation increases exposure to slope instability and debris movement.
- **Evaporation**: Slightly lessened. Ponds may reduce total evaporation, but shallow water remains vulnerable to heat stress.
- **Sedimentation**: Moderately lessened. Sediment management is proposed to restore geomorphic function to a quasi-equilibrium state, which can improve sediment mobilization and reduce long-term instability across the reach. While this enhances ecological resilience, effectiveness may be limited during extreme flow events or debris flows.

### D7.7.2 Sensitivity Reduction

Relative to the baseline, sensitivity reduction is **moderately lessened.** Ecological systems benefit from restoration, but human systems remain vulnerable.

- **Habitat health**: Improved. Wetlands are resilient to flooding and support biodiversity. Sediment restoration helps stabilize the Eel River and its tributaries.
- **Public health**: Increased sensitivity. Water quality may improve through natural filtration, but supply is unreliable during emergencies. Standing water may increase mosquito habitat and algal growth.
- **Agriculture**: Increased sensitivity. Without a large reservoir, irrigation support is minimal and sensitive to seasonal variability.
- **Infrastructure**: Increased sensitivity. Reduced flood buffering and sediment control heighten vulnerability to damage.
- **Landslides and Debris Flow**: Significantly increased sensitivity due to loss of upstream sediment control and flow regulation.
- **Evaporation**: Slightly increased sensitivity to water loss and algal blooms.
- **Sedimentation**: Moderately lessened sensitivity. If restoration efforts are robust and sustained, sediment transport processes may stabilize, supporting habitat complexity and reducing vulnerability to channel degradation. However, sensitivity remains elevated during extreme events.

### D7.7.3 Adaptive Capacity

Adaptive capacity is **moderately lessened** when compared to the baseline condition. While operational control is lost, ecological restoration introduces long-term resilience benefits.

- **Water supply**: Significantly reduced. The ponds cannot support regional water needs during droughts or emergencies.

- **Flood control**: Absent. No operational control exists to manage flows or buffer runoff.
- **Ecological adaptation**: Improved. Wetland creation and sediment restoration support habitat resilience and fish passage.
- **Emergency response**: Moderately supported. Ponds provide targeted support for aerial firefighting, but broader emergency water needs remain unmet.
- **Sediment Management**: Proposed restoration aims to reestablish natural sediment dynamics, which can improve ecological resilience and support long-term geomorphic stability. However, the system lacks operational control to manage sediment transport during high-intensity events, limiting adaptive capacity under extreme conditions.

### D7.7.4 Comparison to Baseline

Relative to the 1922–2019 baseline:

- **Exposure reduction** is **significantly lessened** due to the elimination of Lake Pillsbury's buffering functions.
- **Sensitivity reduction** is **moderately lessened**, with ecological gains offset by increased vulnerability in human systems.
- **Adaptive capacity** is **moderately lessened**, with long-term ecological benefits but limited operational flexibility.

Scenario E represents a **significantly lessened climate resilience scenario**. It prioritizes ecological restoration and sediment transport recovery, which may enhance long-term ecosystem stability, but introduces substantial vulnerabilities for water supply, emergency response, and infrastructure protection.

# D8.0 Conclusions

Maintaining a safe and reliable water supply is essential for climate resilience in Lake County. The historical operations of Scott Dam and Lake Pillsbury provided strong protection against drought, flooding, wildfire, and extreme temperatures. As climate stressors intensify, the ability to store and manage water becomes increasingly critical for public health, agriculture, infrastructure, and emergency response.

This climate vulnerability analysis shows that scenarios which retain or enhance water storage and operational control offer the highest resilience. In contrast, scenarios involving full dam removal without replacement introduce significant vulnerabilities unless paired with robust mitigation strategies.

The table below summarizes how each scenario compares to the historical baseline in terms of **exposure reduction, sensitivity reduction, and adaptive capacity**.

**Table 3: Summary Matrix comparing The Historical Baseline Scenario to The Mitigation Scenarios**

| SCENARIO | EXPOSURE REDUCTION | SENSITIVITY REDUCTION | ADAPTIVE CAPACITY | Overall Rating |
|---|---|---|---|---|
| **Current Operations** | **Slightly lessened** – Reduced reservoir elevation limits flood and drought buffering | **Slightly lessened** – Increased vulnerability in fire suppression and agriculture | **Moderately lessened** – Flexibility retained but constrained by aging infrastructure | **Slightly Lessened** |
| **Scenario A1 – Full Dam Removal** | **Significantly lessened** – No water storage; system is fully exposed to climate stressors. | **Significantly increased** – loss of water supply, fire suppression, and infrastructure protection | **Eliminated** – no operational control or emergency water management | **Significantly lessened (lowest rating)** |
| **Scenario A2 – Dam Removal + Suggested Mitigation** | **Significantly lessened** – Loss of reservoir; mitigation improves ecological exposure. | **Moderately lessened** – ecological gains, but human systems remain vulnerable | **Moderately lessened** – suggested actions may help, but no guaranteed water management | **Moderately lessened** |
| **Scenario B – Current Operations + Fish Ladder** | **Slightly lessened** – similar to historical flood, drought, and heat buffering | **Slightly lessened** – adds fish passage but reduction in capacity | **Slightly lessened** – reduced reliability to support water supply needs | **Slightly lessened** |
| **Scenario C – Lowered Dam + Fish Ladder** | **Moderately lessened** – smaller reservoir limits flood and drought buffering | **Moderately lessened** – improves fish habitat, but reduces water reliability | **Moderately lessened** – retains control, but with diminished capacity | **Moderately lessened** |
| **Scenario D – Reduced Reservoir (Salmon Creek)** | **Significantly lessened** – small reservoir offers minimal buffering | **Significantly lessened** - Improved for fish, but increased vulnerability for agriculture and public health | **Significantly lessened** – limited flow control and reduced water supply | **Significantly lessened** |
| **Scenario E – Dam Removal + Off-Channel Ponds** | **Significantly lessened** – ponds offer limited buffering, but restored floodplain may absorb runoff | **Moderately lessened** – wetland habitat and sediment management enhance ecological resilience; human systems remain vulnerable | **Moderately lessened** – ecological restoration and sediment control improve long-term river stability, but water supply and emergency response remain limited | **Moderately lessened** |

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# Exhibit D-1 Primary and Secondary Climate Stressors

## Appendix D – Climate Vulnerability

Impact Analysis and Adaptation Strategy Evaluation Report

**County of Lake**

SLR Project No.: 102.021249.00001

August 22, 2025

SLR

## Exhibit D-1

**Table E-1 Primary and Secondary Climate Stressors**

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| **Primary Climate Stressors** | | | |
| Changes in Temperature | **Past**: Historical (1961 to 1990) annual average minimum temperature was 37.9°F countywide. The annual average maximum temperature was 67.7°F countywide.<br><br>**Present and Future**: Annual average minimum temperatures are projected to increase to 40.9°F by 2030, 42.3°F by midcentury (2035 to 2064), and 45.9°F by late century (2070 to 2099). Annual average maximum temperatures are projected to increase to 70.8°F by 2030, 72.4°F by midcentury, and 75.8°F by late century. | | |
| Changes in Precipitation Patterns | **Past**: Historical annual average precipitation was approximately 42 inches countywide.<br><br>**Present and Future**: Precipitation is projected to increase to 47 inches by midcentury and 51 inches by late century. Though overall rainfall will increase, it is likely to occur during fewer, intense storm events with longer dry periods between storms. | | |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| **Secondary Climate Stressors (Hazards)** | | | |
| Agriculture and Forestry Pests and Diseases | **Past**: Tree mortality increased from 3,000 dead trees in 2019 to over 1,000,00 by 2024. Sudden oak death was identified in southwestern portions of the county and Mediterranean Oak Borer detected in 2022 in Middletown moving northward into the Mendocino National Forest by 2024<br>**Present and Future**: Warmer temperatures and changing precipitation patterns will create more favorable conditions for agricultural and forestry pests and diseases. | Agricultural Productivity Decline<br>Habitat Loss<br>Power Delivery Disruption<br>Structure & Property Damage | Income Loss<br>Food & Water Insecurity<br>Mental & Behavioral Health |
| Drought | **Past**: Significant droughts in 1976–1977, 2014, and 2020–2021.<br>**Present and Future**: More frequent and severe droughts likely, with longer durations and shorter recovery periods. County reached D3 level (Extreme Drought) in 2020 and fluctuated between D3 and D4 (Exceptional Drought) through 2021. | Water Supply/Quality Reduction<br>Agricultural Productivity Decline<br>Habitat Loss | Food & Water Insecurity<br>Income Loss<br>Mental & Behavioral Health<br>Public Health Risks |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| Extreme Temperatures | **Past**: Multiple heat events included June 2019 (record-breaking temperatures), July 2021 (100°F+ temperatures), September 2022 (prolonged heat wave), July 2023 (temperatures over 100°F) and July 2024, the hottest month ever recorded in California<br><br>**Present and Future**: Extreme heat days projected to increase from 4 days annually to 12 days by 2030, 20 days by midcentury, and 42 days by late century. Warm nights projected to increase from 5 to 77 days annually by late century. | Power Delivery Disruption<br> Agricultural Productivity Decline<br> Structure & Property Damage<br> Emergency Services Strain | Physical Injury<br> Public Health Risks<br> Mental & Behavioral Health |
| Flooding | **Past**:<br>• February 1998 (60+ homes flooded, $5 million in damages)<br>• January 2006 (federal disaster declaration)<br>• December 2014 (state of emergency)<br>• February 2017 (forced evacuations, $5 to 7 million in road damage)<br><br>**Present and Future**: More intense precipitation events likely to cause more frequent and severe flooding, particularly around Clear Lake and along waterways. | Structure & Property Damage<br> Transportation Disruption<br> Water Supply/Quality Reduction<br> Emergency Services Strain | Displacement<br> Income Loss<br> Access to Services Reduction<br> Physical Injury |



Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| <br>Human Health Hazards | **Past**: West Nile virus first detected in 2004; COVID-19 pandemic (2020–present) with 13,860 total cases and 192 deaths in Lake County as of February 2024.<br><br>**Present and Future**: Ongoing concerns with vector-borne diseases and harmful algal blooms in Clear Lake. Warmer temperatures expected to expand ranges and seasons for disease vectors. | <br>Emergency Services Strain<br> <br>Water Supply/Quality Reduction<br> <br>Agricultural Productivity Decline | <br>Physical Injury<br> <br>Public Health Risks<br> <br>Access to Services Reduction |
| <br>Landslides and Debris Flows | **Past**: Spring Valley Landslide (Cache Creek Slide) showed considerable movement between 1993 and 1999. Anderson Springs landslide (~4 acres) and Ettawa Springs landslide (~2 acres) discovered during 2017 winter storms.<br><br>**Present and Future**: More frequent landslides and debris flows likely due to increased wildfire activity followed by intense precipitation events. | <br>Structure & Property Damage<br> <br>Transportation Disruption<br> <br>Power Delivery Disruption<br> <br>Emergency Services Strain | <br>Displacement<br> <br>Physical Injury<br> <br>Access to Services Reduction<br> <br>Income Loss |

Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| <br>Severe weather | **Past:** Lake County has received 13 federal disaster declarations for severe winter storms since 1950. Notable events include winter storms in 1986, 1998 El Niño floods, 2005 to 2006 winter storms ($5 million in damages), 2017 winter storms (two separate federal disaster declarations), 2019 atmospheric river events, and 2022 to 2023 winter storms.<br><br>**Present and Future:** More extreme precipitation events are expected, with increased frequency and intensity of atmospheric rivers. Though damage directly associated with severe weather has historically been limited, secondary hazards like floods, landslides, and power outages will become more significant. | <br>Structure & Property Damage<br> <br>Power Delivery Disruption<br> <br>Transportation Disruption<br> <br>Emergency Services Strain | <br>Displacement<br> <br>Physical Injury<br> <br>Access to Services Reduction<br> <br>Mental & Behavioral Health |
| <br>Wildfires | **Past**:<br>• 2015 Rocky Fire and Jerusalem Fire (69,438 acres, 43 homes destroyed)<br>• 2015 Valley Fire (76,067 acres, 1,280 homes destroyed, 4 deaths)<br>• 2016 Clayton Fire (3,929 acres, 189 homes destroyed)<br>• 2017 Sulphur Fire (2,200 acres, 138 homes destroyed)<br>• 2018 Pawnee Fire (15,185 acres, 12 homes destroyed)<br>• 2018 Mendocino Complex (459,123 acres, 157 homes destroyed in Lake County)<br>• 2020 August Complex (1,032,648 acres)<br>• 2021 Cache Fire (83 acres, 100+ structures destroyed)<br>• 2024 Boyles Fire (81 acres, 33 structures destroyed) | <br>Structure & Property Damage<br> <br>Power Delivery Disruption<br> <br>Habitat Loss<br> <br>Emergency Services Strain | <br>Displacement<br> <br>Mental & Behavioral Health<br> <br>Physical Injury<br> <br>Income Loss |



Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

## Exhibit D-1

| Climate Stressor | Past, Present, Future | Impact on Built and Natural Environment | Impact on Vulnerable Populations |
|---|---|---|---|
| | **Present and Future**: Annual average acres burned projected to increase by 35 percent by midcentury and 50 percent by late century. More extreme fire behavior expected. | Transportation Disruption Water Supply/Quality Reduction | Public Health Risks Access to Services Reduction |

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Source:
County of Lake, 2025. Lake County Climate Vulnerability Analysis. April.5

# Appendix E – Permitting Process

## Impact Analysis and Adaptation Strategy Evaluation Report

### County of Lake

Prepared by:

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SLR Project No.: 102.021249.00001

August 22, 2025

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# Acronyms and Abbreviations

| | |
|---|---|
| CDFG | California Department of Fish and Game |
| CDFW | California Department of Fish and Wildlife |
| CEQA | California Environmental Quality Act |
| CESA | California Endangered Species Act |
| DSOD | California Division of Dam Safety |
| DWR | Department of Water Resources |
| EIR | Environmental Impact Report |
| ESA | Endangered Species Act |
| ERPA | Eel-Russian Project Authority |
| FERC | Federal Energy Regulatory Commission |
| ITP | Incidental Take Permit |
| MND | Mitigated Negative Declaration |
| ND | Negative Declaration |
| NEPA | National Environmental Policy Act |
| NERF | New Eel-Russian Facility |
| NFS | National Forest System |
| NHD | National Hydrology Dataset |
| NMFS | National Marine Fisheries Service |
| NOAA | National Oceanic and Atmospheric Administration |
| NOD | Notice of Determination |
| NOP | Notice of Preparation |
| PG&E | Pacific Gas and Electric |
| PVID | Potter Valley Irrigation District |
| RPA | Reasonable and Prudent Alternative |
| SCADA | Supervisory Control and Data Acquisition |
| SLR | SLR International Corporation |
| USFWS | United States Fish and Wildlife Service |
| USGS | United States Geological Survey |
| USFS | United States Forest Service |
| VAFS | Van Arsdale Fish Station |
| WSE | water surface elevation |
| WY | water year |

# E1.0 Introduction

SLR International Corporation (SLR) was contracted by the County of Lake to conduct an analysis of Pacific Gas and Electric's (PG&E's) proposed decommissioning of Scott Dam. SLR has prepared this appendix (Appendix E) to explain the regulatory process for the decommissioning of Scott Dam (Proposed Action) and alternative Scenarios developed by SLR that will follow the submittal of the final Surrender Application. PG&E plans to submit the final Surrender Application and Decommissioning Plan to the Federal Energy Regulatory Commission (FERC) by July 29, 2025. Following the filing and distribution of the Surrender Application FERC and PG&E will consult with Resource Agencies and Tribes to develop Plans and Measures.

A large volume of information has been prepared for the Surrender Application and will be further developed in the resource reports. This information will be used to prepare federal, state, and local permit applications. Existing Conditions Report previously prepared by SLR (SLR, 2025) have outlined the National Environmental Policy Act (NEPA) and California Environmental Quality Act (CEQA) processes and the following focuses on the major federal and state environmental permits. FERC will be the lead federal agency for the NEPA process and compliance with the Federal Endangered Species Act. FERC will coordinate with the United States Fish and Wildlife Service (USFWS) for compliance with the Section 404 of the Clean Water Act (wetlands) and with the United States Forest Service (USFS) for a Special Use Permit.

PG&E plans to decommission the Potter Valley Hydroelectric Project (PVP; *Federal Energy Regulatory Commission [FERC] Proceeding No. 77*). PVP facilities are shown in Exhibit 1 (Main Report). PG&E's overall project goal is the decommissioning of the PVP for the reason being that the hydropower generation is no longer economically viable. The PVP consists of the following:

- Scott Dam which forms Lake Pillsbury in the Eel River Watershed;
- Cape Horn Dam, which forms Van Arsdale Reservoir in the Eel River Watershed;
- The Van Arsdale diversion, including tunnels and conveyance systems, which diverts water from the Eel River Watershed to the Russian River Watershed;
- The Potter Valley Power house, no longer operational, in the Russian River Watershed.

Based on our impact evaluation we compared a total of seven scenarios for the Scott Dam/Lake Pillsbury area, including existing conditions and the PG&E's proposal. Seven alternate scenarios were added to address more appropriate mitigation alternatives for the future of the Scott Dam/Lake Pillsbury area. The seven scenarios include:

1. Existing Operational Conditions Scenario covers operational conditions during the WY[1] period from 2019-2024 based on operational data (i.e. Spillway gates open at elevation 1,900 ft (PG&E Datum)[2] and operation of the needle valve to provide water supply downstream).

[1] Water year is defined as starting on October 1 and ending on September 30.

[2] All elevations referenced in this document refers to the PG&E datum unless otherwise specified. The PG&E datum is 81.7 feet higher than the Mean Sea Level (MSL) according to the USGS (Porterfield, 1964). MSL was later converted (no elevation change) to the National Geodetic Vertical Datum of 1929 (NGVD29). The PG&E datum is

2. Scenario A1 describes PG&E's proposed complete removal of Scott Dam per the PG&E Surrender Application dated January 2025.
3. Scenario A2 describes PG&E's proposed complete removal of Scott Dam as indicated in Scenario A1 but addresses the County's suggested additional mitigation options (including active sediment management).
4. Scenario B describes the continued operation under current conditions, where the Spillway gates are open at elevation 1,900 ft and operation of the needle valve would provide water supply downstream, and the spillway would be retrofitted with a fish passage structure.
5. Scenario C describes the lowering of Scott Dam to a new spillway height (elevation 1,875 ft), operation of a low-level flow control device and the inclusion of a fish passage structure.
6. Scenario D describes the replacement of Scott Dam with a new earthen dam, placed to restrict flows from only the Salmon Creek and Habematolel watershed for a smaller reservoir footprint. The spillway is assumed to be at elevation 1,885 ft.
7. Scenario E describes the removal of Scott Dam and construction of one or two small off-channel ponds built along the Eel River mainstem and allowing water storage along the river and natural fish passage.

## E1.1 Scott Dam System Overview

Lake Pillsbury is situated within the Lake Mendicino National Forest and is the largest lake within the confines of the Forest with 2,275 acres of surface area and 31 miles of shoreline (Stillwater, 2021, PG&E, 2025). The Lake's main flow contributor is the Eel River's upper watershed[3], located in Lake County, California. Scott Dam, which forms Lake Pillsbury, is a complete barrier to upstream fish migration, preventing access to high value habitat for federally Endangered Species Act (ESA)-listed fish.

Scott Dam was constructed between 1921-22 as a gravity dam approximately 805 feet long and 130 feet high. The dam primarily provides water storage (Lake Pillsbury) for the hydroelectric plant located at the north end of Potter Valley, which has been offline since July 2021 after PG&E discovered a transformer did not meet current operating standards. The Scott Dam has also historically provided water for irrigation to the Potter Valley Irrigation District (PVID) and to meet the minimum instream flow requirements in the Eel River and East Fork Russian River (Stillwater, 2021) (PG&E, 2025). Lake Pillsbury also provides various recreational opportunities such as camping, trails, boating, and swimming.

Water not utilized by the PVID are diverted into the Russian River and stored in Lake Mendicino for municipal and agricultural water supply (Stillwater, 2021). Lake Mendocino and the Russian River watershed are located south of Lake Pillsbury in the Potter Valley and were not historically connected to the Eel River watershed. The Eel River, Rice Fork, and several other smaller streams supply water to Lake Pilsbury. The Scott Dam is directly upstream of Soda Creek and approximately 12 miles upstream of the Cape Horn Dam and Van Arsdale Reservoir. The Cape Horn Dam and Van Arsdale Reservoir allow for the diversion of water from the Eel River to the

78.8 feet higher than the North American Vertical Datum of 1988 (NAVD88). Elevation 1,900 feet (PG&E datum) corresponds to elevation 1,828.3 feet (NGVD29) and 1821.2 feet (NAVD88 datum).

[3] An area of land drained by a river, river system, or other body of water.

Russian River. Cape Horn Dam has a fish ladder allowing fish species to migrate upstream until encountering Scott Dam.

Scott Dam currently operates with an approximate maximum capacity of 55,948 acre-feet at Lake Pillsbury (PG&E, 2025). The releases from Scott Dam are generally between 70 to 300 cubic feet per second (cfs). Water can be released from the dam through two structures: a needle valve[4] located at the base of the dam and the spillway[5]. The flow capacity of the needle valve is approximately 400 cubic feet per second (cfs) but can operate at varying flows for each Water Surface Elevation (WSE). The needle valve is remotely operated via Supervisory Control and Data Acquisition (SCADA[6]) controls. The spillway consists of multiple sliding and radial gates that are typically fully open between November 1 and April 1 and closed the remainder of the year. Flow through the gated spillway will only occur when the WSE within Lake Pilsbury exceeds the spillway invert elevation of 1,900 feet. The top of the spillway gates are all at elevation 1,910 feet. Slide gate #13 and one of the five (5) radial gates area SCADA controlled, whereas the remaining controls are manual. Since 2023, the spillway gates have remained fully open during the whole year.

As part of the PG&E's license surrender, a New Eel Russian Facility (NERF) to maintain water diversions to the Russian River watershed is proposed by PG&E and the Eel-Russian Project Authority (ERPA) as non-project use of project land. On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the ERPA (Eel-Russian Project Authority, 2023). ERPA will be responsible for the construction, maintenance and operation of NERF after the completion of the surrender requirements to be included in a regulatory order to be issues by FERC.

[4] A type of valve with a small port and needle-shaped plunger, used for directing precise flows of water.

[5] A structure used to provide controlled release of water downstream of a dam or levee.

[6] A computer-based system for gathering and analyzing real-time data to monitor and control equipment.

# E2.0 Permitting Evaluation

The permitting process will consist on a Federal Process, anticipated to be led by FERC, and a California Process, anticipated to be led by the California State Water Resources Control Board.

## E2.1 Federal Process

### E.2.1.1 Endangered Species Act Section 7 Consultation

The Endangered Species Act (ESA) of 1973 directs all federal agencies to ensure that any action they authorize, fund, or carry-out does not jeopardize the continued existence of an endangered or threatened species or designated or proposed critical habitat. The US Fish and Wildlife Service (USFW) and National Marine Fisheries Service (NMFS), collectively referred to as the Services, are the agencies primarily responsible for overseeing the ESA. The Services follow a structured process to authorize projects involving potential impacts to endangered species. This includes:

1. **Informal Consultation**

   Informal consultation is defined within the ESA regulations (50 CFR 402.02) as a "process that includes all discussions, correspondence, etc., between the Service and the Federal agency (FERC) or designated non-Federal representative (PG&E) prior to formal consultation.

   During informal consultation, an agency, in the early stages of project planning, approaches USFWS or NMFS and requests an informal consultation when a listed species may be affected by the action. If the agency, after discussions with the USFWS determines that the proposed action is not likely to affect any listed species in the project area, and if USFWS concurs, the informal consultation is complete.

   If the agency, after discussions with the USFWS determines that the proposed action is not likely to affect any listed species in the project area, and if USFWS determines that the proposed action may affect a listed species, that agency may then prepare a Biological Assessment (BA) to assist in its determination of the project's effect on a species.

2. **Biological Assessment**

   A biological assessment (BA) is a study that evaluates the potential impacts of a proposed project or action on biological resources, particularly threatened or endangered and their habitats. It is a crucial component of the ESA consultation process ensuring that federal agencies consider the impact of their actions on protected species.

3. **Formal Consultation**

   If it is determined that the proposed action may affect any listed species or critical habitat, then the agency must begin formal consultation with USFWS and/or NMFS. Formal consultation may last up to 90 days, after which the agency will prepare a biological opinion.

4. **Conclusion of the Biological Opinion**

   The biological opinion will state whether the federal agency has ensured that its action is not likely to jeopardize the continued existence of a listed species or result in the destruction or adverse modification of critical habitat. A biological opinion may include

reasonable and prudent measures necessary or appropriate to minimize the impact of incidental take on the species.

The USFWS and/or NMFS have 45 days after completion of formal conservation to write the biological opinion.

### E.2.1.2 Section 404 of the Clean Water Act

Section 404 of the Clean Water Act regulates the discharge of dredged or fill materials into "waters of the United States, including wetlands". Dredged and fill material refers to materials like soil, sand, rock, or other materials that are excavated or placed in waters or wetlands. Section 404 is administered by the United States Army Corps of Engineers (the Corps) and the Environmental Protection Agency (EPA). The goal is to protect water quality by controlling activities that can impact aquatic ecosystems.

The following steps outline the application process:

1. **Pre-Application coordination**:

   PG&E will start by consulting with the appropriate USACE District office in California to discuss your project. This would be the USACE Sacramento District office.

2. **Application form and project information**:

   PG&E will need to complete and sign the standard individual permit application form (ENG Form 4345). This includes providing a detailed project description, outlining activities involving dredged or fill material, and submitting site plans and drawings. PG&E will also need to list adjoining property owners and other required authorizations.

3. **Environmental impact analysis and mitigation**:

   This involves preparing a wetland delineation and jurisdictional determination. PG&E must demonstrate efforts to avoid and minimize impacts and propose a compensatory mitigation plan for any unavoidable effects. An alternatives analysis and an environmental questionnaire may also be necessary.

4. **Coordination with other agencies**:

   PG&E must also obtain a Water Quality Certification under Section 401 from the California State Water Resources Control Board or a relevant Regional Water Quality Control Board. Consultation with agencies regarding endangered species and historic properties may also be required if applicable.

5. **Additional requirements**:

   These can include a pre-filing meeting request with the Water Board for Section 401 certification and a public notice and comment period. conducted by the USACE.

### E.2.1.3 USFS Special Use Authorization

The Forest Service requires a special use permit for all uses and occupancies on National Forest System (NFS) lands. Examples of authorized land uses include road rights-of-way. The primary permit application form required for road access on USFS lands is Form FS-7700-40 titled "Application for Permit for Use of Roads, Trails, or Areas Restricted by Regulation or Order". PG&E will need to fill out and submit forms FS-7700-40a Commercial Use Attachment and FS-7700-40b Oversized or Overweight.

Additionally, USFS will be a cooperating agency actively involved with the NEPA process. Under NEPA, a cooperating agency is any Federal, State, Tribal, or local agency designated by the lead agency to participate in the environmental review process because of its jurisdiction by law or special expertise related to a proposed action's environmental impacts. As the primary land owner for which PG&E's recreational facilities are located surrounding the Scott Dam and Pillsbury reservoir, USFS is considered a cooperating agency. As a cooperating agency they must participate early in the process and scoping. USFS may also develop information and prepare environmental analyses at the lead agency's request.

### E.2.1.4 National Historic Preservation Act (Section 106)

Dam removal projects must assess their potential impact on historic properties, potentially including the dam itself and archaeological sites. Section 106 compliance refers to the process outlined in Section 106 of the National Historic Preservation Act (NHPA). This section requires federal agencies to consider the effects of their undertakings on historic properties. The process involves identifying historic properties, assessing potential effects, and consulting with stakeholders to find ways to avoid, minimize, or mitigate any adverse effects.

The following are key aspects of Section 106 compliance:

1. **Historic Properties**:

   These are defined as properties included in or eligible for the National Register of Historic Places.

2. **Undertaking**:

   This refers to any project, activity, or program funded in whole or in part under the jurisdiction of a federal agency.

3. **Consultation**:

   The Section 106 process emphasizes consultation with various parties, including the State Historic Preservation Officer (SHPO), Tribal Historic Preservation Officer (THPO), Indian tribes, and other interested parties.

4. **Four-Step Process**:

   The core of Section 106 compliance involves a four-step process: initiation, identification, assessment, and resolution.

5. **Adverse Effects**:

   If a project is found to have an adverse effect on historic properties, the agency must work with consulting parties to develop measures to avoid, minimize, or mitigate those effects.

6. **Agreement Documents**:

   The agreed-upon measures are often documented in a Memorandum of Agreement (MOA) or Programmatic Agreement which are legally binding.

7. **Advisory Council on Historic Preservation (ACHP)**:

   The ACHP plays a role in providing guidance and commenting on the Section 106 process, particularly in cases of significant or controversial projects.

   In essence, Section 106 compliance ensures that federal agencies consider the impact of their actions on the nation's cultural heritage and work to protect historic properties.

These are defined as properties included in or eligible for the National Register of Historic Places.

## E2.2 California Process

The decommissioning of Scott Dam will involve coordination with multiple federal and California agencies due to its potential impact on water resources, environmental factors, and public safety. The following discusses the major California regulations and permitting requirements for the project. As discussed under the Federal Process, a Clean Water Act Section 401 Water Quality Certification will be required from the California State Water Resources Control Board prior to issuance FERC must license change.

### E.2.2.1 Section 401 Water Quality Certification

The California State Water Resources Control Board acts as the water quality certification agency during FERC licensing. The certification confirms that the proposed dam removal will comply with state water quality standards and may include conditions to protect water quality during and after the removal process. The State Water Board may also require specific actions, such as water quality monitoring during reservoir drawdown and sediment sampling.

PG&E must file a complete application with the Division of Water Rights, including project details, fees, and information demonstrating compliance with the California Environmental Quality Act (CEQA).

A "pre-filing meeting" with the Water Board is often required at least 30 days before submitting the application.

The application includes information to allow the State Water Board to determine that water is available for the proposed project, it wouldn't deprive anyone of higher priority water rights, wouldn't harm public trust resources, is in the public interest, and impacts on water quality and the environment under CEQA have been considered. The regulatory process for obtaining a Water Quality Certification is outlined below:

1. **Application Submittal:**

   The project proponent submits a Water Quality Certification application to the Regional Water Quality Control Board (Water Board).

2. **Environmental Review Determination:**

   The Water Board evaluates the project to determine if it may result in significant environmental impacts under CEQA.

3. **CEQA Document Selection:**

   If the Water Board is the lead agency, it determines whether to prepare an Environmental Impact Report (EIR) or a Negative Declaration (ND)/Mitigated Negative Declaration (MND).

4. **Notice of Preparation (NOP):**

   If an EIR is required, the Water Board issues a Notice of Preparation and distributes it to responsible agencies and interested parties for input.

5. **Draft EIR Development:**

   The Water Board prepares the Draft EIR, incorporating input received during the NOP comment period.

6. **Public Notice and Review:**

   A Notice of Completion is filed with the Office of Planning and Research, and the Draft EIR is made available for public review, typically for a minimum of 30 days.

7. **Final EIR Preparation:**

   The Water Board reviews and responds to public comments, then prepares the Final EIR with necessary revisions.

8. **Project Consideration and Certification:**

   The Water Board certifies the Final EIR, makes CEQA findings, and decides whether to approve the project.

9. **Notice of Determination (NOD):**

   Following project approval, the Water Board files a Notice of Determination with Office of Planning and Research, finalizing the CEQA process.

### E.2.2.2 Division of Safety of Dams

The Division of Safety of Dams (DSOD) in California plays a crucial role in the dam removal process, ensuring public safety throughout the project. The process generally involves site reconnaissance, feasibility studies, community engagement, final engineering, and construction oversight. DSOD's involvement ensures that dam removals are conducted safely and in accordance with regulations.

The DSOD ensures that the following steps are completed:

1. **Site Reconnaissance and Feasibility Study**:

   This step involves a series of planning and evaluation prior to dam removal. For example:

   - Gathering Information - Collecting data on the dam's history, age, legal rights, infrastructure, biological resources, sediment, and community interests.
   - Engineering Analysis - A feasibility study assesses the dam's hydrology, hydraulics, sediment movement, and potential impacts on the surrounding environment.
   - Conceptual Plans - Developing conceptual plans for dam removal, sediment management, and stream restoration.
   - Regulatory Compliance - Permitting requirements are identified and coordinated with relevant agencies.
   - Cost Estimation - Financial feasibility is assessed, including potential funding sources.

2. **Community Engagement**:

   The community must be involved throughout the dam removal process to ensure the affected communities are informed. This can be demonstrated through (**1) Stakeholder meetings**, where community members and stakeholders are engaged to address concerns and **(2) Pre-permitting Meetings**, where a consultation with local, state, and federal agencies is conducted to streamline the permitting process.

3. **Final Engineering and Permitting:**

   The final engineering and permitting phase involves the following:

   - Detailed Plans - Final engineering plans are developed for dam removal, stream restoration, and other related construction activities.
   - Permitting - All necessary permits are obtained from relevant agencies.
   - Addressing Agency Comments - Any concerns or requirements raised by regulatory agencies are addressed.
   - Cost Estimation - A final cost estimate is developed.

4. **Construction Oversight**:

   To ensure successful execution of the project, the contractor employed to complete the project shall be subject to the following:

   - Contractor Selection - A qualified contractor is selected to carry out the removal work.
   - Construction Oversight - The engineer oversees the construction process, ensuring it aligns with approved plans and specifications.
   - Addressing Issues - Any issues that arise during construction are addressed promptly.

5. **DSOD's Role**:

   The role of DSOD for dam removal is as described below:

   - Reviewing and Approving Plans - DSOD reviews and approves plans for dam removals, ensuring they meet safety standards.
   - Independent Analysis - DSOD conducts independent analyses of dam performance and stability.
   - Construction Oversight - DSOD oversees construction to ensure compliance with approved plans.
   - Annual Inspections - DSOD inspects dams annually to ensure they are safe and functioning as intended.
   - Emergency Action Plans - DSOD reviews and approves dam owners' Emergency Action Plans (EAPs).

6. **Post-Construction**:

   Following removal of the dam the following activities can be expected:

   - Final Inspections - DSOD conducts a final inspection after dam removal is complete.
   - Cost Affidavit - Dam owners submit an affidavit detailing the actual construction costs.
   - Filing Fees - Additional fees may be required if the final cost exceeds the estimate.

### E.2.2.3 California Department of Fish and Wildlife (CDFW):

Lake or Streambed Alteration Agreement: Notification to the CDFW and a subsequent agreement are required for any activity that may substantially divert or obstruct the natural flow of, or substantially change or use any material from the bed, channel, or bank of any river, stream, or lake. Information developed for the Surrender Application will be used to prepare the agreement.

### E.2.2.4 California Endangered Species Act (CESA)

Projects must comply with CESA to protect listed species and their habitats, potentially requiring permits or consultations. PG&E will need to consult with the California Department Fish and Service to ensure that no California listed threatened or endangered species are affected by the project.

California ESA compliance permitting, involves obtaining authorization to "take" harass, harm, pursue, hunt, shoot, wound, kill, trap, capture, collect, or attempt to engage in any such conduct) a species listed as threatened or endangered under CESA. This typically involves an Incidental Take Permit (ITP) from the California Department of Fish and Wildlife (CDFW) and can be triggered by projects that may impact listed species.

Key aspects of California ESA compliance permitting include:

- **Listing of Species** – CESA lists various plant and animal species as threatened or endangered.
- **“Take”** – The definition of “take” under CESA is broad and includes various actions that could harm or disturb listed species.
- **Incidental Take Permits (ITPs)** - ITPs are the primary mechanism for authorizing the “take” of listed species when it is incidental to an otherwise lawful activity. If a project has already secured federal ESA authorization (e.g., a federal ITP or Section 7 consultation), a Consistency Determination from CDFW may be sufficient for CESA compliance.
- **Application Process** – Applications for CESA permits must be submitted to the appropriate Regional Manager at CDFW and include detailed information about the project, the species potentially impacted, and measures to minimize and mitigate those impacts.
- **Fees** – There are application fees associated with CESA permits.
- **Impact Analysis** – Permit applications require an analysis of potential impacts to listed species and whether the project would jeopardize their continued existence.
- **Minimization and Mitigation** – Permittees must implement measures to minimize and fully mitigate the impacts of the authorized take.

In essence, compliance with California’s ESA involved a comprehensive process of assessment, permitting, and mitigation to ensure that projects minimize harm to listed species and contribute to their conservation.

# E3.0 References

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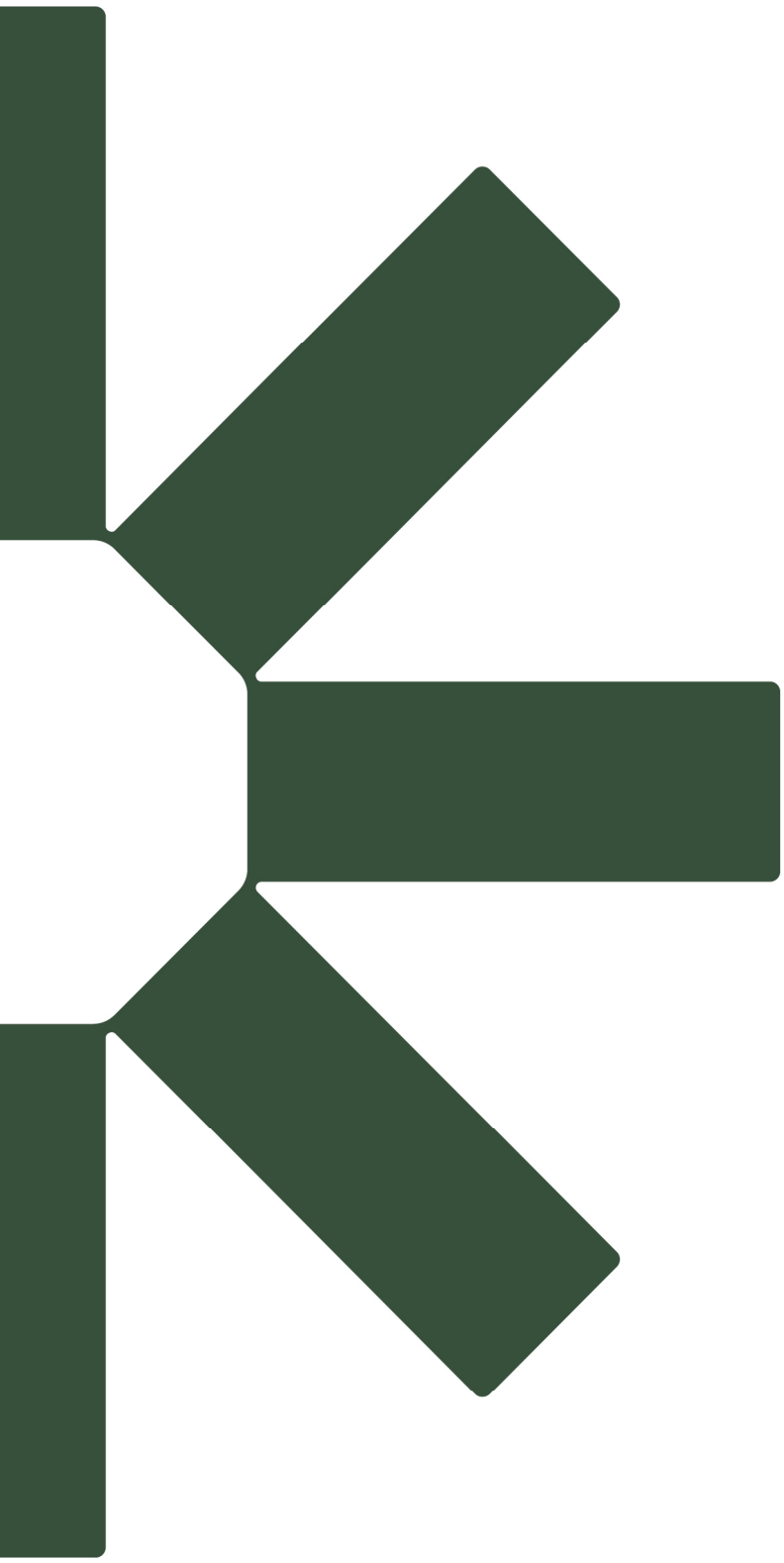

Making Sustainability Happen

# ATTACHMENT 3

**SLR International Corporation**
2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

August 21, 2025

County of Lake
255 N. Forbes Street
Lakeport, CA 95453

SLR Project No.: 102.021249.00001

**RE: SLR Comments on PG&E's Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 2025**

# 1.0 Introduction

SLR International Corporation (SLR) has been contracted by the County of Lake to analyze the effects of the proposed Scott Dam decommissioning, including providing comments on the Final Application for Surrender of License prepared by Pacific Gas & Electric Company (PG&E).

PG&E has submitted a Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 25, 2025 (*Final Application for Surrender of License; PG&E, 2025b*) for the Potter Valley Hydroelectric Project (*Federal Energy Regulatory Commission [FERC] Proceeding No. 77*) to FERC. The Final Application for Surrender of License includes information on the decommissioning process for the Potter Valley Hydroelectric Project, and for the construction of a New Eel Russian Facility, which is identified as non-project use of project lands by the Eel-Russian Project Authority (ERPA). The publicly available contents were published in five electronic documents, consisting of Volume IA, Volume IB, Volume II Parts 1 to 3. P&E also submitted two Volumes (III and IV) with privileged information to the FERC. PG&E hosted a virtual town hall to present the contents of the Final Surrender Application on August 11, 2025.

This letter report summarizes SLR's comments on PG&E's 2025 Final Application for Surrender of License. The comments provided only refer to the Application for Surrender of License aspects mainly included in Volume IA, and Volume II Parts 1 and 2, and not the Non-Project Use of Project Lands by the ERPA. It is SLR's understanding that the County of Lake intends to submit this comment letter to FERC during their official comment period.

Although FERC has primary regulatory authority in the Decommissioning Proceeding filed under FERC Docket P-77, the County would like SLR to provide recommendations on environmental mitigation and economic impacts to be considered by both FERC and PG&E in connection to the Environmental Impact Statement (EIS) process required by the National Environmental Policy Act (NEPA) and the Environmental Impact Report (EIR) process required by the California Environmental Quality Act (CEQA).

The focus of SLR's comments is on the existing known or documented conditions and effects within the County of Lake; impacts outside of the County may likewise be noted, where relevant to the County of Lake's concerns.

The position of the County of Lake Board of Supervisors is that PG&E has not demonstrated the full effects of dam removal, proposed environmental mitigation measures or addressed potential economic impacts to the County[1].

**Organization of the Comment Letter:**

This letter is organized as follows:

**1.0 – Introduction:** Provides an overview of the Final Application for Surrender of License and Application for Non-Project Use of Project Lands dated July 25, 2025; background information on work conducted by PG&E and others for the Potter Valley Hydroelectric Project, and Interventions by the County of Lake.

**2.0 – General Comments by SLR:** This section lists general comments on the Final Application for Surrender of License, and the general decommissioning process.

**3.0 – Comments on PG&E Application for Surrender of License:** This section is subdivided into 17subsections that correlate with the Affected Environment resource areas identified by PG&E. In particular, the resource areas directly match those listed in *Volume II, Section 2.2.3 Affected Environment* of the Final Application for Surrender of License. A summary correlation table is included for the reader.

## 1.1 Background Information

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in PG&E's Potter Valley Hydroelectric Project. Scott Dam is located on the Eel River in the County of Lake, within the Mendocino National Forest.

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The Final Application for Surrender of License considers removal of Scott Dam and draining Lake Pillsbury. The proposed decommissioning of Scott Dam is also considered by some interested parties as part of a broader initiative to restore the Eel River's ecosystem and improve fish passage[2].

Scott Dam is a concrete, gravity-type, ogee-shaped structure. It is 130 feet high and 805 feet long. The spillway crest elevation is 1,900 feet[3]. There are five radial gates and 26 steel slide gates surmounting the spill crest that allow storage of water 10 feet above the spill elevation.

At its maximum capacity, Lake Pillsbury has a surface area of approximately 2,275 acres with an elevation of 1,910 feet. The initial design capacity of the lake is 76,876-acre feet at the elevation of 1,910 feet. PG&E reduced this capacity in 2023 due to concerns of dam instability, to a maximum elevation at the spillway crest of 1,900 feet. Therefore, the reservoir is currently operated for a storage capacity of 66,876 acre-feet (ac-ft).

1) On January 25, 2019, PG&E filed a notice of withdrawal with FERC, discontinuing its efforts to relicense or sell/transfer the Potter Valley Hydroelectric Project. FERC issued a notice soliciting applications, which did not result in a viable new applicant[4].

[1] County of Lake Comments on Pacific Gas & Electric Company's (PG&E's) Draft Application for Surrender of License and Application for Non-Project Use of Project Lands dated March 3, 2025.
[2] Friends of the Eel River, n.d. https://eelriver.org/about-us/
[3] Scott Dam Area elevations are based on PG&E's datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. Elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.
[4] FERC Accession No. 20190125-5100 and FERC Accession No. 20190301-3038.

2) Since July 2021, the Potter Valley Powerhouse has been offline. PG&E determined a transformer did not meet operating standards (PG&E, 2025). No plans were made to return the Potter Valley Powerhouse to service.
3) The Project License expired April 14, 2022. The default license term for most hydroelectric projects under FERC jurisdiction is 40 years. Currently PG&E operates the project under annual licenses (PG&E, 2025).
4) FERC issued a Notice of Authorization for Continued Project Operation, on April 21, 2022.
5) On July 8, 2022, PG&E proposed to file an application for surrender of license by January 2025, which was acknowledged by Commission staff on July 29, 2022.
6) In 2023, the Scott Dam spillways were permanently opened due to seismic risk with the approval of the Division of Safety of Dams (DSOD, a Division of the California Department of Water Resources). Lake Pillsbury reservoir had 20,000 AF of reduced capacity. PG&E submitted a long-term flow variance request to FERC. A Water Quality Certification for the lower flow is pending with the State Water Resources Control Board[5].
7) On December 12, 2023, a Joint Exercise of Powers Agreement was executed by Mendocino County Inland Water and Power Commission, County of Sonoma and Sonoma County Water Agency to create the Eel-Russian Project Authority (ERPA)[6].
8) On June 6, 2024, PG&E documented progress in developing a surrender plan among the consulting parties.
9) Prior to filing their final application with FERC, PG&E provided a Draft Application for Surrender of License to interested parties and agencies in January 2025, for a 30-day comment period.

Several non-governmental organizations (NGOs), governmental agencies, and working groups have an interest in the Decommissioning process.

Further, a Conservation Easement for 5,660 acres of PG&E-owned land, of which approximately 2,234 acres are within the FERC Project boundary, was recorded on June 28, 2022, to permanently protect beneficial public values on lands owned by PG&E (PG&E, 2025, PG&E, 2022a). The conservation easement is held by Mendocino Land Trust. The easement permanently restricts development and protects conservation values (habitat, recreation, cultural resources) regardless of PG&E's future use. If PG&E ceases power operations, the land could be sold or transferred, but any new owner must comply with the conservation easement. Mendocino Land Trust is to enforce conservation restrictions; they do not own the land but can limit certain uses.

## 1.2 Interventions and Comments by the County of Lake

On July 31, 2023, the County of Lake filed a motion to intervene on the Potter Valley Hydroelectric Project Decommissioning with FERC using FERC Rule 214 subdivision (b). The County's interests in the proceedings include:

[5] State Water Resources Control Board (SWRCB), 2017. Study Requests and Comments on the Pre-Application Document and Scoping Document 1 for Potter Valley Hydroelectric Project, FERC Project No. 77, Mendocino and Lake Counties. August.
[6] The Agreement is available here: https://www.sonomawater.org/media/PDF/Water%20Resources/Potter%20Valley%20-%20ERPA/for%20signature_%20JEPA%20FINAL%20-%20Eel%20Russian%20Project%20Auth%20-%20signed.pdf

1) The Potter Valley Project (PVP) does not just generate power but also provides water supply for agriculture, recreation, and wildfire suppression.
2) The County has an interest in this matter as this project creates Lake Pillsbury on the Eel River. This lake has 31 miles of shoreline, used for recreational activities such as fishing, boating, sailing, and waterskiing.
3) The County opposes Decommissioning of Scott Dam, in part, because PG&E has not outlined mitigation measures that would be put in place to address environmental and safety concerns raised by the County.
4) If FERC provides approval for the Decommissioning of Scott Dam, the County of Lake requests the following mitigation measures:
    a. Establishing fire protection measures that directly address wildfire risks resulting from removal of Scott Dam;
    b. Establishing water rights for the County of Lake and its residents on Scott Dam, and implementing water storage measures for the County of Lake;
    c. Financial consideration of lost County of Lake property tax collections, and any and all revenue losses to result from the removal of Scott Dam;
    d. Additional monetary consideration for loss of economic development opportunity in the County of Lake;
    e. Full restoration and revegetation of the area, restoring flora and fauna in areas due to the loss of Scott Dam and Lake Pillsbury; and
    f. Constructing road and other infrastructure improvements necessary to safely complete the decommissioning plan.
5) The County asserts that any financial burden of these mitigation measures should be the responsibility of PG&E, as PG&E should remain responsible for the PVP and all consequences throughout the project lifecycle including Decommissioning.
6) Additionally, FERC should maintain PG&E's Licensee status until these mitigation efforts are addressed. For example, FERC required PacifiCorp (the project owner) to stay Co-Licensee surrounding Decommissioning of the Klamath Dams.

On December 23, 2023, the County of Lake issued a comment letter to PG&E regarding the PVP Initial Draft Surrender Application. This letter expressed concern regarding the lack of sufficient consideration for the effects of decommissioning on the County of Lake. These effects include loss of property, environmental concerns, regional loss of recreation, loss of water supply for wildfire suppression, and more. Additionally, the rapid timeline proposed by PG&E and lack of risk mitigation measures with respect to the County of Lake was an issue. The County is concerned about the need to manage large amounts of sediment and the lack of communication between PG&E and the County. Lake Pillsbury is an important source of economic activity as well as local government tax revenues.

PG&E documents several meetings with the County of Lake in their Final Application for Surrender of License; nevertheless, no substantive formal response to comments has been provided by PG&E to address the County of Lake's concerns, including the most recent County of Lake Comments on PG&E's Draft Application for Surrender of License and Application for Non-Project Use of Project Lands dated March 3, 2025.

## 1.3 Regulatory Process

PG&E has summarized the process to be followed by FERC and the State Water Resources Control Board as required by NEPA and CEQA. The process described is similar to what has been followed in other decommissioning processes.

A final approval of the Application for Surrender of License would be documented in a FERC Order with conditions as specified in the EIS (*led by the FERC*) and EIR (*led by the State Water Resources Control Board*), Management Plans (*prepared by PG&E and others*), and any other conditions.

# 2.0 General Comments by SLR

In accordance with 18 Code of Federal Regulations Ch. I § 6.1, an application for the Surrender of a license for a dam must include a detailed plan outlining the disposition of project works and any required restoration measures. The plan should address environmental, public safety, and operational concerns associated with cessation of the project's use. The Application for Surrender of License filed with FERC has not yet included all the necessary environmental documentation and stakeholder consultations to ensure compliance with federal regulations. Based on SLR's independent review:

1) PG&E has acknowledged but not fully addressed or requested an evaluation of Federal and California Environmental review processes. The County of Lake intends to participate in both processes.
2) PG&E's Final Application for Surrender of License states Scott Dam Decommissioning in the form of Rapid Dam Removal as the Proposed Action. The only Alternative considered was Phased dam removal. For the benefit of the County of Lake as well as other stakeholders no other alternatives were presented in the Final Application for Surrender of License. FERC needs to request PG&E to evaluate and include other decommission options such as partial removal of Scott Dam to best benefit the community and minimize environmental impacts.
3) PG&E has included the request for the use of NPUPL for FERC to approve the construction NERF in their final application, along with a comparison of outflows under existing and proposed conditions. However, the statement made by PG&E that they will be able to meet their flow requirements post-removal is overly optimistic. PG&E needs to consider what keeping Scott Dam in its existing state would do to meet minimum flow requirements compared to proposed action. Under existing conditions, PG&E sometimes falls out of compliance with FERC. Therefore, removing Scot Damm would only make it more challenging to meet minimum flows in the future, which is consistent with PG&E's modeling.
4) Specific studies are not included in the Final Application for Surrender of License even though PG&E acknowledged in the Draft Surrender Application additional studies would be conducted as part of the final Decommissioning per the future FERC order to be issued.
5) In the Final Application for Surrender of License, there are references to facilities and features that will be left in place (*including piezometers and block building*) and there is no detailed discussion with respect to the plan to leave in place/restore.
6) The County of Lake has not been engaged in meaningful stakeholder consultation, and therefore the stakeholder consultation has not been adequate from the County's perspective.

7) PG&E has stated in the Final Application for Surrender of License that various impacts resulting from the Scott Dam Decommissioning are unavoidable adverse impacts. However, the SLR review indicates these impacts are not sufficiently evaluated to be considered unavoidable.

8) PG&E should more thoroughly evaluate these impacts to establish that they are unavoidable and identify and evaluate options for minimizing unavoidable impacts. The County proposes PG&E include additional restoration scenarios that will likely minimize the unavoidable impacts to the County and enhance recreation, ecosystem, and fire suppression benefits.

9) To the benefit of stakeholders, including the County, PG&E should make all relevant data available to stakeholders and firms providing contracted technical support. Scott Dam has been in place since 1921, and there is limited publicly available data. A majority of relevant data is managed by PG&E. The various reports and studies completed reference a variety of PG&E supplied data not made available for this review.

Unavoidable adverse impacts are defined in both NEPA and CEQA. The definitions include:

- NEPA states "Effects that can not be avoided due to constraints in alternatives. These effects do not have to be avoided by the planning agency, but they must be disclosed, discussed, and mitigated, if possible [40 CFR 1500.2(e)]" [7].
- CEQA states "A project impact is considered significant and unavoidable if it would result in a substantial adverse change in the environment that cannot be feasibly avoided or mitigated to a less-than-significant level if the project is implemented. If a lead agency proposes to approve a project with significant unavoidable impacts, it must adopt a statement of overriding considerations to explain its actions (CEQA Guidelines, Section 15093(b))"

PG&E has indicated in the final application that several impacts are unavoidable; nevertheless, PG&E has failed to identify mitigation strategies and offsets. PG&E would need to comply with NEPA and CEQA with respect to defining the unavoidable impacts.

Clear understanding of the impacts expected to result from the proposed decommissioning is important to the County, as following the decommissioning process, the County of Lake will need to revise its General Plan and the Local Area Plan.

The Conceptual Decommissioning Plan does not specify how Scott Dam and the Eel River flows will be managed if the first high-flow season does not occur the November following the initial low-flow season (*i.e. if it is a low-flow winter that does not produce much precipitation*).

It is unclear how PG&E will determine when a wet season with enough precipitation is anticipated to conduct the sediment flushing needed during the first high-flow season described in the Conceptual Decommissioning Plan. PG&E should consider using the definition of the historic precipitation data in forecasting expected wet and dry seasons. In the absence of a high precipitation season, a long slow bleed of sediments may occur for many years, and PG&E would need to address these scenarios in the Sediment Management Plan.

Several plans to manage and mitigate environmental effects are briefly described in the application. PG&E needs to provide more detailed information on how proposed mitigation measures will be implemented. Additional comments are anticipated to be provided when these details become available. PG&E should also provide a schedule for when Management Plans will

[7] US Forest Service, nd. NEPA Glossary. https://www.fs.usda.gov/Internet/FSE_DOCUMENTS/stelprdb5298903.pdf

become available for public review and explain how the measures proposed in different management plans will be related to each other.

# 3.0 Comments on PG&E Surrender Application

SLR comments are presented in the following subsections, organized by the resource areas in the draft PG&E application. For the reader's reference, this table outlines the corresponding sections in the PG&E application and this comment letter. Any comments that were not related to a specific resource area in the PG&E Final Application for Surrender of License are presented in Section 3.19.

| Topic | PG&E Vol IA | PG&E Vol II | SLR Comments |
|---|---|---|---|
| Water Use and Hydrology | 6.2.1 | 2.2.3.1<br>2.2.4.2 | 3.1 |
| Water Quality | 6.3.1.2 | 2.2.3.2<br>2.2.4.3 | 3.2 |
| Fish and Aquatic Resources | 6.3.1.3 | 2.2.3.3<br>2.2.4.4 | 3.3 |
| Botanical Resources | 6.3.1.4 | 2.2.3.4<br>2.2.4.5 | 3.4 |
| Wildlife Resources | 6.3.1.5 | 2.2.3.5<br>2.2.4.6 | 3.5 |
| Geology and Soils | 6.3.1.6 | 2.2.3.6<br>2.2.4.7 | 3.6 |
| Geomorphology | 6.3.1.7 | 2.2.3.7<br>2.2.4.8 | 3.7 |
| Land Use | 6.3.1.8 | 2.2.3.8<br>2.2.4.9 | 3.8 |
| Recreation Resources | 6.3.1.9 | 2.2.3.9<br>2.2.4.10 | 3.9 |
| Aesthetic Resources | -- | 2.2.3.10<br>2.2.4.11 | 3.10 |
| Cultural Resources | 6.3.1.10 | 2.2.3.11<br>2.2.4.12 | 3.11 |
| Tribal Resources | 6.3.1.11 | 2.2.3.12<br>2.2.4.13 | 3.12 |
| Socioeconomic Resources | 6.3.1.12 | 2.2.3.13<br>2.2.4.14 | 3.13 |

| Topic | PG&E Vol IA | PG&E Vol II | SLR Comments |
|---|---|---|---|
| Air Quality | -- | 2.2.3.14<br>2.2.4.15 | 3.15 |
| Noise | 6.3.1.13 | 2.2.3.15<br>2.2.4.16 | 3.16 |
| Traffic | -- | 2.2.3.16<br>2.2.4.17 | 3.17 |
| Marine Resources | 6.3.1.14 | 2.2.3.17<br>2.2.4.18 | 3.18 |
| Others | -- | -- | 3.19 |

## 3.1 Water Use and Hydrology

- PG&E needs to discuss the local beneficial uses of Lake Pillsbury water, including drinking, recreational, and others, and how loss of this water supply for the various beneficial uses will be mitigated. PG&E notes in the application there are no expected effects on water reliability or cost in the Scott Dam Area because there are no known surface or authorized water diversions, but PG&E did not acknowledge the potential recharge to groundwater from Lake Pillsbury, which will result in reduced water reliability and increased cost for well owners, even though it refers to this effect as part of impacts on campgrounds/recreational facilities. Additional related comments are provided as part of the Geology and Soils, and Socioeconomic Resources sections herein.
- Water from Lake Pillsbury is currently used for fire suppression, and removing the lake will affect this resource and water use. Additional related comments are provided as part of the Land Use section.
- PG&E should evaluate the water supply and the effects on groundwater recharge as part of the final surrender application. Water supply insecurity has been a concern during drought conditions in recent years even with Scott Dam in place. As part of the NEPA Process, PG&E may be required to prepare a Water Supply Management Plan and Public Drinking Water Management Plan. The Final Application for Surrender of License does not include any details on these plans. It is currently unknown what the anticipated groundwater levels will be in the vicinity of Lake Pillsbury. Drinking water should be made available to well owners that will be affected by the reduced groundwater recharge resulting from draining the lake. Long-term effects are not expected to be known until several years have passed. PG&E needs to address in more detail than currently presented in Section 2.2.4.7.2 the groundwater table changes anticipated post-construction.
- PG&E notes "removal of the dam would restore flows in the Eel River to unimpaired natural conditions. High flows are similar under existing conditions (*i.e. No-Action*) and the Proposed Action (refer to Section 2.2.3.1, Figure 2.2.3.1-18), but low flows would vary substantially. […] Because there are no water users in the Eel River between Scott Dam and the Van Arsdale Diversion, the Proposed Action would have no effect on water users in the river reach." PG&E needs to evaluate the current beneficial uses of Lake Pillsbury to include recreational, ecosystem, and fire suppression.

- The mass balance approach PG&E used to determine unimpaired flows and flood frequency is unconventional per standard industry practice. PG&E needs to develop a flood flow frequency analysis based on the USGS Guidelines for Determining Flood Flow Frequency – Bulletin 17C[8] and perform an uncertainty analysis to account for the raw hydrology data and unimpaired natural flow assumptions. Active USGS gages along the Eel River and within the County of Lake are limited and restricted in the information provided. Streamflow gages near Scott Dam only record periods where flow is controlled or impaired by dam operations and there are no known or active precipitation gages within the entire Eel River watershed. The streamflow gages used in the hydrologic analyses are likely to incur levels of uncertainty based on unimpaired data available. PG&E should further analyze the raw hydrology data and assumptions to perform an uncertainty analysis.
- The January 2025 Final Application for Surrender of License does not provide any insight on validating or calibrating the hydrology assumptions for unimpaired flows. PG&E needs to add additional streamflow gages upstream of the Scott Dam to understand inflows to the impoundment, validate and calibrate unimpaired natural flows, and predict future climate change flows. Note that, the California State Water Resources Control Board, as part of the 2022 California Stream Gaging Prioritization Plan, have highlighted the Smokehouse Creek-Lake Pillsbury, Lake Pillsbury-Eel River, and Rice Creek-Rice Fork watersheds that supply Lake Pillsbury as high-priority watersheds with significant gaps in their stream gaging network.[9] The Plan also highlights that the Eel River reach between Scott Dam and Cape Horn Dam are not defined as well-gaged but as almost well-gaged or un-gaged. PG&E needs to justify their assumptions on what unimpaired flows are expected to look like with technical references and provide rationale for their assumptions.
- PG&E should conduct further analysis into the operations, storage, and streamflow gage relationship to better understand the flow relationship to operations and assumptions for unimpaired flows and water supply. Further analysis over the entire period of record will also provide a level of confidence in the data and assumptions made for the hydrology and hydraulics. Operations of Scott Dam have changed due to varying regulations and restrictions over the life of the dam. Storage capacity has likewise changed, due to the accumulation of sediment within Pillsbury Reservoir and permanently opening the spillway gates due to seismic concerns. SLR recommends verifying the assumption of no sediment management or removal of sediment within the Lake Pillsbury Reservoir. Changes in operations and storage can affect the stage discharge relationship.
- PG&E needs to further develop the hydrology, building on past analyses to evaluate future climate change scenarios as part of the environmental analysis for dam removal.
- PG&E should develop a hydraulic model to better define the proposed action during varying seasons and recurrence intervals. This would also support quantifying the sediment yield.
- PG&E needs to analyze sediment grain size distribution and determine the grain size and flow correlation for mobility (*i.e., At what flows will sediment move?*) to support changes in the longitudinal profile as well as core sampling the entire depth of sediment within Lake Pillsbury. Tributaries upstream and downstream of Scott Dam are likely sediment sources

[8] https://pubs.usgs.gov/tm/04/b05/tm4b5.pdf
[9] SB 19 Stream Gaging Plan | California State Water Resources Control Board

that account for and will continue to transport sediment into the Eel River system under all conditions.

- PG&E needs to develop a risk and uncertainty analysis for the proposed Scott Dam removal to better understand potential consequences and articulate the level of uncertainty or confidence in the data assumptions. The hydrology informs the hydraulics and the function of the existing and proposed infrastructure and the assumptions under proposed conditions. If hydrologic information is limited or inconclusive, there is a lower level of confidence in the models and assumptions made to fill in gaps within the data.
- PG&E notes "The Flood Monitoring Plan would establish protocols for monitoring water levels and flood risks, enabling proactive measures to protect infrastructure and surrounding areas." The details of the proposed plan will need to accommodate the findings of the additional hydraulic modelling for defining the monitoring requirements.
- PG&E needs to reevaluate claims made that the proposed removal of the dam will not harm their ability to meet downstream flow requirements. Under existing conditions, with operational control of the needle valve and spillway, they have reported noncompliance to FERC, most often during the dry season. Under proposed conditions, they would have no operational control or storage. During dry seasons, and especially droughts, the likelihood of noncompliance is higher under proposed conditions for a variety of hydrologic and climate-related factors. PG&E's modeling results concur with this: graphs of flow projections under proposed conditions show flows hovering near and going below 10 cubic feet per second (cfs) during the dry season every year. Currently, 20 cfs is their all-season minimum flow requirement. Therefore, a more thorough assessment of the proposed dam removal's impact on flows is necessary.

## 3.2 Water Quality

- PG&E notes they expect a short-term adverse effect due to the suspended sediment and turbidity following removal of the dams. Based on the level of uncertainty of the models, and lack of grain size analysis data, it appears that the anticipated period of several days up to several months, and additional effects with remobilization for 1 to 3 years may be optimistic. PG&E should conduct further studies to improve the sediment grain-size distribution dataset and update the sediment transport models with improved data. PG&E needs to validate their claim that "suspended sediment concentrations might be less than modeled" with their own modeling, an explanation, and references.
- PG&E should consider the above suggested additional data collection in development of short term and long-term sediment mitigation measures, including engineering controls and maintenance activities.
- PG&E acknowledges there will be impacts to water temperatures during and following the removal of Scott Dam. Given the uncertainty of the existing models, these may be underestimated, and additional evaluation with improved data should be conducted by PG&E in order to facilitate the methodologies for temperature monitoring. PG&E should update the Summer Water Temperature Monitoring Plan from 2005 due to operational, climate, and water use changes that have taken place since that time.

## 3.3 Fish and Aquatic Resources

- The Final Application does not include the estimated current population of all fish species in Lake Pillsbury. A list of the fish species that can be found in Lake Pillsbury can be found in Table 2.2.3.3-16 (Volume II) of the Final Application. For the fish species in Lake

Pillsbury, PG&E includes the 2018 fish assemblage data in Figure 2.2.3.3-22 of the Final Application, showing the percentage composition of each fish species in a graph. However, PG&E only provides population numbers in Lake Pillsbury for rainbow trout (stocking data). PG&E needs to include the lake-related fish population for estimating the ecological impacts due to the decommissioning of the Lake.

## 3.4 Botanical Resources

- PG&E anticipates long-term changes in habitat conditions due to altered hydrology and land use, some impacts to sensitive habitats and species may be unavoidable despite mitigation. The information in the Restoration Plan will be key to better understand if PG&E is proposing adequate mitigation to prevent avoidable impacts.

## 3.5 Wildlife Resources

- The information PG&E provides in the numerous wildlife management plans will be key to assess if adequate mitigation is established to prevent and mitigate avoidable impacts. Stakeholder input will be needed on PG&E plans.

## 3.6 Geology and Soils

- PG&E needs to evaluate impacts on surface and groundwater interactions following dam removal and its effects on groundwater wells as part of the Surrender Process. It is anticipated stored groundwater will be used, and without surface water replenishment from the reservoir, groundwater levels will be lower and affect existing wells. As a mitigation, PG&E should establish a well monitoring program pre- and post-dam removal, and funding to support well owners to repair or deepen existing wells to allow access to groundwater. PG&E acknowledges the adverse impact, nevertheless there are mitigation options that should be considered and not just deemed unavoidable.
- PG&E should conduct additional hydrogeological assessments and evaluate the radius of impact of the groundwater level drawdown. Once the system is better understood, PG&E should contact residents and landowners in the impacted radius to notify of these potential impacts, and request information on existing wells in the affected area. A detailed groundwater monitoring plan should be developed prior to decommissioning of Scott Dam. In the absence of detailed groundwater studies, all well owners identified in Appendix 2.2.3.6-A should be notified of the potential impacts.
- Suggested mitigation strategies similar to those established for the Lower Klamath Projects should be considered by PG&E. For example, in that instance, the Water Board required the following conditions to be met by the Renewal Corporation. Note that the distances or number of locations listed below are for information only, and site-specific criteria applicable to this area need to be developed based on further studies:
    a) Monitor groundwater levels within a 2.5-mile range of the high-water marks, before, during and after reservoir drawdown;
    b) Contact all residents and landowners within 1,000 feet of the reservoirs;
    c) Identify potentially affected groundwater wells;
    d) Monitor groundwater levels at a minimum of 10 locations within the 1,000 feet radius for at least two months before drawdown activities, and continue monthly monitoring for at least two years; and

e) Mitigate groundwater impacts.

- The removal of the abutment structure will require more detailed evaluations including abutment removal sequence and phasing plan, temporary excavation support systems for worker safety, slope stability analysis for each stage of the proposed removal, permanent slope support systems, and ground movement monitoring and alarm systems. During removals, there is a high risk of triggering landslides. Proper restoration and re-vegetation of these slopes to their original condition may involve complete removal of the abutment.
- Upon removal, the existing slopes all around the formally inundated areas will be subject to higher risk of landslides. This risk may be heightened during or soon after water release due to rapid drawdown conditions. The slope conditions of currently submerged areas need to analyzed and remedial grading should be developed accordingly.
- Alternative options should be further evaluated, as they can provide a well-balanced remedy to many of the above-mentioned geotechnical and geologic concerns.
- PG&E needs to conduct targeted and dam-specific subsurface investigations as a crucial part of this process in the absence of existing documentation to support the post-removal conditions.
- Debris management needs to be considered as part of the removal process, including any disposal or reuse of the demolished concrete of the dam, other structures, material found at recreational facilities, and any material (*including solid waste*) discovered during sediment removal. Land disposal of waste in California is regulated under California Code of Regulations Title 27, CEQA and County ordinances. PG&E should detail their planned permitting process for disposal of non-hazardous waste, especially if they plan to dispose on PG&E lands as it had been described in their January 2025 Draft Application for Surrender of License.
- PG&E needs to evaluate risks associated with triggering slides when removing the dam and when cleaning out sediments. The Final Application to Surrender License does not provide details on evaluating these risks, it notes the exposed reservoir bed would be susceptible to erosion and proposes to define specific mitigation measures in management plans that will be prepared in the future.
- PG&E acknowledges Lake Pillsbury is designated as mercury-impaired on the State of California's 303(d) list because of elevated mercury concentrations in fish. Geosyntec[10] conducted an environmental sediment characterization and found no indication of significant mercury, or other contamination. Since the sediment studies were only conducted for shallow sediments, additional studies are recommended to evaluate metal concentrations and mobility, including but not limited to mercury, for the deeper sediments impounded.
- Additional investigations and planning are needed to verify and protect against a potential release of Naturally Occurring Asbestos (NOA) which is known to occur within the Lake Pillsbury watershed. A NOA management and mitigation plan would need to be developed to mitigate the potential impacts during the dam decommissioning process. The Final Application for Surrender of License cites prior studies on Lake Pilsbury sediments, nevertheless, does not provide specific references. A Geosyntec[11] study was conducted, but it only tested shallow sediments in the lakebed as the target depth of 20 feet could not

[10] Geosyntec, 2020. Lake Pillsbury and Van Arsdale Reservoir Sediment Characterization. April 1.
[11] Geosyntec, 2020. Lake Pillsbury and Van Arsdale Reservoir Sediment Characterization. April 1.

be met. Any available study should be properly cited, and additional studies should be conducted.

## 3.7 Geomorphology

- PG&E's proposed conceptual approach lacks sufficient baseline studies on the amount and quality of sediments impounded in Lake Pillsbury. The proposed Sediment Management Plan needs to include such additional studies needed.
- PG&E needs to complete a comprehensive sediment risk analysis for the proposed action. The United States Bureau of Reclamation recommends an iterative, risk-based approach for evaluating and managing sediment impacts from dam removal projects in its *Dam Removal Analysis Guidelines for Sediment* (Bureau of Reclamation, 2017).
- PG&E plans to develop a Scott Dam Slope Stability Monitoring Plan. That plan should include measures to address and reduce the potential for reactivating the landslide and to protect worker safety. PG&E should provide the details to better understand the proposed plan.
- The application notes "Water surface elevations would continue to decline after the dam is removed. Without the impoundment, the denuded slopes surrounding Lake Pillsbury and the exposed reservoir bed would be susceptible to water and wind erosion, especially as the soils dry." PG&E should provide the details to better understand the proposed plan.
- The PG&E Final Application for Surrender of License generally acknowledges sediment impacts to water quality, erosion and transport following dam removal. Nevertheless, no clear mitigation alternatives such as sediment stabilization or removal are proposed.
- To analyze the effects of sediment transport, PG&E relies on models by Stillwater Sciences (2021a,b)[12], though the model assumptions differ from the Proposed Action. Because the method of dam removal heavily affects sediment erosion and transport modelling, PG&E should model the final removal approach and determine sediment-related impacts.
- PG&E will need to evaluate sediment grain size distribution. Additional field studies would be needed to properly evaluate the relative proportions of coarse and fine sediment impounded behind Scott Dam, which will affect the transport of sediments. If the lake is drained, there would be increased landslide hazards of the currently submerged slopes. These risks would be increased in a similar fashion to lowered safety factors for any slope under drawdown conditions. This may involve long-term maintenance efforts in the wet seasons. Such slope failures or slumps would increase the volume of materials that have to be managed, in addition to the already inordinate amounts of sediments that must be mitigated. Other geotechnical issues to be looked at include the stability of submerged slopes when the lake is drained.
- Potential scouring at or near the toes of future slopes needs to be addressed as it can be a triggering factor for landslides.

[12] Stillwater Sciences. 2021c. Hydraulic model development and potential flood implications following the proposed Scott Dam removal, Eel River, California. Technical memorandum. August 2021. As referred to in PG&E, 2025.

## 3.8 Land Use

- PG&E noted that "The Proposed Action may result in unavoidable adverse effects to local fire suppression to properties near Lake Pillsbury due to the replacement of Lake Pillsbury with the Eel River or other sources as a water source, resulting in potentially longer fire response times." The loss of Lake Pillsbury water availability would have significant wildfire suppression adverse impacts on infrastructure, property, and human life. Below is an analysis of the key impacts that PG&E need to include in their analysis:
    - Keeping small wildfires small is vital in the County of Lake due to the history of wildfires in the area. Federal, state and local fire resources have all worked together in the past to combat the many large campaign fires that have brought stress to the communities that have been affected.
    - Dry lightning around the Lake Pillsbury area poses the greatest risk for large scale wildfires in the area due to their unpredictability. Lightning fires can spark a large-scale wildfire in very remote areas of the Mendocino National Forest and have the potential to grow rapidly with fuel and topography alignment. Dry Lightning is a natural occurring event in California and is always a threat to the wildland every year.
    - The County is already prone to having water availability problems due to the wildland rural community interface that affects the populace that call the area home. Water availability directly correlates with fire department resources efficiency to fight fire in a rural area. Large fires that develop near Lake Pillsbury can grow (and historically have grown) exceptionally large due to the difficult access and topography aspects that promote fire growth.
    - Since its creation, Lake Pillsbury has always been utilized as a water resource because it reduces flight or drive time to resupply water to combat fires in the surrounding area. Extended fire attack operations that take place around Lake Pillsbury benefit from and rely on the lake as a primary water source because of reduced drive time for ground resources to refill their apparatus with water. Additionally, not having a body of water available to fire resources will always change fire strategies. Mitigating fire threats in rural locations always makes fire attack operations difficult for firefighter safety.
    - In the Lake Pillsbury area, the first suppression resource on scene of a fire will most likely be a fire engine. There exists a direct mutual aid agreement allowing both US Forest Service and CAL FIRE to respond regardless of jurisdiction. The first responder could be one of the helicopters from CAL FIRE. Because of the limited resources in the more rural areas, the helicopter resource becomes a critical initial attack resource. The closest fire station to Lake Pillsbury is a USFS station 2.6 miles/9 min drive to Lake Pillsbury. The closest CAL FIRE helicopter air support is in Willits, Ca. The estimated flight time for a Sikorsky S-70 Firehawk helicopter from Willits to Lake Pillsbury is approximately 8 to 10 minutes, depending on specific conditions and flight paths. CAL FIRE's helicopter attack (Helitac) philosophy incorporates specific goals for air resources, enhancing their effectiveness in wildfire management. Their number one goal is a rapid response. Specifically, it is used to achieve a quick initial attack on wildfires, keeping most fires at 10 acres or less. Helicopters can reach remote fires in approximately 20 minutes, allowing for timely intervention.
    - The distance from Clear Lake to Lake Pillsbury is approximately 20 miles. Given that the Sikorsky S-70 Firehawk has a cruise speed of around 150 to 160 knots (approximately 173 to 184 mph), the estimated flight time for a Sikorsky S-70 Firehawk

from Clear Lake to Lake Pillsbury is approximately 6 to 7 minutes, depending on specific conditions and flight paths. Given the spacing between fire stations and the windy roads, the next fire engine to respond to a fire in the surrounding areas adjacent to Lake Pillsbury will be 20+ minutes away. Rotary wing and fixed wing aircraft play a pivotal role in helping control the spread of a fire. Helicopters can get water from many sources, sources as small as a swimming pool. However, smaller water sources may only allow for one or two dips.

- Another issue with water sources for the fire service is cross-contamination. In California, aircraft used for firefighting must adhere to strict guidelines to prevent cross-contamination of water from different sources, such as lakes or reservoirs. Firefighting aircraft are required to identify and use designated water sources that have been approved for firefighting purposes. To prevent contamination, aircraft must avoid collecting water from multiple sources in a single mission. This means they should not dip from different lakes or reservoirs in one operation. For CAL FIRE's fixed wing resources, the CL-415 Super Scooper is utilized, and these aircraft are able to scoop water from a larger water source. The scoopers response time is a little slower than the Helicopters, but they can dump more water on a fire than a helicopter. It would take about 20-25 minutes to fly from a nearby airbase to wildfires adjacent to Lake Pillsbury. For fires north of Lake Pillsbury there are fewer larger lakes and greater distances between fire stations.
- Removing Scott Dam, eventually leading to Lake Pillsbury ceasing to exist, would increase fire behavior in the area due to the exposed lakebed. Invasive plant species are already well established in the area that can contribute to erratic fire behavior. Previous fires like the August Complex and the Mendocino Complex have already burned an exceptionally sizable portion of the Mendocino National Forest. Additionally, invasive brush and grass species will grow in these burn scars and create a bigger problem for wildfire, and in return make wildfire difficult to mitigate with ground and aerial resources. Ecosystems will be affected when taking a water source from the area, which means drier fuel moisture content that can increase fire behavior with certain types of fuel models. The largest fire in the State of California was the Mendocino Complex Fire. Without the water from Lake Pillsbury, the potential exists for a similar situation. This makes Lake Pillsbury an important resource for the fire service and its loss will have long-term adverse wildfire suppression impacts.

- There are additional Wildfire Suppression-related adverse Impacts on infrastructure, property, and human life, as noted below (these should be addressed by PG&E):
  - Increased Wear on Aviation Equipment: Helicopters would need to fly an additional 40 miles to access water, leading to increased operational wear and tear on rotors, engines, and other critical components. This could shorten equipment lifespan and increase maintenance costs, reducing overall readiness.
  - Strain on Fuel Infrastructure: The additional flight distance increases fuel consumption, which could strain local aviation fuel supplies and necessitate more frequent refueling, delaying firefighting operations.
  - Delay in Other Emergency Responses: With helicopters occupied for longer periods, there would be reduced availability for other critical missions, such as evacuations or medical response.
  - Delays in Fire Containment: The additional round-trip distance to collect water translates to slower response times for suppressing fires, increasing the likelihood of

wildfire spreading. This could result in greater destruction of homes, businesses, agricultural lands, and critical infrastructure (*e.g., power lines, roads*).

- Increased Costs of Fire Damage: Larger, uncontrolled wildfires typically lead to greater property losses and more extensive rebuilding efforts, which can strain local and state resources.
- Increased Risk to Residents: Delayed fire suppression increases the danger to residents, especially in rural areas where evacuation routes are limited. Lives could be lost or disrupted due to a lack of timely intervention.
- Helitac Crew Safety: Longer flight distances increase fatigue for Helitac crews, raising the risk of accidents during firefighting operations. The additional flight time also increases exposure to hazardous conditions, such as smoke, turbulence, and heat.
- Health Impacts from Smoke: Slower containment of wildfires leads to prolonged exposure to wildfire smoke for residents, exacerbating respiratory and cardiovascular issues, particularly among vulnerable populations.
- Loss of Natural Resources: Delayed fire suppression can lead to the loss of forests, watersheds, and wildlife habitats, which are critical for local ecosystems and the area's economy.
- Economic Hardship: The destruction of property and infrastructure could result in significant economic hardship for affected communities, including loss of jobs, reduced tourism, and increased insurance premiums.

- The decommissioning project will require several road improvements to ensure safe execution during the project and to maintain accessibility for emergency wildfire response, recreation, and other purposes afterward. Below is a breakdown of the necessary road improvements that PG&E will need to incorporate in future road improvements:
  - Fire Access Road accessibility around Lake Pillsbury is a concern for emergency response and must be upgraded to allow rapid emergency response. Roads always pose concern for emergency vehicles due to the difficulty in turning fire engines, if needed. Constructing or upgrading fire access roads with all-weather surfaces will ensure year-round usability. Adding turnouts and passing lanes on long, narrow roads will improve emergency vehicle safe passage.
  - Encroaching vegetation will also need to be cleared from high priority areas to reduce fire risk and improve visibility for emergency responders.
  - Recreational access roadways post-decommissioning, to include trailhead and parking access may be changed or require modifications to parking lots and trailheads. Improvements may need to be made, including creating or improving parking facilities and access points for hiking, fishing, and other activities, and installing clear signage to direct visitors to new recreation areas.
  - If the decommissioning affects current road alignments (*e.g., roads become submerged or rerouted due to reservoir changes*), new road sections may need to be built. Any unpaved or poorly maintained roads in the area must be upgraded to meet CAL FIRE standards for emergency access. The decommissioning project will require significant upgrades to roads and related infrastructure to ensure safety during construction and maintain accessibility afterward. Improvements such as road widening, weight-bearing capacity increases, emergency access enhancements, erosion control, and vegetation management will be essential. These upgrades will not

only support the decommissioning project but also ensure the long-term usability of the area for wildfire response, recreation, and other community needs.

- The removal of Lake Pillsbury as part of the PG&E Scott Dam Decommissioning Project raises valid concerns about wildfire spread, particularly the loss of the Lake's role as a barrier to prevent small fires from becoming larger events, according to United States Forest Service and Lake Pillsbury Fire Protection District personnel. The lake has frequently acted as a natural firebreak, slowing or stopping the spread of wildfires. Without the lake, vegetation would replace the reservoir basin, potentially increasing fuel loads and creating a continuous pathway for fire spread. Below is an analysis of the technical considerations to address wildfire concerns and the feasibility of retaining or reconfiguring the lake to meet both wildfire mitigation and seismic safety goals that must be considered by PG&E:
    - Vegetation Management: Implementing rigorous fuel reduction measures (*e.g., controlled burns, mechanical thinning*) in the lake basin and surrounding areas to reduce fire risk.
    - Establishing permanent firebreaks where the lake currently acts as a barrier.
    - Strategic Water Storage: Constructing smaller reservoirs, ponds, or water tanks within the region to provide accessible water for firefighting.
    - Installing dry hydrants and ensuring sufficient water storage for aerial and ground crews.
    - Infrastructure Upgrades: Setting up permanent firefighting staging areas with water access near the former lakebed.
    - Enhanced Detection Systems: Installing advanced fire detection systems, such as remote cameras, infrared sensors, and drones, to monitor lightning-related ignitions.

## 3.9 Recreation Resources

- PG&E is planning to remove several of the existing camping facilities. This would result in a significant recreational impact. To reduce recreation impacts, PG&E should also include recreational restoration in the Restoration Plan they intend to develop. The plan should be developed to offset the loss of recreational facilities, such as swimming opportunities, hiking, picnicking, off-road vehicle paths, camping and fishing. This plan needs to be developed in coordination with both USFS/Mendocino National Forest (MNF) staff and the County of Lake.
- The recreational features surrounding Lake Pillsbury that will continue to operate are:
    - Pogie Point Campground
    - Sunset Point Campground
    - Oak Flat Campground
    - Navy Camp Campground
    - Trout Creek Campground (Cape Horn Dam Area)

    These recreational features can be found in Map 2.2.3.9-2, though PG&E should provide additional details on any improvements or changes to the facilities to maintain its functionality if Lake Pillsbury is drained.

- PG&E's Final Application and environmental reports need to address loss of revenue generated by visitor usage and similar losses that will be experienced in the area.
- Recreation needs to be quantified as value revenue loss. PG&E should provide compensation to the County of Lake for the loss of recreation. Other locations offering similar lake-based recreational opportunities are a 45-minute to 2-hour drive from Lake Pillsbury, and include Clear Lake, Lake Sonoma, and Lake Mendocino. PG&E should discuss the potential for overcrowding at these other facilities due to closure of recreational facilities at Scott Dam.
- Recreation opportunities in the Project vicinity are concentrated around Lake Pillsbury, which inundates land primarily owned by PG&E but is surrounded by public land managed by the US Forest Service.
- Lake Pillsbury is the largest waterbody in the MNF and provides a variety of reservoir-based recreation opportunities such as boating, wind surfing, fishing, and swimming. There are a variety of developed recreation facilities in the immediate area of Lake Pillsbury, including family campgrounds, group campgrounds, and day-use opportunities.
- All Project recreation facilities (including campgrounds and day-use areas), ancillary facilities, and associated access roads in the Scott Dam area would be removed and the sites would be restored.
- PG&E has stated in the Final Application that recreation will shift from lake-based to riverine-based and acknowledges the "unavoidable adverse effects" due to the loss of these recreation facilities. However, they have not provided details on how they plan to assist in this transition and the support they will provide the County of Lake.

## 3.10 Aesthetic Resources

- PG&E needs to incorporate the various proposed restoration activities to improve the aesthetic aspects of the decommissioning project. This consideration needs to be addressed within the NEPA and CEQA process. PG&E did not detail specific restoration plans or activities, once prepared the plans should be available for stakeholder comment.
- SLR recommends that PG&E include photo modelling/artist renderings of visual impacts following dam removal.
- PG&E will need to prepare an ecosystem restoration plan that considers management of invasive species, and reintroduction of native and endangered species, along with management and stabilization of the lake sediments. Monitoring should be considered to evaluate the progress of revegetation, and any needed adaptation until vegetation is established in the areas affected by the Project.
- PG&E would need to properly address the restoration of all areas and items referenced in their preliminary restoration plan to the landowners' and the County's agreement.
- PG&E does not describe in this section how the Scott Dam area recreation facilities would be restored such as the campground, day-use areas, and associated roads and trails.
- Removing water bodies has a significant visual impact. Anything that is restored should be visually appealing. Small ponds and hiking trails could help offset these visual impacts and should be included in a restoration plan.
- PG&E argues that no adverse effects to visual resources have been identified for Phase 2 of the Proposed Action. During Phase 2, visual resources will be impacted until final

restoration is implemented. PG&E should note this impact and propose offsets. PG&E argues the overall impact would be temporary adverse effects during construction and long-term benefits as the rivers return to natural conditions and vegetation recovers. PG&E needs to provide a better justification for the offsets indicated with respect to the aesthetic aspects of the decommissioning project over a sufficient timeframe to allow for seed collection and responsible revegetation, as well as mitigation of invasive species.

## 3.11 Cultural Resources

- PG&E's proposed Historic Properties Management Plan will need to be reviewed for compliance and consistency with Federal and State environmental review requirements.

## 3.12 Tribal Resources

- PG&E's proposed Tribal coordination will need to be reviewed for compliance with and meeting Federal and State environmental review requirements.

## 3.13 Socioeconomic Resources

- PG&E has not quantified Operation and Maintenance costs associated with areas that will be restored, where sediments will be disposed of, and where dam debris will be contained.
- PG&E states that the community way of life for residents surrounding Lake Pillsbury will likely be adversely affected both during and post construction. PG&E should identify proper compensation for current residents that may be impacted.
- PG&E states that river-based recreation, in the long term, may provide similar economic opportunity to reservoir-based recreation. PG&E also provided a study that "estimated that decommissioning of the Project would extend the rafting and kayak season on the Eel River by approximately 6 weeks…", which appears to be for a portion of the Eel River below Cape Horn Dam, and not for the Scott Dam area. With the absence of road infrastructure to access the river, any potential rafting and kayaking activity near the Scott Dam area will be further reduced. Given that the loss of Lake Pillsbury will cause lost revenue from reservoir-based recreation for the County of Lake, and that the potential increase in river-based recreation may happen outside of county limits, PG&E will need to work with the County of Lake to identify the potential revenue loss for the County caused by the change in recreational activities from reservoir-based to river-based and conduct additional studies to identify mitigation measures associated with recreational activities for the watershed.
- PG&E states that "for recreationists who prefer lake-based activities", Lake Mendocino or Clear Lake "can provide substitute recreation opportunities." As many lake-based recreationalists from higher population centers currently travel to Lake Pillsbury from the west, most will likely go to Lake Mendocino instead. This would result in a loss of tourism and revenue to the County of Lake. Furthermore, Lake Pillsbury appeals to recreationists due to its remoteness, which these alternative lakes may not provide. PG&E has not accounted for this revenue loss.
- PG&E also states that "potential adverse effects can be offset with restoration of vegetation and site aesthetics (see the Restoration Plan) and with investment in river-based recreation infrastructure by local businesses." PG&E has not yet provided a Restoration Plan and appears to assume that restoration will involve investment by local communities. River based recreation infrastructure as part of the Restoration Plan should

be prepared and set up funded by PG&E. Furthermore, PG&E has not indicated if the lands will remain under PG&E ownership or if it will be transferred to other parties, therefore it is not clear who could or should develop the river-based recreation infrastructure.

- PG&E ties the hypothetical fish abundance to an increase in recreational activities by stating that if "fish abundance in the Eel River were to improve in response to the Proposed Action and return to natural conditions, then economic opportunity associated with commercial and recreational fishing could also increase." This statement should be supported by additional studies to be performed to identify whether restoring the river is going to lead to additional fishing and if this would overcome the loss of lake fishing at Lake Pillsbury.
- On Volume II page 2.2.4.14-26, PG&E details there are approximately 300 recreational homes and cabins surrounding Lake Pillsbury. The study provides an average assessed value of the properties at $253,000 and then goes on to apply 1.0 to 1.112% annual property tax rate or "up to approximately $285 annually per home". There is math problem where 1.112% of $253,000 is $2,833 and the annual taxes paid is nearly $850,000 not the $85,000 value presented by PG&E.
- In Volume II Section 2.2.3.13.6 of the application - Local Government Finance, PG&E have identified current revenue from property tax, lodging tax, sales tax and licenses, but have not evaluated how the lodging tax, sales tax, and licenses will be affected long-term. PG&E should consult with the County of Lake to obtain the correct tax information and incorporate it into the analysis.
- In Volume II Table 2.2.3.13-3 of the application, PG&E shows that the population of Lake County has a higher unemployment rate, poverty rate, and lower median income than the state average. PG&E has failed to quantify the effect of dam removal on these key economic indicators. These impacts need to be quantified, with mitigation measures identified and implemented by PG&E.
- PG&E should evaluate if this is an Environmental Justice community and recommend strategies to mitigate impacts on this group. This evaluation should be focused on the population in the region impacted by the Proposed Action, and not on County averages.
- In Volume II Sections 2.2.4.7 and 2.2.4.10.2, PG&E acknowledges the removal of Lake Pillsbury reservoir could reduce groundwater levels in the vicinity of the reservoir, thereby affecting local groundwater wells; and that lower groundwater levels may result in "higher pumping costs; the need for newer, stronger pumps; and in worst-case scenarios, the need to drill deeper wells." PG&E fails to account for this economic impact in the socioeconomic resources (Volume II Section 2.2.4.14) and incorrectly states "there are no expected effects on water reliability or cost in the Scott Dam Area" because the evaluation was only related to surface water and not the main source for the area which is groundwater. PG&E should list the potential effects on water reliability and cost as an adverse effect and proposed mitigation or funding options for the affected users.

## 3.14 Air Quality

- The construction plans proposed by PG&E should address emissions from construction vehicles and other greenhouse gas aspects. No specific plans or statements on addressing emissions from construction were identified in the application.

- Additional details on the monitoring of airborne NOA and other asbestos will be provided due to the presence of serpentinite rock in project construction areas and potential building demolition.

## 3.15 Noise

- The construction plans proposed by PG&E should address noise and vibration from construction equipment and vehicles.
- Demolition activities associated with the Project would generate ground borne vibration from the operation of heavy equipment. As with noise, vibration levels would vary depending on the type of equipment used and the distance to receptors.

## 3.16 Traffic

- PG&E documented existing road infrastructure, transit and aviation facilities in the vicinity of Scott Dam and Lake Pillsbury area. However, the impact on access road infrastructure was limited to three access roads. PG&E should include all access roads and other infrastructure to facilitate the future restoration and use of the facilities.
    - Scott Dam Area and Cape Horn Dam Roads documented and listed, but there were no specific plans on how the infrastructure would be restored.
    - The Transportation and Circulation Element of the Lake County General Plan also includes goals and policies that may be applicable to the Project (Lake County 2008). The relevant policies are listed below:
        - Policy T-1.8. Level of Service. County maintained roadways should be improved and maintained to provide an adequate peak period Level of Service (LOS) of "C" or better for existing and anticipated traffic volumes if roadway upgrades are feasible, such as roadway widening, addition of lanes via re-striping, and other safety and operational improvements. The County shall allow a limited number of County roadway segments to operate at a level of service of "E" or better where improving the segments to LOS C are deemed infeasible due to cost, negative community and/or environmental impacts, and/or constructability issues. This "E" level of service for certain roadways shall not include any State Highway unless approved by Caltrans.
        - Policy T-1.9. Truck Routes. To reduce heavy truck traffic in residential areas and near noise sensitive land uses, the County shall ensure truck routes are designated in a manner such that traffic noise impacts are minimized.
        - Policy T-1.10. Construction Methods. The County shall utilize road construction methods that seek to reduce air, water, and noise pollution associated with road and highway development.
- Regarding the impacts of dam removal, PG&E will need to prepare inundation maps with respect to releases during dam drainage and removal, and evaluate the potential effects on road infrastructure, including road easements, culverts, bridges and other utilities.
- Once these effects are sufficiently evaluated, road and other infrastructure improvements deemed necessary to provide for consistent opportunity for ingress/egress by emergency services (*including United States Forest Service personnel*) and residents of/visitors to the area would need to be conducted prior to removal of Potter Valley Project Dams and Equipment. The application notes "[*sediment*] deposition can reduce the capacity of the

river and floodways to transport water during periods of high rainfall, which can potentially cause localized flooding in areas that were previously unaffected. These changes can heighten the risk of flooding at downstream structures, such as bridges and road crossings. A Sediment/Channel Monitoring and Response Plan is proposed to monitor problematic levels and interventions.

- Roadwork improvements and modifications will be needed to accommodate the anticipated increased construction traffic and heavy equipment movement on local roads during the dam decommissioning. The general construction measures will include speed limits and dust control. PG&E proposes preparing a Construction Transportation Management Plan and following the General Construction Measures to mitigate transportation network effects. The construction transportation plan needs to include non-construction-related traffic that is expected to be present in the vicinity of Lake Pillsbury. The general maps presented in the Application do not provide the level of detailed required for assessment. Roadwork improvements or modifications to accommodate heavy equipment need to be specified.
- There is no detailed assessment of road capacity, load-bearing limits, or required upgrades to support increased construction traffic.
- There is no detailed assessment of road capacity, load-bearing limits, or required upgrades to support increased construction traffic.

## 3.17 Marine Resources

- Impacts on Marine Resources are outside of the scope of this review.

## 3.18 Others

**Cumulative Effects due to Climate Change**

PG&E's assessment of climate vulnerability in the Final Application for Surrender of License identifies several climate-related stressors. However, its evaluation lacks the structure, depth, and geographic specificity necessary to support informed decision-making. Specifically, PG&E's assessment:

- Does not follow a standardized vulnerability assessment framework.
- Omits key geographic areas, including Lake County, from its hydrologic and socioeconomic analysis.
- Fails to utilize Cal-Adapt or other state-supported climate modeling tools.
- Provides limited evaluation of social vulnerability and equity impacts.
- Focuses primarily on fish passage and geomorphic restoration, without assessing broader resilience strategies.

To strengthen its climate vulnerability assessment and the overall cumulative effects of the Proposed Action, PG&E should:

1) Adopt a structured framework such as the APG to evaluate exposure, sensitivity, and adaptive capacity.
2) Use Cal-Adapt and other state-supported tools to model future climate conditions and stressors.

3) Include the County of Lake in its hydrologic and socioeconomic impact analysis, given Lake Pillsbury's role in regional water storage, drought mitigation, fire suppression and the economic benefits it provides.
4) Evaluate impacts on vulnerable populations, including Tribal communities, low-income residents, and those dependent on agriculture and recreation.
5) Quantify and compare greenhouse gas emissions under existing and proposed conditions, including long-term carbon sequestration potential.
6) Assess recovery timelines and resilience for geomorphic and ecological systems.
7) Explore alternative water storage or conservation strategies to offset the loss of reservoir capacity.
8) Engage with local stakeholders and Tribal communities to co-develop adaptation and resilience strategies.

Without these additions, PG&E's climate vulnerability assessment remains incomplete and insufficient to address the full scope of risks associated with dam removal and climate change in Lake County. We request FERC to include the climate vulnerability assessment as required plan in the order to be issued by the FERC.

**Power System Impacts:**

- PG&E will need to discuss the existing power grid in the area and provide mitigation measures for any impact on the power grid due to Scott Dam and Pillsbury Lake removal. PG&E notes that the project powerhouse has not generated power since May 2021, and the decommissioning would permanently eliminate generation capabilities. The Final Application for Surrender of License does not explicitly state whether the vicinity of Lake Pillsbury is connected to the electrical grid.

# 4.0 Closure

SLR appreciates the opportunity to support the County of Lake on this important project. Please contact the undersigned if you have questions.

Regards,

**SLR International Corporation**

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# ATTACHMENT 4

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STATE OF CALIFORNIA
CALIFORNIA NATURAL RESOURCES AGENCY
DEPARTMENT OF WATER RESOURCES
DIVISION OF SAFETY OF DAMS

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101 County Lake
Type of Dam Gravity Type of Spillway Gated overpour
Water is 13.6 feet below the spillway crest at elevation 1900', and 38.18 feet below the dam crest.
WSE = 1886.4
Weather Conditions Sunny and warm
Contacts Made Matthew Colwell (PG&E)
Reason for Inspection Maintenance Inspection

## Important Observations, Recommendations or Actions Taken

In a letter dated March 17, 2023, PG&E submitted the results of a simplified seismic stability analysis of the dam to DSOD. Based on the analysis, the dam may become structurally unstable when subjected to seismic loading resulting from ground motions with an estimated return period of 850 years. The analysis also indicated that the potential for seismic instability is lower when the reservoir level is at or below the spillway crest elevation. As an interim risk reduction measure, PG&E, with concurrence from DSOD, has implemented a [redacted] foot restriction on the maximum reservoir operating level. [redacted]

PG&E's reported long-term plan for this facility is to decommission the dam as a part of surrendering the FERC license for the Potter Valley Hydroelectric Project. Details of the decommissioning are being developed.

## Conclusions

From the known information and visual inspection, the dam, reservoir, and the appurtenances are judged safe for continued use.

## Observations and Comments

**Dam**

The visible portions of the upstream and downstream faces of the concrete appeared weathered but in acceptable condition. PG&E recently made surface repairs to areas of poorly consolidated concrete on both downstream training walls and the downstream face as a part of routine maintenance (Photo 2).

Vegetation control along the downstream face, toe, and abutments is generally satisfactory. PG&E continues to remove vegetation annually and no concerning vegetation was observed.

There were no apparent signs of fresh movement on the upstream left abutment, which is continually monitored by survey monuments and inclinometers. The most recent instrumentation report, which was for calendar year 2022, indicated that the landslide continues to creep gradually and parallel with the dam.

The lower seepage [redacted] was inspected during this inspection. [redacted] The upper seepage [redacted] was not inspected during this inspection.

**Spillway**

The spillway approach and exit channel were clear of obstructions, and the log boom located several hundred feet upstream was in place.

All [redacted] gates were cycled during this inspection. [redacted] gates [redacted] were cycled [redacted], which performed satisfactorily (Photo 3). [redacted] gate [redacted] was cycled [redacted] which is scheduled to be retired sometime in the future. The [redacted] gates were cycled

Photos taken? Yes X No
cc for Owner/Book

Inspected by M. Holmes MH 11/1/2023
Date of Inspection October 18, 2023
Date of Report November 1, 2023 RG 11/1/2023

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STATE OF CALIFORNIA
CALIFORNIA NATURAL RESOURCES AGENCY
DEPARTMENT OF WATER RESOURCES
DIVISION OF SAFETY OF DAMS

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102 County Mendocino
Type of Dam Earth / Gravity Type of Spillway Overpour
Water is 0.27 feet above the spillway crest at elevation 1490.5 and 28.23 feet below the dam crest.
WSE = 1490.77
Weather Conditions Sunny and cool
Contacts Made Matthew Colwell (PG&E)
Reason for Inspection Maintenance Inspection

## Important Observations, Recommendations or Actions Taken

PG&E's reported long-term plan for this facility is to decommission the dam as a part of surrendering the FERC license for the Potter Valley Hydroelectric Project. Details of the decommissioning are being developed.

The aquatic vegetation on the water side of the retaining wall needs to be removed as a part of annual maintenance (Photo 1).

## Conclusions

From the known information and visual inspection, the dam, reservoir, and the appurtenances are judged safe for continued use.

## Observations and Comments

**Dam**

***Embankment***: The visible portions of the upstream and downstream faces, the crest, and the abutment contact remain in satisfactory condition with no indication of surficial distress or instability. The rock riprap protecting the lower reaches of the downstream face is in good condition (Photo 4). The concrete retaining wall appeared in satisfactory condition; the aquatic vegetation on the water side of the retaining wall needs to be removed as a part of annual maintenance (Photo 1). Vegetation control on both upstream and downstream faces is satisfactory. Rodent control is satisfactory, and no burrowing activity was noted.

***Concrete Gravity***: The downstream face was not visible due to water flowing over the spillway. Past inspection reports note that the stepped chute on the downstream face has erosion and deep pits in the concrete; this is not an immediate dam safety concern. The visible portions of the left abutment contact remain in satisfactory condition with no indication of surficial distress or instability (Photo 3).

**Spillway**

The spillway approach, control section, and channel were clear of obstructions. The reservoir was full and spilling at the time of inspection (Photos 2 & 3).

PG&E, along with their engineering consultant, GEI, have completed an engineering study to analyze and evaluate potential improvement alternatives for the spillway at Van Arsdale. The study was completed as part of PG&E's Spillway Assessment and Improvement Program (SAIP). PG&E has prioritized long-term spillway improvements at this dam within their overall dam portfolio. PG&E will provide results for the evaluation and provide a plan and schedule for further actions to improve the spillway. PG&E will also provide a plan and schedule for developing, implementing, and maintaining interim risk-reduction measures, as necessary, until long-term improvements are completed.

**Outlet**

[redacted] gates were fully cycled during the February 8, 2023, inspection and were found to be in satisfactory operating condition.

Photos taken? Yes X No
cc for Owner/Book

Inspected by M. Holmes MH 11/1/2023
Date of Inspection October 18, 2023
Date of Report October 31, 2023 RCB 11/1/2023

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

**Observations and Comments**

The original [redacted] valve is reportedly buried under silt and is no longer operable. [redacted].

Seepage Two 90-degree v-notch weirs, designated E3A (lower elevation) and E3B (upper elevation), are located at the toe of the embankment. Weir E3B is nearest the right groin and captures runoff and flow from groundwater springs within the adjacent hillside (Photo 5). Weir E3A also captures precipitation and may intercept spring runoff or high tailwater. It is not known if any of the flow to Weir E3B is true seepage or what percentage of the flows at E3A are seepage.

Flow at E3A was approximately 6 gpm, and flow at E3B was approximately 52 gpm. All seepage was clear and within historical ranges. Continue to remove vegetation surrounding the weirs to ensure flow through the weirs are unobstructed and accurate weir measurements can be taken.

Instr. Instrumentation consists of the following:

- Five (5) open standpipe piezometers located on the embankment portion of the dam.
- Six (6) survey monuments along the crest of the concrete retaining wall, installed in 2022.
- Two (2) seepage-monitoring weirs. The two 90-degree v-notch weirs are designed to monitor seepage through the downstream face and left groin.
- One (1) reservoir staff gauge.
- One (1) automatic reservoir water level monitoring [redacted]

The most recent combined DSSMP/DSSMR for record year 2022 was submitted in March 2023. I reviewed the report and am generally in agreement with the owner's evaluation of the data/plots as described below:

*Piezometers*: In 2022, all piezometer readings were within the historical range and at or below their respective Threshold and Action levels. Piezometers designated 1-2, 1-3, and 2-2 appear to be unreliable; 1-2 because it reads above the reservoir elevation; 1-3 because the tip is located at elevation 1498.5 ft, which is above the normal max water surface elevation at 1494.3 ft, and the readings are above the tip elevation; and 2-2 because it reads below its tip elevation and should be recorded as dry.

*Survey Monuments*: The six survey monuments were installed along the crest of the retaining wall in December 2022 in response to the 2020 Part 12D recommendation R-2. An initial survey was completed on December 28, 2022, with the initial positions. Movements will be plotted upon completion of a second survey.

*Weirs*: Overall, the weir seepage readings were within the historical range. On January 12, 2022, weir E3B exceeded the Threshold Level during a rain event. It is believed that subsurface flow from the adjacent hillside is the major contributor to the weir flow and that the reading is within normal range.

The owner's conclusion from their March 2023 submittal were that "Based on the instrumentation evaluations included in this DSSMR and the visual inspections by PG&E, FERC, and DSOD, the dam is judged to be performing satisfactorily. The project is suitable for continued safe and reliable operation. The dam surveillance and monitoring program is adequate for purposes of monitoring the normal performance of the dam." Based on the data submitted, I agree with the owner's conclusion.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 1: Vegetation on the water side of the concrete retaining wall needs to be removed as a part of annual maintenance.

## INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 2: View of the concrete overpour, retaining wall, and embankment as seem from the left abutment.

Photo 3: View of the concrete overpour as seen from the concrete retaining wall.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Van Arsdale Dam No. 97-102

Date of Inspection October 18, 2023

Photo 4: View of the downstream face of the embankment as seen from the toe.

Photo 5: View of weir E3B (upper); continue to clear vegetation and debris surrounding the weir.

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

[redacted]. All gates were cycled with no issues.

PG&E is operating the dam and gates with the year-round [redacted] reservoir restriction. [redacted] The gate configuration is in compliance with the Certificate of Approval.

Outlet

There is no upstream control for this dam. [redacted] valve [redacted] valve were last successfully cycled in DSOD's presence on April 27, 2022, with no apparent issues.

A lower sluice blocked on the upstream end [redacted] gate has been abandoned with the controls removed because it is buried under approximately 14-feet of silt. The sluice outfalls at the center base of the spillway chute.

Seepage

Seepage is monitored at three drainpipes located below the left abutment, labeled E1A, E1B, and E1C. E1A was dry, and E1B and E1C were estimated to be flowing less than 1 gpm. All flows were clear and within historical limits.

Minor historical seepage has been reported on the upper left downstream face at two locations along construction joints. No live seepage was observed at either joint and no standing water was visible, which is typical for the current reservoir elevation.

Seepage in the [redacted] measured through a v-notch weir [redacted]. PG&E routinely inspects and clears the sump and weir of sludge buildup. Mr. Colwell removed sludge buildup on the weir and tested the sump pump during this inspection, which was found to be in satisfactory operating condition. The weir reads accurately during low flows but reads erratically during high flows. There is currently no staff gage, and the weir cannot be read manually.

Instr.

Instrumentation consists of the following:

- Thirty (30) piezometers, located within the dam and left abutment.
- Three (3) drainage pipe type weirs located at the left abutment.
- One (1) v-notch weir [redacted].
- Sixty-two (62) survey monuments on the dam and abutment areas for measurement of vertical and horizontal movement. Ten (10) of the monuments were installed in the slide area near the left abutment.
- Eleven (11) inclinometers located in the dam and left abutment area (six are no longer read).
- Two (2) seismographs
- Five (5) tiltmeters; one on the left abutment and four [redacted]
- Three (3) sets of crack pins installed on the outside face of the dam to measure cracking.
- Two (2) crack meters, located in the upper [redacted]
- One (1) bubbler-type gage to monitor reservoir level.
- One (1) tailwater sensor
- One (1) automatic reservoir water level monitoring [redacted]

The most recent combined DSSMP/DSSMR was submitted in May 2023 for record year 2022. In October 2020 the monitoring data management system was replaced, creating inconsistencies

# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

between some old and new instrumentation data set. I reviewed the report and am generally in agreement with the owner's evaluation of the data/plots as described below:

Piezometers:

- Piezometers 4C, 5F, 6C, and 6F ([redacted] were replaced in July 2020. A new vibrating wire sensor was installed for piezometer 4C.
- Piezometer 2C [redacted] has been reading above its threshold level by 2 feet since the dam's remote data collection system was replaced in October 2020. PG&E plans to re-validate the readings of these sensors throughout 2023.
- Piezometer 6C [redacted] exceeded the threshold and action level in 2022; erratic readings in the winter and spring were caused by an inconsistent valve closure schedule. PG&E plans to re-validate the readings throughout 2023.
- All other piezometers were below the Threshold Levels, Action Levels, and within the historical range.

Seepage:

- 2022 [redacted] weir readings continued to exceed the temporary action level of 4 gpm, which was established on limited data since the installation of the weir in 2014. Flow readings are unreliable during high flows due to the intermittent operation of the [redacted] sump pump. During low flows, the weir trends with the reservoir elevation and have been consistent since 2017. PG&E plans to evaluate potential alternatives for improving the reliability of the weir data for 2023.
- Readings from Weir E-1A reached threshold level on February 8, 2022. Readings from E-1B and E-1C were below Threshold levels in 2022 and within historical trends.

Survey Monuments:

- In 2016, PG&E completed their initial comparative LiDAR survey of the left abutment hillside, and it shows that the downstream hillside is creeping. The movement appears to be surficial in the upper ten to twenty feet of the slope; this matches what other instruments have indicated over recent history.
- Horizontal movements for the survey monuments located on the dam crest were within the Threshold Levels. Between 2021-2022, survey points L, M, N, P, Q, and S did not move. Survey point 218 moved slightly downstream (to the West and North), about 0.01 feet.
- Survey point L, located near the right abutment, has the largest downstream movement since 1969, equal to 0.092 feet.
- In 2022, the vertical movements were in the upward direction between 0.006 to 0.017 feet. Since 1970, the vertical movements range between 0.035 feet downward, to 0.031 feet upwards.
- The maximum horizontal movement of the crib wall since 1993 is approximately 0.21 feet.
- In 2022, the monuments within the left abutment landslide moved an average of 0.95 feet horizontally. The maximum downslope movement between 2021 and 2022 was approximately

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

0.02 feet at Monument 702. Overall, the movement within the slide is parallel with the dam.

Inclinometers

- Inclinometer SCTD, located in the left abutment slope above the crib wall, indicates movement of 0.5 inches in the uphill direction from 1986 to 2022. However, the upper 10 to 20 feet indicates surficial movement in the downhill direction. Surveys on monument 41, near SCTD, show apparent downhill movement. The uphill trend indicates the inclinometer is founded in the upper portion of the deep-seated slide.
- Inclinometer SCTE, located in the left abutment slope, indicates movement of 2.5 inches to the north (downhill) from 1986 to 2022. This movement is consistent with nearby survey monuments. The greatest deflection is seen in the upper 10 feet, indicating surficial movement downhill.
- Inclinometer SCTH, located upstream of the dam in the left abutment, show approximately 0.4 inches of downhill movement since 2014. The instrument is relatively shallow, with most of the movement within the top 20 feet of fill material.
- Erratic readings for SCTH and SCTG began occurring in July 2019. PG&E removed the automated inclinometers in July 2022.

Seismograph:

- There was no significant seismic activity that triggered seismic monitoring within the vicinity of Scott Dam during the 2022 record year.

Tiltmeters:

- There are four tiltmeters [redacted]. Three of the four tiltmeters are measuring transverse (upstream/downstream) movement. Tiltmeters 1T and 2T continue to respond to reservoir water surface fall by tilting upstream and reservoir rise by tilting downstream, as expected. Tiltmeter 2L measuring longitudinal (north/south) movement, did not measure any apparent change during 2022.
- There is one tiltmeter installed in the left abutment hillside. The hillside tiltmeter was determined to be not functional during a July 2021 site visit. A new hillside tiltmeter was installed at the same location in July 2022. The hillside tiltmeter continues to exhibit the apparent trends of cyclic year-to-year movement along the axis of the dogleg portion of the dam.

Crack Pins and Crack Meters:

- The three (3) sets of crack pins have deteriorated and no longer provide reliable measurements of change in crack width and were abandoned in place in 2017.
- Both [redacted] crack meters continue to exhibit a marked response to reservoir rise and fall. Both crack meters record a positive displacement (opening of the cracks) as the reservoir rises and a negative displacement (closing of the cracks) as the reservoir empties. CM-2 positive displacement was approximately 0.012 inches and CM-1 was approximately 0.011 inches.

The owner's conclusion from their May 2023 submittal were that "Based on the instrumentation evaluations included in this DSSMR and the visual inspections by PG&E, FERC, and DSOD, the dam is judged to be performing satisfactorily. This dam needs to be closely monitored under normal, flood,

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

**Observations and Comments**

and seismic conditions to detect any unusual instrumentation readings and trends relative to past behavior. The project is suitable for continued safe and reliable operation. The dam surveillance and monitoring program is adequate." Based on the data submitted, I agree with the owner's conclusion.

Photo 1: View of the upstream face as seen from the right abutment.

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# INSPECTION OF DAM AND RESERVOIR IN CERTIFIED STATUS

Name of Dam Scott Dam No. 97-101

Date of Inspection October 18, 2023

Photo 2: View of the downstream face as seen from the left abutment.

[redacted]

# Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

## County of Lake

255 N. Forbes Street
Lakeport, CA 95453

Prepared by:

**SLR International Corporation**

2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

SLR Project No.: 102.021249.00001

June 6, 2025

Revision: 00

Making Sustainability Happen

**SLR International Corporation**
2175 N California Blvd, Suite 205, Walnut Creek, California, 94596

June 6, 2025

County of Lake
255 N. Forbes Street
Lakeport, CA 95453

**RE: Existing Conditions Report - Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project, Proposed Scott Dam Decommissioning by Pacific Gas and Electric (PG&E)**

To whom it may concern:

On behalf of the County of Lake, SLR International Corporation (SLR) has prepared this Existing Conditions Report as part of our effort to support the Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project. SLR is conducting a comprehensive evaluation aimed at assessing the potential impacts of decommissioning Scott Dam and draining Lake Pillsbury on various resources and stakeholders in the surrounding community. The grant project, funded by the Department of Water Resources (DWR) Agreement Number 4600015984, dated June 12, 2024 with the County of Lake aims to provide a thorough understanding of the existing conditions that would be affected by the removal of the dam and lake. The following tasks define Grant scope of work:

1. Project Administration
2. Existing Condition Evaluation
3. Impact Analysis and Adaptation Strategy Evaluation

This document intends to meet the requirements of the deliverable under the DWR agreement Task 2 “Existing Conditions Evaluation” and has been prepared by SLR International Corporation (SLR). The material and data in this report were prepared under the supervision and direction of the undersigned.

Sincerely,

**SLR International Corporation**

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# Executive Summary

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in PG&E's Potter Valley Hydroelectric Project (facilities shown in Exhibit 1). Scott Dam is located on the Eel River in Lake County, within the Mendocino National Forest. Pacific Gas and Electric Company (PG&E), the owner, is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam.

The decommissioning process considers removal of Scott Dam and draining Lake Pillsbury. The removal of Scott Dam and loss of Lake Pillsbury is expected to have complex impacts on various aspects of the environment, economy, and community. SLR scope of work is to identify these potential impacts and develop adaptation strategies to mitigate or adapt to them.

This Existing Conditions Report is the first deliverable for the Grant and provides a summary assessment of the present conditions in the vicinity of Scott Dam and Lake Pillsbury. The report identifies existing conditions including documented proposed actions in the following areas as related to the proposed decommissioning: Recreation, ecosystem, wildfire suppression, power, geotechnical, sediment, water supply, groundwater, hydrology, roadways and economic aspects.

The summaries referenced in this report include a detailed review of the information provided by PG&E and other stakeholders starting in 2019 to 2025 through the most recent version of the Draft Application for Surrender of License published in January 2025, prior to submittal to the Federal Energy Regulatory Commission (FERC) in July 2025.

**Recreation:** Lake Pillsbury attracts numerous visitors each year, and these visitors generate revenue for the local economy.

**Ecosystem:** Preliminary research identified 15 total species within 0.5 miles of Lake Pillsbury's perimeter. Among the list of species identified, 3 of 15 are considered endangered according to State criteria.

**Wildfire Suppression:** Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts.

**Power:** Lake County receives its electrical power through PG&E's broader transmission grid rather than directly from the Potter Valley Project's generation facilities (which are now disconnected). In the vicinity of Lake Pillsbury, some residences and commercial establishments are connected to the grid, most of the area's power primarily relies on independent solar arrays and generators.

**Geotechnical:** PG&E has reported that Scott Dam is unstable under seismic conditions and therefore, has reduced Lake Pillsbury capacity for safety considerations. There is a landslide mapped on the south bank of Lake Pillsbury, just upstream of the left abutment.

**Sediment:** An estimated 21,000,000 cubic yards of sediment have been impounded in Lake Pillsbury since construction of Scott Dam.

**Infrastructure/Water Supply:** The water supply system in the vicinity of Lake Pillsbury consists of both surface and groundwater. Four small water systems supply 147 residences and provide water for recreational purposes including hotels, resorts and campsites, and a state-operated water system supplies water for seven residences.

**Groundwater:** There are well completion records for 38 groundwater wells in the vicinity of Lake Pillsbury (DWR, 2024b). Reported planned uses include domestic (29), irrigation-agriculture (2), and public (1) water supply, test wells (3), and unknown (3). The groundwater level varies from 8

feet to 109 feet below ground surface. The average well depth in the area surrounding Lake Pillsbury is 150 feet.

**Hydrologic and Hydraulic Modeling:** Scott Dam provides a reliable water supply for the Eel River and Russian River watersheds, and is a major water supply source for irrigation, municipal and domestic water supply, recreation, and the support of salmon and steelhead populations. The Eel River continues 12 miles downstream of Scott Dam before entering the Van Arsdale Reservoir created by Cape Horn Dam.

**Restoration:** It is currently anticipated restoration of the remnant inundation zone of Lake Pillsbury and Van Arsdale Reservoir will take place following removal. PG&E expects riverbed profiles to return to pre-dam conditions within a few years of dam removal. As part of the proposed activities, reservoir sediment flushing would be conducted in a high-flow season as part dam removal. PG&E proposes to remove and restore recreational facilities (e.g., campgrounds; day-use facilities; recreation access roads and trails; kiosk; and boat ramps) located on Forest Service and PG&E lands.

**Roadways:** The primary roads around Lake Pillsbury are paved, but most are gravel and dirt roads. Based on the flood zone map, the 100-year storm event (1% annual chance floodplain) would flood the northern portion of Lake Pillsbury. Another significant area of inundation would be the southeastern portion of the lake, though no major access roads would be affected.

**Economic Conditions:** Five major economic sectors were identified in the area: agriculture; manufacturing; outdoor recreation and tourism; retirement and vacation housing; and the Federal government. The value of recreation at Lake Pillsbury was once estimated at $4,480,000 per year (1989 dollars; FERC, 2000). In 1999, the employment associated with Lake Pillsbury was estimated at 152 jobs per year (FERC, 2000).

**Community Feedback:** Comments from interested parties are split between dam decommissioning and those who prefer for Scott Dam to not be removed. Even for community members supporting removal, feedback provided on the plans is expected to continue due to the magnitude of the changes in the area.

SLR has completed the evaluation of the existing conditions as summarized in this report. The recommended next steps to move forward with evaluating the potential impacts of removing Scott Dam are in accordance with the grant work plan.

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Exhibit 3A: Identified Species Near Lake Pillsbury (0.5 Mile Radius)

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# Appendices

**Appendix A Site Photographs**

**Appendix B Past Hydraulic Modeling Technical Aspects**

# Acronyms and Abbreviations

| | |
|---|---|
| 4WD | 4-wheel drive |
| afa | acre-feet per annum |
| ac-ft | acre-feet |
| BACEI | Bay Area Council Economic Institute |
| BFEs | base flood elevations |
| BMP | Best Management Practices |
| BO | Biological Opinion |
| CDFW | California Department of Fish and Wildlife's |
| CEED | Center for Environmental Economic Development |
| CEQA | California Environmental Quality Act |
| cf | cubic feet |
| cfs | cubic feet per second |
| cy | cubic yards |
| CNDDB | California Natural Diversity Data Base |
| DREAM | Dam Removal Express Assessment Models |
| DSOD | Division of Safety of Dams |
| DWR | Department of Water Resources |
| EIR | Environmental Impact Report |
| EIS | Environmental Impact Statement |
| ERPA | Eel-Russian Project Authority |
| ESA | Endangered Species Act |
| FEMA | Federal Emergency Management Agency |
| FERC | Federal Energy Regulatory Commission |
| FIRM | Flood Insurance Rate Map |
| FIS | Flood Insurance Studies |
| FFA | flood frequency analysis |
| FIRO | Forecast Information Reservoir Operations |
| FS | factor of safety |
| HEC-RAS | Hydrologic Engineering Center’s River Analysis System |
| HEC_ResSim | Hydrologic Engineering Center Reservoir System Simulation |
| HEC-SSP | Hydrologic Engineering Center’s statistical software package |
| mg/L | milligrams per liter |
| MOU | Memorandum of Understanding |

| | |
|---|---|
| MW | megawatt |
| NEPA | National Environmental Policy Act |
| NERF | new Eel Russian Diversion Facility |
| NFHL | National Flood Hazard Layer |
| NGOs | non-governmental organizations |
| NAVD88 | North American Vertical Datum of 1988 |
| NGVD29 | National Geodetic Vertical Datum of 1929 |
| NOA | Naturally Occurring Asbestos |
| NWI | National Wetlands Inventory |
| NWIS | National Water Information System |
| PAD | Pre-Application Document |
| PG&E | Pacific Gas and Electric Company |
| PLSS | Public Land Survey System |
| PVP | Potter Valley Project |
| PVID | Potter Valley Irrigation District |
| RM | river mile |
| RPA | Reasonable and Prudent Alternative |
| SFHA | special flood hazard area |
| SFID | School Facilities Improvement Districts |
| SLR | SLR International Corporation |
| SPCC | Spill Prevention Control and Countermeasures Plan |
| SWPPP | Stormwater Pollution Prevention Plan |
| SWRCB | State Water Resources Control Board |
| USACE | United States Army Corps of Engineers |
| USDA | United States Department of Agriculture |
| USFS | United States Forest Service |
| USGS | United States Geological Survey |

# 1.0 Introduction

SLR International Corporation (SLR) has been contracted by the County of Lake to analyze the effects of the proposed Scott Dam decommissioning, perform modelling of existing conditions plus one or two decommissioning scenarios, and perform climate change modelling. This report summarizes SLR's findings of existing conditions within the County based on the review of publicly available and County of Lake provided limited information. The focus of this evaluation is on the existing known or documented conditions and effects within Lake County; other impacts outside of the County may be noted, where relevant to the County of Lake's concerns.

In 1922, Lake Pillsbury was created by the construction of Scott Dam, one of two dams included in Pacific Gas and Electric Company (PG&E's) Potter Valley Hydroelectric Project (*facilities shown in Exhibit 1*). Scott Dam is located on the Eel River in Lake County, within the Mendocino National Forest. PG&E, the owner, is proposing to decommission the Potter Valley Project (PVP), which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The Federal Energy Regulatory Commission (FERC) is the lead agency providing oversight of the PVP decommissioning. An overview of the regulatory requirements of the decommissioning process is included in Figure 1.

The "Lake County Resource Assessment, Impact Analysis, and Adaptation Strategy Evaluation Project" is a comprehensive evaluation project aimed at assessing the potential impacts of decommissioning Scott Dam and draining Lake Pillsbury on various resources and stakeholders in the surrounding community. The grant project, funded by the Department of Water Resources (DWR) Agreement Number 4600015984, aims to provide a thorough understanding of the existing conditions that will be affected by the removal of the dam and lake.

The removal of Scott Dam and loss of Lake Pillsbury are expected to have complex impacts on various aspects of the environment, economy, and community. SLR's scope of work is to identify these potential impacts and develop adaptation strategies to mitigate or adapt to them.

The grant scope includes evaluating the existing conditions of the areas that will be affected by the dam removal and lake loss, including water supply, wildfire suppression, utilities, ecosystem and environment, recreation, sediment, and infrastructures. This evaluation will improve the understanding of how resources will be impacted and identify potential adaptation strategies to minimize the negative effects.

The Existing Conditions Report, as part of this grant project, aims to provide a detailed assessment of the existing conditions that will be affected by the dam removal and lake loss. This report will serve as a foundation for the impact analysis and adaptation strategy evaluation, which are critical components of the overall project. As part of the Existing Conditions Evaluation, SLR has focused on the various aspects of the Dam Decommissioning Analysis completed by PG&E, their consultants, and others. Additional analysis is proposed as part of future work of the grant.

## 1.1 Report Organization

The report is organized into several sections to facilitate understanding and navigation:

**1.0 Introduction:** This section provides an overview of the report's purpose, scope, and key findings.

**2.0 Background Information:** This section provides historical context and relevant background information for Scott Dam and Lake Pillsbury.

**3.0 Summaries of Existing Conditions:** This section summarizes existing conditions with consideration given for categories of technical and economic aspects of concern to the County of Lake.

**4.0 Next Steps to be Completed:** This section outlines the next steps required to move forward with Task 3 of the grant scope of work.

**5.0 Remarks:** This section specifies professional scope and remarks of the report.

**6.0 References:** This section provides a list of sources cited in the report.

**Tables:** Tables with supporting information are attached to this report.

**Figures:** Figures created by SLR present relevant information.

**Exhibits:** Figures and diagrams created by others are included as exhibits.

**Appendix A:** Presents general existing features of Scott Dam and Lake Pillsbury in photographic format.

**Appendix B:** This appendix provides detailed information on existing information and models relevant to hydraulic aspects. This appendix supports the report's findings but is not essential for understanding the main message.

# 2.0 Background Information

The construction of Scott Dam started in 1920 and was completed in 1922, when Lake Pillsbury started to fill. Snow Mountain Water and Power Company was the original owner. They acquired a Department of Agriculture Power permit in 1920. Then, the Federal Power Commission (*precursor to FERC*) granted a 50-year license for the Project in 1922. In 1930, PG&E acquired the Project from Snow Mountain Water and Power and assumed the license (PVID, 2024; McMillen Jacobs Associates, 2021).

Scott Dam is a concrete, gravity-type, ogee-shaped structure. It is 130 feet high and 805 feet long. The spillway crest elevation is 1,900 feet[1] built in 1922. Additionally, each of the five radial gates surmounting the spill crest is 32 feet wide by 10 feet high, made of 26 steel slide gates, each 10 feet high and varying width from 7.5 feet to 10.08 feet. The storage releases are 72-inch diameter, riveted-steel outlet pipe passing through the dam at inert elevation of 1,812 feet. The storage releases are controlled by a 42-inch Lauren-Johnson needle valve.

At its maximum capacity, Lake Pillsbury has a surface area of approximately 2,275 acres, with an elevation of 1,910 feet. The initial design capacity of the lake is 76,876-acre feet at the elevation of 1,910 feet. PG&E reduced this capacity in 2023 due to concerns of dam instability, to a maximum elevation at the spillway at 1,900 feet. Therefore, the reservoir is currently operated for a storage capacity of 66,876 acre-feet (ac-ft).

PG&E is proposing to decommission the Potter Valley Hydroelectric Project, which includes Potter Valley Powerhouse, Cape Horn Dam and Scott Dam. The decommissioning process considers removal of Scott Dam and draining Lake Pillsbury. PG&E prepared an Updated Draft Application for Surrender of License and Application for Non-Project Use of Project Lands in January 2025 (mentioned as “Draft Application for Surrender of License”) which has been circulated to agencies and interested parties for comments (PG&E, 2025). The proposed decommissioning of Scott Dam is also considered by some interested parties as part of a broader initiative to restore the Eel River’s ecosystem and improve fish passage.

In addition to power generation, the PVP has been providing water supply for agriculture and recreation in Mendocino and Sonoma Counties. A key aspect of the Potter Valley Project is the Cape Horn Dam, where water diversions to the Russian River basin occur. As part of the license surrender a New Eel Russian Facility to maintain water diversions is proposed by PG&E as a non-project use of project lands by the Eel-Russian Project Authority (ERPA). Since these diversions take place downstream of the Lake County's boundary, it is expected that the impact on the County of Lake will be minimal.

Several non-governmental organizations (NGOs), governmental agencies, and working groups have an interest in the Decommissioning process. A list of entities/groups identified to date are summarized in Table 1.

Existing features of Scott Dam and Lake Pillsbury area are presented in photographic format in Appendix A to this report.

[1] Scott Dam Area elevations are based on PG&E’s datum, which equals National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet. Elevations for Potter Valley Project (PVP) facilities in the Cape Horn Dam area are in relationship to the North American Vertical Datum of 1988 (NAVD88) datum.

# 3.0 Summaries of Existing Conditions

The summary of findings discussed below was prepared with consideration given for categories of technical and economic aspects of concern to the County of Lake.

## 3.1 Recreation

In this section, SLR has identified existing recreation facilities and the recreation experiences that currently exist on and around Lake Pillsbury. Recreation has relied on the scenic qualities of Lake Pillsbury for many decades. Lake Pillsbury is the largest waterbody in the Mendocino National Forest[2] and attracts numerous visitors each year who generate revenue for the local economy. Recreation facilities and access roads proposed to be removed are presented in Exhibit 2 as provided in the Initial Draft Surrender Application (PG&E, 2023). Table 2 summarizes existing facilities, which includes six campgrounds with 153 campsites. Figure 2 shows the location of recreational facilities in the vicinity of Lake Pillsbury.

Recreation taking place in and around the lake, includes camping, boating (*motorized and non-motorized*), swimming, fishing, picnicking, wind surfing, bird watching, and stargazing. Additional recreational activities include hiking, nature study, hunting, bicycling, and backpacking (USFS, 2018). Lake fishing includes largemouth bass, Sacramento pikeminnow, bluegill and Rainbow trout (Fish Brain, n.d.). Whitewater rafting has historically occurred downstream of Lake Pillsbury in October and November when PG&E released water from Pillsbury Reservoir to make room for winter rain and snowmelt (California Creek). There are currently 6 campgrounds, 2 with group camping facilities along with 3 boat ramps around Lake Pillsbury (summarized in Table 2 and Figure 2). According to the Forest Service's website "The campgrounds at Lake Pillsbury are operated and maintained by Pacific Gas and Electric," excepting Lake Pillsbury Resort, which is privately owned.

The annual visitation estimate for the Mendocino National Forest is 254,000 individuals, based on National Visitor Use Monitoring Data Collected FY 2010, FY 2011, FY2012, FY2013, FY2014, FY2015, FY2016, FY2017, FY2018, FY2019 (Visitor Use Report Pacific Southwest Region (R5) United States Department of Agriculture (USDA) Forest Service Region 5) A visit is defined as "A National Forest Visit is defined as the entry of one person upon a national forest to participate in recreation activities for an unspecified period. A National Forest Visit can be composed of multiple Site Visits" (USFS, 2018).

Below is recreational information as outlined in the Draft Application for Surrender of License by PG&E (2025); This information is consistent with the recreational facility information provided by the County of Lake:

- PG&E is planning to remove the existing camping facilities. This would result in a Significant Recreation Impact. PG&E should reconsider the removal of existing recreation facilities. To reduce recreation impacts, PG&E should include recreational elements in the Restoration Plan that they plan to develop. The plan should be developed to offset the loss of recreational facilities, such as swimming opportunities, hiking, picnicking, off-road vehicle paths, camping, and fishing. This plan needs to be developed in coordination with both the Mendocino National Forest and the County of Lake.

[2] https://www.fs.usda.gov/recarea/mendocino/recarea/?recid=25166

- Other locations offering similar lake-based recreational opportunities are a 45-minute to 2-hour drive from Lake Pillsbury, and include Clear Lake, Lake Sonoma, and Lake Mendocino.
- Recreation opportunities in the Project vicinity are concentrated around Lake Pillsbury, which inundates land primarily owned by PG&E but is surrounded by public land managed by the US Forest Service.
- PG&E's Draft Application for Surrender of License has acknowledged and stated various plans to mitigate construction related impacts on these recreational resources. The following proposed mitigation plans are associated with the removal of recreation facilities:
    - Construction Recreation Plan, Public Safety Plan, East Branch Russian River Diversion Plan, Water Quality and Erosion Control, Restoration Plan, and Fisheries and Aquatic resource measures.

  These proposed plans will become available for the County of Lake's review after the submittal of the Final Surrender Application in July 2025.

  It is important to note that PG&E has stated in their Draft Application for Surrender of License the unavoidable adverse effects to recreation resources include the following:
    - Short-term loss of recreational opportunities during construction at Lake Pillsbury from the drawdown of Lake Pillsbury and restricted recreation use.
    - Loss of recreation opportunities at Lake Pillsbury with the removal of PG&E recreation facilities and ancillary features (permanent loss).
    - Loss of reservoir-based recreation at Lake Pillsbury with the transition of a lacustrine to riverine environment.
    - Potential short-term, temporary effects to fishing opportunities in the Eel River from high suspended sediment load and aquatic biota mortality.
    - Potential reduction in fishing, recreation watercraft, and swimming opportunities in the East Branch Russian River during low summer flows because water would no longer be diverted to the East Branch Russian River under the Proposed Action.

## 3.2 Ecosystem

An ecosystem is defined as a large community of living organisms (*plants, animals and microbes*) in a particular area[3]. Lake Pillsbury has its own unique ecosystem with threatened and endangered species, including Tule Elk, and fish that attracts visitors to the area. SLR has conducted further research on these species to understand the existing ecosystem surrounding Lake Pillsbury as discussed below.

### 3.2.1 Wildlife Resources

In order for SLR to understand the ecosystem aspects in the area, a data search was conducted using the California Natural Diversity Data Base (CNDDB). The CNDDB was run for the Lake Pillsbury quadrangle. Animal and plant species are listed in Table 3 and shown on Exhibit 3A, 3B,

[3] https://education.nationalgeographic.org

and 3C. Exhibit 3B also demonstrates contours explaining the likelihood of Tule Elk to be present within the area surrounding Lake Pillsbury.

The focus of the ecosystem evaluation is on threatened and endangered species. Verifying the presence or absence of these species within the project study area and determining potential impacts will require additional protocol surveys. Factors related to soils and water are being evaluated in other subsections of this report.

The predominant plant communities surrounding Lake Pillsbury and Van Arsdale Reservoir are yellow pine forest, mixed evergreen forest, foothill woodland, and chaparral (FERC, 1978).

SLR conducted a preliminary database review through the California Department of Fish and Wildlife's (CDFW's) BIOS Viewer, part of the CA Fish and Wildlife Wetland – CA Natural Diversity Database. Preliminary research identified 15 total species within 0.5 miles of Lake Pillsbury's perimeter as shown on Exhibit 3A. Among the list of species identified, 3 of 15 are considered endangered according to State criteria. Bald eagle, steelhead, and Humboldt marten are the 3 endangered species located within the 0.5-mile distance radius. Additional information on all species types and names are available in Table 3.

The preliminary database review also located Tule Elk in areas surrounding Lake Pillsbury, as shown in Exhibit 3B. Exhibit 3B demonstrates the likelihood of elk presence within the outlined contours on the map. Contours from the database suggest there is a 50% likelihood elk would be within the parameters of 0.5 miles, around the southern area of Lake Pillsbury. The second contour demonstrates a 90% likelihood of elk presence within its outlined contours, which exceeds the 0.5-mile distance radius scope for this database review.

PG&E has also identified rare, threatened, and endangered resources in the vicinity of the Project in the Draft Application for Surrender of License (PG&E, 2025).

- The PG&E Study Area for wildlife habitats and common wildlife species includes (1) area within 0.5 mile of FERC Project boundary, and also the Eel River between Scott Dam and Van Arsdale Reservoir, and (2) areas within 0.25 miles of the Eel River from Cape Horn Dam downstream to the Eel River estuary outlet into the Pacific Ocean, as will as the East Branch Russian River between the Potter Valley Powerhouse and Lake Mendocino.
- The Draft Application for Surrender of License provides a list of wildlife habitats and associated common wildlife species that occur in the study area.
- PG&E provides a list of special-status terrestrial wildlife species known to occur in the study area.
- PG&E has provided CONFIDENTIAL special-status terrestrial wildlife occurrences documented in the study area to FERC (these files are unavailable for review).
- Species know to occur on Lake Pillsbury and Van Arsdale reservoirs include:
    - Bald Eagle
    - Northern Spotted Owl
    - Northern Goshawk
    - Fisher-West Coast
    - Pacific Marten
    - Special—Status bats

- Black-Tailed Deer
- Tule Elk
- Western bumble bees
- Monarch butterflies

- There are six active bald eagle territories at Lake Pillsbury in the vicinity of the Scott Dam construction area.
- PG&E's Draft Application for Surrender of License has acknowledged and stated various plans to mitigate construction related impacts on these wildlife resources. The following proposed mitigation plans are associated with the wildlife resources:
  - Bald Eagle Conservation Plan, Northern Spotted Owl Management Plan, Other Raptors Measure, Other Species-status Birds, Game Birds Construction Measures, Special-status Bat Construction Measure.

  Additionally, PG&E has proposed a Construction Site Water Diversion Dewatering and Drawdown Plan and a Post-dam Removal Aquatic Species Management and Monitoring Plan to minimize the short-term impact of increased suspended sediment. These proposed plans will become available for the County of Lake's review after the submittal of the Final Surrender Application in July 2025.

### 3.2.2 Botanical Resources

- There are four special-status plant species that are known to occur in the FERC Project boundary [see Exhibit 3C, specifically the area of the Lake Pillsbury] or are adjacent to Project features, as mentioned in the Draft Application for Surrender of License including:
- Two populations of three-fingered morning glory (Calystergia colling ssp. Tridactylosa). One population located along the tunnel between Cape Horn Dam and Potter Valley Powerhouse; and one population located along the southern shoreline of Lake Pillsbury. One population of Green's narrow-leaved daisy (Erigeron greeni).
- PG&E's Draft Surrender Applications has acknowledged and stated various plans to mitigate construction related impacts on botanical resources. The following proposed mitigation plans are associated with the botanical resources:
  - Special-Status Plant Construction measures, General Construction Measures

  These proposed plans will become available for the County of Lake's review after the submittal of the Final Surrender Application in July 2025.

Riparian and Wetland Resources

- PG&E's study area includes the wetland and riparian habitats associated with the floodplains along the streams, littoral zones, and the Eel River estuary. There CALVEG alliances (standards to classify existing vegetation)[4] identified in the study area include herbaceous vegetation alliances, shrub-dominated vegetation alliances, tree-dominated vegetation alliances, and aquatic features. PG&E has used the National Wetlands Inventory (NWI) to identify the 10 NWI classification types and the areas that are affected.

[4] https://www.fs.usda.gov/detail/r5/landmanagement/resourcemanagement/?cid=fsbdev3_046815

### 3.2.3 Fish and Aquatic Resources

- The Draft Application for Surrender of License provides an overview of fish and aquatic resources in the Project Study Area, which includes the Eel River from upstream of Lake Pillsbury downstream to the ocean, including the East Branch of the Russian River to Lake Mendocino.
- The Eel River downstream of Lake Pillsbury, including the lower Eel River and estuary, provides aquatic habitat for several special-status aquatic species, including anadromous salmonids listed as threatened under the Endangered Species Act (ESA) -- Chinook salmon (Oncorhynchus tshawytscha), Coho salmon (O. Kisutch), steelhead (O. mykiss) threatened Pacific eulachon (Thaleichthys pacificus), and endangered tidewater goby (Eucyclogobius newberryi) – and species proposed for listing as threatened under the ESA – northwestern pond turtle (Actinemys marmorata) and longfin smelt (Spirinchus thaleichthys).
- Significant short-term impacts on aquatic life (*i.e. high mortality*), particularly to salmonids, are expected as a result of high suspended sediment concentrations. However, one of the most important species (Steelhead, [*Oncorhynchus mykiss irideus*]) are able to tolerate such conditions due to their broad spatial distribution and flexibility within their lifecycle (Stillwater Sciences, 2021c).
- PG&E's Draft Surrender Applications has acknowledged and stated various plans to mitigate construction related impacts on fish and aquatic resources. The following proposed mitigation plans are associated with the fish and aquatic resources:
    - Construction Site Water Diversion, Dewatering and Drawdown Plan, Construction Water Quality and Temperature Monitoring Plan, Water Quality best management practices (BMPs), Stormwater Pollution Prevention Plan (SWPPP), Hazardous Materials Handling Measures, Spill Prevention Control and Counter – measures (SPCC) Plan.

  These proposed plans will become available for the County of Lake's review after the submittal of the Final Surrender Application in July 2025.

## 3.3 Wildfire Suppression

SLR review confirms wildfires are common in Lake County. Lake Pillsbury serves as a crucial water resource for the region, including for fire suppression efforts (Friends of the Eel River, 2023); if the lake is drained, this regional resource will be lost, meaningfully reducing the immediacy of suppression efforts. Exhibit 1 shows in green the land owned by the U.S. Forest Service.

The Mendocino National Forest and surrounding areas have a long history of significant wildfires. Notable recent fires include:

a) August Complex Fire (2020):
    - Burned 1,032,648 acres
    - Largest wildfire in California history
    - Destroyed 935 structures and caused significant environmental damage
b) Mendocino Complex Fire (2018), which is shown on Exhibit 4:
    - Burned 459,123 acres

- Largest wildfire in California history at the time
- Destroyed 280 structures and led to significant evacuations

c) Redwood Valley Fire (Mendocino Lake Complex, 2017):

- Burned 36,523 acres
- Destroyed over 500 structures and resulted in several fatalities

These fires demonstrate the region's susceptibility to large-scale wildfire events and underscore the importance of effective fire suppression resources (Cal Fire, 2021).

Lake Pillsbury currently serves as a critical water source for regional fire suppression efforts. Its importance is highlighted by the following factors:

a) Proximity to fire-prone areas: Lake Pillsbury is centrally located within the Mendocino National Forest, providing quick access to water for firefighting efforts.

b) Size and capacity: With a surface area of approximately 2,000 acres and a capacity of 80,000 acre-feet, Lake Pillsbury offers a substantial and reliable water source for firefighting (PG&E, 2022b).

c) Accessibility for various firefighting methods: The lake is suitable for multiple water collection methods, including:

- Helicopter bucket operations
- Fixed-wing aircraft water scooping (*e.g., Canadair CL-415 "Super Scooper"*)
- Ground-based drafting for fire engines and water tenders

Figure 3 shows how the helicopters and water scooping aircraft operate.

## 3.4 Power

The Lake County receives its electrical power through PG&E's broader transmission grid rather than directly from the PVP's generation facilities. When fully operating, this hydroelectric project represented less than 0.1% of PG&E's total generation capacity, and it has not been generating power since July 2021, as PG&E determined it would not be cost-effective to repair an equipment malfunction (PG&E, 2023).

While Scott Dam and Lake Pillsbury are physically located within the County, the project's 9.4-megawatt hydroelectric generation capacity historically served other regions through two powerhouses - the Potter Valley (*7.5 MW*) and Cape Horn (*1.9 MW*) facilities, which together produced approximately 19,900 megawatt-hours each year.

The Indian Valley Dam Hydroelectric Plant, owned by Yolo County Flood Control Water Conservation District, currently operates in the Lake County, along with the adjacent Indian Valley Reservoir. Although the reservoir is in the County of Lake, it was built by neighboring Yolo County, which owns all water rights to the 300,600 acre-feet (370,800,000 m3) of water.

Features of the Indian Valley Hydroelectric Plant are as follows[5]:

[5] *Indian Valley Dam*. (2024, June 5). Yolo County Flood Control & Water Conservation District. https://ycfcwcd.org/indian-valley-dam/

Capacity and Turbines. The plant has a total generation capacity of 2.5 megawatts, achieved through the use of two turbines, each capable of generating 1,400 kilowatts, along with a smaller 100-kilowatt turbine.

Reservoirs: The infrastructure includes the Indian Valley Reservoir, which has a maximum capacity of 300,600 acre-feet of water. This reservoir plays a crucial role in water storage and management, allowing for controlled releases to optimize power generation and maintain ecological flow requirements.

Water Management Features: The plant's infrastructure includes diversion channels that direct water from Indian Creek to the powerhouse, ensuring a consistent flow for electricity generation. Additionally, the dam's gates control water flow into the spillway, which can convey over 39,000 cubic feet of water per second.

Powerhouse: The powerhouse where the turbines and generators are located, convert the kinetic energy of flowing water into electrical energy. The larger generators are primarily used during the irrigation season, while the smaller generator is utilized to maintain minimum habitat flows.

Transmission Lines: High-voltage transmission lines connect the plant to the local power grid, facilitating the distribution of generated electricity to consumers. The lines are owned and operated by PG&E.

Regulatory Compliance: The operations of the Indian Valley Hydroelectric Plant are subject to regulations from FERC.

"The Geysers" (*partially in Lake County*) is the world's largest geothermal complex and supplies significant renewable power to the region as part of the multiple generating units operated by Calpine Corporation in the geothermal field. The energy transmission infrastructure is connected to PG&E's regional transmission system. Major transmission lines cross through the county and local distribution networks serve populated areas through a mix of overhead and underground lines, and substations throughout the county. Rural areas are served by longer distribution lines.

Figure 4 presents a general overview of the area's power supply, while some residences and commercial establishments are connected to the grid, most of the area's power primarily relies on independent solar arrays and generators.

## 3.5 Geotechnical

Based on the geologic map of the area (USGS/Ohlin, 2010), the dam appears to be founded on Ophiolitic mélange and sedimentary rocks of Split Rock (*Early Cretaceous or younger*), mapped as green in Exhibit 5 and Figure 5. Bedrock appears to dip steeply (66 degrees) to the southwest at the dam. This unit locally includes blocks and lenticular bodies of ultramafic rock (spsr), mapped as purple in Exhibit 5 and Figure 5. There is a landslide mapped on the south bank of Lake Pillsbury, just upstream of the left abutment. The landslide area has a localized ultramafic rock intrusion in its easterly portion.

The original dam was intended to be constructed with a straight crest, with a spillway section 485 feet long at an elevation of 1,900 feet, and a total crest length of 679 feet, with abutments at either end rising 10 feet higher. After the dam had been completed to 515 feet from the north abutment, it was decided to connect with the south abutment by constructing the remainder of the dam at an angle of 44 degrees with the original structure. This was presumably to avoid the landslide area and to connect with the solid rock area further downstream (USGS/ DWR, 1964; McMillen Jacobs Associates, 2021). Citing stability concerns, PG&E made the determination to leave the

spillway gates permanently open, and the current reservoir capacity is operated at least 20,000 AF lower than the initial maximum capacity.

It is suspected the dam foundation would have been placed on solid bedrock throughout the progression of its construction from north to south, which led to this dog-leg geometry. The available geologic mapping (Exhibit 5) supports the high likelihood that the mélange extends from the north bank to the south bank underwater, beneath the present-day alluvium/sediment deposits. It is quite unlikely the dam's foundation would have been constructed on top of alluvium or landslide deposits. However, while this has not been investigated appropriately, this adjacent landslide issue is being presented as the main reason to justify dam removal.

A landslide near the left abutment (*just upstream of the abutment*) has been known since the 1920s. In the 1970s, inclinometers installed on that slope reportedly sheared. As far as we know, this adjacent slide has not affected the dam structure for over a century because the dam is situated on a different formation than the slide. There is no information that the dam itself is under any verifiable distress. If, as speculated by the applicant, the abutment was under earth pressures and/or if the southern end of the dam was found on materials other than bedrock, there would have been mention of distress, compression cracks, settlement, or other visible indications of distress at the dam. The landslide is likely located adjacent and upstream of the dam and extends under the dam (Miller Pacific Engineering Group, 2018).

Inasmuch as can be determined through the publicly accessible redacted Study, the seismic slope stability analyses of Gannett Fleming (2023) refer to low factor of safety (FS) values, but no outputs of the analysis have been provided to the County's reviewers. This evaluation appears to have applied incorrect seismic parameters for the simplified (*pseudostatic*) analysis. The Miller Pacific Engineering Group approach to pseudostatic analysis is reasonable (*with a 0.4 reduction factor for the pseudostatic coefficient*), but the Gannett Fleming (2023) pseudostatic analysis appears to have used ground accelerations directly as the pseudostatic coefficients without adjustment. Peer review of the unredacted analysis is recommended.

Draining the lake increases the fire hazard, which will result in slope failures around the lake area. The area would be prone to greater instability and require additional maintenance, earthwork, and disposal. Post-burn instability of slopes can persist for many years.

The landslide depth 110 ft; volume of 8 million cubic feet (cf); mass of 520,000 tons; seismic displacement analysis estimating 3' to 19' movement – all are relevant to the adjacent area and may not be directly applicable to the dam location.

The geologic information points to the presence of Serpentinite (USGS/Ohlin, 2010) which has a Naturally Occurring Asbestos (NOA) hazard. Demolition of the dam abutments which were cast on bedrock would likely involve earthwork and other activities that could release fine particles from this formation. When these become airborne, they would impact the health of the population within an area of considerable extent, depending on the wind conditions and other factors.

Serpentinite and NOA would be a hazard should demolition of the dam abutments proceed. They were cast on bedrock, and earthwork and other activities could release fine particles that become airborne. This could affect the health of the population, potentially within an area of considerable extent, depending on wind conditions and other factors.

## 3.6 Sediment

The watershed reporting to Lake Pillsbury is approximately 183,000 acres, nearly 8% of the total Eel River drainage basin (*2.3 M acres*). An estimated 21,000,000 cubic yards of sediment have been impounded in Lake Pillsbury since construction of Scott Dam (Stillwater Science, 2021a).

Sediment thicknesses range from less than 5 feet in the north to over 55 feet to the south in the Rice Fork (Exhibit 6). Approximately 12,000,000 cubic yards of sediment could be available for downstream transport after the removal of Scott Dam (Stillwater Science, 2021a).

Grain size distribution is not well characterized, but the majority of impounded sediment is expected to be fine-grained (*silt and clay particles of less than 0.002 inches in diameter*). These particles can be transported as suspended sediment and, under high flow conditions, can be transported as far as the Eel River estuary in Humboldt County or to the Pacific Ocean (Geosyntec, 2020; Stillwater Science, 2021a).

Gravel is expected to be no more than 10% of total sediment accumulation (*2,000,000 cy*) and is likely deposited in the upstream reaches of the lake. Gravel, therefore, is not expected to mobilize beyond the reservoir footprint and would likely be redeposited entirely upstream of the current dam location (Stillwater Science, 2021f).

Possibly as much as 30% of the deposit is sand (Geosyntec, 2020; Stillwater Science, 2021a). A sand pulse can create a deeper deposit further downstream following dam removal than a gravel pulse, and the portion of the Eel River between Scott Dam and the County line could experience temporary sand bar creation. However, most sand deposition is expected in Van Arsdale Reservoir. Sand deposition in the reach upstream from Van Arsdale could be as much as 5 feet but is expected to be transient and would be removed during subsequent high flow events (Stillwater Science, 2021f).

Removal of Scott Dam under any scenario involving natural sediment erosion will result in extremely high ($10^5$ mg/L) suspended sediment concentrations for durations ranging from a few days in one season, to several hundred days over a period of years, depending on the removal alternative selected (Stillwater Science, 2021f).

The removal of Scott Dam using the vertical notching (*or similar rapid removal*) method is estimated to result in a four-day release of high suspended sediment at an approximate concentration of 600,000 mg/L(Stillwater Science, 2021f).

Sediment transport modelling was performed by Stillwater Sciences using the Dam Removal Express Assessment Models (DREAM). The results provided simulated bed profiles following dam removal (Exhibit 7). In general, the models showed bed profiles are expected to return to pre-dam conditions within a few years (even within a few months for some scenarios) of dam removal (Stillwater Sciences, 2021f).

Some alternatives to managing the lake deposits in such a way that variable amounts of erosion of fine sediment would occur have been evaluated by Jacobs (2018) and others. Generally, these involve mechanical removal or stabilization of most sediment prior to or during removal.

Significant short-term impacts on aquatic life (*i.e. high mortality*), particularly to salmonids, are expected to result from high suspended sediment concentrations. However, one of the most important species (Steelhead, [*Oncorhynchus mykiss irideus*]) are able to tolerate such conditions due to their broad spatial distribution and flexibility within their lifecycle (Stillwater Sciences, 2021c).

As discussed in geotechnical aspects, NOA is known to occur in the Lake Pillsbury watershed and near Scott Dam (USGS/Ohlin, 2010). Release of impounded sediment could redistribute NOA to downstream receptors.

The environmental sediment characterization found no indication of significant mercury or other contamination. Detected chemicals were below freshwater sediment screening levels with the exception of nickel, which was elevated, but considered consistent with regional background concentrations in the shallow sediments evaluated (Geosyntec, 2020). Despite the presence of naturally occurring mercury within the area and a mercury impaired water body (SWRCB, 2018), concentrations of total mercury and its organometallic cation (methylmercury) were below established screening levels. Since the sediment studies were only conducted for shallow sediments, additional studies are recommended to evaluate metal concentrations and mobility, including but not limited to mercury, for the deeper sediments impounded.

## 3.7 Infrastructure/Water Supply

In 2023, PG&E reduced Scott Dam's capacity by 20,000 ac-ft, due to concerns regarding seismic risk, following concurrence from Division of Safety Dam (DSOD) (PG&E, 2023). Following capacity reduction, FERC requested license evaluation (SWRCB, 2024). PG&E requested amendment to the license by adjusting minimum instream flow requirements for two areas: (1) setting the minimum instream flows in the East Branch Russian River to the lowest levels specified in the FERC license, and (2) reducing minimum instream flows below Lake Pillsbury. PG&E aims to conserve water in Lake Pillsbury during spring and summer, maintain safe facility operations, and ensure cooler water releases to support threatened salmonid species (SWRCB, 2025). "Existing conditions," therefore consider the reduction of the storage capacity with respect to the conditions that had been maintained for the historical operations.

The water supply system in the vicinity of Lake Pillsbury consists of both surface and groundwater. Four small water systems supply 147 residences and provide water for recreational purposes including hotels, resorts and campsites (DWR, 2023). Additionally, a state-operated water system supplies water for seven residences as shown in Exhibit 8 (DWR, 2023).

PG&E holds three licensed and two claimed pre-1914 water rights on the Eel River for power generation, irrigation, and domestic use. Water is diverted from the Eel River to the Potter Valley Powerhouse in the East Branch Russian River Watershed. After generating power, a portion of the outflow is directed to the Potter Valley Irrigation District (PVID), while the remainder flows into the East Branch Russian River, contributing to Lake Mendocino and benefiting downstream users. The licensed and pre-1914 claimed water diversions are as follows (PG&E, 2023):

- **License 1424** (1920): Permits the diversion and storage of up to 102,366 "acre-feet per annum" (afa) at Lake Pillsbury for hydropower generation and incidental fish and wildlife protection and enhancement.
- **License 1199** (1927): Permits the diversion and storage of up to 4,500 afa at Lake Pillsbury for irrigation within the PVID service area.
- **License 5545** (1930): Permits the storage of up to 4,908 afa at Lake Pillsbury and direct diversion of up to 40 cubic feet per second (cfs) from the Eel River for PVID irrigation.
- **SWDU 1010 (Claim pre-1914)**: Permits the direct diversion of up to 340 cfs from the Eel River for power generation and irrigation.
- **SWDU 4704 (Claim pre-1914)**: Permits the storage of up to 1,457 afa in Van Arsdale Reservoir for power generation, irrigation, and domestic use.

Downgradient of Scott Dam, several communities rely on the water supply due to the active management of Lake Pillsbury. In particular, approximately 600,000 people depend on the water in the counties of Lake, Mendocino, Sonoma and Marin (Lake Pillsbury Alliance, 2024a; County of Lake, 2023c). Even with the reservoir in place, the City of Ukiah raised concerns in 2024 due to reduced water supply for the Russian River due to the effects on ecology, economy and the way of life (MendoFever, 2024).

Per a Memorandum of Understanding (MOU)[6] for water diversions and the operation of a new Eel Russian Diversion Facility (NERF), the Parties agree that NERF will operate for an initial term of 30 years, beginning on the date operation begins, and extended an additional 20 years upon the satisfaction of several conditions regarding finding and effects on the ecosystem.

## 3.8 Groundwater

The area surrounding Lake Pillsbury is within the northern California Coast Ranges largely underlain by the Franciscan Complex, composed of a variably metamorphosed marine sedimentary and volcanic rocks of Early Jurassic to early Late Cretaceous age (Exhibit 5, USGS/Ohlin, 2010). The northeast portion consists of Quaternary alluvium and lake deposits, which define the Gravelly Valley Groundwater basin (DWR, 2004). These deposits consist of poorly sorted sands, silts, gravels and boulders from alluvial fans (USGS/Ohlin, 2010).

Well completion records in California, maintained by the DWR, are documents that provide essential information about groundwater wells. SLR reviewed hydro-geological data, such as lithology descriptions, aquifer characteristics, and depths to groundwater encountered during drilling. SLR downloaded available well construction reports from the DWR website (DWR, 2024b).

There are well completion records for 38 groundwater wells in the vicinity of Lake Pillsbury. Reported planned uses include domestic (29), irrigation-agriculture (2), and public (1) water supply, test wells (3), and unknown (3). Figure 6 shows the general location of groundwater wells (*where available*) based on the information provided by DWR, which are approximated by the centroid of each respective township[7], providing a generalized reference point for spatial orientation.

DWR groundwater well records indicate the presence of alluvial deposits in the area of the Gravelly Valley Groundwater Basin. Based on driller's logs (DWR, 2024b), the primary top layers are mainly described as clay and silt with occasional gravels, and average approximately from 1 foot to 50 feet. The sequence layers are interlayers of clay, silt, sand, gravel, and occasional boulders layers. Boulders were generally encountered at depths of approximately 200 feet, though a few were found as shallow as approximately 20 feet. These logs would be generally consistent with deposits within an alluvial fan, showing interbedded layers of sand, silt, gravels and occasional boulders.

[6] The Memorandum of Understanding (MOU) was entered into by the California Department of Fish and Wildlife (CDFW), California Trout, Eel-Russian Project Authority (ERPA), Humboldt County, Mendocino County Inland Water and Power Commission (IWPC), Round Valley Indian Tribes (RVIT), Sonoma County Water Agency (Sonoma Water), and Trout Unlimited.

[7] Township is a land measurement unit within the Public Land Survey System (PLSS), typically a square area measuring six miles on each side, divided into 36 sections, each one square mile in size.

The groundwater level varies from 8 feet to 109 feet below ground surface in the surrounding area of Lake Pillsbury (for the wells shown in Figure 6). The average well depth in the area surrounding Lake Pillsbury is 150 feet. The well depth varies from 20 feet to 440 feet.

## 3.9 Hydrologic and Hydraulic Modeling

SLR reviewed available studies completed for the Eel River reach downstream of Scott Dam in regard to the PVP system. We find the level of confidence in the results of these modeling efforts has been limited by a lack of comprehensive data.

The PVP system has provided hydroelectric power and continues to deliver water from the upper Eel River watershed to the headwaters of the Russian River. According to the Potter Valley Irrigation District[8], the project was initiated in the early 1900s when the Cape Horn Dam was constructed to create the Van Arsdale Reservoir, which serves as a forebay for the diversion tunnel leading to Potter Valley. The over-one-mile tunnel diverts water from the Van Arsdale Reservoir before flowing through a penstock that drops over 450-feet into the Potter Valley Powerhouse, which started supplying power in 1908. A second penstock was constructed in 1912 and by 1917 the Potter Valley Powerhouse supplied 9.4 megawatts of power. The initial setup of the Potter Valley Powerhouse was only able to run at limited capacity during the winter flows, as the water supply from the Eel River diversion was insufficient to run the powerhouse during the summer. This initiated the planning and construction of Scott Dam which was completed in 1921[9], according to the US Army Corps of Engineers (USACE).

Lake Pillsbury had an approximate storage capacity of 76,876 acre-feet (ac-ft) with a current usable capacity of 66,876 ac-ft. The Eel River continues 12 miles downstream of Scott Dam before entering the Van Arsdale Reservoir created by Cape Horn Dam. Cape Horn Dam is an earthen and concrete, gravity overflow spillway dam that has a pool and weir fish ladder. The Van Arsdale Reservoir had an initial storage capacity of 1,100 ac-ft but has since filled with sediment and is closer to 390 ac-ft. The Van Arsdale Reservoir facilitates the Van Arsdale Diversion to the Potter Valley Powerhouse. The Potter Valley Powerhouse is located in the Upper Russian River watershed and supplies water to Lake Mendocino which is a major water supply source for irrigation, municipal and domestic water supply, recreation, and the support of salmon and steelhead populations. Water that is not diverted toward the Russian River is released over Cape Horn Dam through the Eel River approximately 150 miles northwest to the Pacific Ocean. Exhibits 1 and 2 show existing PVP facilities and features as well as the Eel River and Russian River Watersheds. (PG&E, 2023)

SLR's technical review of existing hydraulic modeling documents (summarized in Appendix B) provides the general opinion that the data used in the modeling and reporting is presumed to be insufficient to provide a high level of confidence that the removal of Scott Dam will provide a reliable water supply and that the long-term impacts are not understood from a hydrologic and hydraulic perspective especially regarding understanding changes in flood frequency and impact. The lack of adequate data is affirmed in the assumptions, limitations, and recommendations made in many of the reviewed reports and studies summarized above. The data and analyses of concern primarily include the limited data used to determine unimpaired flows (there is no gage data with unimpaired hydrology and no precipitation gages in the area), the uncertainty of pre-

[8] The Potter Valley Project - https://www.pottervalleywater.org/potter_valley_project.html

[9] United States Army Corps of Engineer's National Inventory of Dams: National Inventory of Dams (https://nid.sec.usace.army.mil/#/dams/system/CA00398/summary)

dam conditions on the riverine hydraulics including the alignment and longitudinal profile, hydraulic contributions and timing from tributaries particularly upstream of Scott Dam, and the limited data for the Scott Dam impoundment sediment gradation and sediment transport throughout the system. The unimpaired flow assumptions are used as a basis for many of the analyses and should those change, it would create a compounding impact on all hydraulic and sediment transport assumptions. A higher level of uncertainty in the data creates a higher level of risk and lower confidence in predictions under future conditions.

There is also a concern that many of these reports do not address the entirety of the project and only limited components. This has created piecemeal information that has built on assumptions developed from PG&E efforts that were not completed or fully vetted for confidence.

## 3.10 Roadways

The Mendocino National Forest Motor Vehicle Use Map shows the primary roads around Lake Pillsbury are paved, but most are gravel and dirt roads. The main access road is Elk Mountain Road, also known as County Road 301, which is a paved two-lane roadway without lane markers or shoulders. County Road 301 connects to National Forest Primary Road M1, running from the southwest to the northern part of Lake Pillsbury. Many of the surrounding dirt roads are suitable for 4-wheel drive (4WD) vehicles and are not maintained for standard passenger vehicles. Some of 4WD roads are open seasonally, from September 9 to May 17, located mostly to the North and Northwest of Lake Pillsbury (USFS, 2013). The access roads in the vicinity of the Lake and along the Eel River are shown on Figure 7.

PG&E maintains a few private roads, including the Gage E2 Access Road, which is a gated unpaved narrow road west of Scott Dam; Scott Dam Road, which is an unpaved one-way road partially accessible to the public but mostly gated; and the Upper Scott Dam Access Road, which is a gated portion of the Scott Dam Road that provides access to the north side of Scott Dam. These private roads are exclusively used by PG&E.

PG&E (2025) provided several potential decommissioning activities, including a proposed haul route for material transport. Their proposal included the construction of temporary roads to provide access and road improvements to facilitate equipment access. PG&E also considered the potential use of helicopters for transporting equipment and materials, if necessary.

Based on the flood zone map available on the County of Lake website and the 1973 inundation map of Scott Dam, the 100-year storm event (*1% annual chance floodplain*) would flood the northern portion of Lake Pillsbury, including the northern access road, Hull Mountain Road. Another significant area of inundation would be the southeastern portion of the lake, though no major access roads would be affected. The creeks and rivers that flow into Lake Pillsbury will experience backup, such as Salmon Creek, Smokehouse Creek, Soda Creek, Eel River and the Rice Fork. However, the western portion that runs parallel to Soda Creek, Elk Mountain Road, would also be inundated. The southern portion of the Eel River would be flooded downstream of Scott Dam and it likewise appears the crossing road over the Eel River, Elk Mountain Road will be inundated as shown in Figure 8 (Hydrocomp, 1973).

## 3.11 Restoration

Restoration of the drained lake footprint, as well as sites where recreational facilities will be removed will be critical to minimizing visual scarring and resulting economic impacts to the County of Lake and the broader region.

Proposed Decommissioning of the Project would include removal of Scott Dam, followed by site restoration. It is currently anticipated that restoration of the remnant inundation zone of Lake Pillsbury and Van Arsdale Reservoir will take place following removal. Detailed restoration is not discussed in Surrender Application. Restoration plans will be required as part of California Environmental Quality Act (CEQA/National Environmental Policy Act (NEPA). A conceptual restoration plan is anticipated to be included in the Draft Final Surrender Application (PG&E, 2023).

PG&E proposes to remove and restore recreational facilities (*e.g., campgrounds; day-use facilities; recreation access roads and trails; kiosk; and boat ramps*) located on Forest Service and PG&E lands. The timing for the proposed removal and restoration of the facilities that are not located within the Scott Dam is not included in the Draft Surrender Application.

PG&E proposes to abandon in place and cap underground Project facilities, such as piping; the Project will leave in place facility access roads on private lands. A plan needs to address restoration to meet US Forest Service and/or the County of Lake needs.

As discussed in Section 3.6, PG&E expects riverbed profiles to return to pre-dam conditions within a few years of dam removal (PG&E, 2025). As part of the proposed activities, reservoir sediment flushing would be conducted in a high-flow season as part of dam removal.

In the case of similar dam removal projects (such Lower Klamath and Elwha River), restoration plans included revegetation of the areas disturbed by project deconstruction and of the reservoir footprints (FERC, 2022). For the Lower Klamath project, the revegetation time was estimated to be 5 years following the complete removal.

## 3.12 Economic Conditions

An economic analysis should consider factors such as agricultural, commercial, residential, municipal and recreational elements, and develop a detailed list of affected entities in the above categories including employment, total revenues and tax revenues.

Economic analyses of varied level of detail and geographic focus have been completed over several decades, documenting the economic resources in the affected region and the projected economic impacts of the PVP. The focus on Lake County and Lake Pillsbury in these analyses is limited and/or analysis needs to be updated with the latest information. In the following section we have presented summaries of four relevant regional focussed economic studies followed by County of Lake specific tax revenue information that can be used in estimating future loss of tax revenue to the County with respect to the decommissioning of Scott Dam and Lake Pillsbury.

### 3.12.1 Summaries of Regional Focus Economic Studies

**FERC Environmental Impact Statement (EIS) completed in 1978 regarding License Renewal:**

As reported in the Environmental Impact Statement (FERC, 1978), the PVP project area had five major economic sectors: agriculture; manufacturing; outdoor recreation and tourism; retirement and vacation housing; and the Federal government. The estimated value of recreation at Lake Pillsbury was $4,480,000 per year (1989 dollars; FERC, 2000). In 1999, the employment associated with Lake Pillsbury was estimated at 152 jobs per year (FERC, 2000). PG&E's Re-licensing Pre-Application Document (PAD) submitted in April 2017 summarized the economic activity in the region as of 2015, with the primary sectors in Lake County being health care and social assistance (20.9%), government (17.2%) and retail trade (12.2%), followed by other

services (except public administration), construction, accommodation and food services (PG&E, 2017). PG&E's Re-licensing PAD submitted in April 2017 details economic indicators for Lake, Mendocino and Sonoma Counties. The report also shows major employers in the Lake County. The PAD was prepared for relicensing purposes, and documents economic indicators at the time.

**Cost Analysis report prepared by CEED in 2024 for the Friends of the Eel River:**

The last regional detailed cost analysis pertaining to PVP related dam removal was performed by the Center for Environmental Economic Development (CEED) in 2004 on behalf of the Friends of the Eel River. The report solely looks at the benefits of restoring fishing and rafting trips on the overall Eel River watershed. It neglects specific impacts to Lake Pillsbury and the County of Lake. It also does not address losses of property values, tax revenue loss, and impacts of loss of water resources and impacts to the water basin at Lake Pillsbury or Lake County.

**Economic analysis report prepared by Economic Forensics in 2023 for the Sonoma County:**

The most recent economic analysis was performed in 2023 (Economic Forensics and Analytics, 2023) and estimates the effects of the removal of the PVP on the Sonoma County economy. This preliminary analysis focused on loss of water resources and demonstrated that all water demand sectors would be affected, including government, wholesale, retail and healthcare jobs.

| Sonoma County: 10% Loss of Water Availability, Annual Loss | | | |
|---|---|---|---|
| **Sector** | **Incomes Lost** | **Jobs Lost** | **Tax Revenues Lost** |
| Agriculture | $6,174,100 | 53.1 | $32,600 |
| Commercial | $50,912,500 | 242.2 | $2,416,200 |
| Residential | $75,200 | 0.3 | $2,600 |
| Municipal | $7,897,500 | 1.4 | - |
| Total | **$65,059,300** | **297.0** | **$2,451,400** |

As presented in the table above, this study shows annual lost income of $65M and annual tax revenue loss of $2.45M. The study outlines that, while the project will have short-term positive economic impacts, those effects will be mainly due to employment created during dam removal (*assumed by Economic Forensics and Analytics to occur over a 4-year period*). The long-term impacts related to cumulative losses at 10% annual average loss of water are presented in Exhibit 18. As based on this exhibit, long-term impacts to Sonoma County are detrimental. The income losses will also affect tax revenue, employment, real estate prices and tax revenue from real estate.

**Study prepared by Bay Area Council Economic Institute in 2023:**

An analysis by the Bay Area Council Economic Institute (2023) estimated the impacts of dam removal in terms of positive impacts from project spending and job creation, with the impact of Scott Dam estimated at 831 to 907 direct, indirect, and induced full-time equivalent job-years for the region (Humboldt, Lake, Marin, Mendocino, and Sonoma counties), not specifically to Lake County. These jobs are expected to be transitory during the decommissioning process. This study noted the nonquantifiable benefits as improved ecosystem for fish passage and noted potential benefits due to improved fishery and recreation, which are long-term benefits.

### 3.12.2 County of Lake Specific Tax Revenue Information

In the areas immediately surrounding Lake Pillsbury, county tax revenues are related to secured and unsecured property taxes, forest service cabins, sales tax and PG&E unitary tax payment related to property they occupy. SLR has reviewed the tax revenue information provided by the County of Lake and summarized information as presented below.

The tax revenue for year 2024 is approximately $872,000 showing a 3.5% increase from the actual 2023 values. These costs are broken down in the table below.

**2024 Taxes Due**

| Tax Source | Taxes collected |
|---|---|
| Current Secured (2024) | $ 581,976 |
| Current Unsecured (2023) | $ 6,700 |
| Current Unsecured Forest Service (2024) | $ 64,380 |
| Transient Occupancy Tax (TOT) + Sales Tax (2019-2020 estimate) | $ 16,400 |
| PG&E (2023) | $ 202,356 |
| **Total** | **$ 871,812** |

**Source:** Lake County Tax Collector, May 2025

More details of the above presented 2024 property taxes and Forest Service Cabins for 2024 are provided in tables below and show that the numbers are in general agreement with the 2023 values. As seen from the table above, majority of the income is from two sources: property taxes and PG&E taxes.

Focusing in on Lake County, based on County tax collector data, properties pay taxes to the County at a rate of $1.0 per $100 in net taxable value. Properties with a Lake Pillsbury address (570 unique assessment numbers) generated $501,462.45 in taxes due to Lake County 2024, with a total net taxable value of $50,146,245.00.

Other taxes and fees for Lake Pillsbury properties include Upper Lake High Schools Bonds, Upper Lake School Facilities Improvement Districts (SFID) Bonds, Upper Lake Unified School Bonds, and Mendocino Community College Bonds. See Tables below.

**2024 Taxes Due, Lake Pillsbury Parcels**

| Tax | Tax Rate Per $100 in Net Taxable Value | Taxes Due |
|---|---|---|
| County | 1.0 | $501,462.45 |
| Upper Lake High School Bonds 1999 | 0.000001 | $0.50 |
| Upper Lake SFID Bonds | 0.076999 | $38,612.11 |
| Upper Lake Unified School Bonds | 0.031815 | $15,954.03 |
| Mendocino Community College Bond | 0.021000 | $10,530.71 |
| **Total Taxes Due** | | $566,559.80 |
| **Total Taxes Due Plus Direct Charges, Fees, and Penalties** | | **$581,976.20** |

**Source:** Lake County Tax Collector, October 2024

A review of 2024 County Tax Parcel data shows that there are 68 Forest Service cabins that generated a total of $58,234.07 in taxes due to Lake County in 2024, with total net taxable value of $5,823,407.00.

**2024 Taxes Due, Forest Service Cabins**

| Tax | Tax Rate Per $100 in Net Taxable Value | Taxes Due |
|---|---|---|
| County | 1.0 | $58,234.07 |
| Upper Lake High School Bonds 1999 | 0.02604 | $1,516.42 |
| Upper Lake SFID Bonds | 0.04332 | $2,522.70 |
| Upper Lake Unified School Bonds | 0.01619 | $942.81 |
| Mendocino Community College Bond | 0.02 | $1,164.68 |
| **Total Taxes Due** | | $64,380.68 |
| **Total Taxes Due Plus Direct Charges, Fees, and Penalties** | | **$68,399.61** |

**Source:** Lake County Tax Collector, October 2024

Additional taxes not captured in the tables above for the 2024 tax year include the PG&E unitary tax at approximately $202,000, and sales and occupancy taxes of approximately $16,400. These values have been provided from actual Tax Collector data for various years as documented in the table below:

**2023 Taxes (Actual)**

| 2023 Tax Source | Taxes collected |
|---|---|
| Current Secured | $ 547,016.12 |
| Current Unsecured | $ 6,700.34 |
| Current Unsecured Forest Service | $ 70,501.00 |
| Property Tax Total | $ 624,217.46 |
| TOT + Sales Tax (2019-2020 estimate) | $ 16,400.00 |
| PG&E (2023) | $ 202,356.00 |
| **Total Taxes** | **$ 842,973.46** |

**Source:** Lake County Tax Collector, May 2025

If Scott Dam is removed the Forest Service cabin occupancy, and the property values associated with properties located with a Lake Pillsbury address will be negatively affected. These impacts have not been evaluated and should be included in an economic evaluation. In addition, PG&E will likely stop or reduce the unitary tax payments. Mitigation measures should be established to address potential losses of income to the County of Lake as a result of dam removal.

Further, if there is no dam, property owners that purchased property in the last few years will face hardship, as their property values will likely be reduced. They will have a hard time refinancing their properties since the lending agencies will not give them a similar valuation. This may force foreclosures further deflating the market in this area causing a spiralling downward effect in property values and in turn taxes.

In addition, there are long term costs associated with the dam removal that affect the County of Lake's expenditures. These are related to infrastructure costs associated with flooding and stream inundation during severe rain events. As part of the dam removal process, PG&E should develop and document these long-term costs.

## 3.13 Community Feedback

The following are a list of public comment resources SLR has reviewed[10]:

- Comments from interested parties on the Draft Initial Surrender Application (Nov 2023)
  - Specific comments with respect to impacts to the Lake County (Sierra Club, County of Lake Board of Supervisors, Lake Pillsbury Alliance, US Forest Service)
  - 53 comment letters
  - 500+ emails
- Draft PG&E application

[10] SLR downloaded and reviewed comment letters and emails from the *Potter Valley Project (FERC Project No. 77) Surrender Application and Decommissioning Plan Stakeholder Website* https://www.pottervalleysurrenderproceeding.com

- Local community websites cited in Table 1
- CalWild is a California-based public benefit corporation, which goal is to protect and restore California's landscape. As part of the PVP decommissioning process their role is to develop collaborative consensus, their work is funded by grants from the California Coastal Conservancy and the Resources Legacy Fund.

The majority of comments (*in aggregate*) were from pro-decommissioning groups, including the Two-Basin Partnership, Friends of the Eel River, Free the Eel, Native Fish Society, and more. There appear to be concerns about inadequate community outreach and impact assessment by PG&E regarding the proposed Scott Dam Decommissioning Plan and potential draining of Lake Pillsbury. Despite some partnerships being established from across the region to work collaboratively to prepare for FERC's consideration of dam removal, and attendant needs should Decommissioning proceed, it is notable input and feedback from the Lake County community, ground zero for the proposed Decommissioning, is lacking.

Commenters who did not appear to favor decommissioning align with the concerns raised in the December 21, 2023, letter from the Lake Pillsbury Alliance regarding inadequate stakeholder engagement. Specifically, the County of Lake and county stakeholders (including the Lake Pillsbury Fire Protection District, local property owners, and businesses) do not appear to be explicitly included in the consultation process described in PG&E's June 2024 status update, nor in a multitude of public documents aligned with the Scott Dam Decommissioning Plan.

This community outreach issue is ongoing, with multiple forums in and out of Lake County continuing to share information and gauge various ideas and proposals on how to move forward with PG&E's surrender application, including ways to save Lake Pillsbury, albeit, without the hydroelectric generation capability, or to construct dams and hydroelectric projects at other locations within adjoining counties, including Mendocino and Sonoma.11

CalWild summarized findings resulting from over 100 meetings [with respect to PG&E's Draft Surrender Application of License] to identify stakeholder comments (CalWild, 2025). The topics identified are the loss of recreation opportunities with limited mitigation, the lack of terrestrial restoration, in particular in Gravelly Valley, and the need for improvement on fire resiliency. The summary of the engagement also highlights a need to invest and help the US Forest Service and Tribes to protect cultural resources, landscapes and important sites. Furthermore, the letter notes that "many stakeholders have expressed concerns that the Mendocino Land Trust's conservation easement will limit restoration, recreation enhancements, and other much needed work both on the Lake Pillsbury footprint and elsewhere."

# 4.0 Next Steps to be Completed

Based on the existing conditions summarized in this report, we recommend the following steps to move forward with evaluating the potential impacts of removing Scott Dam. We suggest conducting preliminary impact analysis for selected items listed below as part of Task 3 "Impact Analysis and Adaptation Strategy Evaluation" of the DWR grant. SLR will continue to closely coordinate with the County of Lake to identify priorities of the evaluation.

[11] Huettl, M., & Huettl, M. (2024, November 29). *Decommissioning Looms: Potter Valley faces urgent water challenges - MendoFever – Mendocino County News.* MendoFever – Mendocino County News. https://mendofever.com/2024/11/29/decommissioning-looms-potter-valley-faces-urgent-water-challenges/

- Recreational impacts will be evaluated, we understand there may be a loss of existing campsites on Forest Service lands, and loss of the lake may affect the type of fishing or whitewater rafting users can access. The change in recreational activities will also be considered as part of revenue generated by visitor usage, and losses are anticipated to be experienced in the area.
- The existing ecosystem would be affected by Decommissioning Scott Dam and draining Lake Pillsbury. Additional information regarding Pillsbury Lake's ecosystem and existing species needs to be evaluated. A list of threatened and endangered species (defined as “special-status species” occurring in the project boundaries was released in the Draft Application for Surrender of License. Maps of the species occurrences documented in the study area are confidential. SLR plans to conduct reviews and complement information generated by PG&E.
- To address potential negative impacts of Scott Dam removal on wildfire suppression and vegetation management, several mitigation strategies and recommendations should therefore be considered and adopted.
- Impacts associated with power generation appear to be limited. Further evaluation will be conducted if required by the County of Lake.
- A limited geotechnical evaluation focusing on adaptation strategies for the County of Lake following dam removal will be conducted. It is anticipated that detailed stability assessments for sediments remaining in place is conducted by PG&E as part of the surrender process.
- Impacts associated with sediment release, both within the reservoir footprint and downstream, can be very significant. These may affect water quality, water use and infrastructure, aquatic life, overall river health, flood plains and flood stage, channel morphology and upstream topography. Based on other documented dam removal projects, sediment dams with large watersheds which have been in place for many years typically impound large volumes of sediment, much of which can be mobilized during dam removal.
- We will further evaluate the local use of Lake Pillsbury water for beneficial uses, including drinking, recreational, and others, and how loss of this water supply will be mitigated.
- Because of the presence of NOA, air quality is anticipated to be impacted during dam removal as serpentine rock may be disturbed, and following the dam removal as NOA-containing soils and sediments are mobilized.
- SLR proposes to conduct additional hydrogeological review, including geolocation of known wells, and qualitatively evaluate potential impacts to groundwater due to the drainage of Lake Pillsbury. It is expected groundwater storage is reduced, and groundwater levels to be lower due to the loss of the lake.
- Further hydrologic and hydraulic modeling will be conducted to understand the effects of Scott Dam decommissioning on existing infrastructure, due to the potential for flooding during high precipitation events, and the effects on water supply, given the loss of the storage capacity and hydrologic control. Considerations to future climate scenarios is to be included in these models.
- We will evaluate potential impacts on roadways should the dam be removed.

- Consideration will be given to evaluate proposed restoration efforts, and how the restored environment can be beneficial for future users. Based on the proposed restoration strategies, mitigation can be proposed to reduce the anticipated impacts to the recreation, ecosystem and other aspects.
- SLR will gather information to conduct an economic analysis and impact assessment on the region should Scott Dam be removed.
- We will evaluate different strategies and aspects and provide preliminary cost analysis for one or two County-selected adaptation scenarios. We will consider factors such as feasibility, cost-effectiveness, and scalability when selecting adaptation strategies, and include as part of this analysis.
- We have summarized existing outreach efforts being conducted as part of the decommissioning process. SLR will consider the community concerns to suggest paths for mitigation and adaptation.

By following these steps, we will evaluate the potential impacts of removing Scott Dam and losing Lake Pillsbury and recommend effective adaptation strategies to minimize the negative consequences.

# 5.0 Remarks

The services described in this work product were performed in accordance with generally accepted professional consulting principles and practices. No other representations or warranties, expressed or implied, are made. These services were performed consistent with our agreement with our client. This work product is intended solely for the use and information of our client unless otherwise noted. Any reliance on this work product by a third party is at such party's sole risk.

Opinions and recommendations contained in this work product are based on conditions that existed at the time the services were performed and are intended only for the client, purposes, locations, time frames, and project parameters indicated. The data reported and the findings, observations, and conclusions expressed are limited by the scope of work. We are not responsible for the impacts of any changes in environmental standards, practices, or regulations subsequent to performance of services. We do not warrant the accuracy of information supplied by others, or the use of segregated portions of this work product.

This work product presents professional opinions and findings of a scientific and technical nature. The work product shall not be construed to offer legal opinion or representations as to the requirements of, nor the compliance with, environmental laws rules, regulations, or policies of federal, state or local governmental agencies.

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# Tables

## Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

June 6, 2025

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

## Parties with Direct Involvement

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| Potter Valley Project | The organization supports Congressman Jared Huffman's collaborative Ad Hoc process, which will develop recommendations to inform FERC relicensing of the Potter Valley Project. | The Potter Valley Project promotes a Two-Basin Solution which incorporates the following elements: (1) improve fish passage, (2) minimize impacts to water supply, (3) respect tribal rights, (4) minimize impact to Lake County businesses/residents, (5) pursue public funding. | -Two-Basin Solution Partnership |
| Huffman Ad Hoc Committee | Congressman Jared Huffman has convened stakeholders called Congressman Jared Huffman's Potter Valley Project Ad Hoc Committee. They complement the formal FERC relicensing process. The Committee has no formal authority, yet it also has no constraints – thereby enabling constructive dialogue among stakeholders and consideration of a broad range of scenarios for the future of the Potter Valley Project. The Ad Hoc Committee serves as a valuable platform for regional water planning while also addressing salmonid population recovery. | The Ad Hoc Committee identified two key topics that are fundamental to the Potter Valley Project: (1) fish passage above Scott Dam and (2) water supply options. At its January 2018 meeting, the Ad Hoc Committee formed two technical working groups to examine these issues in a focused manner. Since 2018, **the Fish Passage and Water Supply Working Groups** have examined these issues and have been reporting their findings to the full Ad Hoc Committee for consideration. | - Two-Basin Solution Partnership<br>**Participants:**<br>-Bear River Rancheria<br>-California Department of Fish and Wildlife<br>-California Trout<br>-City of Ukiah<br>-Coyote Valley Band of Pomo Indians<br>-Dry Creek Rancheria<br>-Friends of the Eel River<br>-Humboldt County<br>-Inland Water and Power Commission<br>-Lake County<br>-Mendocino County<br>-National Marine Fisheries Service<br>-North Coast Regional Water Quality Control Board<br>-Pacific Coast Federation of Fishermen's Association<br>-PG&E<br>-Potter Valley Irrigation District<br>-Round Valley Indian Tribes<br>-Russian Riverkeeper<br>-Sonoma County |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| | | | -Sonoma Water<br>-SWRCB<br>-The Nature Conservancy<br>-Trout Unlimited<br>-U.S. FWS<br>-U.S. Forest Service<br>-Wiyot Tribe |
| Fish Passage Working Group | The Fish Passage Working Group has been developing information and recommendations on fish passage for the Ad Hoc Committee. The Ad Hoc formed this working group to develop detailed technical information and work to support Ad Hoc group's mission. | -Identify existing information and best available science<br>-Develop objectives for fish passage<br>-Identify viable solutions (near term, longer term) | Huffman Ad Hoc Committee |
| Water Supply Working Group | The Water Supply Working Group is responsible for collecting data and providing recommendations regarding water supply for the Ad Hoc Committee. The Ad Hoc formed the working group to develop and analyze detailed water supply scenarios and recommendations for managing water supply issues and concerns to inform the Ad Hoc group's discussions. | -Identify water supply issues on Eel and Russian Rivers<br>-Develop objectives for water supply<br>-Identify viable solutions (near term, longer term)<br>-Assess consequences, opportunities, and challenges of potential futures for the Potter Valley Project | Huffman Ad Hoc Committee |
| Friends of the Eel River | Friends of the Eel River engages in projects that support recovery of the Wild and Scenic Eel River, its fisheries, and its communities. | No additional information available. | **Consultants Used:**<br>- Miller Pacific Engineering Group<br><br>**Affiliated With:**<br>- CEED<br>Indigenous Tribes:<br>- Wiyot Tribe<br>- Bear River Band Rancheria<br>- Cahto Tribe of Laytonville Rancheria<br>- Round Valley Indian Tribes |
| Two-Basin Solution Partnership | The Two-Basin Solution Partnership develops licensing strategies that will materially benefit both basins. | In 2018, Congressman Jared Huffman convened a diverse stakeholder group in a collaborative Ad Hoc process designed to develop recommendations to inform the FERC relicensing process for the Potter Valley Project. The Two- | - Huffman Ad Hoc Committee<br><br>**Participants:** |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| | | Basin Partnership is a direct outgrowth of that collaborative effort, which brought together diverse stakeholders to develop a compromise solution for the future of this aging hydroelectric project. | - California Trout<br>- County of Humbolt<br>- The Mendocino County Inland Water and Power Commission<br>Round Valley Indian Tribes<br>- Sonoma County Water Agency<br><br>**Affiliated Consultants**:<br>- Stillwater Sciences |
| Notice of Intent (NOI) Parties | The NOI Parties came together to form a partnership to reduce uncertainty regarding the fate of the Project after PG&E's withdrawal, and to create a positive outcome that meets the shared objectives for stakeholders in both the Eel and Russian river basins. | The NOI Parties are committed to acquiring the Project and meet the shared objectives of a Two-Basin Solution.<br><br>The NOI Parties intend that a Regional Entity will be formed to file a new license application for the Project, and the new license will reflect modifications to achieve the Shared Objectives of a Two-Basin Solution for the mutual benefit of the Eel River and Russian River basin stakeholders. | **Participants:**<br>- Mendocino County Inland Water and Power Commission<br>- Sonoma County Water Agency<br>- California Trout<br>- County of Humboldt<br>- Round Valley Indian Tribes |
| Joint Proponents | No additional information available. | Proponents are committed to the coequal goals of (1) improving fish migration and habitat on the Eel River with the objective of achieving naturally reproducing, self-sustaining, and harvestable native anadromous fish populations and (2) maintaining material and continued water diversion from the Eel River through the existing tunnel to the Russian River to support water supply reliability, fisheries, and water quality in the Russian River basin. | - Seek to receive support for the proposal from the National Marine Fisheries Service and representative government and non-governmental entities in the Russian and Eel River Basins<br>**Participants:**<br>- California Department of Fish and Wildlife<br>- California Trout<br>- Humboldt County<br>- Mendocino County Inland Water and Power Commission<br>- Round Valley Indian Tribes<br>- Sonoma County Water Agency<br>- Trout Unlimited |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| Eel-Russian Project Authority (ERPA) | A joint powers authority formed to discuss the Eel-Russian Project and ensure continuation of water diversions while prioritizing the restoration of fish migration. | ERPA will have the power to negotiate with the Pacific Gas and Electric Company (PG&E) as the utility moves ahead with plans to surrender operations of the Potter Valley Hydroelectric Project and to decommission the Scott and Cape Horn dams on the Eel River. The new authority will also have the legal capacity to own, construct and operate a new water diversion facility near Cape Horn Dam. | **Participants:**<br>- County of Sonoma<br>- Sonoma County Water Agency (Sonoma Water)<br>- County of Sonoma<br>- Mendocino County Inland Water and Power Commission<br>-Round Valley Indian Tribes |
| Eel River Forum Members | The Eel River Forum, led by California Trout and comprised of 22 public agencies, tribes, non-profit conservation organizations and other stakeholders, released the Eel River Action Plan. California Trout and partners are now taking action on the priorities outlined in the plan to recover the Eel River watershed and its native fish. | The goal of this forum is to achieve consensus among a coalition of agency, tribal, and conservation partners regarding priority recovery actions and policy reform needed to recover salmonid populations in the Eel River basin, California's third largest watershed. | **Project Partners:**<br>- California Trout<br>- CA Department of Fish and Wildlife<br>- CA State Parks<br>- Coastal Conservancy<br>- Eel River Recovery Project<br>- Eel River Watershed Improvement Group<br>- Environmental Protection Information Center<br>- Friends of the Eel River<br>- Friends of the Van Duzen River<br>- Humboldt County Resource Conservation District<br>- Mendocino County Resource Conservation District<br>- National Marine Fisheries Service<br>- North Coast Regional Water Quality Control Board<br>- Pacific Gas and Electric Company<br>- Potter Valley Irrigation District<br>- Round Valley Indian Tribe<br>- Salmonid Restoration Federation<br>- Sonoma County Water Agency<br>- US Bureau of Land |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| | | | Management<br>- US Fish and Wildlife Service<br>- US Forest Service<br>- Wiyot Tribe |
| U.S Fish and Wildlife Services | U.S Federal Agency that works to conserve fish, wildlife, plants, and their habitats | No additional information available. | No additional information available. |
| Sonoma County/Sonoma County Water | The local government agency that ensures water supply in Sonoma County and plays an important role in the decommissioning of PGE'S Scott Dam. | No additional information available. | **Consultants Used:**<br>-EnviroAnalytics<br>**Participants in:**<br>-Huffman Ad Hoc Committee<br>-Two-Basin Solution Partnership<br>-NOI Parties<br>-Joint Proponents |
| State Water Resource Control Board (SWRCB) | A government agency that regulates water quality. It is a part of the department of CalEPA. | No additional information available. | No additional information available. |
| California Department of Water Resources (DWR) | The California Department of Water Resources manages California's water resources, systems, and infrastructure. | No additional information available. | No additional information available. |
| Federal Energy Regulatory Commission (FERC) | FERC is an independent agency that regulates the interstate transmission and sale of electricity, natural gas, and oil in the US. | Assist consumers in obtaining reliable, safe, secure, and economically efficient energy services at a reasonable cost through appropriate regulatory and market means, and collaborative efforts. | No additional information available. |
| National Marine Fisheries Services | NOAA Fisheries, also known as the National Marine Fisheries Service, is an office of the National Oceanic and Atmospheric Administration within the Department of Commerce, a federal government agency. | NOAA Fisheries is responsible for the stewardship of the nation's ocean resources and their habitat. | No additional information available. |
| California Department of Fish and Wildlife (CDFW) | The CDFW state government agency that is part of the Department of CA Natural Resources Agency. This agency manages California's diverse fish, wildlife, and plant resources, and the habitats upon which they depend. | No additional information available. | -Joint Proponents<br>-Huffman Ad Hoc Committee |
| CalFire | CalFire is a state agency part of the Department of CA Natural Resources Agency and responsible for fire prevention, fire protection, and forest fire management in California. | No additional information available. | No additional information available. |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| USDA Forest Service | This federal government agency is within the U.S. Department of Agriculture that manages the conservation, use, and stewardship of natural and cultural resources on national forests and grasslands in the United States. | No additional information available. | No additional information available. |
| U.S. Department of Interior Bureau of Land Management | This federal government agency manages public lands across the country. | No additional information available. | No additional information available. |

## Parties with Indirect Relationship

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| Cardno, Inc | Environmental consulting firm | No additional information available. | - Now owned by Stantec<br>- Consultant used by PG&E |
| Stantec | Environmental consulting firm | No additional information available. | - Owns Cardno<br>- Consultant used by PG&E |
| Stillwater Sciences | Stillwater Sciences is a consulting firm that specializes in evaluating natural resources & conservation such as: watershed, rivers, and fish. | Stillwater Sciences specializes in science-based, technical approaches to natural and water resource management.<br>Using a collaborative and targeted approach, we integrate habitat and species information to understand critical ecosystem processes and identify effective measures for restoring and managing rivers and their floodplains as functioning ecosystems within the context of current conditions and future climate change. | **Affiliated With:**<br>- Huffman Ad Hoc Committee<br>- Two-Basin Solution Partnership |
| California Trout | Nonprofit organization working to improve fish habitat throughout CA | No additional information available. | **Affiliated With:**<br>- Huffman Ad Hoc Committee<br>- Free the Eel<br>- Two-Basin Solution Partnership<br>- Notice of Intent (NOI) Parties |
| McBain Associates | Environmental Consulting Firm | No additional information available. | **Affiliated With:**<br>- Potter Valley Project Planning Agreement Parties |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| McMillen Jacobs Associates | Civil engineering consulting firm | No additional information available. | **Affiliated With:**<br>-Two-Basin Solution Partnership<br>- Potter Valley Project Planning Agreement Parties |
| Center for Environmental Economic Development (CEED) | Based in Arcata | Sustainable initiatives | **Affiliated With:**<br>- Friends of the Eel River |
| University of California, Davis (UC Davis) | UC Davis is a California university that performs academic research which provides supporting scientific facts related to the project. | No additional information available. | **Affiliated With:**<br>-Mendocino County Russian River Flood Control and Water Conservation Improvement District |
| EnviroAnalytics | Environmental consulting | No additional information available. | **Affiliated With:**<br>- Sonoma Water |
| Mead & Hunt | Architectural and engineering firm | No additional information available. | PG&E |
| M3 | Environmental Consulting | No additional information available. | No additional information available. |
| University of California Cooperative Extension (UCCE) | Across California, the University of California's Cooperative Extension offices are problem-solving centers—the bridge between local issues and the power of UC research. | No additional information available. | UC Davis |
| Miller Pacific Engineering Group | Miller Pacific Engineering Group provides services that span the range of Geotechnical Planning, Geological Hazards Analysis, Geotechnical Exploration and Testing, Foundation Engineering, Construction Monitoring, Geo-Civil Engineering and Distressed Property Evaluation. | No additional information available. | **Affiliated With:**<br>-Friends of the Eel River |
| Princeton Hydro | Provides Princeton Hydro is an environmental firm which provides nature-based solutions for watershed management and ecological restoration | No additional information available. | No additional information available. |
| Ross Taylor and Associates | oss<br>Ross Taylor and Associates provide fish passage assessments, biological sampling, technical writing, workshops and training | No additional information available. | No additional information available. |

# Table 1- List of entities associated with PVP Project

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

| Organization | About | Mission | Affiliates |
|---|---|---|---|
| Cal Poly Humboldt - River Institute | The River Institute is a branch within the Cal Poly Humbolt's Department of Environmental Science and Management. This institute educates students and researches different projects pertaining to aquatic ecosystems. | The mission of the Cal Poly Humboldt River Institute is to conserve and restore river ecosystems locally, nationally, and internationally through multi-disciplinary research, education, policy reform, improved management practices, and creative restoration strategies. | No additional information available. |
| Riverbend Sciences | Riverbend Sciences is a consulting firm dedicated to the study of freshwater aquatic ecosystems and watersheds. | No additional information available. | No additional information available. |
| Native Fish Society | The Native Fish Society exists to cultivate a groundswell of public support needed to revive abundant wild, native fish. | No additional information available. | No additional information available. |
| Bay Area Council Economic Institute | Bay Area Council Economic Institute has been the leading think tank focused on the economic and policy issues facing the San Francisco/Silicon Valley Bay Area | No additional information available. | No additional information available. |
| Lake Pillsbury Alliance | The Lake Pillsbury Alliance is a 501(c)(3) non-profit organization formed to preserve Lake Pillsbury. | No additional information available. | No additional information available. |

**Table 2 – Recreational Facilities**

[Identified by SLR as part of the Impact Evaluation on Scott Dam for the Benefit of County of Lake]

Analysis of Effects of Proposed Scott Dam Decommissioning

| Facilities | Recreation Facilities/Opportunities |
|---|---|
| **PG&E Maintained and Operated /US Forest Service Property** | |
| Fuller Grove Campground | 30 campsites, vault toilets, tables, and fire rings. |
| Fuller Grove Group Campground | 12 tables, 12 fire rings, 1 food storage locker and 1 toilet building. Group camping with capacity of up to 60 people. Boating-motorized, lake and pond fishing. |
| Fuller Grove Day-Use Area and Boat Launch | 20 parking spaces for cars with trailers; 1 boarding float |
| Navy Campground | 20 campsites, 20 tables, 20 fire rings and 2 vault toilets. Boating-motorized, lake and pond fishing. |
| Oak Flat Campground | 12 campsites, 12 tables, 12 stoves, 4 vault toilets. Lake and pond fishing, OHV riding and camping. |
| Pogie Point Campground | 50 campsites, 50 tables, 50 stoves, 4 toilets. Lake and pond fishing, boating-motorized |
| Pogie Point Day-Use Area | Provides day-use access (number of parking spaces not reported) |
| Sunset Point Campground | 25 campsites, boat ramp, tent camping, camping trailer, picnic tables, toilets, drinking water |
| Pillsbury Pines Day-Use Area and Boat Launch | Day-use area, accessible restroom, picnic tables and pedestal grills |
| Lake Pillsbury Low Level Boat Launch | 75 parking spaces for cards with trailers, 0 boarding floats, 2 carry down walkways, 25 dry storage capacity |
| Trout Creek Campground | 16 campsites, picnic tables, fire grills and a double vault toilet |
| Trout Creek Group Campground | No information available |
| **Other/Private** | |
| Lake Pillsbury Resort | 10 cabins, 38 campsites, 1 full RV hook up, country store, beach and swim area, coin showers, flush toilets, fuel service at the store and on the lake, marina, launch ramp, boat slips |

Note: According to the US Forest Service website "The campgrounds at Lake Pillsbury are operated and maintained by Pacific Gas and Electric".

## Table 3 - Identified Species Near Lake Pillsbury (0.5 Mile Radius)

| Scientifc Name | Common Name | Element Type | Endangered (Y/N) |
|---|---|---|---|
| Rana boylii pop. 1 | Foothill yellow-legged frog | Animals - Amphibians | N |
| Haliaeetus leucocephalus | Bald eagle | Animals - Birds | Y |
| Pandion haliaetus | Osprey | Animals - Birds | N |
| Setophaga petechia | Yellow warbler | Animals - Birds | N |
| Oncorhynchus mykiss irideus pop. 48 | Steelhead | Animals - Fish | Y |
| Eumops perotis californicus | Western mastiff bat | Animals - Mammals | N |
| Martes caurina humboldtensis | Humboldt marten | Animals - Mammals | Y |
| Pekania pennanti | Fisher | Animals - Mammals | N |
| Antrozous pallidus | Pallid bat | Animals - Mammals | N |
| Corynorhinus townsendii | Townsends big-eared bat | Animals - Mammals | N |
| Lasiurus frantzii | Western red bat | Animals - Mammals | N |
| Myotis thysanodes | Fringed myotis | Animals - Mammals | N |
| Arctostaphylos manzanita ssp. elegans | Konocti manzanita | Plants - Vascular | N |
| Hesperolinon adenophyllum | Glandular western flax | Plants - Vascular | N |
| Sidalcea oregana ssp. hydrophila | Marsh checkerbloom | Plants - Vascular | N |

Note: Data collected by SLR from CNDDB via BIOS Viewer @ CDFW on October 30, 2024

Red - Endangered species identified within a 0.5 mile radius of Lake Pillsbury's perimeter

# Figures

## Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

SLR Project No.: 102.021249.00001

June 6, 2025

## California State Water Resources Control Board (SWRCB) Process

1. Certification of Applicant
   - Water Certification Applicant filed with Water Board
2. Environmental Effects
   - Water Board Decides Project has the potential to have significant effect of the environment.
3. EIR
   - Water Board Decision to prepare an EIR of Negative Declaration
4. Notice of Preparation
   - Water Board send out Notice of Preparation to Responsible Agencies
5. Draft EIR
   - Water Board Prepares Draft EIR
6. Notice of Completion
   - Water Board files Notice of Completion and give Public Notice of Availability of Draft EIR
7. Public Review Period
   - Public Review Period, minimal of 30 days
8. Comment Response
   - Response to Comments and Preparation of Final EIR
9. Approval of EIR
   - Consideration and Approval of Final EIR by Water Board
10. Findings of Feasibility
    - Water Board prepares Findings of Feasibility of reducing or avoiding significant environmental effects
11. Decision of Project
    - Decision of Project
12. File Notice of Determiniation
    - File Notice of Determination with Governor's Office of Planning and Research (OPR)

## Federal Energy Regulatory Commission (FERC) Process

1. License Surrender Application
   - License Surrender Application is Submitted to FERC
2. Public Notice
   - FERC Issues a Public Notice, with a 30-day Comment Period
3. Draft NEPA
   - FERC prepares the Draft NEPA Document
4. NEPA Comments
   - Draft NEPA Document Released, 30-60 Day Comment Period
5. Mandatory Conditions Filed
   - Agencies file Mandatory Conditions, 60-Day Window to file following close of Comment Period
6. Final NEPA Document
   - FERC Releases Final NEPA Document within 90 days of receiving conditions from Agencies
7. Decision on Surrender Application
   - FERC Issues Decision on Surrender Application

**Notes:** The decommissioning of Scott Dam requires regulatory approval from the California State Water Board and the Federal Energy Regulatory Commission (FERC). The State Water Board is responsible for issuing a Water Quality Certification for the Project's license surrender and decommissioning and will likely act as the California Environmental Quality Act (CEQA) lead. FERC must approve the Surrender of the Hydropower License and will act as the Federal lead for compliance with the National Environmental Policy Act (NEPA). A Water Quality Certification must be issued prior to the FERC license surrender being authorized.



COUNTY OF LAKE

**DECOMMISSIONING REGULATORY PROCESS IN CALIFORNIA**

| | | |
|---|---|---|
| OCTOBER 2024 | Rev **1.0** | Figure No. **1** |
| Project No. 102.021249.00001 | | |

SLR

1197

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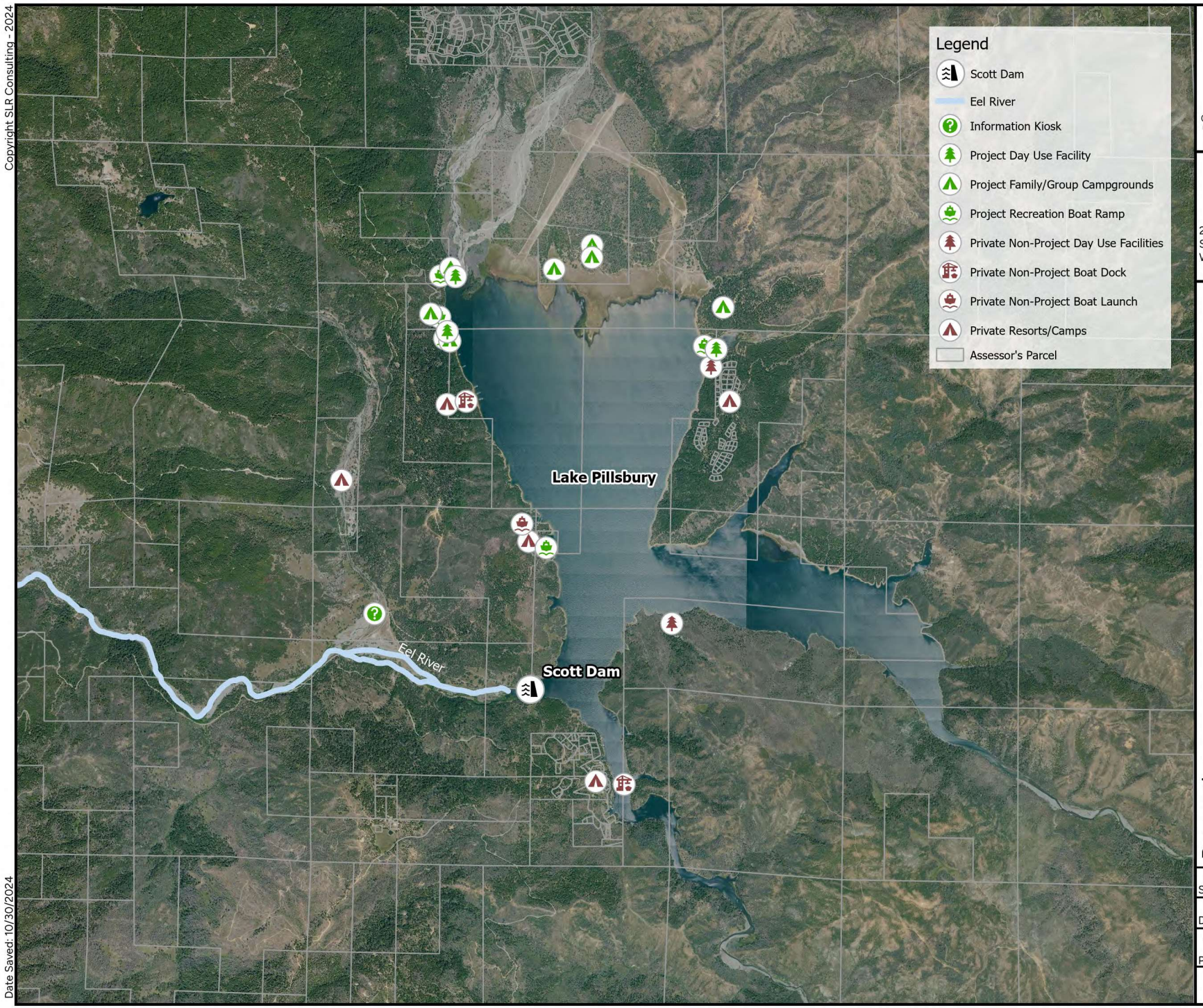


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Date Saved: 10/30/2024

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1 in. = 4,000 ft

SLR
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SUITE 205
WALNUT CREEK, CA 94596

Recreation

Potter Valley Project Decommissioning
County of Lake

SCALE 1:48,000

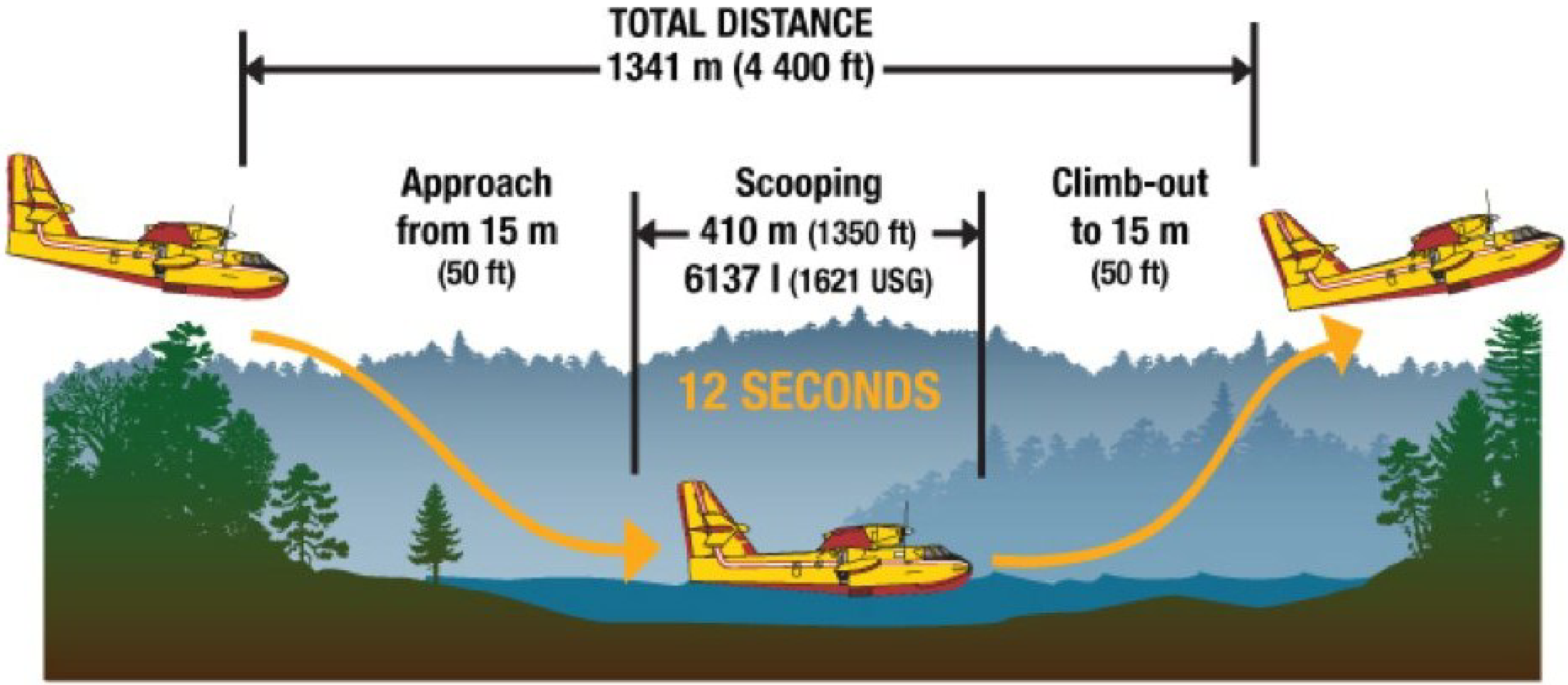
DATE 10/30/2024

102.021249.00001
PROJ. NO.

**FIG. 2**



Sources:
California Division of Regional Assistance
California Department of Water Resources
PG&E Initial Surrender Application




| | | | |
|---|---|---|---|
| COUNTY OF LAKE | | | |
| **SUPER SCOOPER TYPICAL USE** | | | |
| OCTOBER 2024 | Rev **1.0** | Figure No. **3** | |
| Project No. 102.021249.00001 | | | |
| SLR | | | 1199 |

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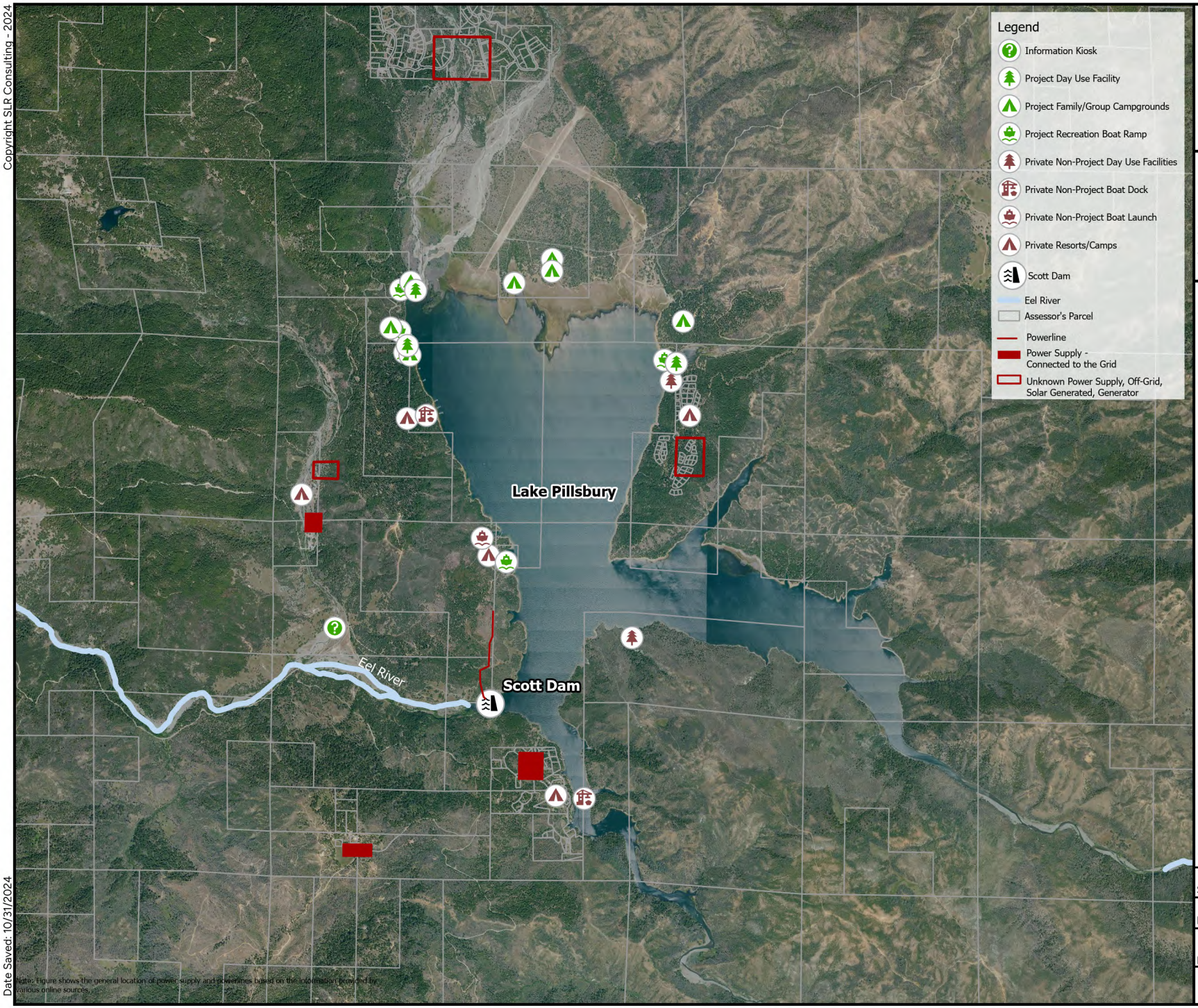


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Note: Figure shows the general location of power supply and powerlines based on the information provided by various online sources.

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1 in. = 4,000 ft

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WALNUT CREEK, CA 94596

# Power Supply

Potter Valley Project Decommissioning
County of Lake

SCALE 1:48,000

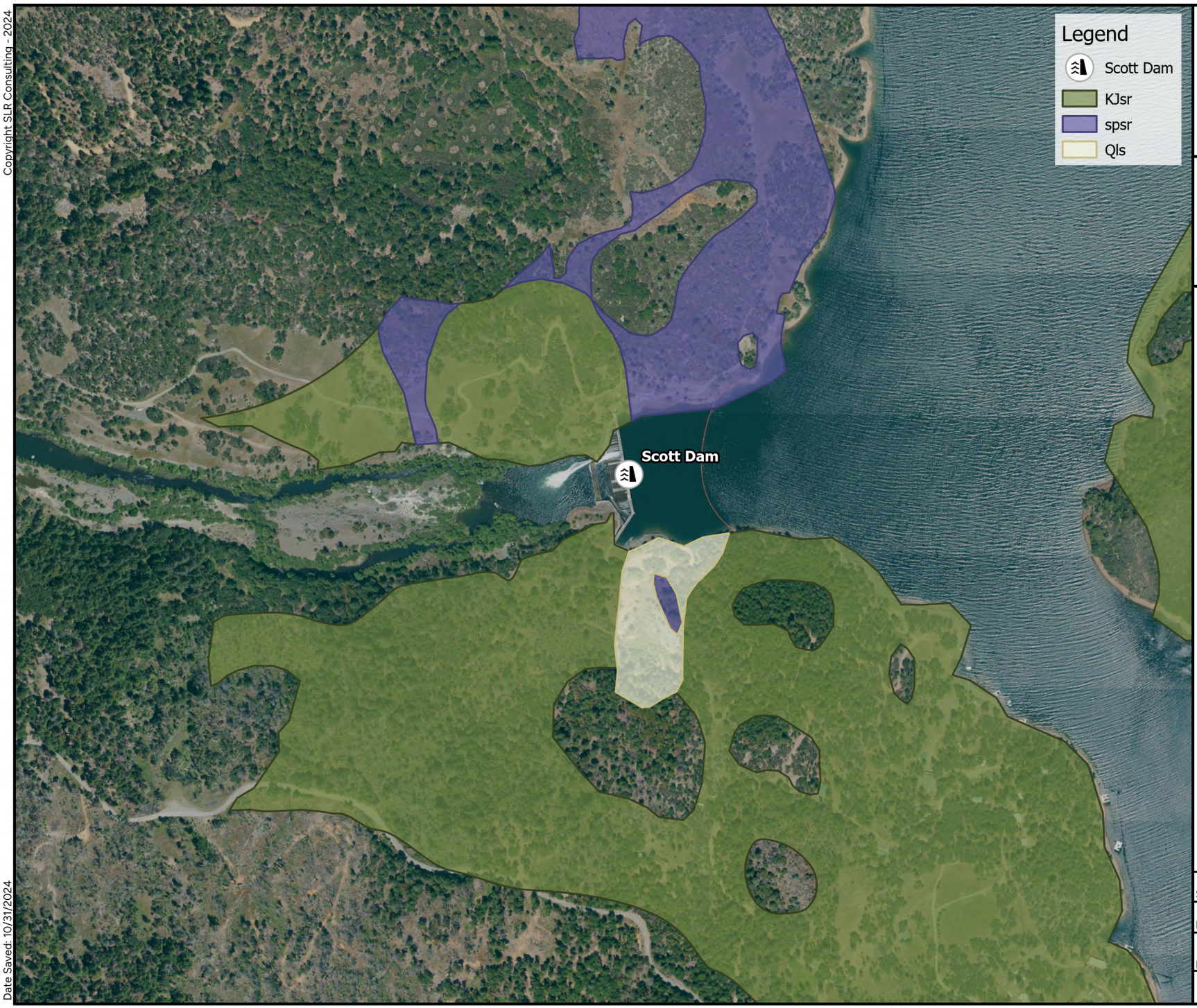
DATE 10/31/2024

102.021249.00001
PROJ. NO.

**FIG. 4**

1200

Sources:
California Division of Regional Assistance
California Department of Water Resources
PG&E Initial Surrender Application
Redfin

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# Geological Units in the Vicinity of Scott Dam

Potter Valley Project Decommissioning
County of Lake

SCALE 1:7,200

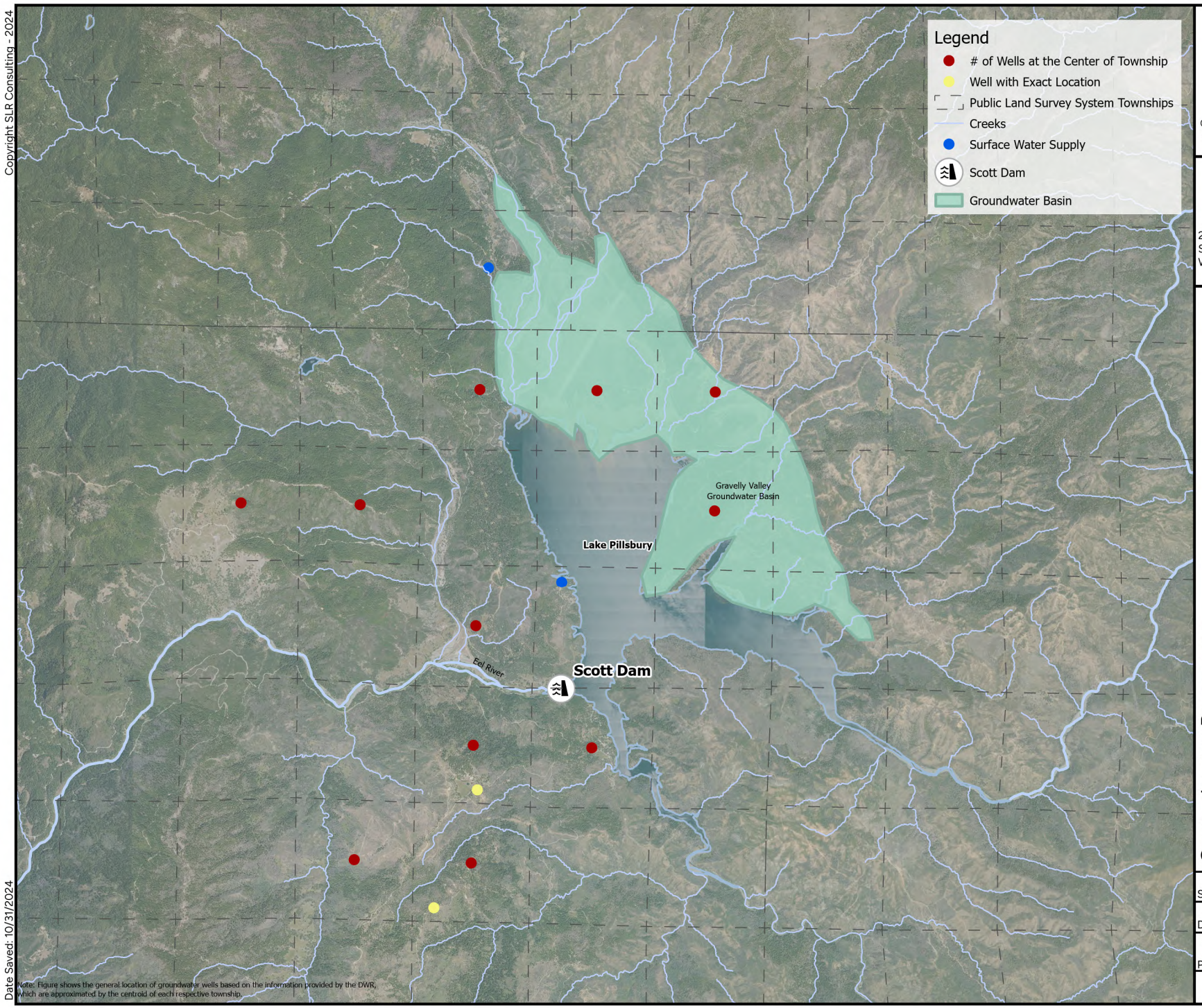
DATE 10/31/2024

102.021249.00001
PROJ. NO.

**FIG. 5**

Sources:
USGS, Geologic Map of the Bartlett Springs Fault Zone
in the Vicinity of Lake Pillsbury and Adjacent Areas of Mendocino,
Lake, and Glenn Counties, Open File Report 2010-1301, 2010.

US National Agriculture Imagery Program

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Note: Figure shows the general location of groundwater wells based on the information provided by the DWR, which are approximated by the centroid of each respective township.

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# Groundwater Resources

Potter Valley Project Decommissioning
County of Lake

SCALE 1:72,000

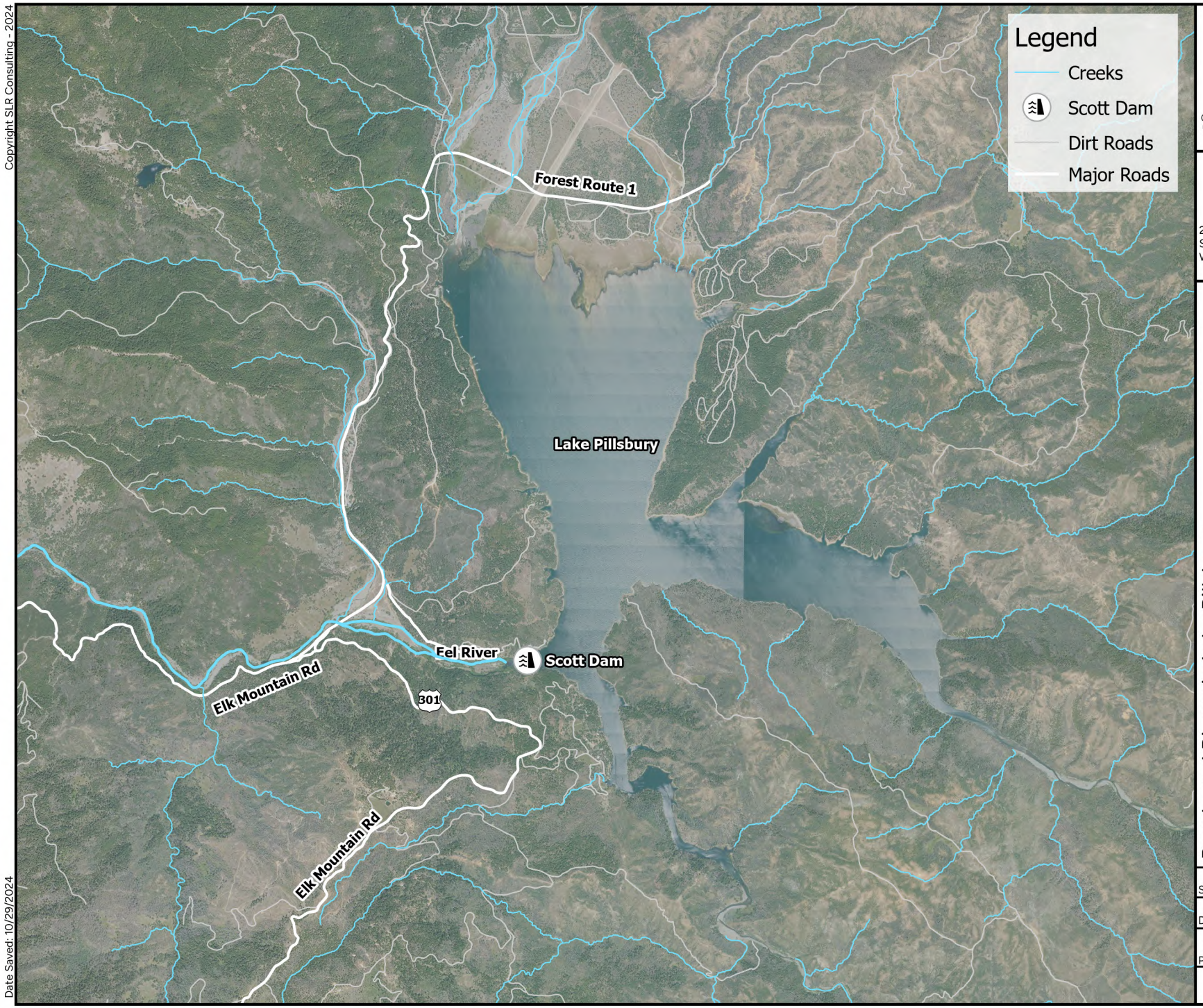
DATE 10/31/2024

102.021249.00001
PROJ. NO.

**FIG.6**

1202

Sources:
Federal Document Management System (FDMS)
Public Records by County of Lake
Mendocino National Forest, US Forest Service
US National Agriculture Imagery Program
California Department of Fish and Wildlife (CDFW)

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Roads and Rivers in Lake Pillsbury

Potter Valley Project Decommissioning
County of Lake

SCALE 1:48,000

DATE 10/29/2024

102.021249.00001
PROJ. NO.

**FIG. 7**

1203

Sources:
California Division of Regional Assistance
California Department of Water Resources
PG&E Initial Surrender Application

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Date Saved: 10/29/2024

Note: Inundation map obtained from "Civil - Hydrologic Engineering Inundation Map for Hypothetical Failure of Scott Dam, Lake County" (PG&E, 2018).

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0 5,000

1 in. = 5,000 ft

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# Inundation Boundary and Roadways

Potter Valley Project Decommissioning
County of Lake

SCALE 1:60,000

DATE 10/30/2024

102.021249.00001
PROJ. NO.

**FIG. 8**



Sources:
Federal Document Management System (FDMS)
Public Records by County of Lake
Mendocino National Forest, US Forest Service
Department of Water Resources
US National Agriculture Imagery Program (NAIP)

# Exhibits

## Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

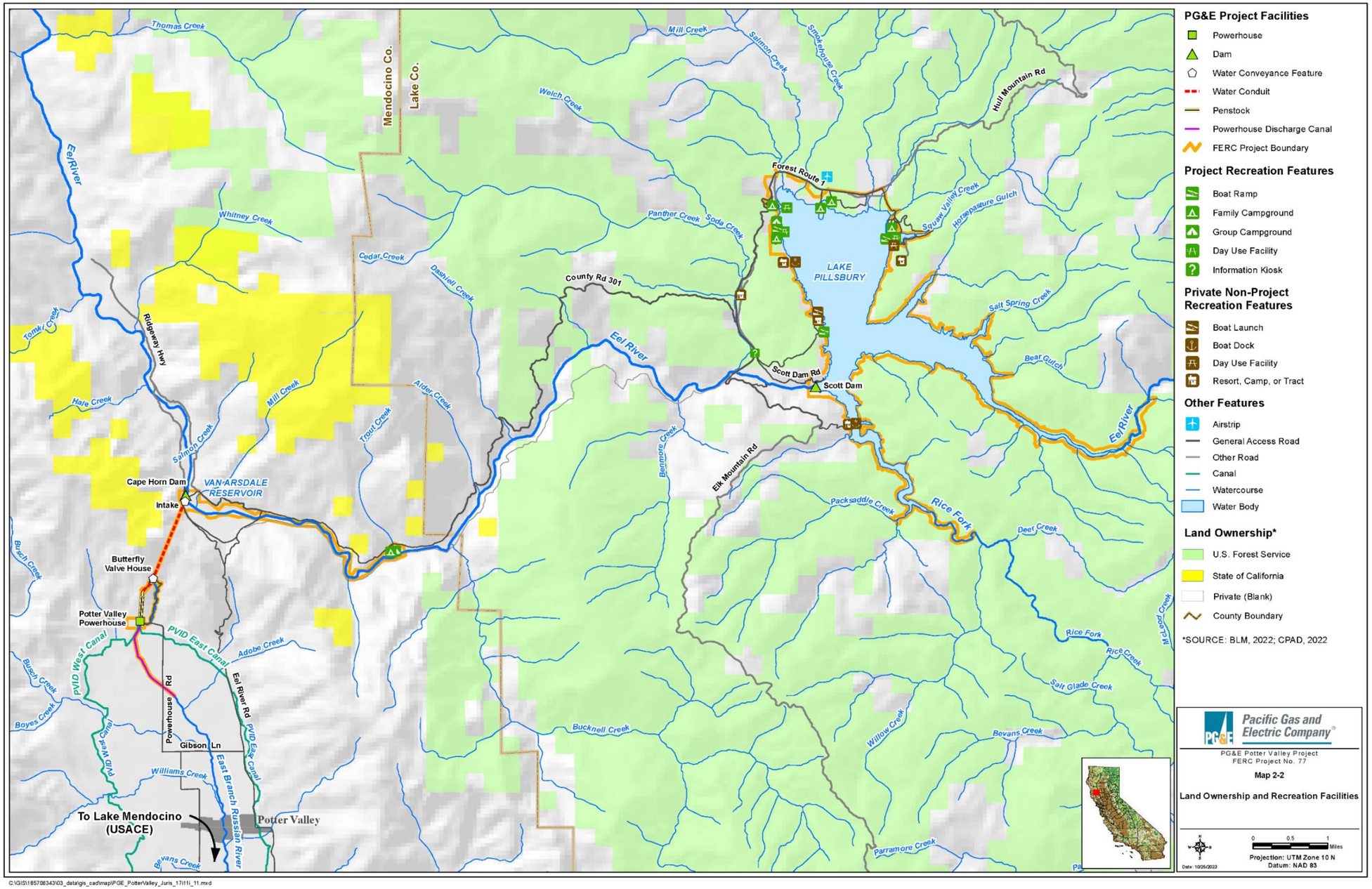
**County of Lake**

SLR Project No.: 102.021249.00001

June 6, 2025

SLR

**Exhibit 1**
Potter Valley Hydroelectric Project Site Plan




**Map 2-2. Land Ownership and Recreation Facilities**



Source:
PG&E, 2023. Initial Draft Surrender Application and Conceptual Decommissioning Plan. November. Page 2-5

**Exhibit 2**
Decommissioning of Project Facilities and Features in the Scott Dam Area.
Initial Draft Surrender Application and Conceptual Decommissioning Plan.

### Other Associated Project Support Facilities and Features

PG&E has reviewed the associated Project support facilities and features in the Scott Dam Area. Table 4-1 provides a summary of decommissioning of Project facilities and features in the Scott Dam Area and the associated land ownership. Decommissioning of Project facilities and features includes leaving the facility in place (L), removal of the facility with restoration (RR), transfer of the facility, if requested by a qualified interested party (T), and removal of the facility with no restoration (X).

**Table 4-1. Decommissioning of Project Facilities and Features in the Scott Dam Area.**

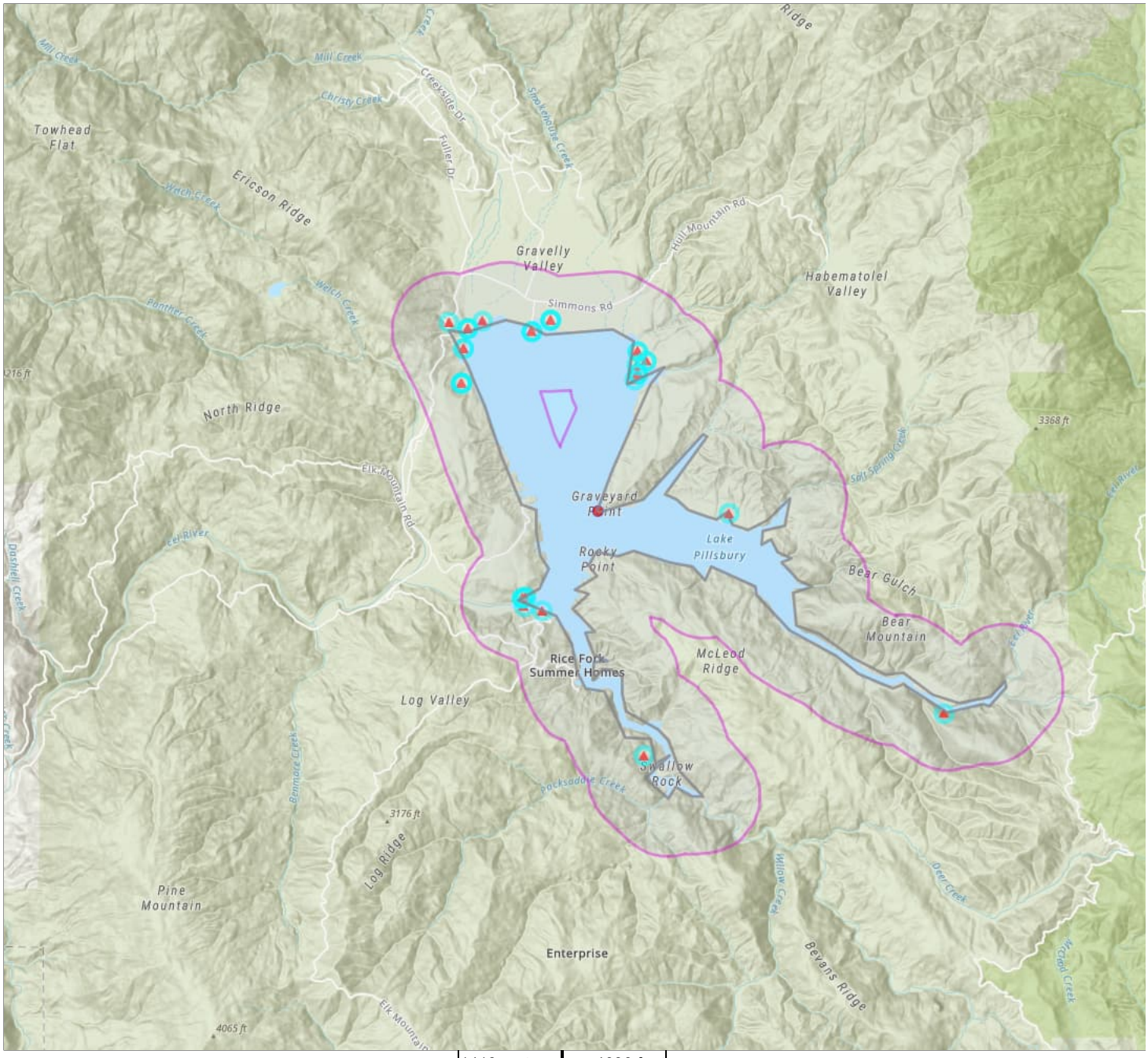
| Project Facility/Feature | Land Ownership | Decommissioning | | |
|---|---|---|---|---|
| | | Rapid Dam Removal | Phase Dam Removal | Potential Transfer |
| **Dam and Associated Facility/Features** | | | | |
| Scott Dam | PG&E | RR | RR | |
| Valve Control House | PG&E | X | X | |
| ***Reservoir*** | | | | |
| Lake Pillsbury (storage reservoir) | PG&E/USFS | RR | RR | |
| ***Reservoir Gage*** | | | | |
| E1—Lk Pillsbury NR Potter Valley CA (11470000) | PG&E | X | X | |
| ***River Gages*** | | | | |
| E2—Eel R BL Scott Dam NR Potter Valley CA (11470500) | PG&E | L (remove equip only) | L (remove equip only) | T (leave equipment) |
| **Leakage Weirs and Piezometers and Associated Trail** | | | | |
| Scott Dam Leakage Weirs | PG&E | RR | RR | |
| Scott Dam Piezometers | PG&E | L (cap) | L (cap) | |
| Scott Dam Piezometers and Leakage Weir Access Trail | PG&E | L | L | |
| **Project Communication Line** | | | | |
| Scott Dam Block Building Communication Line | PG&E | X | X | |
| **Ancillary and Support Facilities** | | | | |
| Scott Dam Block Building | PG&E | L (remove equip only) | L (remove equip only) | |
| Scott Dam Boat Barrier | PG&E | X | X | |
| **Project Facility Access Roads** | | | | |
| Gage E2 Access Rd | PG&E | L | L | Easement |
| Scott Dam Rd | PG&E | L | L | |
| Upper Scott Dam Access Rd | PG&E | L | L | |



Source:
PG&E, 2023. Initial Draft Surrender Application and Conceptual Decommissioning Plan. November. Page 4-12.

**Exhibit 3A**

Identified Species Near Lake Pillsbury (0.5 Mile Radius)




| 1413 meters | 4636 feet |
|---|---|

Endangered Species identified within 0.5 miles of Lake Pillsbury Permitter

**Map Legend**

**Unprocessed Data from CNDDB Online Field Survey Form [ds1002]**

Unprocessed Data from CNDDB Online Field Survey Form _ ds1002

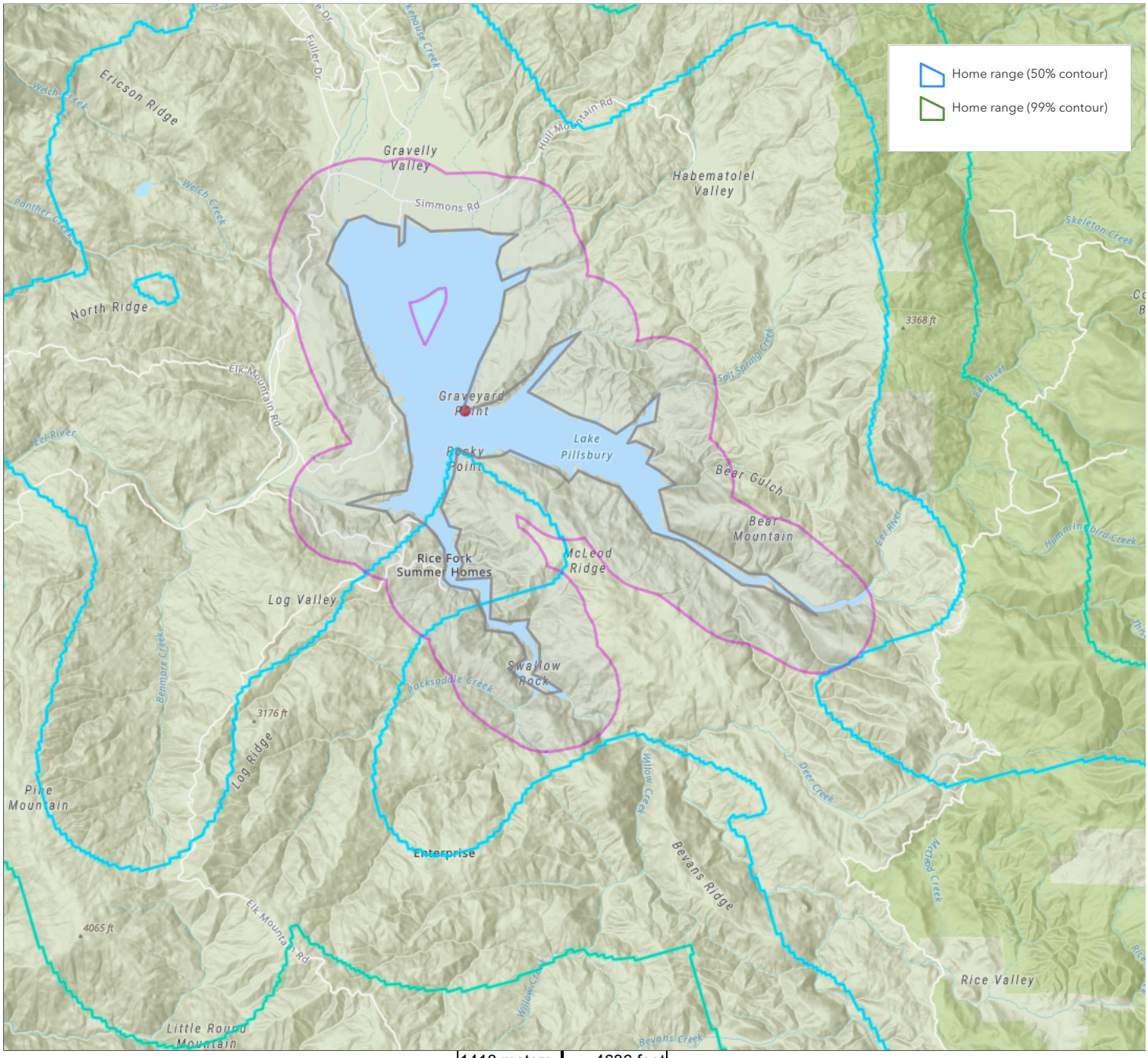
- Animal
- Plant

Source:

California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System. https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

**Exhibit 3B**

Tule Elk Contours




| 1413 meters | 4636 feet |
|---|---|

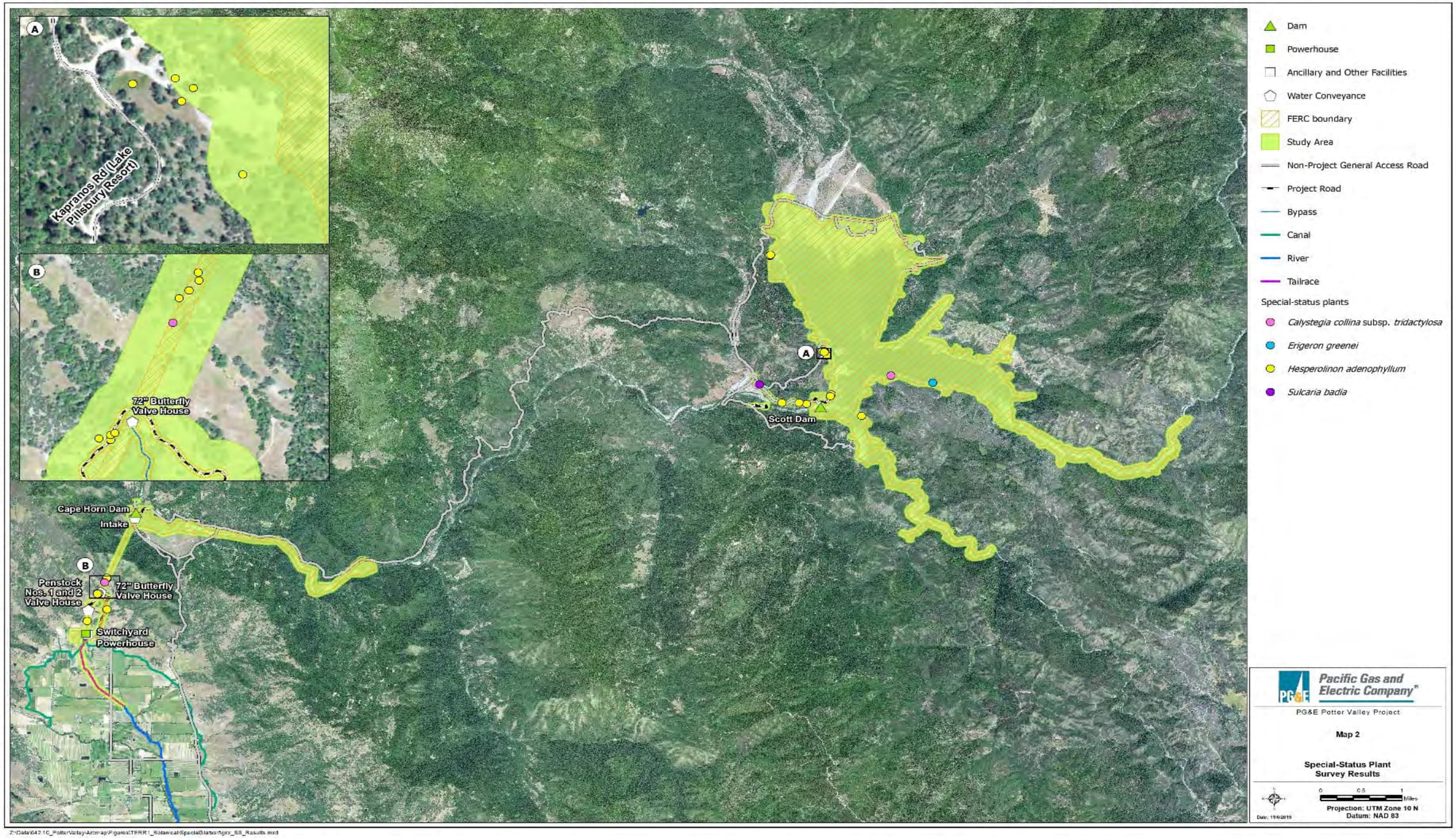
Description: Elks identified nearby Lake Pillsbury that may enter the 0.5 mile radius range.

Source:

California Department of Fish and Wildlife (CDFW), 2024. BIOS – Biogeographic Information and Observation System.https://wildlife.ca.gov/Data/BIOS. Accessed October 29.

**Exhibit 3C**

Special-status species plants documented in the Study Area During the 2018 Botanical Surveys

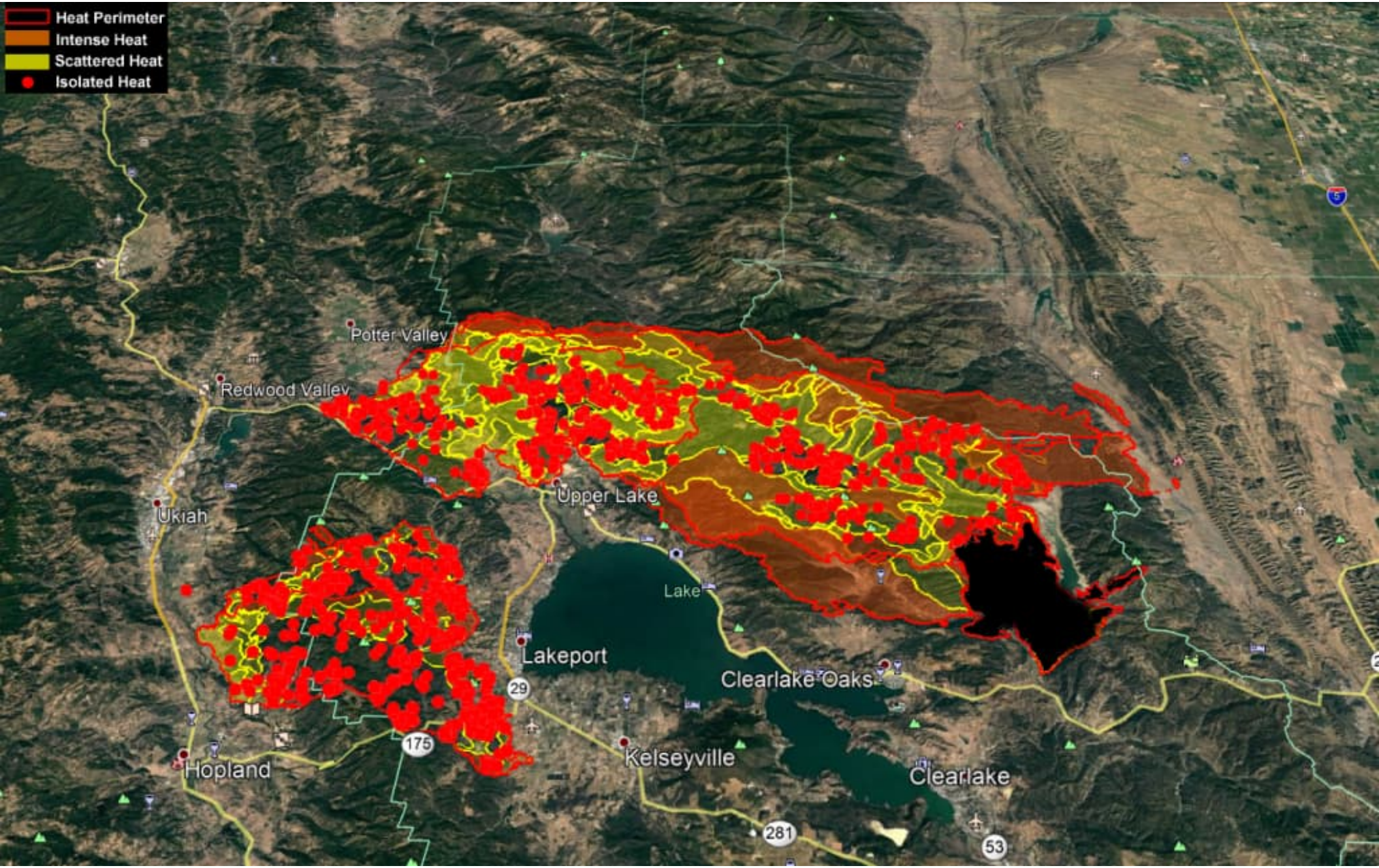



Source:

Cardno and Stillwater Sciences, 2019. TERR 1 – Botanical Resources Data Memorialization, Potter Valley Project (FERC Project No. 77). December 5

**Exhibit 4**

Mendocino Complex Fire 2018 - Impacting Lake Pillsbury within Lake County, California






Source:

CalFire, 2018. Mendocino Complex fire map. http://kymkemp.com/wp-content/uploads/2018/08/Capture-11-1024x641.png. August 5.

**Exhibit 5**

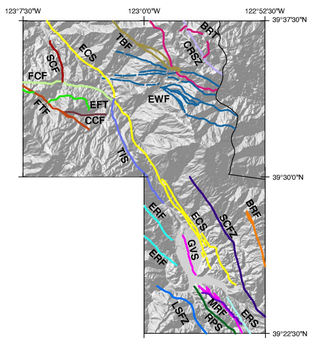
Geologic Map of the Bartlett Springs Fault Zone in the Vicinity of Lake Pillsbury and Adjacent Areas of Mendocino, Lake, and Glenn Counties, California

USGS science for a changing world
U.S. Department of the Interior
U.S. Geological Survey

Open-File Report 2010-1301
*Pamphlet accompanies map*

123°7'30"W 123°0'0"W 122°52'30"W 39°37'30"N 39°30'0"N

MAJOR FAULTS OF THE LAKE PILLSBURY REGION



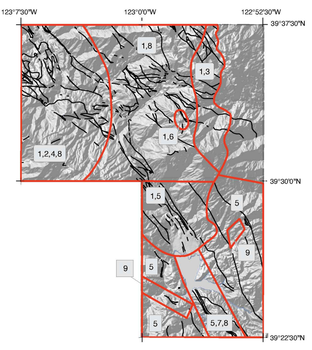

**Main Bartlett Springs Fault Zone**
- Elk Creek Segment- ECS
- The Island Segment- TIS
- Gravelly Valley (creeping?) Segment- GVS
- Eel River Segment- ERS
- Rocky Point Segment- RPS

**Southwestern Bartlett Springs Fault Zone**
- Sanhedrin Creek Fault- SCF
- Northeast and Southwest Ericson Ridge Faults- ERF
- Logan Spring Fault Zone- LSFZ

**Low-angle Faults**
- Board Ridge Thrust- BRT
- East Flank Thrust- EFT

**Other Faults**
- Chimney Rock Shear Zone- CRSZ
- Thatcher Butte Fault- TBF
- Forks Creek Fault- FCF
- Crocker Creek Fault- CCF
- Foot Trail Fault- FTF
- Boardman Ridge Fault Zone- BRF
- Squaw Creek Fault Zone- SCFZ
- McLeod Ridge Fault- MRF
- East-West Faults- EWF

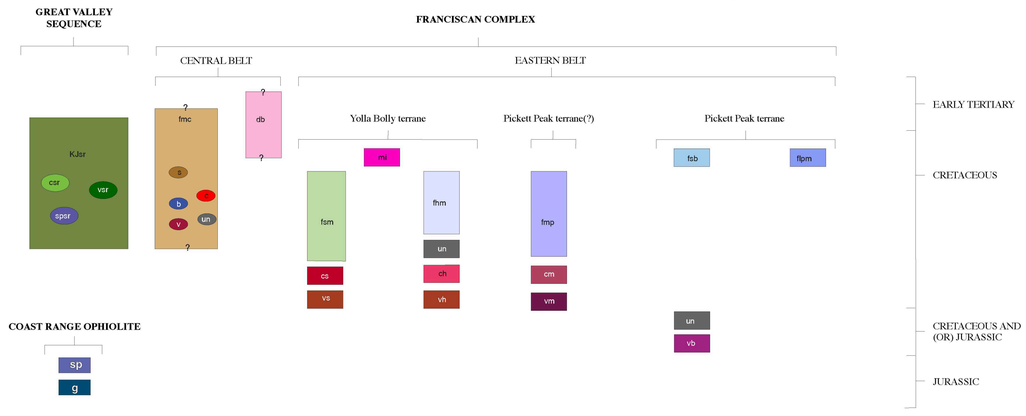
SOURCES OF GEOLOGIC MAP DATA

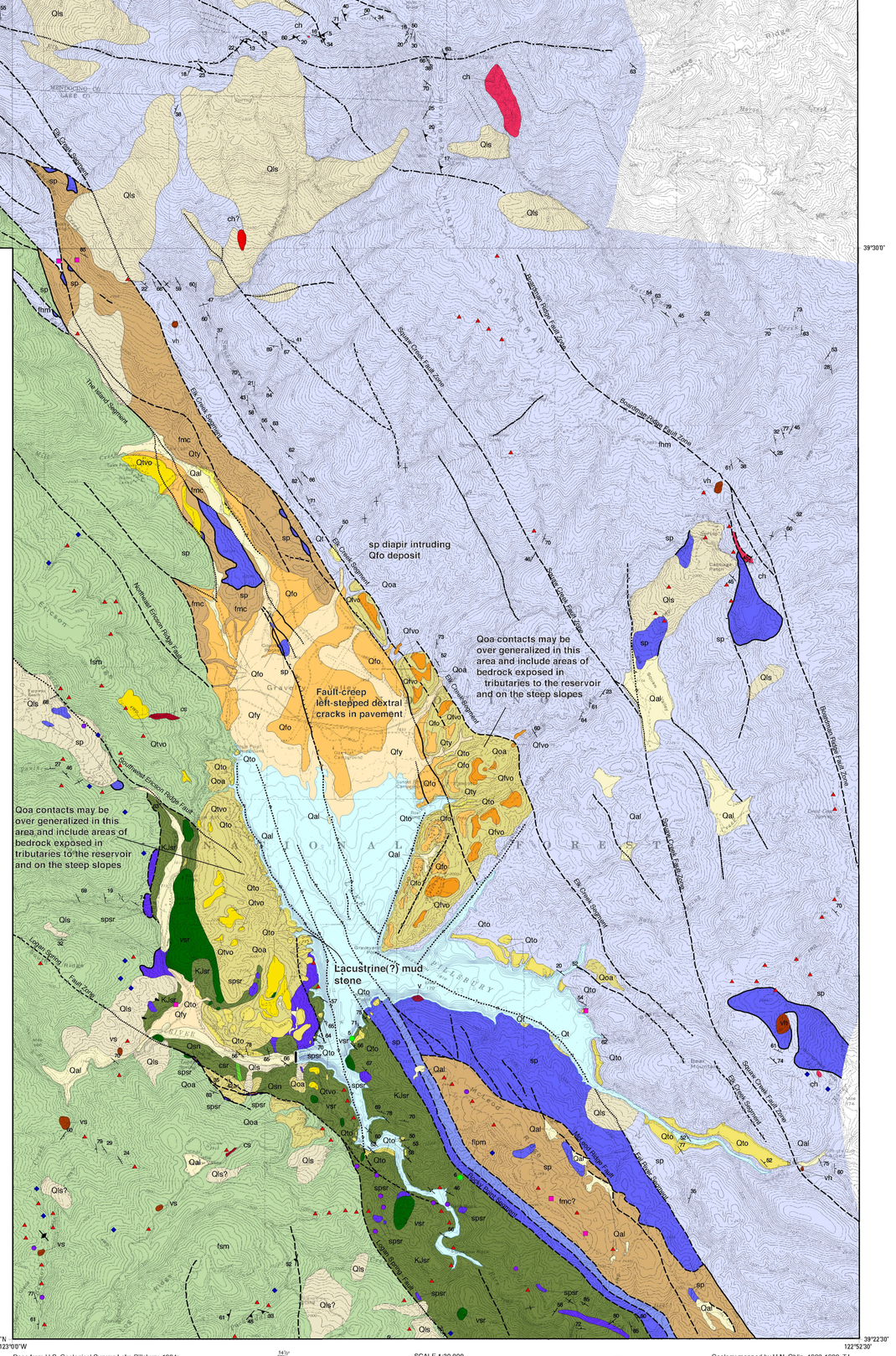



1. Ohlin, H.N., mapping and air photo reconnaissance, 1980-1982
2. CDWR, 1969
3. Lehman, D.H., 1974
4. Jordan, M.A., 1974
5. Etter, S.D., 1979
6. Layman, E.B., 1977
7. Sawyer, T.L., fieldwork, 2008-2009
8. Lienkaemper, J.J., and Brown, 2009
9. McLaughlin, R.J., reconnaissance field work, 2009

CORRELATION OF MAP UNITS

QUATERNARY DEPOSITS




Base from U.S. Geological Survey: Lake Pillsbury, 1994; Hull Mountain, 1973; and Sanhedrin Mountain, 1994; Lake Pillsbury Bathymetry from Pacific Gas and Electricitic Company, 2005
Universal Transverse Mercator projection, Zone 11

SCALE 1:30,000

CONTOUR INTERVAL 40 FEET
DATUM IS MEAN SEA LEVEL

APPROXIMATE MEAN DECLINATION, 2010

MAP LOCATION

Geology mapped by H.N. Ohlin, 1980-1982, T.L. Sawyer, 2007; revised, R.J. McLaughlin, 2008-2009.

GIS database and digital cartography by Barry C. Moring

Edited by Jan L.Zigler

Manuscript approved for publication December 3, 2010

## DESCRIPTION OF MAP UNITS

**Qls** **Landslide deposits (Holocene and late Pleistocene)**—Unsorted, unconsolidated to weakly consolidated rock and soil debris, which has moved downslope by various landslide processes, including rotational slumping, surface creep, flowage, or combinations thereof. Earth flows are common in mélange of the Central Belt of the Franciscan Complex

**Qal** **Alluvium (Holocene and Pleistocene)**—Poorly sorted, unconsolidated sand, silt, gravel, and boulders deposited in and adjacent to active stream channels

**Qt** **Alluvial stream terrace deposits, undivided (Holocene and Pleistocene)**- Poorly sorted, weakly consolidated sand, silt, gravel, and boulder deposits; slightly dissected; deposited along or adjacent to major streams. Locally divided into:

**Qty** **Young alluvial stream terrace deposits and surfaces (Holocene and late Pleistocene)**—Stream terraces that are least dissected and elevated above the active stream channel; unit is delineated by geomorphic expression of terrace surfaces and in places does not include alluvial deposits

**Qto** **Old alluvial stream terrace deposits and surfaces (Pleistocene)**- Alluvial terrace deposits that are elevated above, dissected, and incised by young alluvial terrace and fan deposits. Includes as many as six stepped geomorphic surfaces that occur around margins of Lake Pillsbury basin at elevations between 1,800 and 1,920 ft

**Qtvo** **Very old alluvial stream terrace deposits and surfaces (Pleistocene)**- Alluvial stream terrace remnants that include several stepped geomorphic terrace surfaces that occur between elevations of 2,000 and 2,700 ft

**Alluvial fan deposits and surfaces (Holocene and late Pleistocene)**

**Qfy** **Young alluvial fan deposits and surfaces (Holocene and late Pleistocene)**—Poorly sorted, weakly consolidated sand, silt, gravel and boulder deposits of alluvial fan systems at different elevations, inset into older deposits, distributed around north and east margins of Lake Pillsbury basin. At least two young alluvial fan surfaces occur between the northern shoreline of Lake Pillsbury and elevation 1,920 ft

**Qfo** **Old alluvial fan deposits and surfaces (Pleistocene)**—Alluvial fan deposits that are elevated, dissected, and incised by younger fans and alluvial terrace deposits. At least five stepped old alluvial fan surfaces are recognized around the north and east sides of Lake Pillsbury basin at elevations between 1,900 and 2,000 ft

**Qfvo** **Very old alluvial fan deposits and surfaces (Pleistocene)**- Topographically high, dissected, and uplifted alluvial fan deposits including as many as four stepped fan surfaces mapped at elevations between 2,040 and 2,100 ft along northeast side of Lake Pillsbury basin

**Qoa** **Older alluvial deposits, undivided (Pleistocene)**—Weakly consolidated, moderately organized to unorganized, weakly sorted to unsorted silt, sand, and gravel deposits that locally display clast imbrication, crossbedding, and channels; moderately to extremely dissected; present along the east and west sides of Lake Pillsbury basin and east side of Gravelly Valley. Unit includes deposits characteristic of alluvial fan and braided-stream depositional settings; locally including massive lacustrine(?) mudstone beneath old terraces (**Qto**) along west edge of Lake Pillsbury. Along east and west sides of Lake Pillsbury, unit contacts are highly generalized and encompass significant unmapped bedrock, particularly on steep slopes and in tributaries draining into reservoir. In places, unit may correlate with old strath terraces or thin old alluvial fan deposits cut on bedrock (**Qto**, **Qtvo**, **Qfo**, **Qfvo**)

**Qsn** **Siliceous and calcareous sinter deposits (Pleistocene?)**—Vuggy opalitic and carbonate-veined and sheared serpentinite and argillitic microbreccia, present locally along Logan Spring Fault southeast of Logan Spring, probably related to extinct low temperature thermal seep activity

**Great Valley Sequence (Cretaceous and Jurassic)**

**KJsr** **Ophiolitic mélange and sedimentary rocks of Split Rock (Early Cretaceous or younger)**—A broken sequence of sandstone, locally massive and conglomeratic, thinly interbedded argillite and turbiditic arkosic wacke, and mafic to ultramafic rocks. Unit is in places penetratively deformed into block-in-matrix mélange, comprised of mafic and ultramafic rocks, with argillite and sandstone matrix; elsewhere matrix is sheared serpentinite with blocks of sandstone and argillite, chert and

**s** (Tithonian) to Middle Cretaceous (Albian or Aptian) age (Lucas-Clark, 2007). Radiolaria from a chert block in the map area (fossil locality 2, table 1) are Late Jurassic (Tithonian) in age. Radiolarian ages from cherts elsewhere in the Central Belt (McLaughlin and Pessagno, 1978; Murchey and Jones, 1984) range from early Jurassic (Pliensbachian) to early Late Cretaceous (Cenomanian). White mica from blocks of blueschist gives potassium-argon ages of 151 to 155 m.y. (Lehman, 1974). A block of diorite from O'Neil Ridge that has undergone pumpellyite grade metamorphism gives a Pb-U age of 161 m.y. (J. Mattinson, written commun., 1982). The inferred age of mélange-forming tectonic mixing, based on the ages of youngest mélange components, is Coniacian or younger north of the map area and Campanian or younger (possibly Eocene or younger) south of the map area. Individual blocks

**c** stone and argillite with abundant lenses and blocks of interleaved radiolarian chert, and porphyritic pillow basalt and tuff. Radiolarian fauna from the chert lenses indicates an Early Cretaceous age in the report area (fossil locality 4). Locally overlying this chert is well-bedded, medium- to coarse-grained, lithic metasandstone with interbeds of chert pebble conglomeratic grit, and rare lensoid bodies of tuffaceous volcanic rock and chert. The structurally highest part of the Metasandstone of Hull Mountain consists of a broken unit of metasandstone and sheared argillite containing a molluscan fauna of Cenomanian age (fossil locality 3). All the metasandstone is reconstituted to Textural Zones 1 to 2 of Blake and others (1967), containing the blueschist mineral assemblage (lawsonite + white mica ± sodic amphibole ± jadeite + pumpellyite). Chert and volcanic rocks are also metamorphosed to blueschist grade,

**v** metavolcanic rocks. Questionably correlated with the South Fork Mountain schist of the Pickett Peak terrane of the Eastern Belt (Blake and others, 1982; Irwin and others, 1974) by Etter (1979). Unit was considered by Etter (1979) to have been metamorphosed to blueschist grade ~137 Ma, based on a whole-rock K-Ar age for the metasandstone (Suppe and Foland, 1978). A recent Pb/U age on detrital zircons from these rocks provides a depositional age of 123 Ma, and an $^{40}Ar/^{39}Ar$ determination on phengitic white mica provides a metamorphic age of about 120 Ma (Dumitru and others, 2009). The Pb-U and $^{40}Ar/^{39}Ar$ ages are here considered more reliable than the older K-Ar ages

**b** **Blueschist and associated rocks**—High-grade blocks of glaucophane schist, eclogite, and garnet amphibolite

**un** **Blocks of unknown composition**—Mapped from air photos

**Eastern Belt (Cretaceous and Jurassic)**

**Yolla Bolly Terrane**

**fsm** **Metasedimentary rocks of Sanhedrin Mountain (Cretaceous and Jurassic)**—Composed of fine- to coarse-grained, thin-to medium-bedded metasandstone reconstituted to textural zone 2 of Blake and others (1967), with sheared interbedded argillite. Quartzofeldspathic sandstone predominates in the northeastern part of the unit with local sheared lenses of conglomerate and tuffaceous greenstone. Higher on Sanhedrin Mountain, metasandstone with a high percentage of lithic grains occurs with abundant thick manganiferous metachert lenses and less common metavolcanic rocks including tuff, pillow basalt, volcanic breccia, and diabase. Unit may include minor thin zones of undivided mélange in a few areas, as suggested by a few areas with scattered blocks of high-grade blueschist and serpentinite

**Blocks and intercalated lenses of metavolcanic rocks**

**Blocks and lenses of metachert associated with metavolcanic rocks**

**Intrusive rocks of Monkey Rock (Cretaceous)**—Metagabbroic dike and sill-like bodies intrusive into metasandstone and argillite of Hull Mountain

**fhm** **Metasandstone of Hull Mountain (Cretaceous)**—The Metasandstone of Hull Mountain consists largely of structurally stacked slabs of broken formation, possibly with minor undivided zones of mélange (suggested by a few scattered blocks of blueschist or serpentinite). The broken formation of these stacked slabs is composed largely of fine-grained, thin-bedded, lithic metasandstone and interbedded argillite; well-bedded, medium- to coarse-grained, lithic metasandstone; and intercalations of chert pebble conglomerate, grit, and rare lenses of volcanic rock and chert. Unit locally includes zones of broken metasand-

**un** **Blocks of unknown composition**—Mapped from air photos

**ch** **Blocks and interleaved lenses of chert**

**vh** **Blocks and interleaved lenses of metavolcanic rocks**

**Pickett Peak Terrane(?)**

**fmp** **Metasedimentary rocks of Mendocino Pass (Early Cretaceous)**- Composed of slightly sheared, fine- to medium-grained, thin- to medium- bedded metasandstone reconstituted to textural zone 2 of Blake and others (1967), interbedded with black argillite. Metasandstone beds are locally graded, and interbedded chert and greenstone lenses are present at one locality. Metasandstone includes Valanginian Buchias at one locality (fossil locality 1, table 1). This unit is distinguished by development of a moderate foliation, in contrast with structurally overlying schistose metamorphic rocks of Black Butte and Bald Mtn., and by its intact coherent aspect, in contrast with structurally underlying disrupted rocks in mélange of the Central Belt. Unit may correlate with Valantine Spring Formation unit of Pickett Peak Terrane (Blake and others, 1992). East of Bald Mountain unit includes the following units:

**cm** **Metachert lens interbedded with metagreenstone**

**vm** **Metagreenstone lens interbedded with metachert**

**Pickett Peak Terrane**

**fsb** **Metamorphic rocks of Black Butte and Bald Mountain (Early Cretaceous)**—Metasandstone metamorphosed to lawsonite grade and reconstituted to textural zones 2 and 3 of Blake and others (1967). In the Bald Mountain area unit consists of medium- to coarse-grained metasandstone with lenses of stretched-pebble conglomerate that structurally overlies finer grained, well-laminated and foliated metasandstone and argillite with blocks of mafic metavolcanic rocks and metagabbro. Radiometric dating of metasandstone in the Black Butte area (Suppe, 1969) yielded K-Ar ages for metamorphism of the unit of 123 to 127 m.y.

**un** **Blocks of unknown composition**—Mapped from air photos

**vb** **Metavolcanic rocks**—Blocks of mafic metavolcanic and metagabbroic rocks, possibly intrusive locally. Metavolcanics are more abundant and varied to the north of map area

**flpm** **Metamorphic rocks of Lake Pillsbury (Early Cretaceous)**- Foliated fine-grained pelite and metasandstone (Textural Zone 3 to high Textural Zone 2 of Blake and others, 1967) and rare intercalated mafic

- **Contact**—Solid where location is accurate; long-dashed where approximate; dotted where concealed
- **Photolineament marking resistive bedding**
- **Fault**—Solid where location is accurate; long-dashed where approximate; short-dashed where inferred; dotted where concealed. Queries added where identity or existence may be questionable
- **Fault Scarp**—Showing fault line at base of scarp with hachure pointing up slope
- **Thrust fault**—Solid where location is accurate; long-dashed where approximate; short-dashed where inferred; dotted where concealed. Sawteeth on upper plate.
- **Fault lineament**— Inferred from features on aerial photography; dotted where projected beneath surficial deposits; queried where uncertain

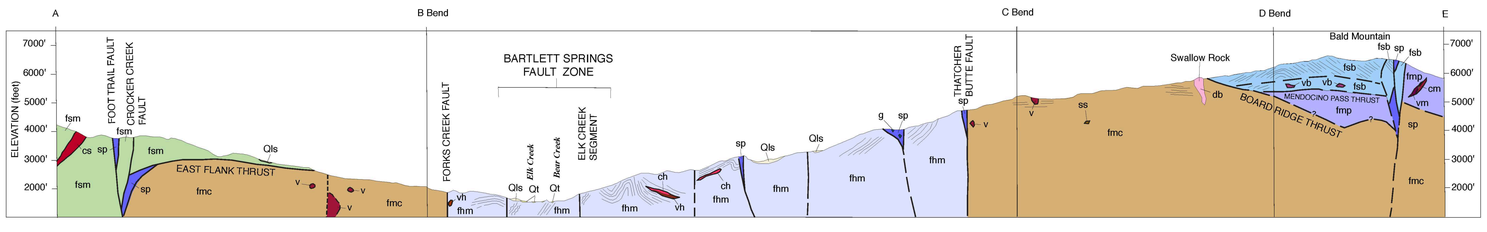
**Strike and dip of beds**
- **Inclined** (35)
- **Overturned** (70)
- **Vertical**

**Strike and dip of metamorphic fabric in sheared or foliated rocks**
- **Inclined** (61)
- **Vertical**

**Location of block or lens too small to map in broken formation or mélange**
- **Blueschist**
- **Serpentinite**
- **Volcanic rock**
- **Chert**

**Fossil Location**—See table 1 for fossil information

**Scott Dam**




# Geologic map of the Bartlett Springs Fault Zone in the Vicinity of Lake Pillsbury and Adjacent Areas of Mendocino, Lake, and Glenn Counties, California

By

Henry N. Ohlin[1], Robert J. McLaughlin[1], Barry C. Moring[1], and Thomas L. Sawyer[2]

2010

[1]U.S. Geological Survey, 345 Middlefield Road, Menlo Park, CA 94025, [2]Piedmont Geosciences, Inc., 10235 Blackhawk Drive, Reno, NV 89506-8527

Any use of trade, product, or firm names in this publication is for descriptive purposes only and does not imply endorsement by the U.S. Government

When this map is printed on an electronic plotter directly from digital file. Dimensional calibration may vary between electronic plotters and between X and Y directions on the same plotter, and paper may change size due to atmospheric conditions; therefore, scale and proportions may not be true on plots of this map.

Digital files available at http://pubs.usgs.gov/of/2010/1301

Suggested Citation: Ohlin, H.N., McLaughlin, R.J., Moring, B.C., and Sawyer, T.L., 2010, Geologic map of the Bartlett Springs Fault Zone in the Vicinity of Lake Pillsbury and Adjacent Areas of Mendocino, Lake, and Glenn Counties, California: U.S. Geological Survey Open-File Report 2010-1301, scale 1:30,000 (http://pubs.usgs.gov/of/2010/1301).

Source:
U.S. Geological Survey (USGS) (Ohlin, H. N., et. al.), 2010.
Geologic Map of the Bartlett Springs Fault Zone in the Vicinity of Lake Pillsbury and Adjacent Areas of Mendocino, Lake, and Glenn Counties, Open File Report 2010-1301.

**Exhibit 6**

Isopach map of sedimentation in Lake Pillsbury, 1922–2015

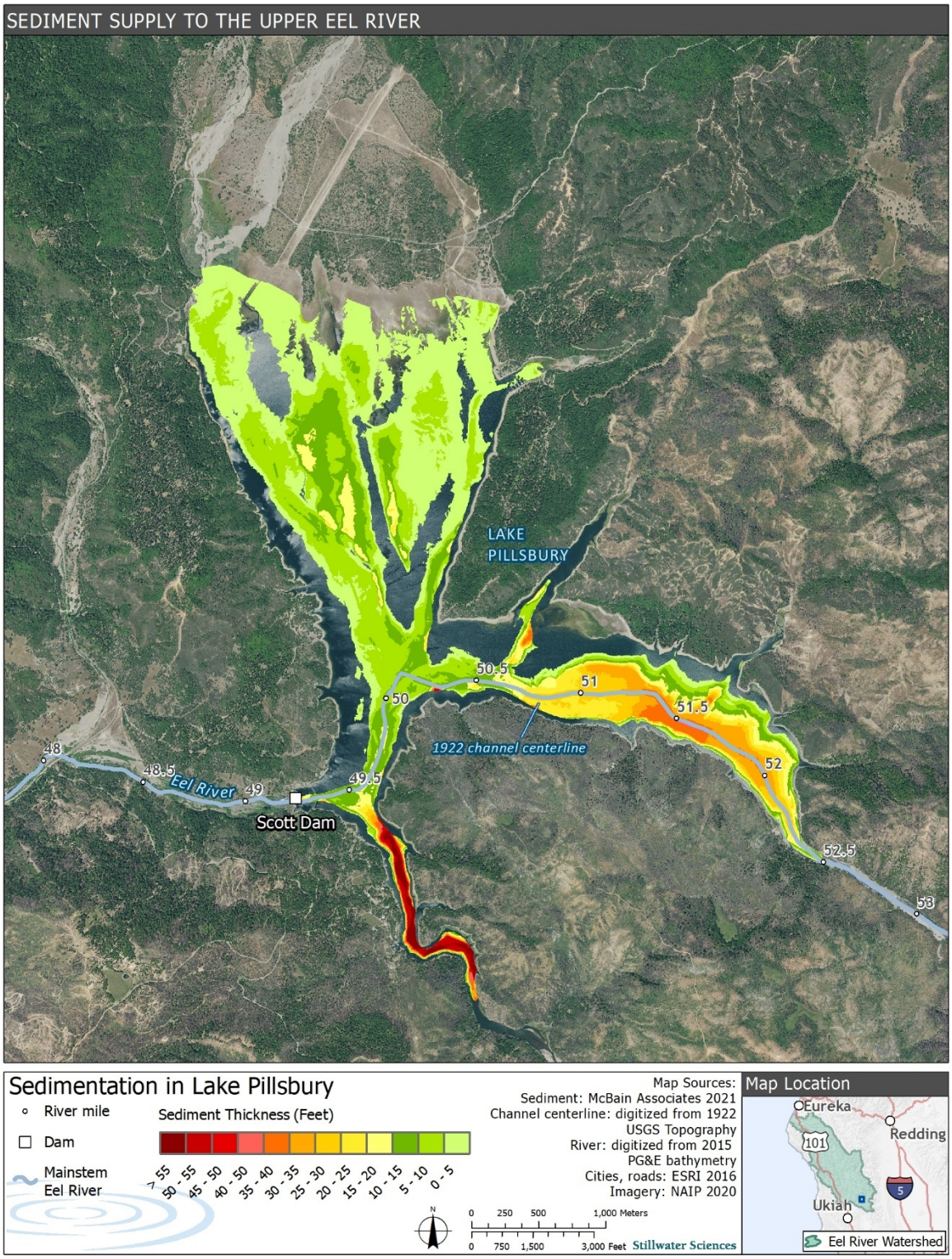



Figure 8. Isopach map of sedimentation in Lake Pillsbury, 1922–2015.



Source:

Stillwater Sciences, 2021a. Sediment Supply to the Upper Eel River. November. Page 23.

**Exhibit 7**

Simulated Bed Profiles at Various Times Following Scott Dam Removal.

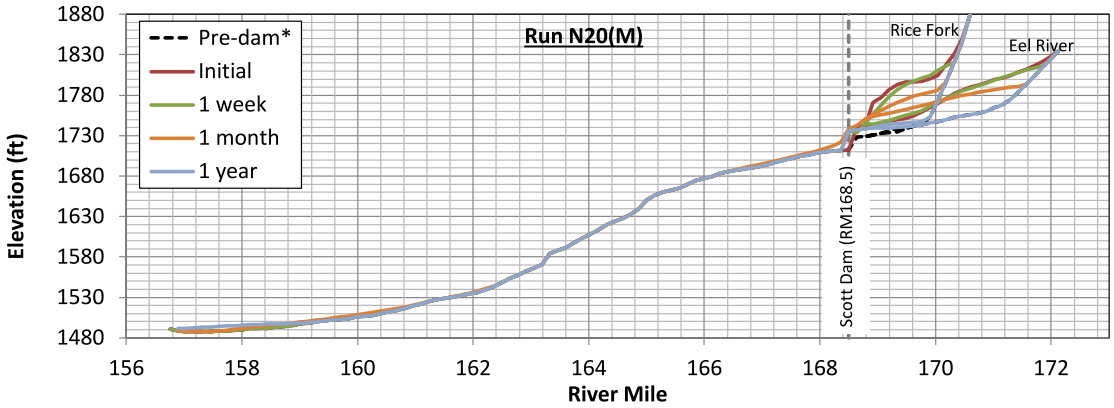



Figure 18. Simulated bed profiles at various times following Scott Dam removal. Temporal variations that are not apparent at this scale are shown in other diagrams below.

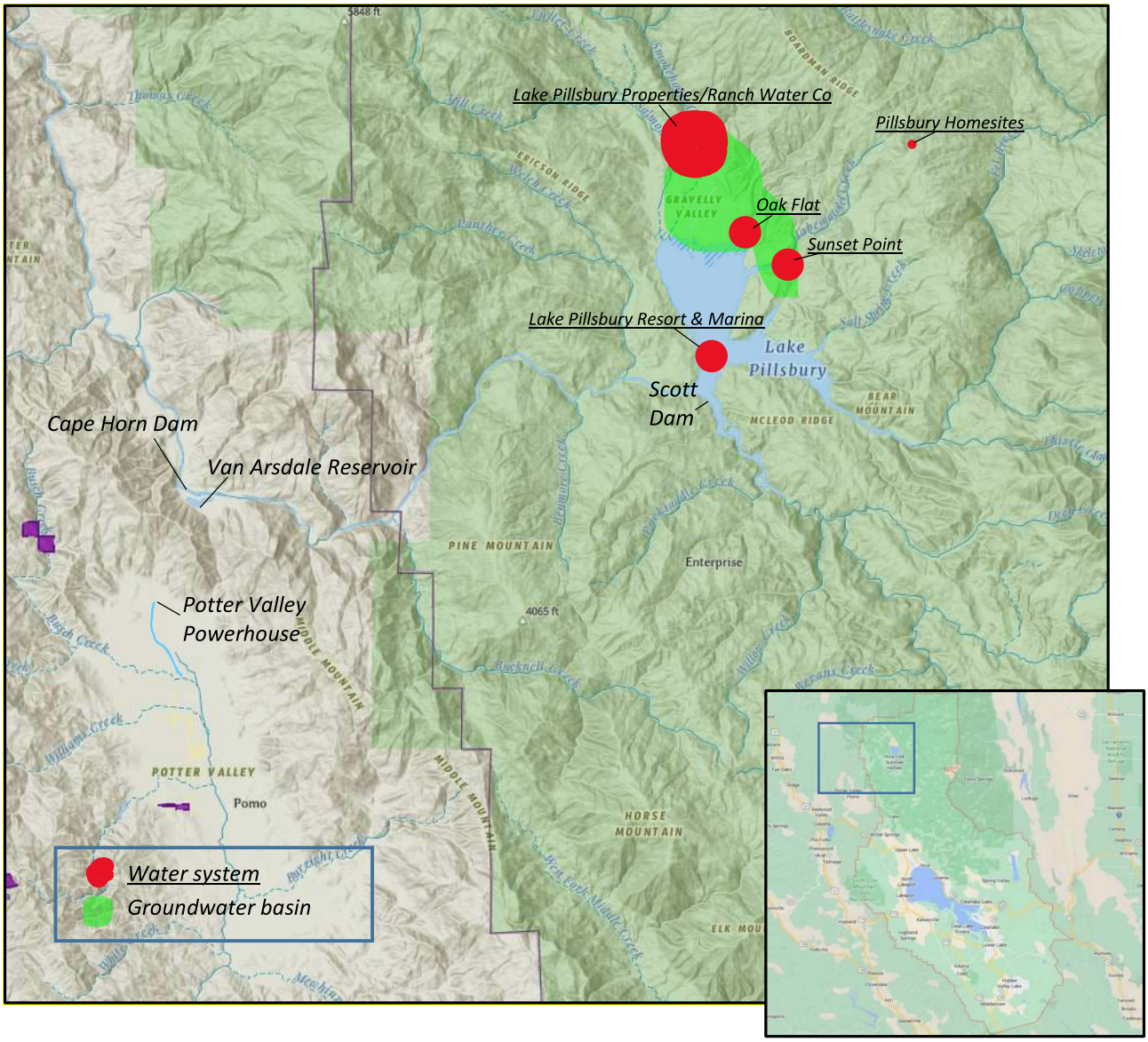
Source:

Stillwater Sciences, 2021f. Analyses and Preliminary Modeling of Sediment Transport Following the Proposed Scott Dam Removal, Eel River, California. November. Page 33

**Exhibit 8:**
Lake Pillsbury Area Map with Water Supply Systems

Lake Pillsbury area is within the North Coast Resource Partnership region. No Lake County participation (Lake County participates with Westside IRWM (with Yolo, Solano, & Napa counites).

**AREA MAP:**

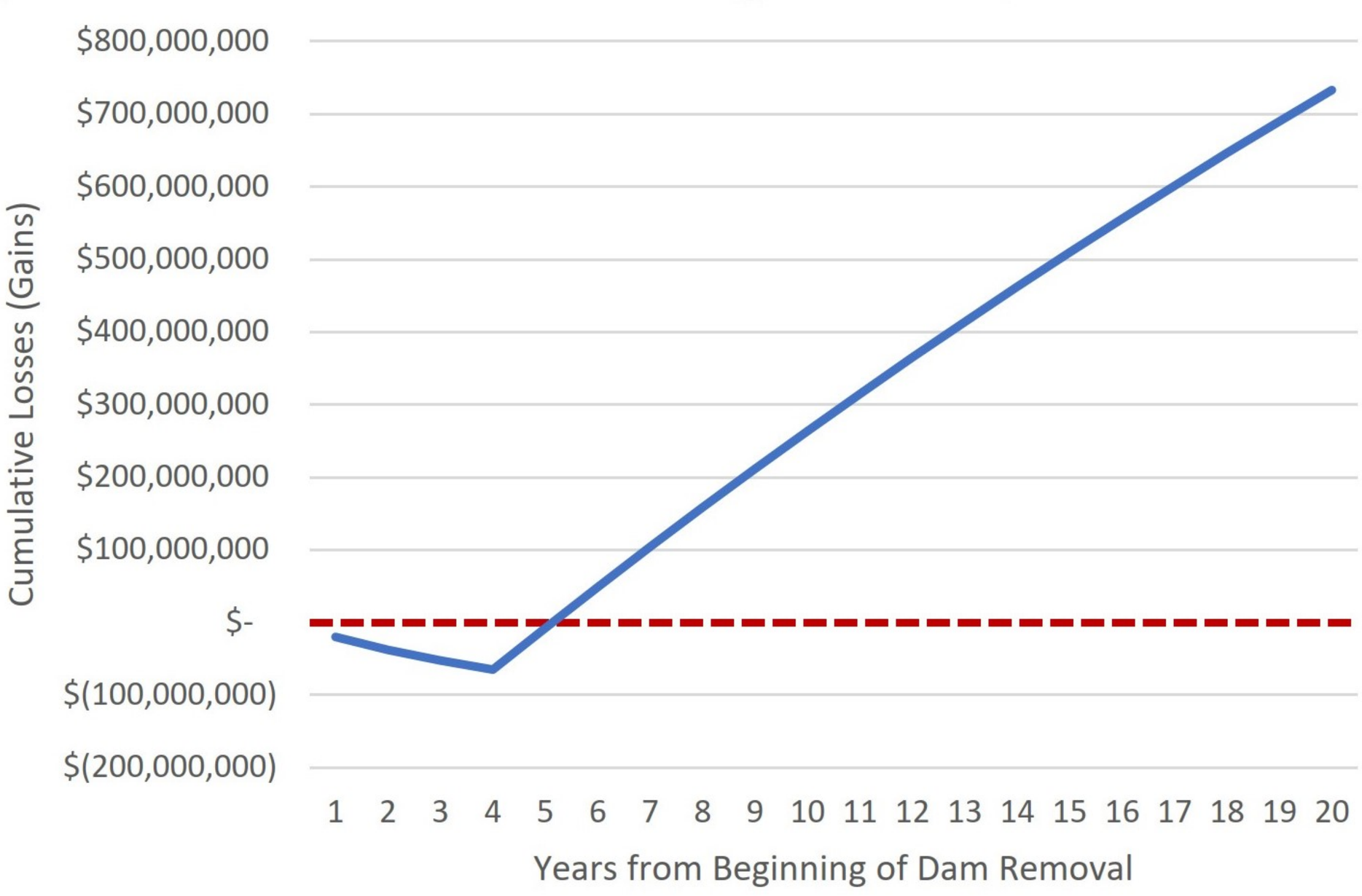



Source:

Pacific Gas and Electric Company (PG&E), 2023. Potter Valley Hydroelectric Project Initial Draft Surrender Application and Conceptual Decommissioning Plan. Prepared by Stantec. November.

**Exhibit 9**

Cumulative Losses at 10% Annual Average Loss of Water, Red Dotted Line = Breakeven




Source:

Economic Forensics and Analytics, Inc., 2023. Economic Impact of Potter Valley (PVP) and Estimated Effects on Sonoma County Economy. October 31.

# Appendix A General Site Photographs

## Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

June 6, 2025

SLR

1217

**Photo 1:** View of upstream face of the dam (Photo from Google Earth)

**Photo 2:** View of the discharge (Photo from Google Earth)

**Photo 3:** View of the downstream face of the dam (Photo from Google Earth)
Note: The kink in the dam alignment at the left abutment (at distance in the image)

**Photo 4:** Aerial view of the dam (Photo from Google Earth)
Note: The kink in the left abutment area presumably designed to avoid the landslide

# Appendix B Past Hydraulic Modeling Technical Aspects

## Existing Conditions – Analysis of Effects of Proposed Scott Dam Decommissioning

**County of Lake**

June 6, 2025

SLR

## Additional Technical Aspects

There are a limited number of active gaging stations within the Eel River according the USGS National Water Information System Mapper[12]. There are 4 active USGS stream gages along the Eel River and there are no USGS active precipitation gages within the Eel River Watershed. PG&E maintains two USGS gages near Scott Dam:

1) USGS gage 11470000[13] (*shown as currently inactive*) records:
   a) Daily observed reservoir storage in acre-feet at midnight based on reservoir elevation for a publication approved record period beginning October 1, 1928, until September 30, 2023; and
   b) Daily minimum and maximum water temperature with a period of record for publication beginning December 16, 1965, through September 10, 1968.
2) USGS gage 11470500[14] (*shown as active*) records:
   a) Daily mean discharge and periodic measured discharges just downstream of Scott Dam in cubic feet per second (cfs) with an approved period for publication record beginning October 1, 1963, until September 30, 2023;
   b) Daily minimum and maximum water temperature with an approved period of record for publication beginning October 3, 1963, until April 30, 1979;
   c) Annual peak streamflow in cfs for the period of approved publication record beginning April 10, 1923, through March 14, 2023;
   d) Field measurements recording streamflow, velocity, channel width, channel area, and other channel observations intermittently between the approved for publication period beginning December 20, 1955, through September 5, 2024; and
   e) Field/lab water quality samples for the approved for publication record period beginning November 11, 1965, until November 15, 1971.

PG&E (2023c) stated operation of Scott Dam is within, “existing regulatory requirements, agreements, and water rights to generate power and deliver consumptive water to local water users.” Scott Dam has 5 radial gates and 26 steel slide gates with all but gate 13 manually operated. Storage release from the dam is controlled by a needle valve that is remotely operated. Regulatory requirements have been noted to adjust dam operations and flow rates based upon a variety of Administrative Orders associated with the FERC Licenses, including:

3) The original license (issued effective April 15, 1922, and expired April 14, 1972);
4) Annual licenses from 1972 to 1983;
5) The 1983 FERC license;

[12] Water Resources of the United States—National Water Information System (NWIS) Mapper - https://maps.waterdata.usgs.gov/mapper/index.html

[13] LK Pillsbury NR Potter Valley CA - USGS Water Data for the Nation - https://waterdata.usgs.gov/monitoring-location/11470000/#period=P1Y&showMedian=true

[14] Eel R BL Scott Dam NR Potter Valley CA - USGS Water Data for the Nation - https://waterdata.usgs.gov/monitoring-location/11470500/?agency_cd=USGS#period=P1Y&showMedian=true

6) The 1983 amended license in 2004, which incorporated the terms of NMFS' Reasonable and Prudent Alternative (RPA);
7) A 2007 operational "reinterpretation" of the terms of the 2002 RPA; and
8) The expiration of the 1983 license on April 22, 2022.

The *Potter Valley Project Huffman Ad-Hoc Committee Water Supply Working Group: Results of Initial Water Supply Modeling for Potter Valley Project and Russian River Alternatives* prepared in 2019 by various consultants and Sonoma Water (Addley, 2019) analyzed nine water supply scenarios for the Potter Valley Project system including Scott Dam. These scenarios were developed using the USACE's Hydrologic Engineering Center Reservoir System Simulation (HEC_ResSim) model to inform and determine the feasibility of meeting the goals of the Two-Basin Solutions Partnership. Two scenarios were identified to best meet those needs (*which notably did not emphasize County of Lake-focused water supply concerns*): Scenario 2 and Scenario 4B. Scenario 2 includes decommissioning Scott Dam with either retaining or removing Cape Horn Dam, assuming a new diversion at Cape Horn can provide a reliable seasonal diversion, while implementing fish flows from the Environmental Impact Report (EIR) developed by Sonoma County Water Agency to comply with the 2008 Biological Opinion (BO) and Lake Mendocino Forecast Information Reservoir Operations (FIRO) activities on the Russian River. Scenario 4B includes retaining the Scott Dam and Cape Horn Dam (unless a diversion alone can maintain seasonal needs) with revised operations under the PVP modified RPA including Eel River flow requirements from the 2002 BO and FIRO and fish flow EIR operations for the Russian River.

The *PVP Feasibility Study: Alternatives Description and Project Plan* (Stillwater et al 2021b) built on the water supply scenarios, presenting 5 alternatives:

1) Alternative 1: Current facilities and hydropower generation
2) Alternative 2: Removal of Scott Dam and hydropower generation
3) Alternative 3: Removal of both Scott and Cape Horn dams and hydropower generation
4) Alternative 4: Removal of Scott Dam and no hydropower generation
5) Alternative 5: Removal of both Scott and Cape Horn dams and no hydropower generation

Alternative 1 was determined to provide the best water supply reliability. Alternative 2 was feasible with concerns about supply reliability. Alternative 3 was determined not likely to provide sufficient water supply reliability. The Parties (*California Trout, Humboldt County, Mendocino County Inland Water and Power Commission, Round Valley Indian Tribes, and Sonoma County Water Agency*) concluded, based on the fish passage improvement potential and assuming the water supply reliability can be resolved in a manner satisfactory to Mendocino County Inland Water & Power Commission and Sonoma Water, Alternative 2 was the best alternative to pursue as the proposed Project Plan.

PG&E presented a proposal to remove Scott and Cape Horn Dams, followed by site restoration (*noting no commitment to a specific site restoration plan has been made, to date*). Scott Dam removal is proposed to be implemented using either a rapid or phased removal approach based upon the "McMillen Jacobs Associates *Scott Dam and Cape Horn Dam Removal Alternatives Technical Memorandum* (2021), existing site conditions, engineering drawings, and technical expertise. The approaches for removal of Scott Dam should be considered preliminary in nature and are subject to change based on further engineering design and resource agency

consultation." The proposed approach for rapid or phased removal is outlined below (PG&E 2023):

Rapid removal of Scott Dam is proposed to continue over a two-year duration and flushing of approximately 12 million cubic yards of sediment within the dam impoundment. The rapid removal includes specific actions during each low and high flow season over the two-year period:

1) Year 1 Low flow season (inflows < 400 cfs) approximately June – October
    - Initiate reservoir drawdown;
    - Mobilization of equipment and road improvements to facilitate dam removal;
    - Construction of the Adit Tunnel in the base of the spillway;
    - Remove the upper portions of the dam through lowering and notching; and
    - Dredge sediments near the new tunnel intake.
2) Year 1 & 2 High flow season (inflows between 1,000 cfs and 5,000 cfs) approximately November – May
    - Initiate sediment flushing through high flows and other means.
3) Year 2 Low flow season approximately June – October
    - Complete final dam removal (approximately 80% of the dam will be removed with the remaining 20% stored and capped or stored in the plunge pool below the final river grade)
4) Restoration of the Eel River channel and previous impoundment and the removal of select support facilities and features (*no commitment has been made to a detailed plan*).

Phased dam removal is proposed to successively lower and notch Scott Dam over an approximate four-year duration where sediment flushes through the dam in three seasonal sediment pulses. The phased dam approach includes specific actions during each low and high flow season over the four-year period:

1) Year 1 Low flow season (inflows < 400 cfs) approximately June – October
    - Initiate reservoir drawdown
    - Mobilization of equipment and road improvements to facilitate dam removal
    - Lower and create a 50-foot wide notch in the dam to elevation 1,850 feet (PG&E datum which equals the National Geodetic Vertical Datum of 1929 (NGVD29) + 81.7 feet)
2) Year 1 & 2 High flow season approximately November – May
    - Flushing of sediment from the reservoir through the lowered notch during high flows with an anticipated release of 1.1 million cubic yards (Stillwater Sciences 2021b)
3) Year 2 Low flow season approximately June – October
    - Lower and notch dam to elevation 1,830 feet (PG&E datum)
4) Year 2 & 3 High flow season approximately November – May
    - Flushing of sediment from the reservoir through the lowered notch during high flows with an anticipated release of 8.5 million cubic yards (Stillwater Sciences 2021b)

5) Year 3 Low flow season approximately June – October
    - Lower and notch dam to elevation 1,810 feet (PG&E datum)
6) Year 3 & 4 High flow season approximately November – May
    - Flushing of sediment from the reservoir through the lowered notch during high flows with an anticipated release of 2.4 million cubic yards (Stillwater Sciences 2021b)
7) Year 4 Low flow season approximately June – October
    - Complete final dam removal (approximately 80% of the dam will be removed with the remaining 20% stored and capped or stored in the plunge pool below the final river grade)
8) Restoration of the Eel River channel and previous impoundment and the removal of select support facilities and features.

Several hydrologic analyses have been completed as part of the initial Relicensing effort, and on notice PG&E no longer planned to pursue relicensing, to understand the existing and uncontrolled (*or unimpaired*) hydrology for the Eel River, and particularly Scott and Cape Horn Dams.

A flood frequency analysis (FFA) was completed by McMillen Jacobs Associates in 2018 using the USGS 11470500 station's annual peak stream flows over the period of record (*presumably between 1923 and 2016*) to estimate the unimpaired peak discharges for the 2-, 10-, 50-, and 100-year events using USGS Bulletin 17B guidelines. This FFA was referenced and used in the 2021 Stillwater Sciences *Analyses and Preliminary Modeling of Sediment Transport Following the Proposed Scott Dam Removal, Eel River, California.*

Work to characterize the hydrology was undertaken by Cardno as part of the initial relicensing of the PVP but work on the Hydrology and Project Operations modeling ended on January 25, 2019, when PG&E filed a notice with FERC that they would no longer be relicensing the PVP. In a memo dated December 30, 2019, Cardno summarized hydrologic and project operations modeling progress up to January 25, 2019. An abridged summary of that progress memorandum (*as it pertains to the proposed Scott Dam removal*) is provided below and in Table AQ 2-1 (Exhibit B-1) of Cardno's Technical Memorandum:

Elements completed by PG&E as part of their initial Relicensing effort (i.e., as of January 25, 2019) include the following:

- Reservoir, stream, and/or diversion gaging information (*PG&E and USGS*) at Lake Pillsbury and powerhouse release locations within the East Branch Russian River were collected. This work was noted to continue through 2019, and the current status is unknown.
- A database of historical gage data was created.
- A temporary stage recorder was noted to have been installed in 2018 to support the generation of rating curves in the Eel River and the Rice Fork above Lake Pillsbury, as well as in Tomki Creek downstream of the Cape Horn Dam. The gage data is meant to inform the development of unimpaired hydrology measured over a two year period (2018-2020). Status of these temporary gages is unknown and any data recorded was not noted for use in the development of unimpaired flows.

Elements initiated during the PG&E relicensing effort but noted to be incomplete (*as of January 25, 2019*), include the following:

- Develop an unimpaired hydrology daily flow dataset for the Project using historical gage data and/or mass balance or proration, as necessary, to generate data where gage data was missing. It is unclear what other elements needed to be included to finalize the unimpaired hydrology.
    - Daily unimpaired hydrology was developed for the Eel River for the 10 watersheds noted in Figure AQ 1-2 (Exhibit B-2) in Cardno 2019. Unimpaired hydrology was developed from estimating travel times for the Eel River from the stream gage data using 15-minute datasets from USGS gages noted in Table QA 1-14 or Exhibit B-3 (Cardno 2019) with a range of available data from October 1, 1992, through September 30, 2017 or October 1, 2007 through September 30, 2017 for the Van Arsdale Reservoir elevation gage. The USGS gages used in the Cardno (2019) 15-minute gage analysis do not all have 15-minute flow data available or are not approved or recorded for publication through USGS. The technical memorandum states, “The analysis utilized a calculated 15-minute inflow to Van Arsdale Reservoir. The 15-minute calculated inflow is used with caution due to travel time from Lake Pillsbury and is only used to determine long term trends in inflow, not instantaneous values (Cardno 2019).”
    - The PG&E E2 gage data (USGS gage 11470500) was noted to vary from the calculated Scott Dam flow release from the needle valve and Van Arsdale inflow from the rating curve provided by PG&E shown in Figure AQ 1-18 and 1-19 respectively (Exhibit B-4 and B-5) (Cardno 2019). Cardno noted the error was “likely from spills at Lake Pillsbury and that the low-level outlet at Lake Pillsbury may have some bias in the 150-220 cfs range.”
    - The Cardno 2019 memorandum references the calculated unimpaired flows were provided in a dss format in Attachment C – Unimpaired Flows. Attachment C was not provided and was stated to be available upon request. The *Scott Dam and Cape Horn Dam Removal Alternatives* (McMillen 2021) references and provides a graph (McMillen 2021, Figure 2) of the unimpaired monthly average discharge (Exhibit B-6) presented as part of the *Results of Initial Water Supply Modeling for PVP and Russian River Alternatives* (Addley 2019) which are noted to be estimates from the Cardno unimpaired hydrology calculations.
- Develop a method to characterize project hydrology effects on the lower Eel River at the furthest downstream location.
- Develop an Existing Operations daily flow dataset using the Project Operations Model.
- Develop a Project Operations model using the USACE Reservoir Simulation version 3.1 or later.
- Evaluate and validate the Project Operations model by comparing the model output simulation to the relevant historical record and provide a validated model.
- Configure the Project Operations existing conditions model to represent the current (as of 2019) operations including the physical constraints and regulatory and contractual requirements since the 2004 FERC license amendment and implementation of the RPA.

Elements that were planned as part of the initial PG&E relicensing effort that had yet to be initiated as of January 25, 2019, include the following:

- Generate climate change inflow hydrology for approximately the year 2050 by adjusting the unimpaired hydrology using the climate change data products developed by the California Water Commission.
- Characterize Lake Pillsbury spills, including antecedent reservoir storage conditions, for the historical gage dataset and modeled Existing Operations hydrology.
- Ramping rates will be characterized for the unimpaired hydrology, historical dataset, and new hydrology data collected upstream of Lake Pillsbury, and modeled Existing Operations hydrology.
- Analyze unimpaired hydrology and modeled Existing Operations at key locations in the Study Area using Indicators of Hydrological Alterations.
- Generate a flood frequency analysis for the unimpaired hydrology using annual peak daily average flow data at key locations in the Study area.

The Federal Emergency Management Agency (FEMA) has developed several Flood Insurance Studies (FIS) for the Lake County, with the most recent effective revision completed October 10, 2024, and previous effective FIS reports published in September 2005 and March 1998. The 2024 FIS reports a summary of Pillsbury Lake stage-frequency relationships for the 10-, 50-, 100-, and 500-year floods which are 1,826.3; 1,829.8; 1,831.0; and 1,834.0 stage feet respectively (FEMA, 2024a). The stage-frequency has remained unchanged since the 1998 FIS (FEMA, 1998). The 2005 and 2024 FISs do not detail the hydrology or hydraulics for the Eel River. The 2005 FIS reports approximate studies for the Eel River, Soda Creek, and Smokehouse Creek with a detailed study of the Lake Pillsbury Perimeter (FEMA, 2005). The special flood hazard area (SFHA) inundation boundaries developed as part of the FIS for the Eel River are defined as a zone A, meaning the area inundated by the 1% annual chance floodplains were determined by approximate methods and no base flood elevations (BFEs) or depths are shown per FEMA Flood Insurance Rate Map (FIRM) panels 06033C0140D and 06033C0125D, per the FEMA National Flood Hazard Layer Viewer[15].

The preliminary July 2024 FEMA FIS for Mendocino County references a more detailed analysis of the Eel River from the Cape Horn Dam downstream approximately 1.8 miles that was completed in 1981. The 1981 study identified the recurrence intervals for the 10%, 2%, 1%, and 0.2% annual chance floods, predicted to be 41,000; 70,000; 82,500; and 112,000 cfs respectively. The recurrence intervals were determined using a USGS Bulletin 17B analysis. (FEMA 2024b)

The Stillwater *Hydraulic Model Development and Potential Flood Implications Following the Proposed Scott Dam Removal, Eel River, California* (2021e) utilized the USGS *StreamStats*[16] program to predict the peak discharges for unimpaired flows. The *StreamStats* program uses regional regression equations developed for estimating magnitude and frequency floods in California assuming undeveloped or unimpaired conditions. The results provided the 1% annual chance or 100-year peak discharge rate of 60,200 cfs immediately downstream of Scott Dam.

[15] FEMA's National Flood Hazard Layer (NFHL) Viewer - https://hazards-fema.maps.arcgis.com/apps/webappviewer/index.html?id=8b0adb51996444d4879338b5529aa9cd&extent=-72.80123646790743,41.867275612089074,-72.79604371125389,41.8692730027783

[16] StreamStats - https://streamstats.usgs.gov/ss/

Stillwater (2021e) also performed a flood frequency analysis (FFA) on four USGS gages along the Eel River (see Table 2 of Stillwater 2021e or Exhibit B-7) using the Hydrologic Engineering Center's statistical software package (HEC-SSP) to perform a USGS Bulletin 17C statistical analysis. The period of record for the USGS gage (11470500) included 1923-2019 and predicted existing hydrology peak discharges of 11,205 cfs for the 2-year; 34,034 cfs for the 10-year; 42,026 cfs for the 25-year; 50,883 cfs for the 50-year; and 56,417 cfs for the 100-year.

A 1-dimensional hydraulic U.S. Army Corps of Engineers' (USACE) Hydrologic Engineering Center's River Analysis System (HEC-RAS) model was developed and reported by Stillwater Science in August and November 2021 as summarized in the *Hydraulic Model Development and Potential Flood Implications Following the Proposed Scott Dam Removal, Eel River, California* (Stillwater 2021e) and *Analyses and Preliminary Modeling of Sediment Transport Following the Proposed Scott Dam Removal, Eel River, California* (Stillwater, 2021f) technical memorandums. Model development included a 49-mile reach of the Eel River from Scott Dam downstream to the Middle Fork Eel River and required gathering and analyzing various data to create the model inputs, boundary conditions, and geometry. Figure 1 of the Stillwater (2021e) technical memorandum or Exhibit B-8 shows the USGS gage locations and model extents. The geometry included topographic and bathymetric data to develop cross sections and the longitudinal profile, roughness coefficients, and assumptions about the geomorphology and sediment transport of the stream channel under existing and proposed conditions. The following summarizes data collection and results used to develop the hydraulic model:

- PG&E collected topographic and bathymetric data in 2018 and in 2019 for Scott and Cape Horn Dams. Bathymetric data for the Van Arsdale Reservoir was also collected in 2006, and used in the development of the channel cross sections. (Stillwater, 2021e)
- The USGS's publicly available LiDAR, taken from the Nation Map web service[17], was also used to develop channel cross sections and validate the topographic data. The LiDAR datasets used are:
    - the 2018-2019 Northern California Wildfire LiDAR with a relative square meter horizontal and 10 cm vertical accuracy and
    - the 2015-2016 Lake County, CA LiDAR with a relative square meter horizontal and 10 cm vertical accuracy
- Additionally, Stillwater completed a supplemental field survey between July 2020 to February 2021 to generate channel cross sections at approximately 1,000-foot intervals from approximately Eel River mile (RM) 119.5 to just downstream of Scott Dam at RM 168.5. A total of 264 cross sections were developed with from the Stillwater surveys, interpolated sections, and PG&E data to characterize the channel geometry and four bridge crossings. The four bridge crossings modeled include the Soda Creek Bridge, Pioneer Bridge, Hearst-Willits Road Bridge, and Eight Mile Bridge. (Stillwater, 2021e)
- The existing conditions Stillwater (2021e) hydraulic model assumed an in-line weir for the Cape Horn dam and did not include detailed dam operations, but used the dam dimensions to simulate the spillway based on data provided by PG&E. The hydraulic model used the impaired 100-year hydrologic FFA results analyzed as part of their study

[17] The National Map - Advanced Viewer

(*described above*) to predict the existing conditions water surface elevations. The existing condition water surface elevations were within 0.02 to 1.5 feet when compared to the recorded water surface elevations. When modeling the FEMA flows under existing conditions, the inundation limits were relatively in agreement, with an exception near Todd Road. The existing conditions water surface elevation profile is shown in Figure 5 of the Stillwater (2021e) technical memorandum (Exhibit B-9)

- The future conditions Stillwater hydraulic model (2021e) assumes Cape Horn Dam remains in place and Scott Dam is removed. The model uses unimpaired hydrology calculated via the USGS *StreamStats* program, and then uniformly increases the impaired discharges at all downstream flow locations by the difference between impaired and unimpaired flows. The channel geometry was assumed to remain unchanged based on the sediment transport model (Stillwater 2021f), predicting the 100-year flows would flush out all sediment released from the removal of Scott Dam. The results of the future conditions model predicted an increase in water surface elevations for the 100-year event to show a maximum increase in flood waters by up to 1.2-feet. Stillwater concludes that risk is not expected to substantially increase following Scott Dam removal during a 100-year flood event and that additional data is expected to refine these conclusions. Limited visualizations of the increased flood profile and inundation area provided in the Stillwater (2021e) report near the Scott Dam. The HEC-RAS model developed was not provided for review and further data could not be extrapolated specific to the Scott Dam flood inundation area.
- Downstream of Cape Horn Dam, the longitudinal profile is relatively uniform at an average slope of 0.29%. There is a low channel gradient reach consisting of 0.18% for roughly 5 miles upstream between Cape Horn Dam and Bucknell Creek presumably from deposition within the Van Arsdale Reservoir. Sediment has been noted to be an issue in function of the diversion and fish ladder at the Cape Horn Dam and Van Arsdale Reservoir with occasional mechanical sediment removal being required. Sediment also passes over Cape Horn Dam during high flow events. Upstream of the Van Arsdale Reservoir and Bucknell Creek is a roughly 0.8% slope for about 4 miles before changing to a 0.33% slope between the Benmore Creek confluence and Scott Dam. (Stillwater 2021f)
- “Gravel and sand have the potential to be deposited in downstream reaches after dam removal. No model was developed to simulate gravel transport dynamics because there is inadequate data to do so.” (Stillwater, 2021f) The risk of not collecting further data or developing more robust modeling would induce a lower level of confidence in the predictions and recommendations for future transport. This could detrimentally affect the Cape Horn Dam and/or diversion operations and deposited sediments have a high potential to change the river channel’s longitudinal profile over time. Sediment aggradation and degradation can change to the predicted flood water surface and inundation areas, increase scour potential and undermine bridge abutments, and/or cause aggradation at crossings creating a constriction and higher risk of damage.
- Documents note no sediment management within Pillsbury Reservoir since inception.

# Appendix B Exhibits

Exhibit B-1: Unimpaired Monthly Average Water Discharge Downstream of Scott Dam

Exhibit B-2: Water Temperature Technical Study Plan Status

Exhibit B-3: Eel River Watersheds for Unimpaired Hydrology

Exhibit B-4: Stream Gage Accuracy Analysis at Lake Pillsbury and Van Arsdale

Exhibit B-5: Scott Dam Spillway Rating Curve

Exhibit B-6: Cape Horn Dam Spillway Rating Curve

Exhibit B-7: USGS Stream Gages Used for Flood Frequency Analyses

Exhibit B-8: Hydraulic Model Study Reach and Potter Valley Project Vicinity.

Exhibit B-9: Existing Conditions Water Surface Elevation

## Exhibit B-1

Unimpaired Monthly Average Water Discharge Downstream of Scott Dam

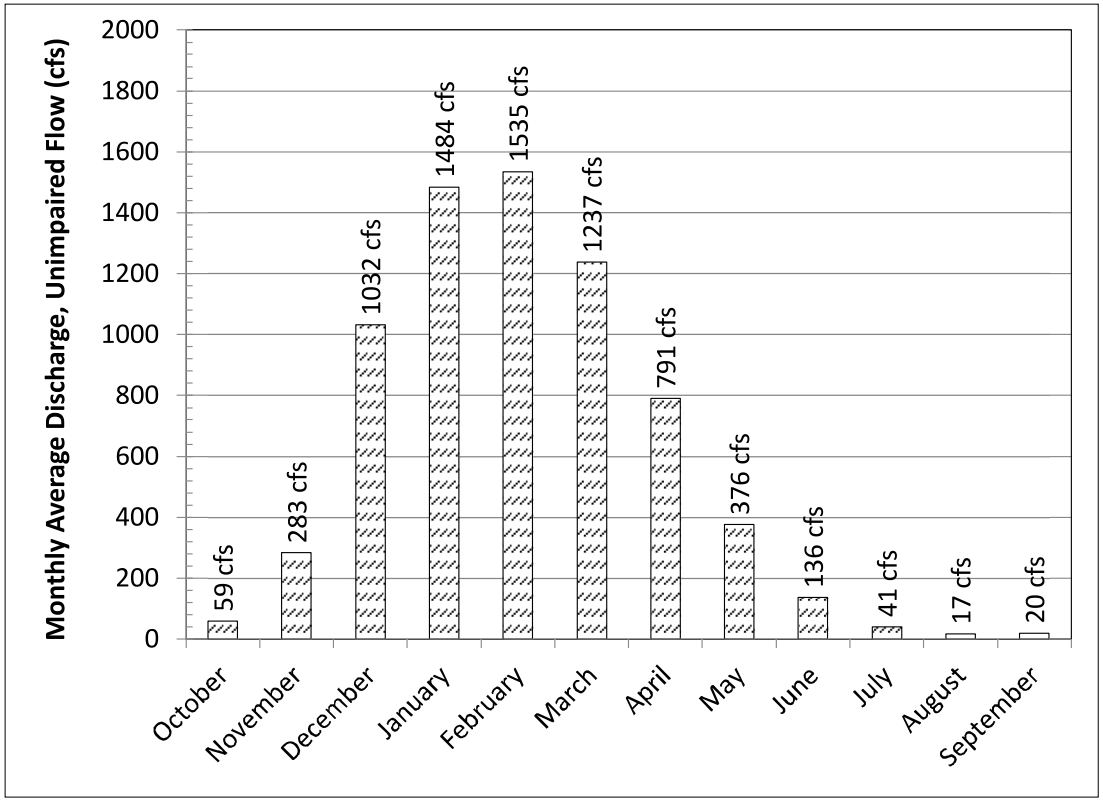



*Figure 2 Unimpaired Monthly Average Water Discharge Downstream of Scott Dam (Addley et al. 2019)*

Source:

McMillen Jacobs Associates, 2021. *Scott Dam and Cape Horn Dam Removal Alternatives Technical Memo*. Prepared for Two-Basin Solution Partners, November. Page 10.

**Exhibit B-2**

Water Temperature Technical Study Plan Status

| Study Elements Completed / Data Collected | Resource Agency Consultation / Technical Working Group Updates | Technical Study Plan Variances | Outstanding Study Elements | Modifications to Ongoing Studies | Proposed New Studies |
|---|---|---|---|---|---|
| **AQ 1 – Hydrology and Project Operations Modeling** | | | | | |
| **On-going Potter Valley Project Monitoring Studies (PG&E)**<br>• Collected reservoir, stream, and/or diversion gaging information (PG&E and USGS) at Lake Pillsbury and powerhouse release locations within the East Branch Russian River. This will continue through 2019.<br>**Hydrology Characterization**<br>*Hydrology Datasets*<br>• Created a database of historical gage data.<br>• Installed a temporary stage recorder and generate a rating curve in the Eel River and the Rice Fork above Lake Pillsbury and in Tomki Creek to collect flow data inform the development of unimpaired hydrology. This collection began in 2018 and will continue for two years.<br>**Water Balance/Operations Model**<br>*Develop Model*<br>• A technical modeling group comprised of relicensing participants with modeling expertise was formed to collaborate on model development. | **Work Group Updates**<br>• September 26, 2017: Conducted Aquatic Resources TWG meeting to discuss implementation of the AQ 1 – Hydrology and Project Operations Modeling TSP. (UVCC)<br>• June 27, 2018: Conducted Aquatic Resources TWG meeting to discuss development of a HEC-ResSim Model and installation of temporary stage recorder. (WebEx)<br>• October 15, 2018: Conducted Aquatic Resources TWG meeting to continue discussion of model development and evaluation. (WebEx) | **Water Balance/Operations Model**<br>• Voluntarily enhanced spatial extent of the model to include the lower Eel River from the Middle Fork Eel to the Scotia Gage (USGS 11477000).<br>• Voluntarily increased the period of record to include 1911 through 2016 (instead of 1975 – 2016). | **Hydrology Characterization**<br>*Hydrology Datasets*<br>• Develop an unimpaired hydrology daily flow dataset for the Project using historical gage data and/or mass balance or proration as necessary to generate data where gage data was missing. **(In Progress)**<br>• Generate climate change inflow hydrology for approximately the year 2050 by adjusting the unimpaired hydrology using the climate change data products developed by the California Water Commission.<br>• Develop an Existing Operations daily flow dataset using the Project Operations Model. **(In Progress)**<br>*Indicators of Hydrological Alterations*<br>• Analyze unimpaired hydrology and modeled Existing Operations hydrology at key locations in the Study Area using Indicators of Hydrological Alterations.<br>*Flood Frequency*<br>• Generate a flood frequency analysis for the unimpaired hydrology and modeled Existing Operations hydrology using annual peak daily average flow data at key locations in the Study area. A second flood frequency analysis was developed from the historical gage data along the Eel River and East Branch Russian River.<br>*Lake Pillsbury Spills and Ramping Rates*<br>• Characterize Lake Pillsbury spills, including antecedent reservoir storage conditions, for the historical gage dataset and modeled Existing Operations hydrology.<br>• Ramping rates will be characterized for the unimpaired hydrology, historical dataset, and new hydrology data collected upstream of Lake Pillsbury, and modeled Existing Operations hydrology.<br>• Biologically suitable stage change and flow ramping rates will be identified in coordination with Study AQ 5 – Instream Flow.<br>**Water Balance/Operations Model**<br>*Develop Model*<br>• Using the USACE Reservoir Simulation Version 3.1 model (or latest version) the Project Operations model will be developed in collaboration with relicensing participants. (**In Progress**) | | |

**Exhibit B-2**

Water Temperature Technical Study Plan Status

| Study Elements Completed / Data Collected | Resource Agency Consultation / Technical Working Group Updates | Technical Study Plan Variances | Outstanding Study Elements | Modifications to Ongoing Studies | Proposed New Studies |
|---|---|---|---|---|---|
| | | | *Model Calibration and Validation*<br>• In coordination with the technical modeling group, PG&E will evaluate the model by comparing the model output simulation to the relevant historical record. A validated model will be produced and provided to the technical modeling group for review and comment prior to making final model simulation runs. (**In Progress**)<br>*Development of Existing Operations Scenario*<br>• The model will be configured to represent how the Project currently operates (existing operations), including all physical constraints, and regulatory and contractual requirements since the 2004 FERC License Amendment and implementation of the RPA. (**In Progress**)<br>*Project Simulation Runs*<br>• The validated model will be utilized to simulate existing operations and evaluating other operational alternatives, including climate change. Through coordination with Study AQ 4 – Fluvial Processes and Geomorphology potential future changes in Lake Pillsbury storage capacity was incorporated into the modeling, as appropriate.<br>• Simulation outcomes will be provided to stakeholders and presented at meetings.<br>*Coordination with Sonoma County Water Agency*<br>• Coordinate with the Sonoma County Water Agency to provide modeling to the FERC proceeding to quantify how Project Existing Operations affect Lake Mendocino storage and Russian River flows downstream of Lake Mendocino. (**In Process**)<br>*Lower Eel River Low Flow Hydrology Analysis*<br>• Develop a method to characterize Project hydrology effects on the lower Eel River at the furthest downstream location. (**In Progress**) | | |



Source:

Two-Basin Partnership, 2020b. Potter Valley Project, FERC Project No. 77 Initial Study Report Attachment 1 Volume 1 Technical Study Summaries. September. Page 3.

**Exhibit B-3:**
**Eel River Watersheds for Unimpaired Hydrology**

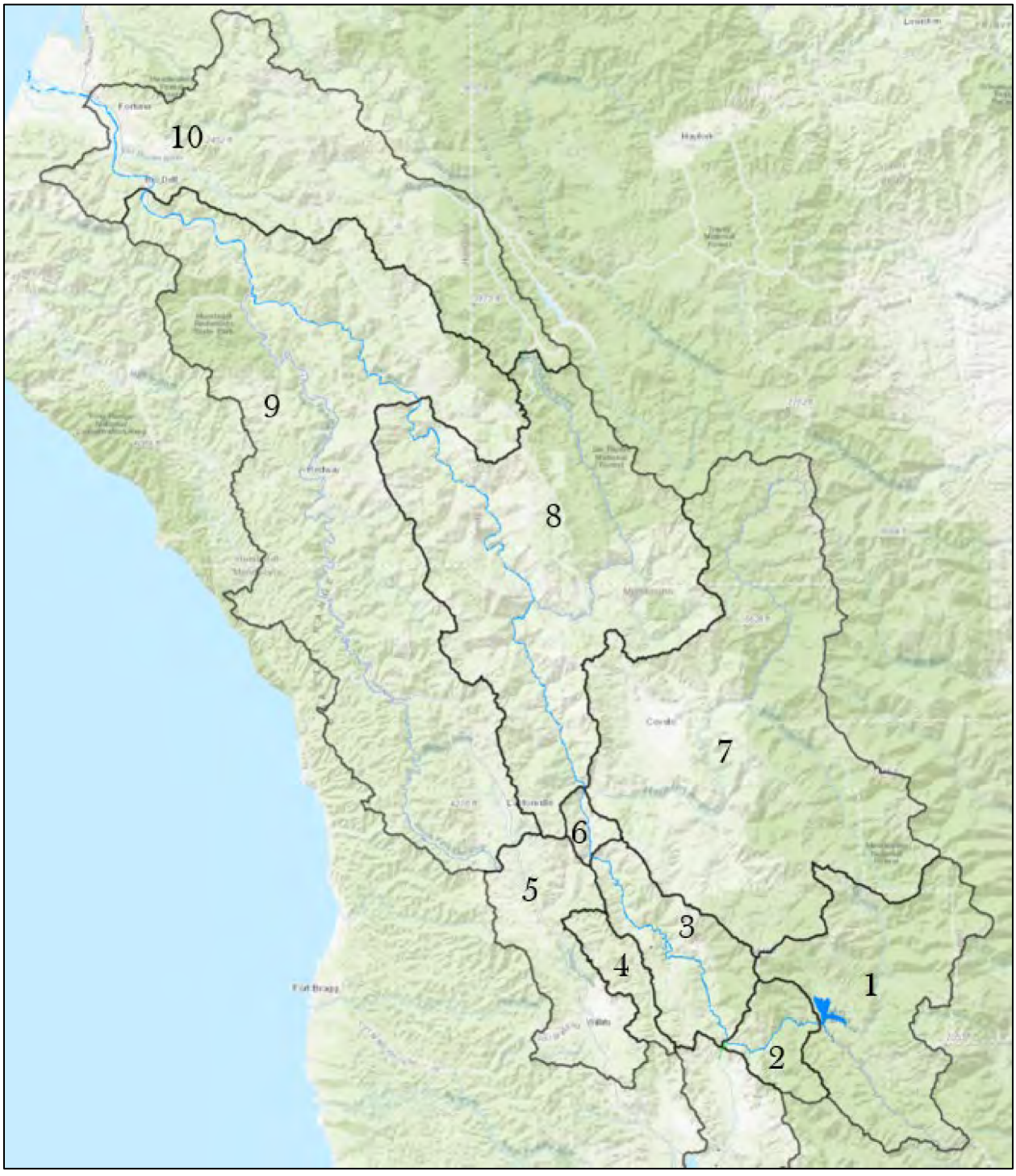



**Figure AQ 1-2. Eel River watersheds for unimpaired hydrology.**

Source:
Two-Basin Partnership, 2020b. Potter Valley Project, FERC Project No. 77 Initial Study Report Attachment 1 Volume 1 Technical Study Summaries. September. Page 8.

**Exhibit B-4**
**Stream gage accuracy analysis at Lake Pillsbury and Van Arsdale**

**Table AQ 1-14. 15-minute gage data used in the stream gage accuracy analysis.**

| Gage | Parameter | Date Range of Available Data |
|---|---|---|
| E1 | Elevation | 10/1/1992 – 9/30/2017 |
| E2 | Flow | 10/1/1992 – 9/30/2017 |
| E3 | Elevation | 10/1/2007 – 9/30/2017 |
| E11 | Flow | 10/1/1992 – 9/30/2017 |
| E16 | Flow | 10/1/1992 – 9/30/2017 |

The analysis utilized a calculated 15-minute inflow to Van Arsdale Reservoir. The 15-minute calculated inflow is used with caution due to travel time from Lake Pillsbury and is only used to determine long term trends in inflow, not instantaneous values. The inflow to Van Arsdale Reservoir is calculated as:

$$\begin{aligned} Inflow \quad & = Release\ to\ Eel\ River\ (E11) \\ & + Release\ to\ Potter\ Valley\ Powerhouse\ (E16) \\ & + change\ in\ storage\ at\ Van\ Arsdale\ (E3) \end{aligned}$$

Source:
Two-Basin Partnership, 2020b. Potter Valley Project, FERC Project No. 77 Initial Study Report Attachment 1 Volume 1 Technical Study Summaries. September. Page 21.

# Exhibit B-5

**Scott Dam Spillway Rating Curve**



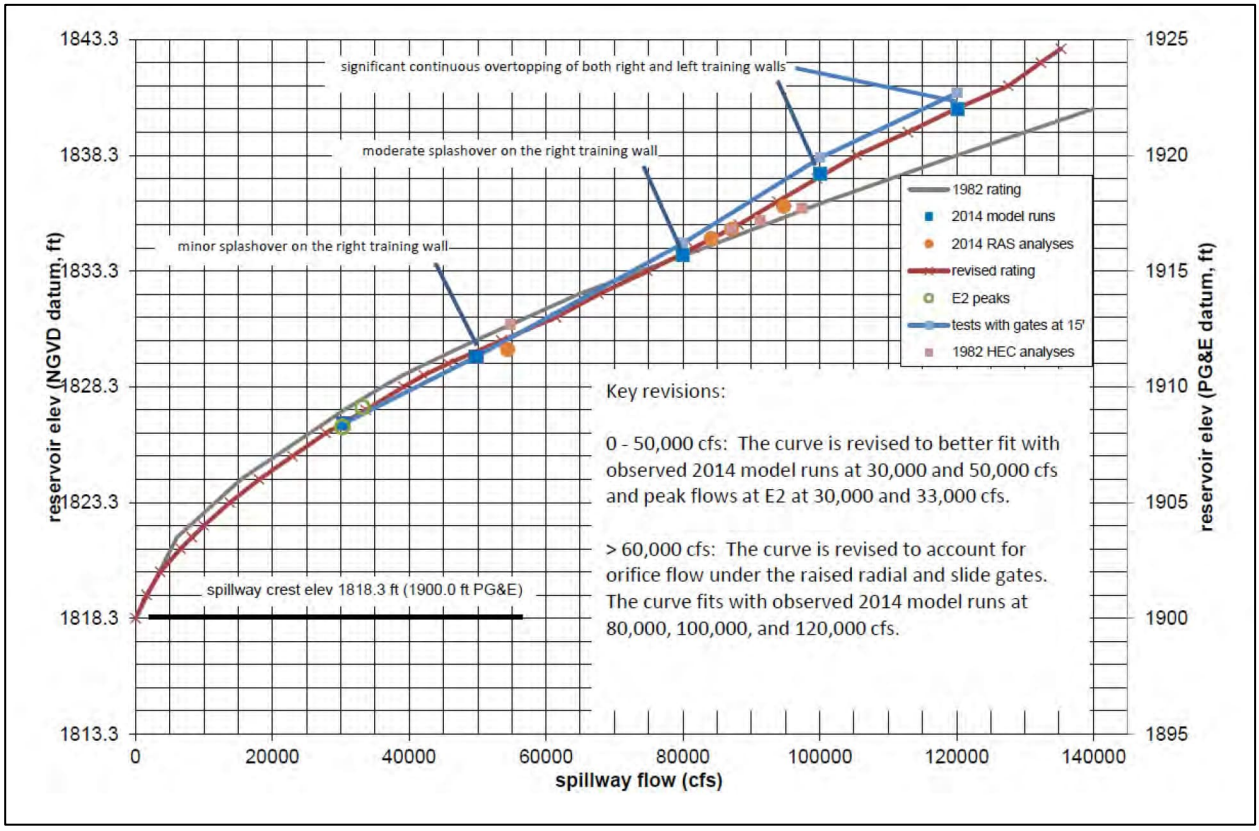



**Figure AQ 1-18. Scott Dam Spillway Rating Curve, 2014 Update.**

Source:
Two-Basin Partnership, 2020b. Potter Valley Project, FERC Project No. 77 Initial Study Report Attachment 1 Volume 1 Technical Study Summaries. September. Page 24.

# Exhibit B-6

**Cape Horn Dam Spillway Rating Curve**

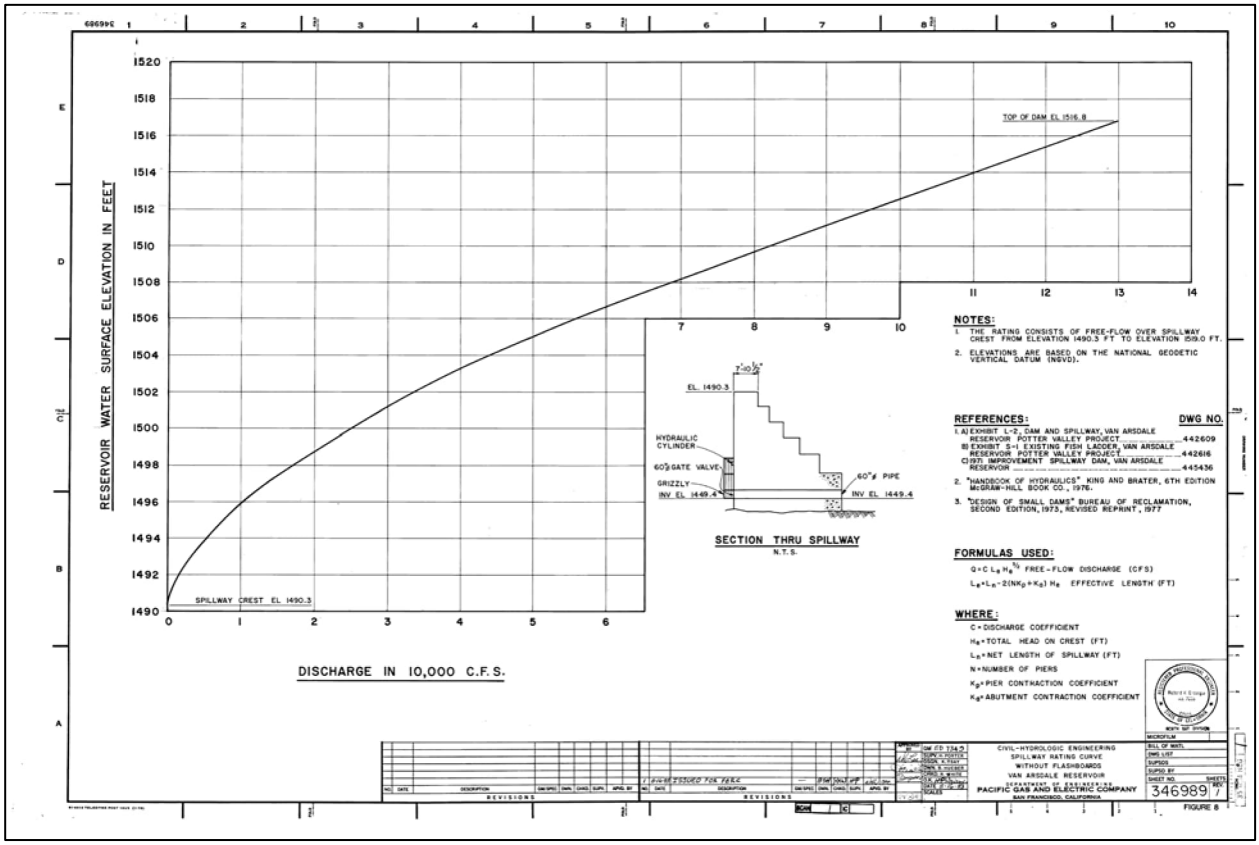



**Figure AQ 1-19. Cape Horn Dam Spillway Rating Curve.**

Source:

Two-Basin Partnership, 2020b. Potter Valley Project, FERC Project No. 77 Initial Study Report Attachment 1 Volume 1 Technical Study Summaries. September. Page 25.

**Exhibit B-7**
USGS stream gages used for flood frequency analyses

Table 2. USGS stream gages used for flood frequency analyses.

| USGS Stream gage ID | Location | Period of record | Drainage area (square miles) |
|---|---|---|---|
| 11470500 | Eel River below Scott Dam | 1923–2019 | 290 |
| 11471500 | Eel River below Cape Horn Dam | 1910–2019 | 349 |
| 11472150 | Eel River above Outlet Creek | 1964–1997 | 528 |
| 11472500 | Eel River above Middle Fork Eel River confluence | 1951–1964 | 705 |

Source:
Stillwater Sciences, 2021e. Hydraulic Model Development and Potential Flood Implications Following the Proposed Scott Dam Removal, Eel River, California. August. Page 7.

**Exhibit B-8**

Hydraulic model study reach and Potter Valley Project vicinity.

Figure 1. Hydraulic model study reach and Potter Valley Project vicinity.

Source:

Stillwater Sciences, 2021e. Hydraulic Model Development and Potential Flood Implications Following the Proposed Scott Dam Removal, Eel River, California. August. Page 3.

**Exhibit B-9**
Existing conditions water surface elevation profile

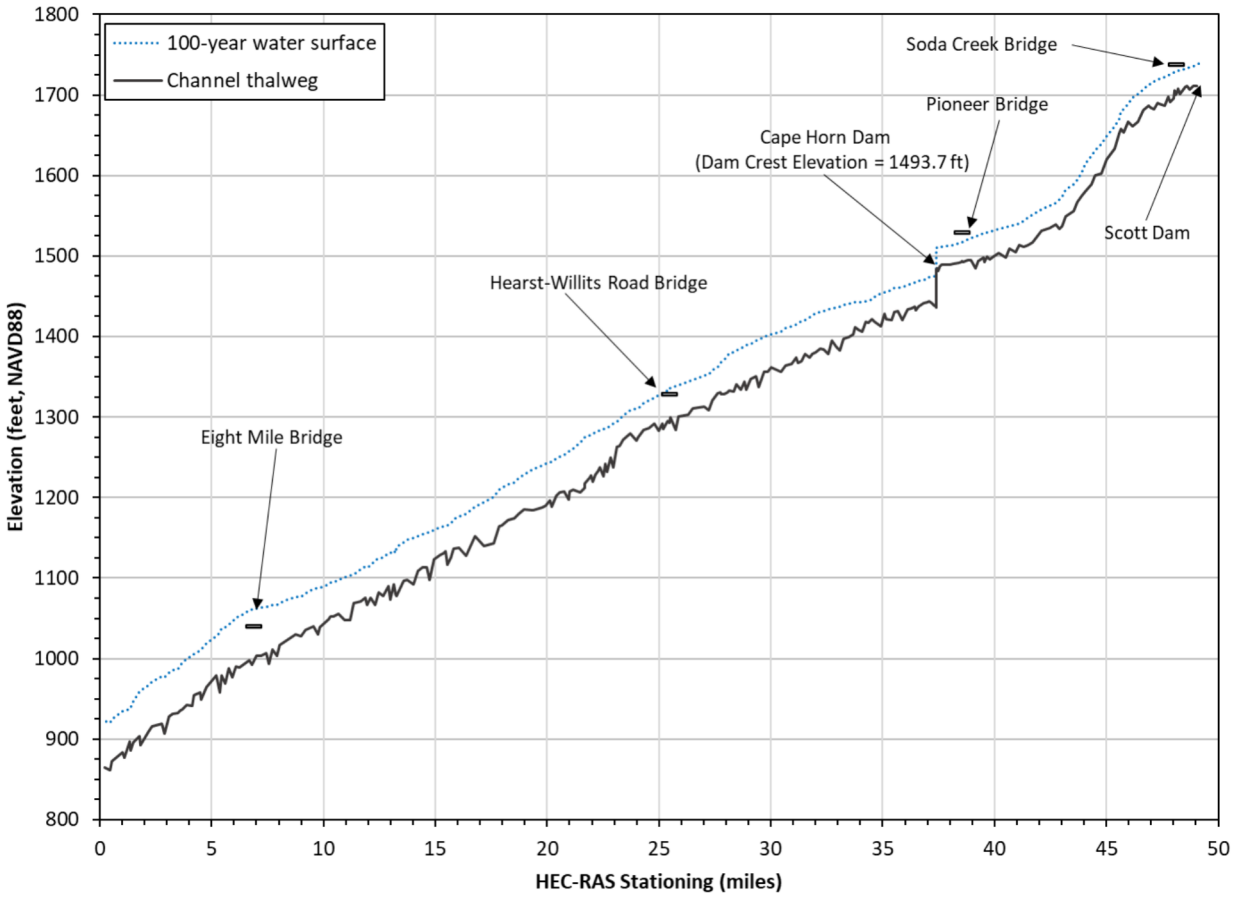
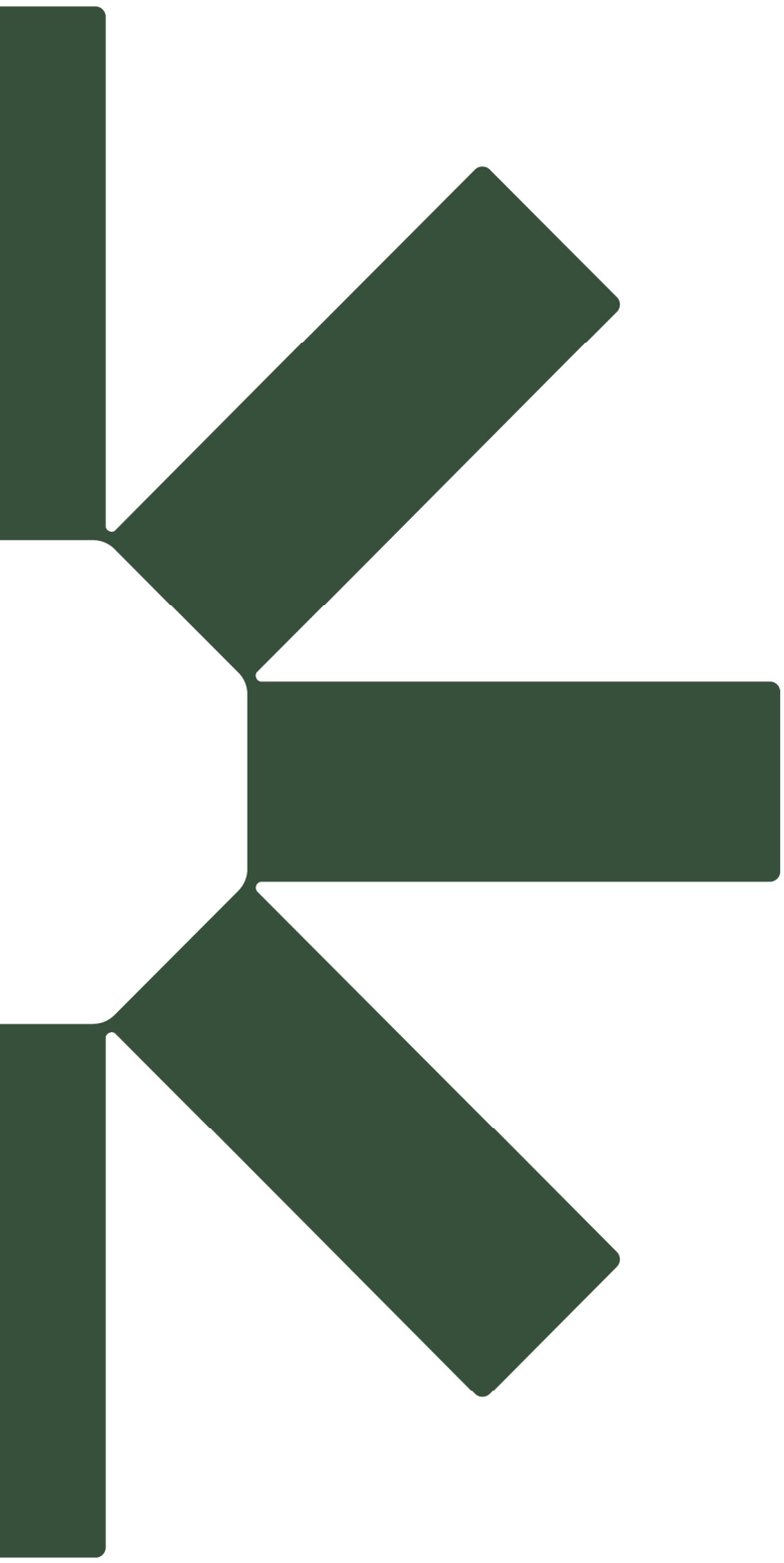



Figure 5. Existing conditions water surface elevation profile. Bold line symbols indicate approximate bridge low chord elevations. HEC-RAS stationing in the Eel River extends from Scott Dam (station 49) downstream to the Middle Fork Eel River confluence (station 0).

Source:
Stillwater Sciences, 2021e. Hydraulic Model Development and Potential Flood Implications Following the Proposed Scott Dam Removal, Eel River, California. August. Page 15.



Making Sustainability Happen

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September 26, 2025

**Via Electronic Submittal (E-File)**

Debbie-Anne Reese, Secretary
Federal Energy Regulatory Commission
888 First Street, N.E.
Washington, DC 20426

**RE: Potter Valley Hydroelectric Project, FERC Project No.77-318**
**Application for Non-Capacity License Amendment**
**Response to Additional Information Request**

Dear Secretary Reese:

On July 28, 2025, the Federal Energy Regulatory Commission (FERC) issued an additional information request in response to Pacific Gas and Electric Company's (PG&E) non-capacity amendment application for the Potter Valley Hydroelectric Project No. 77. PG&E initially filed the amendment request with the FERC on July 31, 2023, and, following an additional information request from FERC on October 4, 2023, supplemented on January 30, 2025.

PG&E's responses to FERC's July 28, 2025, additional information request is included as (Enclosure 1).

If you have any questions concerning these documents, please contact Chadwick McCready, senior license coordinator for PG&E, at (530) 685-5710, or by email at Chadwick.McCready@pge.com.

Sincerely,

Matthew Joseph
Supervisor, Hydro License Compliance

Enclosure:
1. PG&E's Response to FERC's July 28, 2025, Additional Information Request

cc: via email w/enclosure
Joshua Fuller, NMFS – joshua.fuller@noaa.gov
Matt Goldsworthy, NMFS- Matt.Goldsworthy@noaa.gov
Josh Boyce, USFWS – josh_boyce@fws.gov
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# ENCLOSURE 1

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# Potter Valley Hydroelectric Project, FERC No. 77

## Responses to FERC Additional Information Request Regarding PG&E's License Amendment Application

On July 28, 2025, the Federal Energy Regulatory Commission (FERC) issued a Request for Additional Information regarding Pacific Gas and Electric Company's (PG&E) non-capacity license amendment application to revise the requirements pursuant to Article 52 of the Potter Valley Hydroelectric Project (FERC Project No. 77) license. For reference, FERC's comments are copied below, followed by PG&E's responses.

## FERC Additional Information Request #1:

*Under hydrology and water resources, Exhibit E, section 3.3.8.2.3 East Branch Russian River, you indicate that the proposed reduction in minimum flow to the East Branch Russian River will negatively affect water users in the basin, but that contractual obligations to Potter Valley Irrigation District will continue to be met. You expect that from July through September, deliveries to the East Branch Russian River will decrease by an average of 88 percent (compared with the average deliveries from 2014-2023), and an average of 97 percent (compared with the average deliveries from 2004-2013). Further in Exhibit E, section 4.2.2, you state the proposed reduction in flows would result in a cumulative adverse effect to the quantity of water available to East Branch Russian River basin water users.*

*Please identify any users that may be affected by the proposed reduction in flow diversions to the East Branch Russian River and characterize their location and estimated amounts (quantity and percentage) under reduced flows. This may include operators of Commission-licensed hydropower projects that rely on powerhouse releases (e.g., Vecino Vineyards, exemptee for the Hammeken's Power House Canal Project No. 9647 or Eugene McFadden, exemptee for the McFadden Farm Project No. 4658) and agricultural users that are located downstream of Potter Valley Irrigation District's diversions. Also, describe and address any potential effects of the proposed amendment on storage and/or releases from Lake Mendocino through analyses of the last ten years of data displaying the percent contribution of Potter Valley Powerhouse releases made to Lake Mendocino yearly storage, and address any effects to environmental resources below Lake Mendocino (including but not limited to water quantity, water quality, and listed salmonids of the East Branch Russian River) that may result from your proposal.*

**PG&E Response to Additional Information Request #1:**

PG&E does not maintain a database or inventory of water rights holders or water users located downstream of the Potter Valley Powerhouse in its ordinary course of business. Information about water rights on the East Branch Russian River (EBRR) below the Potter Valley powerhouse is available on the California State Water Resource Control Board's (SWRCB) publicly accessible Electronic Water Rights Information Management System (eWRIMS)

(available at https://www.waterboards.ca.gov/waterrights/water_issues/programs/ewrims/). PG&E does not collect or maintain information about water users who may benefit from those water rights (e.g., contractual arrangements), how they divert and use water, or how the SWRCB may implement curtailments.

Table 3-6 of Exhibit E calculates the estimated changed deliveries to the EBRR at the Potter Valley Powerhouse compared to historic conditions. Calculating or estimating the impact of changed deliveries to the EBRR on specific water rights holders or water "users" cannot be calculated without undertaking a detailed study of flow inputs below the powerhouse (e.g. precipitation and tributary flows from Adobe Creek, Burright Creek, Mewhinney Creek, Busch Creek, Williams Creek, Bevans Creek and other sources) and patterns of diversions and usages by all downstream water rights holders and other water users. PG&E does not maintain such information in the ordinary course of its business. Such an analysis for each water right holder or water right user would also require a comprehensive, and unduly burdensome, legal analysis of water rights that reflects priority dates, seasonal patterns of diversions, and other factors.

With respect to the downstream exemptees referenced by Commission staff, any other exemptions issued after issuance of the license for the Potter Valley Project in 1922 were granted subject to the rights of the previously licensed project.[1] Commission issued an exemption for the Hammeken's Power House Canal Project No. 9647 in in 1986.[2] Furthermore, the Commission's eLibrary database indicates Project No. 9647 is currently undergoing implied surrender.[3] The Commission issued an exemption for the McFadden Farm Project No. 4658 in 1981.[4] Thus, the exemptions for both referenced exemptions are subject to the rights of the Potter Valley Project.

With respect to the potential effects of the proposed amendment on storage and/or releases from Lake Mendocino, PG&E does not have a license obligation to develop or maintain this type of information, and PG&E does not maintain such information in the ordinary course of its business. Lake Mendocino and Coyote Dam are located outside the Project boundary and are owned by the U.S. Army Corps of Engineers (the Corps) and operated in coordination with Sonoma Water. PG&E is not in control of how releases to the EBRR below the powerhouse under the amendment impact operations and storage at Lake Mendocino. Lake Mendocino storage and releases are impacted by numerous factors beyond EBRR deliveries, including the Corps' operations, precipitation, input from EBRR tributaries, and diversions and uses by EBRR water rights holders and other water users.

[1] *BES Hydro Co.*, 41 FERC ¶ 61,084 (1987) (finding the exemptee had no right to compensation for loss of head at the exempted project caused by the operation of the previously licensed project), *affirmed on rehearing*, 45 FERC ¶ 61,478 (1988).

[2] Order Granting WH Hammeken Exemption for Hammeken Power House Canal Project (Aug. 27, 1986), Accession No. 19860827-0280.

[3] Notice of Proposed Termination of Exemption by Implied Surrender and Soliciting Comments, Motions to Intervene, and Protests (May 15, 2024), Accession Number 20240515-3041.

[4] Order Granting Exemption From Licensing of a Small Hydroelectric Project of 5 Megawatts or Less, 17 FERC 62,383 (Dec. 9, 1981), Accession Number 19811216-0066.

## FERC Additional Information Request #2:

*In Exhibit E, you identified anticipated adverse effects to some environmental resources in the East Branch Russian River basin (e.g., riparian and wetland vegetation, narrowing of the river channel, and potential reductions in rainbow trout habitat), but did not adequately address all environmental resource areas. For water quality in the East Branch Russian River, you suggest that powerhouse outflow temperatures will be similar to Eel River temperatures immediately upstream of Van Arsdale Reservoir. You do not include information related to expected water quality effects on the East Branch Russian River downstream resulting from your proposal.*

*Provide either a quantitative or qualitative analysis of expected effects to water quality (e.g., water temperatures and dissolved oxygen concentration) in the East Branch Russian River in the reach above Lake Mendocino resulting from the proposed amendment. If no effect is expected, provide your rationale.*

### PG&E Response to Additional Information Request #2:

PG&E is in the process of developing a qualitative analysis of effects to water quality in the East Branch Russian River above Lake Mendocino. PG&E will provide an update to FERC by March 31, 2026.

## FERC Additional Information Request #3:

*In the consultation record included in Appendix E-2, summaries from June 13, 2024, indicate consensus from the consulting parties that trout stocking in the East Branch Russian River can continue under the proposed 5 cubic feet per second release and that no effect on recreational trout fishing is anticipated. Notes from the meeting suggest that data exists in support of this conclusion.*

*Provide a summary of any existing information that supports the conclusion that the recreational fishery in the East Branch Russian River would not be affected by the proposed amendment and citations or direct sources to support this conclusion.*

## PG&E Response to Additional Information Request 3:

On September 24, 2025, the California Department of Fish and Wildlife provided stocking records for the EBRR above Lake Mendocino and reiterated that trout stocking in the East Branch Russian River (EBRR) can continue with under the amended flows. That statement appears consistent with information received from CDFW while PG&E was preparing its License Surrender Application for the Potter Valley Project regarding rainbow trout stocking in the EBRR. Table 1 below summarizes CDFW's annual rainbow trout stocking in the EBRR from 2001 through 2025:

**Table 1: CDFW Annual Stocking Rates of Catchable Rainbow Trout in the Upper East Branch Russian River**

| Year | Number of Fish | Pounds |
|---|---|---|
| 2001 | 7,400 | 4,300 |
| 2002 | 15,075 | 8,250 |
| 2003 | 38,800 | 8,165 |
| 2004 | 33,185 | 4,000 |
| 2005 | 39,650 | 7,985 |
| 2006 | 16,490 | 8,800 |
| 2007 | 7,930 | 4,950 |
| 2008 | 10,620 | 6,700 |
| 2009 | 11,285 | 5,990 |
| 2010 | 12,125 | 6,500 |
| 2011 | 10,630 | 6,000 |
| 2012 | 25,000 | 12,000 |
| 2013 | 23,230 | 11,900 |
| 2014 | 21,300 | 12,000 |
| 2015 | 9,165 | 5,000 |
| 2016 | 8,573 | 5,000 |
| 2017 | 8,540 | 4,600 |
| 2018 | 11,252 | 3,825 |
| 2019 | 10,700 | 5,650 |
| 2020 | 8,505 | 4,110 |
| 2021* | 2,460 | 1,000 |
| 2022 | 6,890 | 3,500 |
| 2023 | 14,502 | 6,050 |
| 2024 | 13,460 | 5,300 |
| 2025 | 11,268 | 4,400 |

*An unprecedented drought occurred in 2021 that impacted CDFW's hatchery programs resulting in reduced stocking of fish statewide.

PG&E is unaware of other information supporting CDFW's conclusion that stocking will continue and recreational trout fishing will not be impacted by the proposed amended flows. PG&E expects that CDFW will comment once FERC notices PG&E's application for protests, interventions, and comments. Please note that, except for a discrete section immediately downstream from the Potter Valley Powerhouse, the EBRR is outside the FERC project boundary and outside the geographic scope of PG&E's recreation mitigation footprint.

# FERC Additional Information Request #4:

*In our request for additional information, dated October 4, 2023, Commission staff indicated that formal consultation under the Endangered Species Act (ESA) is likely necessary with the National Marine Fisheries Service (NMFS) on the proposed amendment. Commission staff requested that you include in your response to our October 4 letter, the submittal of a Biological Assessment (BA) providing an account of any potential effects to federally listed species under NMFS jurisdiction. In response to this request, you state that the proposed Project amendment may reduce available habitat in the Eel River below Scott Dam during the summer but that it would support improved habitat conditions for juvenile steelhead by providing cooler water refugia in the late summer. You state that you received technical assistance from NMFS regarding this matter and that NMFS supports the assertion that this amendment is consistent with the 2002 Biological Opinion analysis and aligns with the goals and objectives outlined in their Marchand October 2022 letters to theCommission.[6]*

*Your January 30, 2025 filing, did not include a BA as requested by Commission staff, but did identify general effects related to the proposed amendment in section 3.3.4.2 on listed species (namely steelhead and Chinook salmon). You state that approving the amendment would provide for the preservation of cold-water storage in Lake Pillsbury, a decrease in Eel River water temperature between the project dams, and a reduction in the risk of bank erosion and sloughing at low reservoir storage levels (at Scott Dam) which would minimize the clogging of the outlet works and any dewatering of Eel River reaches resulting from such clogging.*

*Also, in our request for additional information dated October 4, 2023, Commission staff requested that you address federally listed terrestrial and aquatic species under the jurisdiction of the U.S. Fish and Wildlife Service (FWS). In Volume 1, page 2 of your January 30, 2025 filing, you indicate that there are no federally listed species under FWS jurisdiction.*

*a. Based on FWS's Information for Planning and Consultation (IPaC) website, Commission staff identified several species under FWS jurisdiction that may be present in the project area.[7]Please address any effects of the proposed amendment on these aquatic and terrestrial species.*

*b. Finally, please request to be designated as a non-federal representative for the purposes of conducting ESA consultation with NMFS and FWS and provide to us a BA that quantitatively supports the above conclusions for both NMFS and FWS species. In addition, address any potential effects to listed species in the East Branch Russian River that may also require consultation under the ESA (e.g., the information requested in Nos. 1 and 2above). In our efforts to formally consult, Commission staff must be able to identify any related effects, both adverse and beneficial, to listed species in both the Eel River and the East Branch Russian River that may result from your proposal.*

## PG&E Response to Additional Information Request 4:

PG&E requests that pursuant to 50 CFR section 402.08, FERC, as the lead federal agency for undertakings related to this Project, designate PG&E as its non-federal representative for informal consultation with the USFWS and NMFS under Section 7 of the Endangered Species Act.

## FERC Additional Information Request #5:

On April 28, 2023, the Commission's Division of Dam Safety and Inspections- San Francisco Regional Engineer's letter discussed your seismic stability analyses and encouraged you to further refine the estimated seismic risk potential of Scott Dam. Please provide any updates to risk reduction measures currently implemented or proposed since this letter was issued, if available.

## PG&E Response to Additional Information Request #5:

In May 2022, PG&E retained an engineering consultant to reevaluate the dam's condition and expected performance under normal, flood, and seismic loading conditions. As an early step in the reevaluation process, PG&E performed a simplified, two-dimensional stability analysis of the dam. The analysis provided an initial understanding of the expected performance of the dam under recently updated seismic and flood loading conditions and a preview of possible conclusions from the more sophisticated analyses planned for later phases of the reevaluation.

PG&E submitted the results of the simplified analysis to FERC in a letter dated March 17, 2023. As stated in the letter based on analysis results and identified deficiencies of the dam, PG&E placed a 10-foot restriction on the maximum reservoir operating level as an interim risk-reduction measure. The restriction will remain in place until long-term measures are developed and implemented.

FERC issued two responses to PG&E's letter:

- From the Division of Hydropower Administration and Compliance (DHAC), in a letter dated March 28, 2023, FERC acknowledged the environmental and compliance impacts of the reservoir restriction and stated that PG&E must file an amendment application to receive FERC authorization for the restriction.
- From the Regional Office, in a letter dated April 28, 2023, FERC provided technical comments on the simplified stability analysis.

The subsequent communication between PG&E and DHAC is not discussed in this response. The subsequent communication between PG&E and the Regional Office related to the technical content of the reevaluation program is provided below.

- PG&E submitted the primary results of varied analyses performed as part of the reevaluation program to FERC in letters dated June 12, 2023, and August 21, November 27, December 2, and December 13, 2024.
- In a letter dated February 28, 2025, PG&E submitted a summary of findings and associated risk-reduction measures from all the analyses conducted as part of the multiyear engineering reevaluation program.

A key seismic risk-reduction measure at Scott Dam is to maintain the reservoir restriction established in the March 17, 2023, letter, referenced above. However, PG&E has also implemented additional operational and monitoring risk-reduction measures for both normal and flood flow conditions based on operations and maintenance and/or storm forecasting criteria. The complete list of interim risk-reduction measures considered are detailed in Enclosure 1 of PG&E's February 28, 2025, letter to FERC.

PG&E believes the interim risk-reduction measures described are sufficient to address the potential risks and vulnerabilities associated with continued operation of Scott Dam. PG&E is no longer considering additional analysis or major remedial projects to reduce long-term risks.

*End of Additional Information Request Responses*

Document Content(s)

PGE20250926_77_PVLicense_Amendment_AIRResp.pdf .....1

# Lake Pillsbury Fire Protection District

29501 Fuller Drive, Unit B, Potter Valley, CA. 95469

Larry Thompson
Interim Fire Chief

Tony Gigliotti
Senior Licensing Project Manager
Power Generation
P.O. Box 28209
Oakland, CA 94604

March 1st, 2025

Subject: Fire Suppression Response to DRAFT Application to Surrender

Mr. Gigliotti,

In reviewing over the Fire Prevention & Fire Suppression sections of this Draft to Surrender document, I agree with the most of the Prevention and Fire mitigations stated within the descriptive....Including adhering to Public Resource Codes 4291, 4292 & 4293.

My only suggestion here would be to recommend that PG&E have a Fire Crew, in their Type 6 Engine, on-site during the Construction projects.....thus proving any needed immediate Initial Attack on any potential Fires and be able to provide immediate Basic Life Support -BLS Medical-aid, during any Medical Emergencies.

This PG&E Fire Crew should have common Radio & Cellular communications with both the Lake Pillsbury Fire Protection District and the Primary Emergency Command Center – Howard Forest ECC.

PG&E would need to provide either a Mobile Cellular Tower or preferrably a Star Link System, for immediate Cellular Phone use.

I do request that once a Fire Suppression & Medical Plan is drafted, that Lake Pillsbury Fire Protection District's Fire Officers be able to review it.....prior to a Final.

And, I'm sure that our Mutual-Aid Partners would like to have a Final Plan as well. USFS, Cal Fire, USFS LE, LCSO, Northshore Fire and Potter Valley Fire.

Reading this narrative is not reality:

Therefore, while Lake Pillsbury water used for fire suppression would be lost, it would be substituted with other sources of water such as Clear Lake or Lake Mendocino, located about 20 miles from Lake Pillsbury. Large fires located closer to Lake Pillsbury may involve longer response times due to sourcing water from other locations (such as the river or other, more distant water sources such as Lake Winawa [a much smaller lake] and Lake Sonoma [about 75 miles from Lake Pillsbury]).

Yes, the Water Supply for Fire Suppression would be greatly lost ! Substituting a Water Source, with extended Flight Times, (to and from another Lake) with an Air Tanker's or Helitankers "turn-around" time now being 28-32 minutes away.....is very detrimental to the "effectiveness" of Aerial Water drops.

During both the RANCH Fire - 2018 and AUGUST Complex - 2020, the Super Scoopers, Fire Hawk & Chinook Helicopters were "vital" to the Fire Suppression efforts, thus saving Hundreds of Homes.

During the RANCH Fire, we lost 11 Homes in the Rice Fork area.....before 4 Blackhawk Helicopters and 1 Chinook Helicopter showed up and provided immediate turn-around times for the most effective Fire Suppression, thus saving the additional remaining Homes in the area.

The Lake Pillsbury basin and surrounding forested areas are rated as "Very High" on Cal Fire's Fire Hazard Severity Zone - FHSZ Map.

Not having Lake Pillsbury as a immediate Aerial Fire Tanker water source, greatly endangers the 5 local Communities, (350-400 Homes ) all within our Sphere of Influence.

1. LP Ranch 148 Homes
2. USFS Homesites 71 Homes
3. Westshore 87 Sites
4. Rice Fork 28 Homes
5. Dashiell's 38 Homes

Another large scale "Mega" Fire will be devastating !

This statement is confusing....not true & true content:

The Proposed Action may have minimal effects to fire suppression on larger fires, as other nearby sources of water are available and are currently being used (e.g., Clear Lake and Lake Mendocino). However, the Proposed Action may result in unavoidable adverse impacts to local fire suppression to properties near Lake Pillsbury due to the loss of Lake Pillsbury as a water source. Longer fire response times may occur when sourcing water from other locations (such as the Eel River or other, more distant water sources).

The Proposed Action will have Major (not minimal) effects to Fire Suppression on Larger Fires.....as I just explained. And, yes....it will also have adverse impacts to our Local Fire Responses, as well.

As for the delay(s) in accessing other distant Water Sources.....yes again this will be a significant delay and be detrimental to any/all our Firefighting effectiveness....especially in a sustained Structure Fire and/or Extended Attack Wildland Fire.

Another significant delay is our Mutual – Aid partners responding into our Area to support our Firefighting efforts.

These Agencies include: Northshore Fire Protection District, Potter Valley Fire Protection District, USFS (evenings & winter) & Cal Fire.

All these Agencies currently have at least a 1hr–1½ hr. response time into the Lake Pillsbury Basin areas....over very "rough" road(s).

Again, a very significant delay of any additional Resources/Water and Manpower to assist our soon to be....absent sustainable Water Supply.

Road Improvements, drainage, maintaince and an "all-weather" road surface....needs to be part of the Fire Suppression Mitigation Plan. I will provide soon..... forth coming.

It'll be imperative to have true "two-way" Roads, with an all-weather surface.....providing better access(s) for our Mutual – Aid partners year around

Respectfully,

*Larry Thompson*

Larry Thompson – Interim Fire Chief
Lake Pillsbury Fire Protection District
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**Douglas G. Gearhart**
**Air Pollution Control Officer**
dougg@lcaqmd.net

Wilhelmina Chon
State Water Resources Control Board
Division of Water Resources- Water Quality
Certification Program
P.O. Box 2000
Sacramento, CA 95812-2000

October 30, 2025

Subject: Potter Valley NOP Comments

Dear Wilhelmina Chon:

The Lake County Air Quality Management District has received the Notice of Preparation and Scoping Notice for an Environmental Impact Report for the Potter Valley Hydroelectric Project. After reviewing the notice, we have the following comments:

1. The Scott Dam and all structures related to the project may contain asbestos and must be thoroughly surveyed prior to proceeding with any demolition or preparation activities. Asbestos in building materials is a significant threat to the community, workers, and the public. Recycling of concrete requires thorough testing to ensure no asbestos is within the concrete or materials to be recycled.

2. The project location is in close proximity to mapped serpentine areas. Serpentine is known to contain naturally occurring asbestos. Due to the age of the dam, it is unknown whether serpentine was found in or within the project boundaries of the Scott Dam and facilities. Should serpentine be found within the project boundaries additional precautions are required. A geological review of the project area is recommended due to the potential of serpentine or asbestos containing rock to be in the project area, condition of the serpentine, and potential for downstream or air quality impacts of any naturally occurring asbestos.

3. Construction emissions are of significant concern and should be evaluated due to the size of this project. Use of CALEEMOD is recommended to determine construction emissions and impacts as well as for determine ongoing impacts.

4. The sediment behind the dam and throughout the current lake bed requires a long term management plan. The dust bowl that could result if improperly management could be significant. Remediation and site management should be required for this project to ensure proper mitigation of dust generation. Access controls to prevent off road vehicle activities and other ground disturbing activities will be required.

5. The sediment within the lake bed has the potential to be highly concentrated with

toxics and heavy metals from upstream erosion. Heavy metals settle out of the water column early while other fine particulates continue to wash further downstream. Thorough testing of the sediment from surface to full depth should be required to determine the potential for toxics and heavy metals becoming entrained in dust generating activities or from being washed down stream during dam removal and annual winter runoff.

6. The impact on ground water recharge and holding capacity may be significant. Evaluation of the impacts to groundwater wells and recharge should be required and a long term monitoring program implemented. Aquifer management is essential for home water supplies, fire suppression wells, prescribed fire activities, household needs including dust controls, and many other essential uses. A management plan and contingency measures are recommended to be developed and implemented.

7. The impact of the lake removal could severely impact fire fighting efforts in the National Forest and residential areas. The flight time to another water body could be significant, resulting in increased fire growth, smoke impacts throughout the region and beyond, and the potential for loss of life due to lack of adequate water storage in close proximity to these rural residential areas.

As the local Air Quality Management District responsible for protecting the air resources for the community and Air Basin, we believe these issues must be evaluated and mitigated or prevented to ensure that clean air is maintained in all areas potentially impacted by this proposal.

Sincerely,

Douglas Gearhart, APCO

CC: LCAQMD Board of Directors

MENDOCINO COUNTY INLAND WATER & POWER COMMISSION
P.O. Box 1247, Ukiah CA 95482 ● IWPC@mendoiwpc.com ● www.mendoiwpc.com

September 16, 2025

Dear Mendocino County Supervisors:

The Mendocino County Inland Water and Power Commission (IWPC**)** is a Joint Powers Authority whose member agencies are the Mendocino County Water Agency, the City of Ukiah, the Redwood Valley County Water District, the Potter Valley Irrigation District, and the Mendocino County Russian River Flood Control and Water Conservation Improvement District. We are privileged to have Supervisor Madeline Cline as our Board member and Supervisor Maureen Mulheren as our alternate from the County. IWPC has been working for decades to preserve and improve water supply availability for the landowners and residents within the Russian River basin portion of Mendocino County. Over the past several years our work has become even more important due to the decision of Pacific Gas and Electric (PG&E) to file a license surrender application for the Potter Valley Project (PVP).

Unfortunately, many in our community do not understand PG&E's decision, or its implications, and this is generating confusion and misinformation. IWPC writes to request that the County Board of Supervisors schedule a workshop at which we can work together to educate the public on this very important topic.

Since 1922, our community has benefitted from the free water that PG&E has diverted to the Russian River, from its electric power generation plant using Eel River water. The existence of the PVP's Scott Dam, which creates Lake Pillsbury, has meant that PG&E can divert water from the Eel River into the Russian River year-round, thus providing water at times of the year when water is naturally unavailable. Our entire community--neighborhoods, businesses, and farms--relies on that water.

But as the Federal and State Endangered Species Acts have limited PG&E's diversions, and as the economics of hydrogeneration have changed and new regional and state-wide power supplies have come online, PG&E has found that its operation of the PVP is no longer economically viable. In response, PG&E offered to transfer its license to operate the project, issued by the Federal Energy Regulatory Commission (FERC), to another entity. Any new operator would have to absorb annual losses of more than $6 million, provide hundreds of millions of dollars' worth of deferred work to upgrade the

project and perform the additional work that would be required to comply with the State and Federal Endangered Species Acts. Not surprisingly, no one stepped forward to take over the project.

Because no one stepped forward, PG&E filed a license surrender application. Under a license surrender application, FERC evaluates the environmental and certain other impacts of the surrender and approves the surrender with specific conditions often relating to restoration. FERC cannot mandate that PG&E continue to operate the project. The only viable way to ensure water continues to come from the Eel to the Russian is to either apply for a new water right, or to somehow preserve a portion of the existing supply. A new water right would be very junior and unreliable under California's water rights system. Therefore, IWPC and its members prioritized preserving a portion of the diversion.

In early outreach to PG&E to try to acquire the water right, we learned that PG&E would not voluntarily participate in any transfer of the water right without a two-basin proposal. PG&E serves customers in both basins and did not believe it could choose between those competing interests. We heard this message all the way to the top of PG&E's management, and it was this message that drove us to our current solution.

Building on Congressman Huffman's Two Basin Partnership, IWPC, along with Round Valley Indian Tribes, California Department of Fish and Wildlife, Humboldt County, Sonoma Water Agency, California Trout, and Trout Unlimited came together on a proposal that linked the two basins: The Eel River interests agreed to support continued diversions to the Russian River (something many of them had taken positions firmly against) in exchange for the Russian River interests not opposing FERC's consideration of PG&E's surrender application by supporting the application. Each side compromised to get certainty on the most important element of what it was seeking. For the Russian River interests, IWPC and its members, the need to ensure continued diversions into the future was the most essential requirement for our water supply security.

Our decision to focus on this most essential element was rooted in our assessment of the risk that PG&E's water rights would go to Eel River interests if we did not reach agreement. This is because the Round Valley Indian Tribes would likely have water rights more senior than the PVP's water rights. Round Valley Indian Tribes has a claim for not yet adjudicated water rights on the Eel River. Under the *Winters* Doctrine, coming out of a famous U.S. Supreme Court case, Native American Tribes can assert a reserved water right associated with the purpose for which Congress "reserved" land for use by the Tribe, unless the reservation showed a different intent. Because the reservation date for the Round Valley Indian Tribe is very senior, the Tribes could elect to adjudicate that water right in court and likely become the senior right on the system. Such a court action could take years and tie us up in uncertainty and costs that entire time, while simultaneously destroying our partnership. The Tribes would also likely seek

to acquire PG&E's water rights under the Tribal Lands Policy of the Public Utility Commission, a policy that controls how PG&E will dispose of the property of the PVP. That policy requires utilities to first offer all property to Tribes. While the Policy has not yet been tested on water rights, those rights are a form of real property under California law. The *Winters* Doctrine and the Tribal Lands Policy creates risk for IWPC's members, a risk that we believe our community cannot afford. Importantly, because of the partnership that we have built, the Round Valley Indian Tribe has recently come out in support of raising Coyote Dam at Lake Mendocino, an essential element for our water future that reduces the risk of limited storage.

We understand that Mendo Matters has asked the County to engage on this issue. If Mendocino County were to send a letter to FERC opposing PG&E's surrender application, it would jeopardize all of the progress we have made to preserve a water supply for the Russian River basin from the Eel River. We understand the frustration in our community and the disbelief that in this time of shrinking water supplies Scott Dam could be removed. But our interests are best served by preserving the water supply and investing our energy and resources into developing new storage in the Russian River basin that will not be subject to the uncertainties that Scott Dam faces. We turn to you, the elected leaders of the County, to help us bring these issues to the public.

Now that Mendo Matters has asked you to engage on the issue, we think that a County sponsored workshop can provide desperately needed perspectives so that people can make their own assessment. We know we have a lot of information to bring to the public, and we suspect that Mendo Matters does as well.

Twenty nine years ago the County of Mendocino had the foresight and wisdom to help form IWPC to take on this mission. We ask that you now provide that foresight and wisdom for the next generation to help us create vital community dialogue. We are available to participate, and we thank you for your leadership.

Respectfully,

Janet K.F. Pauli

Janet K.F. Pauli, Chair
Mendocino County Inland Water and Power Commission

CERTIFIED TO BE A TRUE
AND CORRECT COPY OF
Document Recorded 6-28-2022
Series # 2022-008987 Lake County
PLACER TITLE COMPANY
BY

RECORDING REQUESTED BY

**PACIFIC GAS AND ELECTRIC COMPANY**
**Land Department**
**Attention: Paul Coviello**
**1850 Gateway Blvd, Room 7043C**
**Concord, CA 94520**

CERTIFIED TO BE A TRUE
AND CORRECT COPY OF
Document Recorded 6-28-2022
Series # 2022-07882 Mendocino County
PLACER TITLE COMPANY
BY

WHEN RECORDED MAIL TO

**PACIFIC GAS AND ELECTRIC COMPANY**
**245 Market Street, N10A, Room 1015**
**P.O. Box 770000**
**San Francisco, California 94177**

| The undersigned Grantor declares that the documentary transfer tax is $-0- (R&T Code 11911 court-ordered conveyance or decree that is not pursuant to sale) | (Space Above this Line for Recorder's Use) |
|---|---|

LD # 2417-11-10003

A.P.N. Lake County – 001-030-009 (Portion); 001-031-002 (Portion); 001-031-024 (Portion); 001-033-003 (Portion); 001-033-004 (Portion); 001-033-005 (Portion); 001-033-006; 001-034-002 (Portion); 001-034-007 (Portion); 001-035-003 (Portion); 001-035-004 (Portion); 001-035-035; 001-040-001

A.P.N. Mendocino County – 171-170-02; 171-180-04; 171-200-003; 171-200-11 (Portion); 171-210-03 (Portion); 171-210-10 (Portion); 171-230-20; 171-240-08; 171-260-09; 172-290-02; 172-300-02 (Portion); 172-300-04; 173-030-02 (Portion); 173-030-03 (Portion); 173-130-23

Date: June 24, 2022

# DEED OF CONSERVATION EASEMENT AND AGREEMENT
# (EEL RIVER PLANNING UNIT)

Between

PACIFIC GAS AND ELECTRIC COMPANY, a California corporation,
as Grantor

and

MENDOCINO LAND TRUST INCORPORATED, a California nonprofit corporation,
as Grantee

**Note to the County Recorder**: This is a conservation easement within the meaning given to such term in California Government Code §27255 and is to be included in the index developed and maintained pursuant to such section.

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**DEED OF CONSERVATION EASEMENT AND AGREEMENT**
**(EEL RIVER PLANNING UNIT)**

THIS DEED OF CONSERVATION EASEMENT AND AGREEMENT (this "**Easement**") is made and entered into this 24th day of June, 2022 (the "**Effective Date**") by and between PACIFIC GAS AND ELECTRIC COMPANY, a California corporation ("**Grantor**"), and MENDOCINO LAND TRUST INCORPORATED, a California nonprofit corporation ("**Grantee**"), with reference to the following facts:

## RECITALS

A. The Property. Grantor is the owner of approximately 5,660 acres of real property located in Lake and Mendocino Counties, State of California, as more particularly described in the attached Exhibit A (the "**Property**").

B. FPA and FERC Jurisdiction. The Property lies within the boundaries of one or more hydroelectric projects licensed to Grantor pursuant to Part I of the Federal Power Act, 16 U.S.C. §§792-823d ("**FPA**").

1. FPA and FERC Requirements. The FPA requires regulation of the construction, operation, and maintenance of non-federal hydroelectric power projects pursuant to licenses issued by the Federal Power Commission, or its successor, the Federal Energy Regulatory Commission ("**FERC**"). Each such license requires the licensee to undertake appropriate measures on behalf of both developmental and environmental public interest uses of a waterway, including as relevant fish and wildlife protection and enhancement, irrigation, flood control, water supply, and recreation, together with whatever other beneficial public uses the license identifies as a "Project Purpose." The license requires the licensee to acquire and retain all interests in non-federal lands and other property necessary or appropriate to carry out the Project Purposes.

2. FPA and FERC Non-Project Uses. The FPA provides FERC with authority to regulate the use of a licensed project's lands and waters not only by the licensee but also by any other entity. FERC refers to such third-party use as "non-project use of project lands and waters." Even where the third-party use may be compatible with and even promote a specified Project Purpose, such use is "non-project," because it is not in the license as a direct obligation of the licensee. As a FERC licensee for the Property which is the subject of this Easement, Grantor must (except for very minor matters) apply to FERC for approval to convey to a third party any easement over project lands. FERC approval requires conveyance instruments to contain recorded covenants providing that that the non-project use will not interfere with Project Purposes, and requires its licensees to enforce such covenants and protect the project values.

3. Removal of FERC Jurisdiction. FERC jurisdiction and authority over a licensed hydropower project is removed if and when (1) the project is

decommissioned and the project license is surrendered or otherwise terminated; or (2) FERC determines that the project does not require a license to continue to operate, and the license expires or is otherwise terminated. Neither FERC nor the hydropower project license can bestow, remove, or alter water or other property rights; therefore, the end of FERC jurisdiction over the project has no effect on existing property rights in project lands and waters, including any conservation easements on such lands.

C. Grantor Party to Settlement Agreement. Grantor is a party to that certain Settlement Agreement (the "**Settlement Agreement**") as modified and approved by the Public Utilities Commission of the State of California (the "**Commission**") in its Opinion and Order of December 18, 2003 (Decision 03-12-035).

D. Grantor Party to Stipulation. In furtherance of the Settlement Agreement, and to provide additional detail regarding the implementation of the "Land Conservation Commitment" (defined below), the parties to the Settlement Agreement and other interested parties entered into that certain Stipulation Resolving Issues Regarding the Land Conservation Commitment dated September 25, 2003 (the "**Stipulation**").

E. Governing Documents and Beneficial Public Values. The Settlement Agreement and the Stipulation (collectively, the "**Governing Documents**") require Grantor to ensure that approximately 140,000 acres of watershed lands, all owned by Grantor (collectively, the "**Watershed Lands**"), including the Property, are conserved for a broad range of beneficial public values, including the protection of the natural habitat of fish, wildlife and plants; the preservation of open space; outdoor recreation by the general public; sustainable forestry; agricultural uses; and historic values. The obligations of Grantor to convey conservation easements and protect such beneficial public values on the Watershed Lands, as well as certain other obligations related thereto, are set forth in detail in Appendix E of the Settlement Agreement (as further explicated in Section 12 of the Stipulation), and are defined therein as the "**Land Conservation Commitment.**"

F. Stewardship Council and Land Conservation Plan. Pursuant to the Governing Documents, the Pacific Forest and Watershed Lands Stewardship Council, a California non-profit public benefit corporation (the "**Stewardship Council**"), was created to oversee and carry out the Land Conservation Commitment. Pursuant to the Governing Documents, the Stewardship Council developed a plan for protection of the Watershed Lands for the benefit of the citizens of California (the "**Land Conservation Plan**" or "**LCP**"). The LCP includes, among other things, objectives to preserve and/or enhance the beneficial public values identified on each parcel of Watershed Lands.

G. California Civil Code §815. The Legislature of the State of California, as set forth in California Civil Code §815 *et seq.*, has found and declared it to be the public policy and in the public interest of this state to encourage the preservation of land in its predominantly natural, scenic, agricultural, historical, forested, or open-space condition, and that it is "the public policy and in the public interest of this state to encourage the voluntary conveyance of conservation easements to qualified nonprofit organizations."

H. Grantee Qualified Nonprofit Organization. Grantee is a tax-exempt nonprofit organization qualified under §501(c)(3) of the Internal Revenue Code and is eligible to acquire and hold a perpetual conservation easement pursuant to §815.3(a) of the California Civil Code.

I. Grantor's Continuing Hydro Project Activities. Grantor has used and continues to use the Property for the purposes related to the generation of electricity from hydropower facilities and related to the delivery, storage, and consumptive and nonconsumptive use of water as described more fully on attached Exhibit B (the "**Hydro Project Activities**"). In furtherance of the Hydro Project Activities, Grantor has improved portions of the Property with water- and power-related facilities, access roads, recreational facilities, buildings and other structures. The Governing Documents provide that "[c]onservation easements on Watershed Lands will include an express reservation of a right for continued operation and maintenance of hydroelectric facilities and associated water delivery facilities, including project replacements and improvements required to meet existing and future water delivery requirements for power generation and consumptive water use by existing users, compliance with any FERC license, FERC license renewal or other regulatory requirements."

J. Perpetual Protection of Beneficial Public Values. Grantee and Grantor intend through this Easement to ensure the perpetual protection of the beneficial public values on the Property as identified in the LCP, on and subject to the terms and conditions of this Easement. Specifically, the parties intend to assure that the beneficial public values identified in the LCP and set forth on Exhibit D (the "**Beneficial Public Values**") will be protected and preserved in perpetuity and that uses of the Property that are inconsistent with protecting and preserving these Beneficial Public Values will be restricted, all as set forth in this Easement; provided, however, that Grantor shall retain all interests not transferred to Grantee by this Easement, including, but not limited to Grantor's Hydro Reserved Rights described in Section 7 below.

## AGREEMENT

In consideration of the above recitals and the mutual covenants, terms, conditions, and restrictions contained herein, and pursuant to California Civil Code §815 et seq., Grantor and Grantee further hereby agree as follows:

1. Grant of Easement. Grantor hereby grants to Grantee a perpetual "conservation easement" as defined by §815.1 of the Conservation Easement Act of 1979 (California Civil Code §815 et seq.) in gross, in, on, over and across the Property (the "**Conservation Easement**"), subject to and in accordance with the terms and conditions of this Easement.

2. Purpose. It is the purpose of this Easement to protect and preserve in perpetuity the Beneficial Public Values on the Property by restricting any use of the Property that will significantly impair the Beneficial Public Values, all subject to and in accordance with the terms and conditions of this Easement (the "**Purpose**"). As used in this Easement, the terms "impair" and "impairment" mean to diminish in quantity,

quality, value, strength or viability. As used in this Easement, the terms "significant" and "significantly," when used with "impair" and "impairment," respectively, mean a greater than negligible adverse impact, for more than a transient period. The parties agree that Grantor's retention of certain rights specified in this Easement, including the Hydro Reserved Rights, is consistent with the Purpose of this Easement.

3. Baseline Documentation Report. The parties acknowledge that certain existing conditions particularly relevant to the Property are documented in a baseline documentation report (the "**Baseline Documentation Report**"). Grantor and Grantee each have a copy of the signed Baseline Documentation Report, executed by both parties to acknowledge their approval and receipt of the Baseline Documentation Report. The parties agree that the Baseline Documentation Report contains an accurate representation of such existing conditions of the Property as of the Effective Date, and is intended to serve as an objective, though non-exclusive, information baseline for monitoring compliance with the terms of this Easement. The foregoing notwithstanding, if a dispute arises with respect to any of the conditions of the Property, the parties shall not be foreclosed from utilizing any and all other relevant documents, surveys, or other evidence or information to assist in the resolution of the dispute.

4. Commission and FERC. The terms and conditions of this Easement are subject to any conditions imposed by the Commission pursuant to Advise Letter 6269E dated August 17, 2021 and effective August 16, 2021, or by FERC pursuant to any hydroelectric project license for the Property or any applicable orders or regulations that FERC may issue from time to time. Notwithstanding anything to the contrary in this Easement, Grantor, its successors, and assigns have the right to perform any and all acts required by an order of FERC, or its successors, without the prior approval of Grantee or any other person. Grantor expressly reserves the right to comply with all FERC orders and regulations as they may be amended from time to time. In addition, Grantee shall comply with any information requests or reporting obligations required by the Commission or FERC, whether directly to the Commission or FERC, or through Grantor; provided that Grantor shall reimburse the reasonable costs and expenses incurred by Grantee in responding to such requests. Execution of this Easement by Grantor does not imply tacit Commission or FERC approval of a non-project use on the Property nor does it obligate Grantor to seek Commission or FERC approval for non-project uses proposed by Grantee.

5. Rights Conveyed to Grantee. Subject to the terms and conditions of this Easement, Grantor grants and conveys to Grantee the following affirmative rights:

5.1 Identification, Monitoring and Enforcement. The right to identify with Grantor the Beneficial Public Values of the Property, the right to monitor and enforce the protection and preservation of such Beneficial Public Values in accordance with the terms of this Easement, the right to enforce the terms of this Easement, the right to enjoin any activity on the Property or other use of the Property which violates the terms of this Easement, and the right to enforce the restoration of such areas or features of the Property as may hereafter be damaged in violation of this Easement.

5.2 Access. The right for Grantee and Grantee's directors, officers, partners, members, managers, employees, contractors, subcontractors, consultants, representatives, agents, permittees and invitees ("**Grantee's Representatives**") to enter onto the Property at reasonable times, during normal business hours, not more than twice per calendar year and upon not less than ten (10) business days' advance written notice in order to monitor and inspect the Property, to enforce the rights which are granted herein, to determine whether the activities conducted on the Property are in compliance with the terms of this Easement, and to enforce the restoration of such areas or features of the Property as may have been damaged in violation of this Easement, all in compliance with the provisions of Section 10. Grantee will limit the number of Grantee Representatives entering the Property to those who are reasonably necessary to undertake the inspections, and such entry will be for no more days than are reasonably necessary to carry out the inspections. Grantor's representatives shall have the right to accompany Grantee's Representatives during bi-annual monitoring visits or on any other visit permitted by this Section 5.2. Notwithstanding the foregoing, Grantee shall also have the right of entry upon the Property upon not less than twenty-four (24) hours' advance written notice where such entry is necessary to (i) prevent, terminate, or mitigate a violation of the terms of this Easement; or (ii) monitor actions taken pursuant to the bi-annual inspections contemplated by this Section 5.2. All access and entry allowed under this Section 5.2 will be made in a manner that will not unreasonably interfere with the permitted use(s) of the Property by Grantor, its successors in interest, and any occupant(s) or user(s) of the Property and shall comply with any entry and access guidelines established by Grantor and restrictions contained in any Third Party Use Agreements. Without limiting the preceding sentence, with respect to the Hydro Operating Zone (as defined in Section 7.2.5 below), Grantee shall comply with any and all of Grantor's on-site safety and security requirements and any other rules and regulations that may be implemented by Grantor. Grantee agrees to cooperate with Grantor and to abide by any and all orders or instructions issued by Grantor, its employees, agents or representatives. Upon request, if Grantee's employees or other representatives will be entering into restricted areas of the Property, Grantee shall have its employees who will be entering such areas attend PG&E safety presentations, so that such employees understand all safety precautions and protocols concerning high voltage transmission lines and the electrical substation.

5.3 Grantee Signs. Grantee shall have the right, but not the obligation, at its sole cost and expense, to erect, maintain, and/or remove, one or more reasonable, non-illuminated signs or other appropriate markers in locations on the Property visible from any public roads or other adjoining property, bearing information indicating (a) that the Property is protected by the Conservation Easement, and/or (b) the participation of Grantee and of any funder in the stewardship of the Conservation Easement, the wording, size, number, design, and location of which shall be decided upon by Grantee and Grantor, each exercising its reasonable discretion.

6. Prohibited Uses. Grantor will not engage in, or permit others to engage in, the prohibited uses set forth on Exhibit F hereto, except as otherwise provided therein (the "**Prohibited Uses**"), which Grantor and Grantee agree are inconsistent with the Purpose of this Easement.

7. Grantor's Reserved Rights.

7.1 Hydro and Other Reserved Rights. As provided in California Civil Code §815.4, all interests not expressly transferred and conveyed to Grantee by this Easement shall remain in Grantor, including the right to engage in and permit or invite others to engage in all uses of the Property not affected by this Easement nor prohibited by this Easement or by law. In compliance with §815.4, Grantor and Grantee acknowledge and agree that Grantor expressly reserves all rights accruing from the ownership of the Property and not expressly transferred and conveyed to Grantee by this Easement, including without limitation the right to engage in or permit or invite others to engage in all uses of the Property that do not significantly impair the Beneficial Public Values and are not expressly prohibited by this Easement. Without limiting the foregoing, Grantor shall have the right to engage in and permit or invite others to engage in the permitted uses set forth in Exhibit I (the "**Permitted Uses**"). In addition and notwithstanding any other provision of this Easement, Grantor expressly reserves the right to engage in or permit or invite others to engage in those uses set forth in Exhibit C ("**Hydro Reserved Rights**"), subject to the restrictions set forth in Sections 7.3 and 7.4 below.

7.2 Definitions. As used in this Section 7, the following defined terms shall have the meanings set forth below:

7.2.1 Anticipated Significant Actions. As used herein, "**Anticipated Significant Actions**" are (a) those Required Actions (which include Specified Required Actions pursuant to Section 7.2.3), that involve a Prohibited Use and/or that Grantor determines in Grantor's reasonable discretion exercised in good faith are likely to significantly impair one or more of the Beneficial Public Values, (b) Discretionary Actions that Grantor determines in Grantor's reasonable discretion exercised in good faith are likely to significantly impair one or more of the Beneficial Public Values, and (c) Permitted Uses that Grantor determines in Grantor's reasonable discretion exercised in good faith are likely to significantly impair one or more of the Beneficial Public Values. Except as provided in Section 7.3.1, no Grantee notification, consultation or consent shall be required for actions, activities or improvements that are not Anticipated Significant Actions.

7.2.2 Required Actions. As used herein, "**Required Actions**" are those intended actions, activities or improvements that Grantor determines in Grantor's sole discretion exercised in good faith are required on the Property by any one or more of the following: (a) the Commission, FERC, or any other governmental entity having jurisdiction over Grantor's use, ownership, operation, or management of the Property, including the Hydro Project Activities, or (b) any Applicable Law (as defined in Section 8), or (c) any Third Party Use Agreements, or (d) to comply with professional practices,

standards and/or policies governing the Hydro Project Activities. All references in this Agreement to "Required Actions" shall include Specified Required Actions (as defined below) unless otherwise noted.

7.2.3 Specified Required Actions. As used herein, **"Specified Required Actions"** are those Required Actions that require a specified action, activity or improvement on the Property, with respect to which Grantor has no material discretion over the specific details of implementation, including, without limitation, the manner, timing, and location of the Specified Required Action. Without limiting Grantor's notification obligations pursuant to Section 7.3.1 below, no Grantee consultation or consent shall be required with respect to any Specified Required Action.

7.2.4 Discretionary Action. As used herein, a **"Discretionary Action"** is an intended action, activity or improvement that is not a Required Action or a Permitted Use, and does not involve a Prohibited Use.

7.2.5 Hydro Operating Zone. As used herein, a **"Hydro Operating Zone"** is a spatially delineated area of the Property intended to primarily contain (or immediately adjacent to an area of the Property containing) Hydroelectric Facilities and Associated Water Delivery Facilities, as defined and described on Exhibit B hereto. The initial delineated Hydro Operating Zones are set forth on Exhibit G hereto; provided, however, that, subject to Sections 7.3 and 7.4 below, Grantor shall have the right, as a Discretionary Action governed by Sections 7.3 and 7.4 below, to expand, contract, add or remove Hydro Operating Zones from time to time.

7.3 Annual Work Plan Notification, Consultation and Consent Requirements.

7.3.1 Delivery and Contents of Annual Work Plan. No later than February 15th of each calendar year after the Effective Date, Grantor shall prepare and deliver to Grantee an annual work plan for the Property (an **"Annual Work Plan"**). In the Annual Work Plan, Grantor shall inform Grantee of the Anticipated Significant Actions Grantor anticipates undertaking on the Property during such calendar year. The Annual Work Plan shall include the following:

(a) a reasonably detailed description of the Anticipated Significant Actions Grantor intends to commence within such calendar year, together with a bullet point list of those actions Grantor intends to commence during such calendar year that Grantor determines do not constitute Anticipated Significant Actions;

(b) a bullet point list of all actions undertaken by Grantor during the immediately preceding calendar year that Grantor determined did not constitute Anticipated Significant Actions and were not described in a previous Annual Work Plan (or otherwise disclosed to Grantee);

(c) an indication of whether the Anticipated Significant Actions will occur within or outside of a Hydro Operating Zone;

(d) Grantor's determination of which Anticipated Significant Actions are Discretionary Actions;

(e) Grantor's determination of which Anticipated Significant Actions are Required Actions, including a reasonably detailed explanation of the basis for Grantor's determination;

(f) Grantor's determination of which Anticipated Significant Actions are Specified Required Actions, including a reasonably detailed explanation of the basis for Grantor's determination;

(g) Grantor's determination of which Anticipated Significant Actions are Permitted Uses, including a reasonably detailed explanation of the basis for Grantor's determination;

(h) Grantor's estimated timeline for commencement and completion of each of the Anticipated Significant Actions;

(i) a description of Grantor's anticipated efforts to avoid or minimize harm to or impairment of the Beneficial Public Values from the Anticipated Significant Actions;

(j) if and when available, Grantor shall use reasonable efforts to provide copies of any underlying filings (including filings, if any, under the California Environmental Quality Act), permits (e.g., burn permits, stream alteration permits, or timber harvest plans), orders or rulings associated with the Anticipated Significant Actions; and

(k) any Third Party Use Agreement renewals or replacements as contemplated by Section 9.1.2 below.

7.3.2 Review of Annual Work Plan. Grantor and Grantee shall meet (in person or electronically) within sixty (60) days after Grantee's receipt of the Annual Work Plan to review the Annual Work Plan. Grantee has the right to request reasonable additional information regarding actions identified in the Annual Work Plan. As part of the Annual Work Plan review process, Grantor and Grantee will consult on Express Third Party Uses as contemplated by Section 9.1 below and Informal Uses as contemplated by Section 9.2 below. Periodically, at such annual review meetings, the content requirements for the Annual Work Plan as set forth in Section 7.3.1 above may be modified, confirmed by mutual written agreement of the parties.

7.3.3 Anticipated Significant Actions within Hydro Operating Zones. Without limiting Grantor's notification obligations pursuant to Section 7.3.1 above, no Grantee consultation or consent shall be required with respect to any Anticipated Significant Actions within a Hydro Operating Zone.

7.3.4 Anticipated Significant Actions Outside Hydro Operating Zones. The following provisions shall apply with respect to Anticipated Significant Actions outside of a Hydro Operating Zone:

(a) Specified Required Actions. Without limiting Grantor's notification obligations pursuant to Section 7.3.1 above, no Grantee consultation or consent shall be required with respect to any Specified Required Actions outside of a Hydro Operating Zone.

(b) Other Required Actions and Permitted Uses. With respect to Required Actions and Permitted Uses disclosed in the Annual Work Plan that are not Specified Required Actions and are to be undertaken outside of a Hydro Operating Zone, Grantor and Grantee agree that, at or prior to the meeting to review the Annual Work Plan, Grantee may (but shall be under no obligation to) propose alternative methods and practices to avoid or minimize harm to or impairment of one or more Beneficial Public Values by such Anticipated Significant Actions ("**Proposed Methods and Practices**"). Grantor shall implement the Proposed Methods and Practices, to the extent Grantor determines in its sole discretion exercised in good faith that the Proposed Methods and Practices (i) may be implemented in a commercially reasonable manner balancing the harm to Beneficial Public Values with any increased cost or burden to Grantor, (ii) where applicable, will allow for the completion of a Required Action in a timely manner, and (iii) are reasonably likely to avoid potential harm to or impairment of one or more Beneficial Public Values. If Grantor determines that one or more of the foregoing conditions has not been satisfied, Grantor shall specify the reasons for this determination in detail, and Grantor and Grantee shall cooperate in good faith and with diligence to attempt to resolve Grantor's objections to Grantee's Proposed Methods and Practices consistent with this paragraph.

(c) Discretionary Actions. With respect to Discretionary Actions disclosed in the Annual Work Plan that are to be undertaken outside of a Hydro Operating Zone, such Discretionary Actions shall be subject to Grantee's prior written consent, which consent shall not be unreasonably withheld, conditioned or delayed by Grantee. If Grantee fails to grant or deny Grantor's request for consent within one hundred eighty (180) days following Grantee's receipt of Grantor's request for consent, Grantee shall be deemed to have consented to the particular Discretionary Action described in the request. If Grantee withholds its consent to such proposed Discretionary Action to be undertaken outside of a Hydro Operating Zone, Grantee shall specify its objections in detail and, wherever possible, propose commercially reasonable alternatives, methods and/or practices to avoid or mitigate harm to or impairment of the Beneficial Public Values while substantially achieving the purposes of Grantor's proposed Discretionary Action. Grantor and Grantee shall cooperate in good faith and with diligence to attempt to resolve Grantee's objections in a manner that sufficiently mitigates Grantee's objections to its reasonable satisfaction.

7.4 Anticipated Significant Actions Not Identified in Annual Work Plan. If Grantor intends to undertake an Anticipated Significant Action not identified in an Annual Work Plan, Grantor shall notify Grantee (a "**Notice of Action**"), and include the

information required by Section 7.3.1 above. Additionally, Grantor and Grantee shall meet (in person or electronically) within sixty (60) days after Grantee's receipt of the Notice of Action to review Grantor's proposed Anticipated Significant Actions. Any Anticipated Significant Action (other than a Specified Required Action) identified in a Notice of Action which is proposed to occur outside of a Hydro Operating Zone shall be subject to Section 7.3.4 above. Where this Section 7.4 applies, references to the "Annual Work Plan" in Section 7.3.4 above shall be deemed to be references to the applicable Notice of Action except that Grantor shall not be required to provide the list of actions set forth in Section 7.3.1(b) above.

7.5 Emergency Actions. Notwithstanding any other provisions of this Section 7, in the case of an emergency or other exigent circumstance affecting the safety of persons and/or property, Grantor may exercise its Hydro Reserved Rights and take any other remedial actions in an unrestricted manner on all or any portion of the Property within or outside of a Hydro Operating Zone without consultation with Grantee and without Grantee's consent. Grantor shall provide copies of any required notifications to applicable regulatory agencies of the emergency action and shall notify Grantee of those emergency actions taken, such notice to be provided to Grantee as soon as practicable but in any event within thirty (30) days after the emergency action has occurred.

7.6 Water Rights. The Parties acknowledge that Grantor's exercise of water rights relating to water located or flowing on or under the Property, including those described in Exhibit C, are governed by this Section 7.

8. Responsibility for Operations. Nothing in this Easement shall be construed as giving any right or ability to Grantee to exercise physical or managerial control of the day-to-day operations of the Property or of Grantor's activities on the Property. Grantor shall have and retain all responsibility for, and shall bear all costs and liabilities of, the ownership of the Property. In connection with Grantor's use or occupancy of the Property, Grantor shall have and retain all responsibility for, and shall bear all costs and liabilities of, compliance with any present and future applicable laws, ordinances, rules, regulations, permits, licenses, authorizations, orders and requirements, whether or not in the current contemplation of the parties, which may affect or be applicable to the Property or any part of the Property (including, without limitation, any subsurface area), all consents or approvals required to be obtained from, and all rules and regulations of, and all building and zoning laws of, all federal, state, county and municipal governments, the departments, bureaus, agencies or commissions thereof, or any other governmental or quasi-governmental body or bodies exercising similar functions, having or acquiring jurisdiction of the Property (in each case, an "**Applicable Law**"), except as expressly stated otherwise in this Easement. Without placing any limitation on the foregoing sentence, the parties agree as follows:

8.1 Condition of Property. Grantee shall have no duty or responsibility for (a) the operation or maintenance of the Property except to the extent specifically undertaken by Grantee as permitted under this Easement, (b) the monitoring of any

hazardous conditions thereon, or (c) the protection of Grantor, the public, or any other person or entity from any risks relating to conditions on the Property.

8.2 Taxes. Grantee shall have no duty or responsibility for real property taxes and assessments levied on the Property.

8.3 Permits and Approvals. Grantor shall be solely responsible for obtaining any and all applicable governmental permits and approvals for, and otherwise complying with all Applicable Laws relating to, any activity or use of the Property by Grantor which is permitted by this Easement; provided, however, Grantor shall have no responsibility pursuant to this Easement for obtaining permits and approvals required on behalf of unrelated third parties who occupy or use the Property or for an unrelated third party's failure to comply with Applicable Laws. Grantee shall be solely responsible for obtaining any and all applicable governmental permits and approvals for, and otherwise complying with all Applicable Laws relating to, any activity or use of the Property by Grantee which is permitted by this Easement.

8.4 Limitation on Restoration Obligations. Nothing in this Easement shall require Grantor to take any action to restore the condition of the Property after (a) any Act of God, which includes, without limitation, fire, climate change, flood, storm, earth movement, or natural evolutionary changes in the condition of the Property from that described in the Baseline Documentation Report; (b) any prudent action taken by Grantor under emergency conditions to prevent, abate, or mitigate significant injury to the Property or to any person resulting from such causes; or (c) the non-permitted acts of unrelated third parties so long as Grantor has satisfied its obligations under Section 9.3.

9. Third Party Use of the Property.

9.1 Express Third Party Uses. Exhibit H hereto describes the existing third party uses of the Property permitted with the express agreement of Grantor ("**Express Third Party Uses**"). Subject to Section 7 above, Express Third Party Uses shall also include any future third party use implemented by Grantor as a Required Action or as a Discretionary Action approved by Grantee in accordance with Section 7. Grantor retains the right to maintain, renew, and replace all agreements memorializing the Express Third Party Uses ("**Third Party Use Agreements**") and to engage in all activities reasonably required to comply with Grantor's obligations with respect to the Express Third Party Uses, subject to the following conditions:

9.1.1 Increases in Intensity or Expansion of Location or Size or Change in Use. Any (i) increase in the intensity, or (ii) expansion of the location or size, or (iii) a change in the use, of an Express Third Party Use (whether through a new agreement or an amendment to an existing agreement), that Grantor determines in Grantor's reasonable discretion exercised in good faith is likely to significantly impair the Beneficial Public Values shall be subject to Grantee's prior written consent, which consent shall not be unreasonably withheld, conditioned or delayed by Grantee, except

if such change in Express Third Party Use constitutes a Required Action in which case the consultation provisions of Section 7 above shall apply.

9.1.2 Renewal or Replacement of Third Party Use Agreements. All Third Party Use Agreements existing on the date hereof are identified on Exhibit H. As Third Party Use Agreements are renewed or replaced (either with the existing user or a new user), Grantor, in consultation with Grantee as part of the Annual Work Plan consultation in accordance with Section 7.3 above, shall include contractual provisions to bring the continuation of the Express Third Party Use and the preservation of the Beneficial Public Values into alignment to the fullest extent reasonably practicable.

9.1.3 Consultation on Express Third Party Uses. As part of the Annual Work Plan review process under Section 7.3 above, Grantor and Grantee will consult on existing Express Third Party Uses, including recommendations, if any, on how to bring the Express Third Party Uses and the preservation of the Beneficial Public Values into alignment to the fullest extent reasonably practicable.

9.1.4 Enforcement of Third Party Use Agreements. If Grantor or Grantee discovers any default under a Third Party Use Agreement that significantly impairs the Beneficial Public Values (and if Grantee makes such discovery, Grantee gives Grantor written notice thereof), Grantor shall use reasonable efforts to enforce such Third Party Use Agreement or otherwise remedy such violation, at Grantor's sole expense.

9.2 Informal Uses and Public Access. Grantor and Grantee recognize that the Property has been used by third parties for recreational, cultural, and other non-commercial or informal purposes without formal written agreements to conduct such activities (the "**Informal Uses**"). Grantor and Grantee further recognize that access is inherent or may be inherent in the enjoyment of the Beneficial Public Values and the Informal Uses. Consistent with the objectives articulated in the Governing Documents to provide continued reasonable access by the public to the Watershed Lands, Grantor shall allow public access to the Property (other than Hydro Operating Zones) that is substantially consistent with the public access existing on the Effective Date, subject to Section 7 and the following limitations:

9.2.1 Rules and Regulations. Grantor reserves the right to make reasonable rules and regulations to control, limit, or, as necessary, exclude Informal Uses and public access.

9.2.2 Liability Limitation. Grantor and Grantee claim all of the rights and immunities against liability for injury to the public to the fullest extent allowable by law.

9.2.3 Periodic Review of Informal Uses. As part of the Annual Work Plan review process under Section 7.3 above, Grantor and Grantee will consult on Informal Uses, including recommendations made by Grantor or Grantee, if any,

regarding the necessity of controlling, limiting or excluding the Informal Uses to ensure the preservation of the Beneficial Public Values.

9.3 Unauthorized Third-Party Uses. If Grantor or Grantee discovers any unauthorized third-party use or activity on the Property (not including any third party violation covered by Section 9.1.4 above) that violates the terms of this Easement (and if Grantee makes such discovery, Grantee gives Grantor written notice thereof), Grantor shall use reasonable efforts, in consultation with Grantee, to stop or prevent any such unauthorized use of the Property, at Grantor's sole expense; provided that in no event shall Grantor's obligations under this Section 9.3 require Grantor to pursue legal action or incur other substantial costs. If Grantee demonstrates that Grantor's efforts in compliance with this Section 9.3 have not prevented, or are unlikely to prevent, the unauthorized third-party use or activity on the Property that violates the terms of this Easement, Grantee may meet and confer with Grantor to propose additional efforts to prevent such use or activity which Grantee may undertake, at Grantee's sole expense. Grantor shall consider such proposal in good faith and, if Grantor permits Grantee to use such additional efforts, the scope and duration of such efforts shall be determined by Grantor, and Grantee shall comply with any requirements imposed by Grantor in connection with such efforts.

10. Enforcement and Remedies.

10.1 Procedures Upon Violation. If a party hereto (the "**Non-Breaching Party**") determines there is a breach of the terms of this Easement or that a breach is threatened, written notice of such breach (the "**Notice of Breach**") and a demand for corrective action sufficient to cure the breach shall be given by the Non-Breaching Party to the party allegedly breaching this Easement (the "**Breaching Party**"). Within fourteen (14) days after delivery of a Notice of Breach, Grantor and Grantee shall meet at a location in the County where the Property is located or as otherwise agreed to by the parties to discuss the circumstances of the alleged or threatened breach and to attempt to agree on appropriate corrective action. If the parties mutually determine that it is appropriate and desirable, a duly qualified expert in the subject matter of the alleged breach (the "**Consulting Expert**") shall attend the meeting. Grantor and Grantee shall each pay one-half of the costs of retaining the services of the Consulting Expert for such discussion; provided, however, that if Grantor and Grantee are unable to agree upon a Consulting Expert, each party may retain the services of an expert at its own expense. If Grantor and Grantee are unable to agree on appropriate corrective action (or if any such corrective action is required) within thirty (30) days after such meeting, then the Non-Breaching Party may, at its election, deliver a further written notice to the Breaching Party to demand reasonable, particular corrective action to cure the breach (the "**Notice of Easement Violation**"). If a violation is not cured within thirty (30) days after the delivery of the Notice of Easement Violation, or if the cure reasonably requires more than thirty (30) days to complete and there is failure to begin the cure within the 30-day period or failure to continue diligently to complete the cure, the Non-Breaching Party may commence litigation in accordance with Section 10.2 below.

10.2 Litigation. If the parties are not able to resolve a claim or dispute pursuant to Section 10.1 above, the Non-Breaching Party may bring an action at law or in equity in a court of competent jurisdiction to enforce compliance with the terms of this Easement, to recover any damages to which Non-Breaching Party may be entitled for violation of the terms of this Easement, or for any other legal or equitable relief available under California law, including, but not limited to, temporary or permanent injunctive relief, monetary damages and/or any other form of relief required to achieve the restoration of the Property to the condition in which it existed prior to any violation. To the extent that Grantee recovers any monetary damages for the cost of restoring any injury or damage to a portion of the Property that is caused by Grantor's breach of this Easement, all such damages recovered by Grantee (after appropriate costs of suit are reimbursed) shall be applied to the cost of undertaking any corrective action to the applicable portion of the Property. Notwithstanding anything to the contrary contained in this Easement, in no event shall the Breaching Party be liable to the Non-Breaching Party for, and the parties each hereby waive their right to, any indirect, special, punitive, or consequential damages resulting from the Breaching Party's breach of this Easement, whether foreseeable or unforeseeable.

10.3 Emergency Injunctive Relief. If circumstances require immediate action to prevent or mitigate a violation of this Easement and the Non-Breaching Party reasonably determines that irreparable harm would result if the Non-Breaching Party were required to complete the process set forth in Section 10.1, the Non-Breaching Party may proceed immediately to seek an injunction to stop the violation, temporarily or permanently.

10.4 Remedies Cumulative. The remedies described in this Section 10 shall be cumulative and shall be in addition to all remedies now or hereafter existing at law or in equity, including but not limited to, the remedies set forth in California Civil Code §815 *et seq.*, inclusive. The failure of a party to discover a violation or to take immediate legal action shall not bar taking such action at a later time.

10.5 Costs of Enforcement. All costs incurred in enforcing the terms of this Easement, including, but not limited to, costs of suit and reasonable attorneys' fees as set forth in Section 20.11, shall be borne by the Breaching Party, but only to the extent that a breach of this Easement is determined to have occurred. If, after the Non-Breaching Party delivers a Notice of Easement Violation, it is determined that there was no breach of this Easement by the Breaching Party, the Non-Breaching Party shall pay all of the Breaching Party's costs and expenses incurred in connection with the alleged breach.

10.6 No Waiver. Enforcement of this Easement against a party shall be at the discretion of the Non-Breaching Party, and any forbearance by the Non-Breaching Party to exercise its rights under this Easement in the event of any breach of any term of this Easement shall not be deemed or construed to be a waiver by the Non-Breaching Party of such term or of any subsequent breach of the same or any other term of this Easement or of any of such party's rights under this Easement. No delay or omission by the Non-Breaching Party in the exercise of any right or remedy shall impair

such right or remedy or be construed as a waiver. A party's permission to the other party to carry out, or failure to object to, any proposed use or activity by the other party shall not constitute consent to any subsequent use or activity of the same or different nature.

11. Indemnification and Insurance.

11.1 Indemnification by Grantee. Grantee shall, to the maximum extent permitted by law, indemnify, protect, defend and hold harmless Grantor, its parent corporation, subsidiaries, affiliates, and their respective officers, managers, directors, representatives, agents, employees, transferees, successors and assigns (collectively, "**Grantor Indemnitees**") from and against all claims, losses, actions, demands, damages, costs, expenses (including, but not limited to, experts' fees and reasonable attorneys' fees and costs) and liabilities of whatever kind or nature (collectively, "**Claims**") arising out of or in connection with this Easement or the Property to the extent caused by the negligence or willful misconduct of the Grantee Indemnitees.

11.2 Indemnification by Grantor. Grantor shall, to the maximum extent permitted by law, indemnify, protect, defend and hold harmless Grantee, its parent corporation, subsidiaries, affiliates, and their respective officers, managers, directors, representatives, agents, employees, transferees, successors and assigns (collectively, "**Grantee Indemnitees**") from and against all Claims arising out of or in connection with this Easement or the Property except to the extent caused by the negligence or willful misconduct of the Grantee Indemnitees.

11.3 Release. Entry onto the Property by Grantee and Grantee's Representatives shall be at Grantee's sole risk and expense, and Grantee accepts all risk relating to the condition of the Property. Notwithstanding the provisions of Section 11.2, Grantor shall not be liable to Grantee for, and to the maximum extent permitted by law, Grantee hereby waives and releases Grantor and the other Grantor Indemnitees from, any and all liability, whether in contract, tort or on any other basis, for any injury, damage, or loss to Grantee and/or Grantee's Representatives resulting from or attributable to any occurrence relating to the condition of the Property, except if arising solely from Grantor's gross negligence or willful misconduct.

11.4 Insurance. Grantee shall procure, carry and maintain in effect during all access to the Property throughout the term of this Easement the insurance specified in Exhibit E hereto, provided that Grantor reserves the right to periodically review and reasonably modify the insurance requirements specified in Exhibit E in effect to be generally consistent with requirements of other prudent property owners allowing access to their properties by conservation easement holders. All insurance shall be written on forms and with insurance carriers acceptable to Grantor in its commercially reasonable judgment. Prior to Grantee's initial entry onto the Property, and thereafter at least thirty (30) days prior to the expiration date of any policy, Grantee shall provide Grantor with evidence of the insurance coverage, or continuing coverage, as applicable, satisfying the requirements of this Section 11.4 and Exhibit E. Grantee is also responsible for causing Grantee's agents and contractors entering the Property to

comply with the insurance requirements of this Easement at all relevant times, the insurance being specified in Exhibit E. Grantee shall, to the maximum extent permitted by law, indemnify, protect, defend and hold the Grantor Indemnitees harmless against claims, losses, costs (including attorneys' fees and costs), liabilities and damages resulting from the failure of Grantee, or any of Grantee's consultants, contractors or subcontractors, to comply with the insurance requirements set forth in this Section 11.4 and Exhibit E. Except for the right to access the Property under Section 5.2 above, which shall be conditioned upon carrying insurance required herein, no failure to carry such insurance or to provide a certificate thereof by any such deadline shall alter or affect in any manner any of the rights or obligations of the parties under or with respect to this Easement. The foregoing insurance requirements shall not apply in the event that the Grantee is a governmental agency with a self-insurance program reasonably acceptable to Grantor.

12. Grantee Transfer of Easement.

12.1 Voluntary Transfer.

12.1.1 If Grantee desires to assign its interest under this Easement, Grantee shall provide Grantor and the Sierra Nevada Conservancy ("**SNC**") with written notice of such intention to transfer to an assignee which is (a) qualified to hold a conservation easement under §815.3 of the California Civil Code; and (b) willing and with the financial capability (taking into account any stewardship funds to be transferred by Grantee with this Easement) and organizational experience to assume all of the responsibilities imposed on Grantee under this Easement; and (c) acceptable to Grantor in its reasonable discretion. Grantee shall allow the SNC, in consultation with Grantor, a period of not less than sixty (60) days within which to approve the proposed assignee, which approval shall not be unreasonably withheld and shall be based on whether the proposed assignee meets the designation criteria specified in this Section 12.1.1.

12.1.2 Grantee is responsible for identifying a suitable assignee pursuant to Section 12.1.1. However, if a suitable assignee is not identified, then SNC shall have sole discretion to elect to become the assignee of Grantee's interest hereunder.

12.1.3 As conditions to any assignment of Grantee's interest under this Easement, Grantee shall (a) require the assignee to expressly agree in writing to assume Grantee's obligations hereunder, and (b) ensure that such assignee has the resources to fulfill its obligations under this Easement. Notwithstanding anything in this Section 12.1 to the contrary, this Easement shall not be transferred by Grantee to any governmental entity, public agency or Native American tribe without the consent of the Grantor, which consent shall be in Grantor's sole discretion exercised in good faith.

12.2 Involuntary Transfer. If Grantee ever ceases to exist or no longer qualifies under §815.3 of the California Civil Code, the Stewardship Council (or its designee), or if the Stewardship Council (or its designee) shall cease to exist, the Attorney General of the State of California, shall petition a court of competent

jurisdiction to transfer this Easement to an organization that meets all of the designation criteria specified in Section 12.1.

13. Subsequent Property Transfers by Grantor.

13.1 Rights of Grantor. Subject to the provisions of Sections 7 and 9 above, this Section 13, Section 20.12 below, and Exhibit F, Paragraph 1 below, Grantor shall have the unrestricted right to sell, encumber, or otherwise transfer the Property or portions thereof to anyone Grantor chooses. Notwithstanding the foregoing, Grantor shall disclose the existence of this Easement (including reference to the recording information) in any deed or other legal instrument by which Grantor divests itself of a real property interest in all or a portion of the Property, including, without limitation, a leasehold interest, and all such conveyances shall be made expressly subject to the terms of this Easement. Grantor shall notify Grantee periodically of any contemplated grants by Grantor to any third party of any interest in any portion of the Property, whether such interest is a fee, easement, lease, mortgage or other interest. Grantor shall notify Grantee in writing not less than ninety (90) days in advance of any grant of fee interest in any portion of the Property by Grantor to any third party. Additionally, Grantor shall notify Grantee in writing not more than thirty (30) days after any grant by Grantor to any third party of any non-fee interest in any portion of the Property, whether such interest is an easement, lease, mortgage or other non-fee interest. The failure of Grantor to perform any act required by this Section 13 shall not impair the validity of this Easement or limit its enforcement in any way or create any obligation on the part of Grantee. Grantor recognizes that Grantee may incur direct and indirect costs for monitoring and administration of the Conservation Easement in the event fee title to the Property is transferred under this provision. Accordingly, upon Grantor's sale, transfer or conveyance of fee title of the Property, Grantor shall pay, or cause to be paid, to Grantee a one-time payment of a sum representing the increased cost of such Conservation Easement stewardship, as reasonably determined at such time by Grantee. Such one-time payment shall be made concurrently with the transfer of fee title and shall include Grantee's estimated additional cost associated with the proposed Removal of Project Dams, as described in Exhibit J of this Easement and in addition to any reimbursements required pursuant to Section 13.2.4 and Section 17.

13.2 Potential Release of Hydro Reserved Rights.

13.2.1 Conveyance of Entire Property. In the event:

(a) Grantor intends to transfer fee title to the entire Property to an unaffiliated third party;

(b) the Hydro Project Activities and any uses and facilities that are unrelated to the Hydro Project Activities but undertaken as a Required Action at the Property have been formally and permanently terminated by Grantor and, as appropriate, decommissioned pursuant to a regulatory proceeding; and

(c) no Hydroelectric Facilities and Associated Water Delivery Facilities, nor other facilities unrelated to Hydro Project Activities installed pursuant to a Required Action are located at the Property,

then, subject to any final orders or decommissioning requirements issued by the FERC and/or other agency(ies) with jurisdiction over the Hydro Project Activities and such other unrelated uses prior to said transfer of the entire Property, Grantor shall release, relinquish and forever terminate, in a manner that shall be binding upon all successors in interest to the Property, (i) all rights of Grantor described in Exhibit C, (ii) the exceptions to the Prohibited Uses for Required Actions and Specified Required Actions set forth in Exhibit F, (iii) the exceptions to the Prohibited Uses in Exhibit F relating to activities within the Hydro Operating Zone, and (iv) Permitted Uses, to the extent related to the Hydro Project Activities, as set forth in Exhibit I (items (i), (ii), (iii) and (iv) being referred to collectively as the "**Reservations**"). Following such release, relinquishment and termination of Reservations, all Anticipated Significant Actions (except for Prohibited Uses and continuing Permitted Uses) shall be subject to Grantee's consent as Discretionary Actions and the Easement shall be interpreted more restrictively in a manner recognizing the release of Reservations. Additionally, following such release, relinquishment and termination of Reservations, the forest management activities specified in Section 9 of Exhibit I shall continue as Permitted Uses, but shall be subject to Grantee's consent as Discretionary Actions.

13.2.2 Partial Conveyance. In the event:

(a) Grantor intends to transfer fee title to less than the entire Property (the "**Transferred Parcel**") to an unaffiliated third party;

(b) the Hydro Project Activities and any uses and facilities that are unrelated to the Hydro Project Activities but undertaken as a Required Action at the Transferred Parcel have been formally and permanently terminated by Grantor and, as appropriate, decommissioned pursuant to a regulatory proceeding; and

(c) no Hydroelectric Facilities and Associated Water Delivery Facilities, nor other facilities unrelated to Hydro Project Activities installed pursuant to a Required Action are located at the Transferred Parcel,

then, subject to any final orders or decommissioning requirements issued by the FERC and/or other agency(ies) with jurisdiction over the Hydro Project Activities and such other unrelated uses, prior to said transfer of the Transferred Parcel, Grantor shall release, relinquish and forever terminate, in a manner that shall be binding upon all successors in interest to the Transferred Parcel, the Reservations with respect to the Transferred Parcel. Following such release, relinquishment and termination of Reservations, all Anticipated Significant Actions (except for Prohibited Uses and continuing Permitted Uses) on the Transferred Parcel shall be subject to Grantee's consent as Discretionary Actions and the Easement shall be interpreted more restrictively in a manner recognizing the release of Reservations as to the Transferred Parcel. Additionally, following such release, relinquishment and termination of

Reservations, the forest management activities specified in Section 9 of Exhibit I on the Transferred Parcel shall continue as Permitted Uses, but shall be subject to Grantee's consent as Discretionary Actions.

13.2.3 Grantor's Continuing Reserved Rights. Nothing in Section 13.2.2 shall limit the rights of Grantor in this Easement with respect to the portion of the Property retained by Grantor.

13.2.4 Easement Amendment. In the event of a conveyance and release of Reservations pursuant to this Section 13.2, Grantor and Grantee may agree to amend this Easement, or to create a separate Easement for the Transferred Parcel and for the remaining portion of the Property in accordance with Section 17, to reflect the release of Reservations and, where appropriate, to reflect separate ownership of the Transferred Parcel and the remainder of the Property. In accordance with Section 17 below, Grantor shall reimburse Grantee for all reasonable costs incurred in connection with the drafting, review, negotiation, approval and recording of any amendment or separate Easement pursuant to this Section, including costs incurred in consideration of whether an amendment and/or new Easement(s) is/are necessary or appropriate.

13.2.5 Transfer Restrictions Remain Applicable. Nothing herein shall affect Grantor's obligations under Section 1 of Exhibit F.

14. Extinguishment and Condemnation.

14.1 Extinguishment. If circumstances arise in the future such as render the Purpose of this Easement impossible to accomplish, this Easement shall only be terminated or extinguished, whether in whole or in part, by judicial proceedings in a court of competent jurisdiction. Grantor's economic hardship shall not be a reason to extinguish this Easement.

14.2 Condemnation. If all or part of the Property is taken by the exercise of the power of eminent domain by a public, corporate, or other authority, whether permanent or temporary, including a private sale in lieu of eminent domain, so as to abrogate the restrictions imposed by the Conservation Easement, Grantor and Grantee shall join in appropriate actions at the time of such taking to recover the full value of the taking and all incidental or direct damages resulting from the taking. All compensation thereby awarded will belong and be paid to Grantor and Grantee in proportion to their respective interests in the Property as determined pursuant to Section 14.3, it being expressly agreed that the Conservation Easement constitutes a compensable property right. All expenses incurred by Grantor and Grantee in such action shall be paid out of the recovered proceeds. Grantor and Grantee acknowledge that any and all awards to Grantor and Grantee may be subject to the approval of the Commission and/or the FERC.

14.3 Proceeds. Pursuant to California Civil Code §815.2(a) this Easement constitutes a real property interest immediately vested in Grantee. It is acknowledged by the parties that the purposes of establishing the value of this

property right and enforcing the rights of Grantee with respect thereto is to prevent a private windfall and to protect the public investment which is involved in the conveyance of the Conservation Easement. That being the case, the parties stipulate that, for the purpose of determining the ratio for proportionate value of each party's respective interest in the Property at the time of termination or extinguishment of the Conservation Easement, the value of the Conservation Easement shall be the difference between (a) the current fair market value of the fee interest in the Property at the time of termination, as if unencumbered by the Conservation Easement, but taking into account all other existing restrictions on the improvement, construction, alteration, expansion, development, use, maintenance or operation of all or any portion of the Property (e.g., zoning laws, land use laws or other governmental laws, codes, regulations or ordinances, and private restrictions such as covenants, restrictions and agreements), and (b) the current fair market value of the Property at the time of termination, as encumbered by the Conservation Easement, but taking into account all other existing restrictions on the improvement, construction, alteration, expansion, development, use, maintenance or operation of all or any portion of the Property (e.g., zoning laws, land use laws or other governmental laws, codes, regulations or ordinances, and private restrictions such as covenants, restrictions and agreements). The values shall be determined by an appraisal prepared by a qualified appraiser familiar with appraising conservation easements jointly selected by Grantor and Grantee. The cost of the appraisal shall be paid out of proceeds in proportion to the recoveries of Grantor and Grantee. There shall be no restriction on Grantor's or Grantee's use of proceeds received pursuant to this Section 14.3.

15. Estoppel Certificates. Grantee shall, within thirty (30) days after receiving Grantor's written request therefor (not to exceed once during any twelve (12) month period), execute and deliver to Grantor a document certifying, to the actual knowledge of the person executing the document without any duty of investigation, that Grantor is in compliance with any obligation of Grantor contained in this Easement, or otherwise evidencing the status of such obligation to the extent of Grantee's actual knowledge thereof, as may be reasonably requested by Grantor.

16. Notices. Any notice or other communication required or permitted under this Easement shall be in writing and shall be either personally delivered or transmitted by registered or certified mail, return receipt requested, postage prepaid, or by a nationally recognized overnight courier, such as FedEx, UPS, or Airborne Express, addressed to the parties as follows:

If to Grantor: If by registered or certified mail, return receipt requested:

Director, Land Management
Pacific Gas and Electric Company
P.O. Box 770000, Mail Code N10A
San Francisco, CA 94177
Re: Land Conservation Commitment

With a copy to:

Law Department
Pacific Gas and Electric Company
P.O. Box 7442
San Francisco, CA 94120
Attn: Managing Counsel, Commercial and Transactions (Real Estate)
Re: Land Conservation Commitment

If by personal delivery or overnight courier:

Director, Land Management
Pacific Gas and Electric Company
245 Market Street, Room 1051
San Francisco, CA 94105
Re: Land Conservation Commitment

With a copy to:

Law Department
Pacific Gas and Electric Company
77 Beale Street, Mail Code B30A
San Francisco, CA 94105
Attn: Managing Counsel, Commercial and Transactions (Real Estate)
Re: Land Conservation Commitment

If to Grantee: Mendocino Land Trust Incorporated
330 N. Franklin St., Suite 7
Fort Bragg, CA 95437
Attn: Conservation Director

The date of any notice or communication shall be deemed to be the date of receipt if delivered personally, or the date of the receipt or refusal of delivery if transmitted by mail or overnight courier. Either party may change the address for notice by giving notice to the other party in accordance with this Section 16.

17. Amendment. This Easement may not be amended, except by written amendment executed by Grantor and Grantee or their respective successors and assigns and recorded in the official public records of the jurisdiction where the Property is located. If circumstances arise under which an amendment would be appropriate, any such amendment shall be consistent with Grantee's conservation easement amendment policy(ies), and the Purpose of this Easement, including continuing to protect and preserve the Beneficial Public Values, and shall not affect the perpetual duration of this Easement or the qualification of the Conservation Easement as a conservation easement under California Civil Code §815 *et seq.* (or successor thereto). Grantee shall promptly record the amendment in the official records of the County in

which the Property is located, and shall thereafter promptly provide a conformed copy of the recorded amendment to Grantor. The party requesting the amendment shall reimburse the non-requesting party for all reasonable costs incurred in connection with the drafting, review, negotiation, approval and recording of such amendment. Grantor shall be deemed to be the "party requesting the amendment" in connection with any amendment and/or new conservation easement(s) pursuant to Section 13 above and the "reasonable costs incurred" shall include consideration of whether an amendment and/or new conservation easement(s) is/are necessary or appropriate.

18. Hazardous Substances.

18.1 Definitions. The following terms have the meanings ascribed to them below for purposes of this Easement:

18.1.1 **"Environmental Requirements"** means all applicable present and future laws, statutes, regulations, rules, ordinances, codes, licenses, permits, orders, approvals, plans, authorizations, judicial, administrative and regulatory decrees, directives and judgments of all governmental agencies, departments, commissions and boards, relating to the protection of human health or safety, or regulating or relating to industrial hygiene or environmental conditions, or the protection of the environment, or pollution or contamination of the air, soil, surface water or groundwater, including, without limitation, all requirements and regulations pertaining to reporting, licensing, permitting, investigating and remediating emissions, discharges, releases or threatened releases of Hazardous Substances, whether solid, liquid or gaseous in nature, into the air, surface water, or land, or relating to the manufacture, processing, distribution, use, treatment, storage, disposal, transport or handling of Hazardous Substances, whether solid, liquid or gaseous in nature.

18.1.2 **"Hazardous Substances"** means any hazardous or toxic material or waste which is or becomes regulated by any local governmental authority, the State of California or the United States Government under any Environmental Requirements, including, without limitation, any material or substance:

(a) now or hereafter defined as a "hazardous substance," "hazardous waste," "hazardous material," "extremely hazardous waste," "restricted hazardous waste" or "toxic substance" or words of similar import under any applicable local, state or federal law or under the regulations adopted or promulgated pursuant thereto; and all rules and regulations of the United States or California Environmental Protection Agency or any successor agency, or any other state or federal department, board or agency, or any other agency or governmental board or entity having jurisdiction, as any of the foregoing have been, or are hereafter amended from time to time; or

(b) which is toxic, explosive, corrosive, flammable, infectious, radioactive, carcinogenic, mutagenic or otherwise hazardous, and is now or hereafter regulated as a Hazardous Substance by any governmental authority, agency, department, commission, board, agency or instrumentality of the United States, any State of the United States or any political subdivision thereof; or which cause, or are listed by the

State of California as being known to the State of California to cause, cancer or reproductive toxicity; or

(c) the presence of which on the Property poses or threatens to pose a hazard to the health or safety of persons or to the environment; or

(d) which contains gasoline, diesel fuel or other petroleum hydrocarbons; or

(e) which contains lead-based paint or other lead contamination, polychlorinated biphenyls or asbestos or asbestos-containing materials or urea formaldehyde foam insulation; or

(f) which contains radon gas.

18.1.3 "**Necessary Remediation**" means Remediation required by any governmental agency which has jurisdiction over the Remediation pursuant to the Environmental Requirements.

18.1.4 "**Remediation**" refers to the process of, and all work and planning performed in connection with, the investigation, testing for, monitoring, remediation, containment, transportation, removal and disposal or recycling of Hazardous Substances from the Property and any other property to which Hazardous Substances originating on the Property have migrated or may migrate in the future, and the repair and restoration of the Property, and restoration and mitigation of affected natural resources, regardless of whether such actions are required by Environmental Requirements.

18.2 Allocation of Responsibility for Hazardous Substances.

18.2.1 Generally. Grantor shall (as between Grantor and Grantee) bear the cost for the Necessary Remediation of Hazardous Substances.

18.2.2 Environmental Reports. Grantor, as part of the Land Conservation Commitment has prepared certain environmental reports concerning the Property. Copies of these environmental reports have been provided to Grantee.

18.2.3 Grantor Responsibility for the Cost of Necessary Remediation. Grantor shall retain responsibility for the cost of Necessary Remediation of Hazardous Substance releases in soil and groundwater, whether occurring in the past or at any time in the future, which are present on the Property, provided that Grantee did not cause, in whole or in part, such Hazardous Substance contamination.

18.2.4 No Owner or Operator Liability. The parties do not intend this Easement to be, and this Easement shall not be, construed such that it creates in or gives to Grantee any of the following solely as the result of being a holder of the Conservation Easement:

(a) The obligations or liability of an "owner" or "operator" or "arranger," as those terms are defined and used in Environmental Requirements;

(b) The obligations or liabilities of a person described in 42 U.S.C. §9607(a)(3) or (4);

(c) The obligations of a responsible person under any applicable Environmental Requirements;

(d) The right to investigate and remediate any Hazardous Substances associated with the Property; or

(e) Any control over Grantor's ability to investigate, remove, remediate or otherwise clean up any Hazardous Substances associated with the Property.

18.3 Hazardous Substances Indemnification.

18.3.1 By Grantor. Grantor agrees and covenants, at its sole cost and expense, to indemnify, protect, defend and hold Grantee harmless, from and against any and all losses (including diminution in the value of the Property and other consequential damages), costs, claims, demands, actions, suits, orders, causes of action, penalties, fines, taxes, obligations, controversies, debts, expenses, accounts, damages (including, without limitation, punitive damages), judgments and liabilities of whatever kind or nature, and by whomsoever asserted, in law, equity or otherwise, including, without limitation, the actual fees and expenses of experts, attorneys and others and the payment of "response costs" under CERCLA or any other Environmental Requirements, arising from or relating, in whole or in part, to Hazardous Substances present at the Property, alleged to be present there, or otherwise connected in any way to the Property, whether before, on, or after the date of this Easement (collectively, "**Environmental Claims**"), except to the extent caused, in whole or in part, by the negligent or intentional act of Grantee.

18.3.2 By Grantee. Grantee agrees and covenants, at its sole cost and expense, to indemnify, protect, defend and hold Grantor harmless, from and against any and all Environmental Claims, to the extent caused, in whole or in part, by the negligent or intentional act of Grantee.

19. Carbon Rights.

19.1 Promotion of Climate Stability. Grantor and Grantee anticipate that the protection and preservation of the Beneficial Public Values will promote climate stability, especially the ability of the forest to store atmospheric carbon as a means to mitigate global warming, which is recognized as being of public benefit by the 1993 United Nations Framework Convention on Climate Change, the federal Energy Policy Act of 1002, section 1605(a) and (b), the United States Climate Challenge Program, the 2007 reports of the International Panel on Climate Change, and California legislation such as that embodied in Fish and Game Code Section 1356.

19.2 Reservation of Carbon Rights. Grantor exclusively reserves to itself, and to its personal representatives, heirs, successors and assigns, any and all carbon rights and obligations appurtenant to or accruing from the Property as may exist as of the date of recordation of this Easement or as may be granted, discovered, created, declared or developed in the future, including, but not limited to, the right to (subject to and in accordance with Section 7 hereof) use, store, sequester, accumulate, and/or depreciate carbon within or on the Property and the right to trade, sell, transfer, or lease these rights. Grantor and Grantee acknowledge and agree that these carbon rights are consistent with the Beneficial Public Values, and this Easement shall not extinguish or otherwise impair the carbon rights and obligations appurtenant to or accruing from the Property.

19.3 Carbon Certification. In furtherance of Grantor's exercise of the carbon rights reserved hereunder, Grantor may elect to enter into an agreement not inconsistent with this Easement respecting such reserved rights as may be required by a third party that Grantor chooses ("**Carbon Certification Party**") in order to facilitate the sale, transfer or lease of the carbon rights and may record such agreement in the official records of any County where the Property is located. To the extent reasonably required by any Carbon Certification Party and requested by Grantor, Grantee, at Grantor's cost and expense, shall cooperate with Grantor by accommodating Grantor's establishment, verification or certification of the carbon rights in connection with the Property. Grantor agrees to notify Grantee at least thirty (30) days prior to any sale, transfer or lease of these carbon rights or the recording of an agreement with respect thereto, unless Grantor has previously notified Grantee in an Annual Work Plan.

20. General Provisions.

20.1 Governing Laws. This Easement shall be governed by, and construed and enforced in accordance with, the laws of the State of California.

20.2 No Public Dedication. Nothing contained in this Easement shall be construed or deemed to be an express or implied dedication or gift of all or any portion of the Property for use or access by the general public nor shall this Easement or any of the rights granted hereunder be construed as an acknowledgement of any claim of prescriptive or other similar rights in or over the Property.

20.3 Liberal Construction. Any general rule of construction to the contrary notwithstanding, this Easement shall be liberally construed to effect the Purpose of this Easement and the policy and purpose of California Civil Code §815 *et seq.*, while recognizing Grantor's reserved rights. If any provision in this Easement is found to be ambiguous, an interpretation consistent with the Purpose of this Easement, which recognizes Grantor's reserved rights and that would render the provision valid shall be favored over any interpretation that would render it invalid.

20.4 Further Assurances. Each party hereto agrees to execute and deliver to the other party such further documents or instruments as may be necessary

or appropriate in order to carry out the intentions of the parties as contained in this Easement.

20.5 Severability. If any provision of this Easement shall be unenforceable or invalid, the same shall not affect the remaining provisions of this Easement, and to this end the provisions of this Easement are intended to be and shall be severable.

20.6 Entire Agreement. This Easement sets forth the entire agreement of the parties with respect to the Conservation Easement and supersedes all prior discussions, negotiations, understandings, or agreements relating to the Conservation Easement, all of which are merged herein.

20.7 No Forfeiture. Nothing contained herein will result in a forfeiture or reversion of Grantor's title in any respect.

20.8 Successors. The easement created by this instrument shall be a servitude running with the land in perpetuity. The covenants, terms, conditions, and restrictions of this Easement shall be binding upon, and inure to the benefit of, the parties hereto and their respective successors and assigns and shall run with the Property. However, this Easement shall not create or bestow any lien or property right in any third party. Grantor and Grantee agree that no third party beneficiary to this Easement exists and that nothing contained herein shall be construed as giving any person third party beneficiary status or any right of enforcement hereunder.

20.9 Recordation. Grantee shall promptly record this Easement in the official records of the County in which the Property is located, and shall thereafter promptly provide to Grantor a copy hereof showing the recording information. Grantee may re-record this Easement at any time as may be required to preserve its rights in this Easement.

20.10 Termination of Rights and Obligations. A party's rights and obligations under this Easement shall terminate only upon transfer of the party's interest in all or portions of either the Conservation Easement or the Property, except that liability for acts or omissions occurring prior to transfer shall survive the transfer.

20.11 Attorneys' Fees. In the event that any party shall bring an action to enforce its rights under this Easement, or relating to the interpretation hereof, whether for declaratory, injunctive or other relief, the prevailing party in any such proceeding shall be entitled to recover from the other party reasonable attorneys' fees and all costs, expenses and disbursements that the prevailing party incurred in connection with such proceeding, including appeals, remands and any other subsequent proceeding (including, but not limited to, the reasonable costs of discovery, investigation, preparation for trial, professional or expert consultation and testimony). A party shall be deemed to have prevailed in any such action (without limiting the generality of the foregoing) if such action is dismissed upon the payment by the other party of the sums allegedly due or the performance of obligations allegedly not complied with, or if such

party obtains substantially the relief sought by it in the action, irrespective of whether such action is prosecuted to judgment. The non-prevailing party shall also pay the attorneys' fees and costs incurred by the prevailing party in any post-judgment proceedings to collect and enforce the judgment. Any such fees and costs incurred prior to judgment, award, or decree may be included in any judgment, award or decree entered in such proceeding in favor of the prevailing party. Any such fees, costs and expenses incurred by the prevailing party in enforcing a judgment, award or decree in its favor shall be recoverable separately from and in addition to any other amount included in such judgment, award or decree. This provision is separate and several and shall survive the merger of this Easement into any judgment on this Easement.

20.12 Mortgage Liens Subordinate. No provision of this Easement is to be construed as impairing the ability of Grantor to use the Property as collateral for any loan, provided that any lien securing such loan (a "**Mortgage Lien**"), regardless of date, shall be subordinate to the terms of this Easement and Grantee's rights under this Easement. Under no circumstances may Grantee's rights be extinguished or otherwise affected by the recording, foreclosure, or any other action taken concerning any Mortgage Lien.

20.13 Pre-Existing Water Rights. In accordance with Section 12(e) of the Stipulation, this Easement does not impact the authority of third-party holders of water rights to exercise those rights.

20.14 Table of Contents and Captions. The table of contents and captions in this Easement have been inserted solely for convenience of reference and are not a part of this Easement and shall have no effect upon construction or interpretation.

20.15 Incorporation of Recitals. All Recitals are incorporated herein by this reference.

20.16 List of Exhibits. The following exhibits are attached hereto and incorporated herein by this reference:

| | |
|---|---|
| Exhibit A | Property Description |
| Exhibit B | Description of Hydro Project Activities and Hydroelectric Facilities and Associated Water Delivery Facilities |
| Exhibit C | Hydro Reserved Rights |
| Exhibit D | Beneficial Public Values |
| Exhibit E | Insurance Requirements |
| Exhibit F | Prohibited Uses |
| Exhibit G | Hydro Operating Zone(s) |
| Exhibit H | Express Third Party Uses and Third Party Use Agreements |
| Exhibit I | Expressly Permitted Uses |

Exhibit J New FERC Project License – Potential Removal of Project Dams

20.17 Counterparts. This Easement may be executed in two or more counterparts, each of which shall be deemed an original, but all of which together shall constitute one and the same instrument. In the event of a discrepancy between counterparts, the recorded Easement shall be controlling.

[Signature page follows]

IN WITNESS WHEREOF, Grantor has granted to Grantee, and Grantee has accepted, this Easement, and the parties mutually agree to the terms and covenants set forth above, as of the Effective Date.

GRANTOR:

PACIFIC GAS AND ELECTRIC COMPANY,
a California corporation

By: _____

Andrew K. Williams
Vice President
Shared Services

GRANTEE:

MENDOCINO LAND TRUST
INCORPORATED,
a California nonprofit corporation

By: _____

Conrad James Kramer
Executive Director

A notary public or other officer completing this certificate verifies only the identity of the individual who signed the document to which this certificate is attached, and not the truthfulness, accuracy, or validity of that document.

State of California )
County of Mendocino )

On May 27, 2022, before me, Sandy Soria, a Notary Public, personally appeared Conrad James Kramer, who proved to me on the basis of satisfactory evidence to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct.

WITNESS my hand and official seal.

Signature Sandy Soria

SANDY SORIA
Notary Public - California
Mendocino County
Commission # 2362802
My Comm. Expires Jul 24, 2025

A notary public or other officer completing this certificate verifies only the identity of the individual who signed the document to which this certificate is attached, and not the truthfulness, accuracy, or validity of that document.

State of California )
County of Contra Costa )

On June 17, 2022, before me, Paul Coviello, a Notary Public, personally appeared Andrew K. Williams, who proved to me on the basis of satisfactory evidence to be the person~~(s)~~ whose name~~(s)~~ is/~~are~~ subscribed to the within instrument and acknowledged to me that he/~~she/they~~ executed the same in his/~~her/their~~ authorized capacity~~(ies)~~, and that by his/~~her/their~~ signature~~(s)~~ on the instrument the person~~(s)~~, or the entity upon behalf of which the person~~(s)~~ acted, executed the instrument.

I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct.

WITNESS my hand and official seal.

PAUL COVIELLO
Notary Public - California
Contra Costa County
Commission # 2272622
My Comm. Expires Dec 23, 2022

Signature Paul Coviello

02898.357 4825-6416-5970.7

EXHIBIT A

Legal Description of Property

[Follows this page]

Order Number: 1415-14284.2

**EXHIBIT "A"**
**LEGAL DESCRIPTION**

THE LAND DESCRIBED HEREIN IS SITUATED IN THE STATE OF CALIFORNIA, COUNTIES OF LAKE AND MENDOCINO, UNINCORPORATED AREA, AND IS DESCRIBED AS FOLLOWS:

PARCEL 1:
LCP ID#0739

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 31, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN LIBER 99, PAGE 546 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE NORTHEAST ONE-QUARTER, THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER, AND THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 31.
A.P.N. 172-290-02, 172-300-04

PARCEL 2A:
LCP ID # 740 PORTION

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 26 AND 35, TOWNSHIP 18 NORTH, RANGE 11 WEST, OF THE MOUNT DIABLO BASE AND MERIDIAN AS RECORDED IN BOOK 60 PAGE 531 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE WEST ONE-HALF OF THE SOUTHEAST ONE-QUARTER, AND THE EAST ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 26; AND THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 35;

EXCEPTING THEREFROM ALL THAT PORTION OF THE AFORE DESCRIBED LANDS LYING NORTHERLY OF A LINE PARALLEL WITH, AND DISTANT 30 FEET NORTHERLY, AS MEASURED AT RIGHT ANGLES, FROM THE CENTERLINE OF THE EEL RIVER ROAD (SO NAMED WITHIN MENDOCINO COUNTY, SAID ROAD BEING NAMED ELK MOUNTAIN ROAD WITHIN LAKE COUNTY), THE CENTERLINE OF WHICH BEING MORE GENERALLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHWEST CORNER OF SECTION 34 TOWNSHIP 18 NORTH, RANGE 11 WEST, OF THE MOUNT DIABLO BASE AND MERIDIAN, THENCE SOUTHERLY ALONG THE WEST LINE OF SAID SECTION S.3° 57'16"W., 1009.13 FEET TO THE CENTERLINE OF EEL RIVER ROAD (SAID ROAD BEING NAMED ELK MOUNTAIN ROAD WITHIN LAKE COUNTY), THENCE ALONG THE CENTERLINE OF SAID ROAD THE FOLLOWING COURSES:

S.79° 23'41"E., 28.84 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 38'39", A DISTANCE OF 30.79 FEET;

THENCE S.61° 45'02"E., 100.55 FEET;

THENCE S.67° 12'50"E., 87.31 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 38° 38'41", A DISTANCE OF 67.45 FEET;

THENCE S.28° 34'09"E., 66.81 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 27° 39'02", A DISTANCE OF 48.26 FEET;

THENCE S.56° 13'11"E., 66.09 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 11° 56'08", A DISTANCE OF 41.66 FEET TO THE NORTHERLY LINE OF SAID SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 34, BEING THE TRUE POINT OF BEGINNING, THENCE CONTINUING ALONG THE CENTERLINE OF SAID ROAD S.44° 17'03"E., 311.73 FEET;

THENCE ALONG THE ARC OF A 125 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 49° 41'34", A DISTANCE OF 108.41 FEET;

THENCE S.5° 24'31"W., 200.50 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 21° 10'54", A DISTANCE OF 73.94 FEET;

THENCE S.15° 46'22"E., 52.38 FEET;

THENCE ALONG THE ARC OF A 150 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 73° 24'50", A DISTANCE OF 192.20 FEET;

THENCE S.89° 11'13"E., 94.64 FEET;

THENCE N.84° 35'45"E., 79.12 FEET;

THENCE S.85° 43'50"E., 120.41 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 37° 28'15", A DISTANCE OF 130.80 FEET;

THENCE N.56° 47'56"E., 181.50 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 41'56", A DISTANCE OF 82.20 FEET;

THENCE N.72° 29'52"E., 226.17 FEET;

THENCE ALONG THE ARC OF A 75 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 47° 07'21", A DISTANCE OF 61.68 FEET;

THENCE N.25° 22'31"E., 80.62 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 82° 02'48", A DISTANCE OF 143.20 FEET;

THENCE S.72° 34'42"E., 72.69 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 57° 57'38", A DISTANCE OF 101.16 FEET;

THENCE N.49° 27'40"E., 221.43 FEET;

THENCE N.45° 30'03"E., 342.16 FEET;

THENCE N.48° 07'37"E., 236.28 FEET;

THENCE N.47° 51'31"E., 319.26 FEET;

THENCE N.52° 38'18"E., 227.20 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 48° 41'42", A DISTANCE OF 169.98 FEET;

THENCE S.78° 40'00"E., 215.67 FEET;

THENCE S.85° 31'18"E., 196.81 FEET;

THENCE S.84° 57'48"E., 133.77 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 29'00", A DISTANCE OF 135.12 FEET;

THENCE S.69° 28'48"E., 253.56 FEET;

THENCE S76° 16'11"E., 242.40 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 17° 45'40", A DISTANCE OF 154.99 FEET;

THENCE N.85° 58'10"E., 112.91 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 9° 07'43", A DISTANCE OF 63.73 FEET;

THENCE S.84° 54'07"E., 72.20 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 17° 11'05", A DISTANCE OF 119.97 FEET;

THENCE N.77° 54'47" E., 224.51 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 24'00", A DISTANCE OF 107.51 FEET;

THENCE S.86° 41'13"E. 84.37 FEET;

THENCE N.85° 54'13"E., 135.18 FEET;

THENCE N.85° 45'19"E., 269.61 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 29° 00'37", A DISTANCE OF 253.16 FEET;

THENCE N.56° 44'42"E., 135.71 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 07'47", A DISTANCE OF 86.69 FEET;

THENCE N.73° 52'28"E., 182.07 FEET;

THENCE N.71° 01'41"E., 225.64 FEET;

THENCE ALONG THE ARC OF A 250 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 19° 46'36", A DISTANCE OF 86.29 FEET;

THENCE N.51° 15'05"E., 53.82 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 25° 50'31", A DISTANCE OF 90.21 FEET;

THENCE N.25° 24'34"E., 15.25 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 27° 41'06", A DISTANCE OF 48.32 FEET;

THENCE N.53° 05'40"E., 144.67 FEET;

THENCE N.55° 07'35"E., 75.08 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 12'12", A DISTANCE OF 70.52 FEET;

THENCE N.34° 55'23"E., 58.18 FEET;

THENCE N.21° 53'09"E., 56.35 FEET;

THENCE N.27° 13'04"E., 111.53 FEET;

THENCE N.29° 44'08"E., 196.00 FEET;

THENCE N.33° 55'54"E., 113.95 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 37° 47'07", A DISTANCE OF 197.84 FEET;

THENCE N.3° 51'13"W., 37.41 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 26° 45'41", A DISTANCE OF 93.42 FEET;

THENCE N.22° 54'28"E., 32.35 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 16° 13'00", A DISTANCE OF 84.91 FEET;

THENCE N.6° 41'29"E., 13.27 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 49° 15'08", A DISTANCE OF 171.92 FEET;

THENCE N.55° 56'37"E., 49.24 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 23° 16'56", A DISTANCE OF 81.27 FEET;

THENCE N.32° 39'41"E., 183.14 FEET;

THENCE N.30° 25'27"E., 235.16 FEET;

THENCE ALONG THE ARC OF A 150 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 50° 25'05", A DISTANCE OF 131.99 FEET;

THENCE N.80° 50'32"E., 49.72 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 24° 44'45", A DISTANCE OF 43.19 FEET;

THENCE N.56° 05'47"E., 129.12 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 58'18", A DISTANCE OF 73.21 FEET;

THENCE N.35° 07'29"E., 250.33 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 27° 57'05", A DISTANCE OF 97.57 FEET;

THENCE N.7° 10'24"E., 111.84 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 24° 03'55", A DISTANCE OF 84.00 FEET;

THENCE N.31° 14'19"E., 147.14 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 26° 30'23", A DISTANCE OF 138.79 FEET;

THENCE N.4° 43'56"E., 165.63 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 70° 28'10", A DISTANCE OF 122.99 FEET;

THENCE N.75° 12'07"E., 43.14 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 42° 37'10", A DISTANCE OF 74.39 FEET;

THENCE N.32° 34'56"E., 202.60 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 67° 13'02", A DISTANCE OF 117.32 FEET;

THENCE S.80° 12'01"E., 40.45 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 44° 03'36", A DISTANCE OF 76.90 FEET;

THENCE N.55° 44'23"E., 70.33 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 66° 40'27", A DISTANCE OF 116.37 FEET;

THENCE N.10° 56'04"W., 261.41 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 55'48", A DISTANCE OF 36.53 FEET;

THENCE N.31° 51'52"W., 72.93 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 21'55", A DISTANCE OF 30.31 FEET;

THENCE N.14° 29'58"W., 129.97 FEET;

THENCE ALONG THE ARC OF A 75 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 85° 30'59", A DISTANCE OF 111.94 FEET;

THENCE N.71° 01'01"E., 13.82 FEET;

THENCE ALONG THE ARC OF A 50 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 63° 02'47", A DISTANCE OF 55.02 FEET;

THENCE S.45° 56'12"E., 70.60 FEET;

THENCE ALONG THE ARC OF A 350 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 40° 55'35", A DISTANCE OF 250 FEET, TO A POINT ON THE EAST LINE OF THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 26, FROM WHICH THE NORTHEAST CORNER OF THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION BEARS N.70° 58'07"E., A DISTANCE OF 1344.07 FEET MORE OR LESS, SAID POINT BEING THE POINT OF TERMINATION FOR THIS DESCRIPTION.

ALSO EXCEPTING THEREFROM THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER LYING SOUTHEASTERLY OF THE CENTER OF THE COUNTY ROAD TO LAKE PILLSBURY, BEING A PORTION OF SECTION 26, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO MERIDIAN, AS RECORDED IN DOCUMENT NUMBER 02-022795, RECORDS OF THE COUNTY OF LAKE.

THE BEARINGS AND DISTANCES USED IN THE ABOVE DESCRIPTION ARE GRID, BASED ON THE CALIFORNIA COORDINATE SYSTEM OF 1983, (CCS83), ZONE 2 NAD 83 (NSRS 2007) (EPOCH 2011.00). TO OBTAIN GEODETIC BEARINGS, ROTATE GRID BEARINGS BY 0° 40'40" COUNTERCLOCKWISE. TO OBTAIN GROUND DISTANCES, DIVIDE GRID DISTANCES BY A COMBINED SCALE FACTOR OF 0.9998297205.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED JULY 24, 2019 AS DOCUMENT NO. 2019009002 OFFICIAL RECORDS OF LAKE COUNTY.

APN: 001-031-002 PORTION, 001-031-024 PORTION

PARCEL 3:
LCP ID#741

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 35, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER AND THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 35.
A.P.N. 001-031-002 PORTION

PARCEL 4:
LCP ID# 742

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 13, 23, 24, 25, AND 26, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE EAST ONE-HALF OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 13;
THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 23;
THE NORTH ONE-HALF OF THE NORTHEAST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER, THE NORTH ONE-HALF OF THE SOUTHWEST ONE-QUARTER, AND THE SOUTHWEST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 24;
THE NORTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 25;
THE EAST ONE-HALF OF THE NORTHEAST ONE-QUARTER, THE NORTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER, AND ALSO ALL THAT PORTION OF THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER WHICH LIES EAST AND SOUTH OF A STRAIGHT LINE DRAWN FROM THE NORTHEAST CORNER OF SAID SUBDIVISION DIRECTLY TO THE SOUTHWEST QUARTER CORNER THEREOF, ALL OF SAID SECTION 26.

A.P.N. 001-030-009, 001-031-002 PORTION

PARCEL 5A:
LCP ID # 743 PORTION

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 16, 17, 20,21, 22, 27 AND 28, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO MERIDIAN, AS RECORDED FEBRUARY 28, 1924, IN BOOK 65 AT PAGE 448 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTH ONE-HALF AND THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 16;

THE SOUTHEAST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, AND THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 17;

THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER AND THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 20;

THE NORTH ONE-HALF OF THE NORTH ONE-HALF, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER, THE WEST ONE-HALF OF THE SOUTHEAST ONE-QUARTER, AND THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 21;

THE NORTH ONE-HALF OF THE NORTHWEST ONE-QUARTER, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, AND THE WEST ONE-HALF OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 22;

THE WEST ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 27;

THE NORTHEAST ONE-QUARTER, THE EAST ONE-HALF OF THE NORTHWEST ONE-QUARTER, AND THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 28.

EXCEPTING THEREFROM ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SAID SECTION 16, AS DESCRIBED IN THE UNRECORDED DEED FROM PACIFIC GAS AND ELECTRIC COMPANY TO GEORGE R. WINKLER, DATED MAY 11, 1945, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE NORTH ONE-HALF OF THE SOUTHEAST ONE-QUARTER AND THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 16.

ALSO EXCEPTING THEREFROM
THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER AND SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 20;
THE SOUTHWEST ONE-QUARTER, THE WEST ONE-HALF OF THE SOUTHEAST ONE-QUARTER, AND THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 21;
THE WEST ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 27;
THE NORTHEAST ONE-QUARTER, THE EAST ONE-HALF OF THE NORTHWEST ONE-QUARTER, AND THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 28.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED OCTOBER 1, 2021 AS DOCUMENT NO. 2021016684 OFFICIAL RECORDS OF LAKE COUNTY.

APN: 001-034-002 PORTION, 001-034-007 PORTION AND 001-035-004

PARCEL 6:
LCP ID# 0747

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 22, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED SEPTEMBER 2, 1907 IN BOOK 39, PAGE 585 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE EAST ONE-HALF OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 22.
A.P.N. 001-035-004 PORTION

PARCEL 7A:
LCP ID # 748 PORTION

ALL THOSE CERTAIN PARCELS OF LAND SITUATE IN SECTIONS 28, 29, 30, 32, 33, 34, AND 35, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN LIBER 99, PAGE 546 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTH ONE-HALF OF THE SOUTH ONE-HALF OF SAID SECTION 28; THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER; THE SOUTH ONE-HALF OF THE SOUTHEAST ONE-QUARTER, THE NORTHWEST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER; THE SOUTH ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 29, LESS A PORTION OF THE NORTHWEST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 29 CONVEYED BY THE PARTY OF THE FIRST PART TO THOMAS D. ORR BY DEED DATED FEBRUARY 3RD, 1906; THE SOUTHEAST ONE-QUARTER, AND ALL THAT PORTION OF THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 30 THAT LIES WEST AND SOUTH OF A STRAIGHT LINE DRAWN FROM THE NORTHWEST CORNER OF SAID SUBDIVISION DIRECTLY TO THE SOUTHEAST CORNER THEREOF, THE NORTH ONE-HALF OF THE NORTHWEST ONE-QUARTER, AND THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, OF SAID SECTION 32, ALSO ALL THAT PORTION OF THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 32 THAT LIES WEST AND NORTH OF THE COUNTY ROAD LEADING FROM POTTER VALLEY TO GRAVELLY VALLEY; THE NORTH ONE-HALF OF THE NORTHEAST ONE-QUARTER, AND THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, OF SAID SECTION THIRTY THREE 33; THE NORTHEAST ONE-QUARTER, THE EAST ONE-HALF OF THE NORTHWEST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, AND THE NORTHWEST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER, OF SAID SECTION 34; THE NORTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER; THE SOUTH ONE-HALF OF THE NORTHWEST ONE-QUARTER; THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER, THE NORTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, OF SAID SECTION 35.

EXCEPTING THEREFROM ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SAID SECTIONS 29, 30, AND 32, AS RECORDED IN BOOK 1797, PAGE 706 OF OFFICIAL RECORDS OF THE SAID COUNTY OF MENDOCINO, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

COMMENCING AT THE SOUTHEAST CORNER OF SAID SECTION 29, AND RUNNING THENCE (A) SOUTH 89° 32'43" WEST 1328.89 FEET ALONG THE SOUTH LINE OF SAID SECTION 29 TO THE EAST 1/16 CORNER COMMON TO SECTIONS 29 AND 32; THENCE (B) NORTH 0° 06' 21" WEST 464.28 FEET LEAVING SAID SOUTH LINE AND RUNNING ALONG THE EAST 1/16 LINE OF SAID SECTION 29 TO THE TRUE POINT OF BEGINNING OF THE HEREINAFTER DESCRIBED PARCEL; THENCE LEAVING SAID EAST 1/16 LINE AND RUNNING ALONG A LINE WHICH LIES 30.00 FEET NORTHERLY OF AND PARALLEL TO THE EXISTING CENTERLINE OF EEL RIVER ROAD THE FOLLOWING COURSES:

(1) FROM A TANGENT BEARING SOUTH 64° 36' 02" WEST ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 930 FEET AND A CENTRAL ANGLE OF 6° 20' 27", AN ARC DISTANCE OF 102.92 FEET; THENCE

(2) SOUTH 58° 15' 35" WEST 196.33 FEET; THENCE

(3) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 900 FEET AND A CENTRAL ANGLE OF 5° 19' 02", AN ARC DISTANCE OF 83.52 FEET; THENCE

(4) SOUTH 63° 34' 37" WEST 491.36 FEET, THENCE

(5) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 2030.00 FEET AND A CENTRAL ANGLE OF 5° 07' 25", AN ARC DISTANCE OF 181.53 FEET; THENCE

(6) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 688.37 FEET AND A CENTRAL ANGLE OF 24° 39' 30", AN ARC DISTANCE OF 296.25 FEET; THENCE

(7) SOUTH 33° 47' 43" WEST 142.07 FEET; THENCE

(8) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 655.00 FEET AND A CENTRAL ANGLE OF 15° 48' 45", AN ARC DISTANCE OF 180.77 FEET; THENCE

(9) SOUTH 17° 58' 58" WEST 125.57 FEET; THENCE

(10) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 70.00 FEET AND A CENTRAL ANGLE OF 25° 33' 52", AN ARC DISTANCE OF 31.23 FEET; THENCE

(11) SOUTH 43° 32' 50" WEST 119.96 FEET; THENCE

(12) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 555.00 FEET AND A CENTRAL ANGLE OF 23° 56' 47", AN ARC DISTANCE OF 231.96 FEET; THENCE

(13) SOUTH 67 29' 38" WEST 212.11 FEET; THENCE

(14) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 970. 00 FEET AND A CENTRAL ANGLE OF 7° 09' 58", AN ARC DISTANCE OF 121.32 FEET, THENCE

(15) SOUTH 74° 39' 35" WEST 142.01 FEET; THENCE

(16) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 340.00 FEET AND A CENTRAL ANGLE OF 36° 48' 30", AN ARC DISTANCE OF 218.43 FEET; THENCE

(17) NORTH 68° 31' 54" WEST 27.54 FEET; THENCE

(18) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 180.00 FEET AND A CENTRAL ANGLE OF 19° 15' 28", AN ARC DISTANCE OF 60.50 FEET; THENCE

(19) NORTH 49° 16' 26" WEST 110.61 FEET; THENCE

(20) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 1430.00 FEET AND A CENTRAL ANGLE OF 12° 55' 27", AN ARC DISTANCE OF 322.56 FEET; THENCE

(21) NORTH 62° 11' 53" WEST 319.59 FEET; THENCE

(22) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 1520.00 FEET AND A CENTRAL ANGLE OF 15° 21' 15", AN ARC DISTANCE OF 407.33 FEET; THENCE

(23) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 2503.90 FEET AND A CENTRAL ANGLE OF 9° 56' 29", AN ARC DISTANCE OF 434.35 FEET; THENCE

(24) NORTH 56° 47' 07" WEST 197.80 FEET; THENCE

(25) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 3030.00 FEET AND A CENTRAL ANGLE OF 3° 28' 25", AN ARC DISTANCE OF 183.69 FEET; THENCE

(26) NORTH 60° 15' 32" WEST 137.84 FEET; THENCE

(27) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 820. 00 FEET AND A CENTRAL ANGLE OF 13° 36' 46", AN ARC DISTANCE OF 194.82 FEET; THENCE

(28) NORTH 46° 38' 46" WEST 111.74 FEET; THENCE

(29) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 66.40 FEET AND A CENTRAL ANGLE OF 61° 27' 56", AN ARC DISTANCE OF 71.24 FEET TO A POINT ON A LINE LYING 30.00 FEET NORTHERLY OF AND PARALLEL TO THE EXISTING CENTERLINE OF VAN ARSDALE ROAD; THENCE ALONG SAID LINE 30.00 FEET NORTHERLY OF AND PARALLEL TO THE EXISTING CENTERLINE OF VAN ARSDALE ROAD THE FOLLOWING COURSES:

(30) NORTH 41° 59' 43" WEST 458.05 FEET; THENCE

(31) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 200.00 FEET AND A CENTRAL ANGLE OF 21° 15' 18", AN ARC DISTANCE OF 74.19 FEET; THENCE

(32) NORTH 20° 44' 24" WEST 60.58 FEET; THENCE

(33) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 340.00 FEET AND A CENTRAL ANGLE OF 14° 28' 47", AN ARC DISTANCE OF 85.93 FEET; THENCE

(34) NORTH 35° 13' 12" WEST 21.97 FEET; THENCE

(35) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 220.00 FEET AND A CENTRAL ANGLE OF 14 ° 28' 20", AN ARC DISTANCE OF 55.57 FEET; THENCE

(36) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 1030.93 FEET AND A CENTRAL ANGLE OF 7° 06' 07", AN ARC DISTANCE OF 127.79 FEET; THENCE

(37) NORTH 27° 50' 59" WEST 10.23 FEET; THENCE

(38) ALONG A CURVE TO THE RIGHT, WITH A RADIUS OF 220.00 FEET AND A CENTRAL ANGLE OF 18° 05' 03", AN ARC DISTANCE OF 69.44 FEET; THENCE

(39) NORTH 9° 45' 56" WEST 51.74 FEET; THENCE

(40) ALONG A CURVE TO THE LEFT, WITH A RADIUS OF 130.00 FEET AND A CENTRAL ANGLE OF 20° 37' 23", AN ARC DISTANCE OF 46.79 FEET; THENCE LEAVING SAID LINE LYING 30 FEET NORTHERLY OF AND PARALLEL TO THE EXISTING CENTERLINE OF VAN ARSDALE ROAD; THENCE

(41) NORTH 56° 27' 18" EAST 111.52 FEET; THENCE

(42) NORTH 56° 27' 18" EAST 158.27 FEET; MORE OR LESS, TO THE CONTOUR LINE WITH AN ELEVATION OF 1495.3 FEET ON THE SOUTHERLY BANK OF THE VAN ARSDALE RESERVOIR (VERTICAL DATUM FROM U.S.C.&G.S. MONUMENTS N1062 AND S1062), THENCE ALONG SAID CONTOUR LINE THE FOLLOWING COURSES:

(43) SOUTH 63° 31' 11" EAST 93.74 FEET; THENCE

(44) SOUTH 63° 31' 11" EAST 230.78 FEET; THENCE

(45) SOUTH 67° 36' 39" EAST 197.20 FEET; THENCE

(46) SOUTH 76° 05' 01" EAST 146.77 FEET; THENCE

(47) SOUTH 64° 58' 08" EAST 240.15 FEET; THENCE

(48) SOUTH 69° 07' 21" EAST 272.10 FEET; THENCE

(49) SOUTH 53° 05' 56" EAST 428.51 FEET; THENCE

(50) SOUTH 81° 13' 25" EAST 163.98 FEET; THENCE

(51) NORTH 75° 28' 56" EAST 238.09 FEET; THENCE

(52) NORTH 66° 32' 13" EAST 295.75 FEET; THENCE

(53) NORTH 80° 26' 23" EAST 179.17 FEET; THENCE

(54) NORTH 89° 59' 30" EAST 252.00 FEET; THENCE

(55) SOUTH 88° 19' 48" EAST 240.24 FEET; THENCE

(56) SOUTH 80° 01' 54" EAST 620.13 FEET; THENCE

(57) NORTH 82° 14' 56" EAST 385.88 FEET; THENCE

(58) SOUTH 74° 52' 58" EAST 317.77 FEET; THENCE

(59) SOUTH 77° 48' 53" EAST 247.96 FEET; THENCE

(60) SOUTH 74° 20' 34" EAST 403.33 FEET; THENCE

(61) NORTH 88° 34' 46" EAST 46.05 FEET, MORE OR LESS,

TO THE EAST 1/16 LINE OF SAID SECTION 29; THENCE

(62) SOUTH 0° 06' 21" EAST 137.29 FEET LEAVING SAID CONTOUR LINE, AND RUNNING ALONG SAID EAST 1/16 LINE TO THE POINT OF BEGINNING.

ALSO EXCEPTING THEREFROM ALL THAT PORTION OF THE SOUTH ONE-HALF OF THE SOUTH ONE-HALF OF SECTION 28, THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SECTION 29, AND THE NORTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SECTION 33, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, IN THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE NORTHWEST CORNER OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 29, THENCE ALONG THE WEST LINE OF SAID SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION S.0° 35'40"W., 28.00 FEET TO THE CENTERLINE OF EEL RIVER ROAD, THENCE ALONG THE CENTERLINE OF EEL RIVER ROAD THE FOLLOWING COURSES:

S.88° 50'29" E., 78.17 FEET;

THENCE S. 73° 17'04" E., 536.17 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 7° 39'15", A DISTANCE OF 53.44 FEET;

THENCE S. 80° 56'19" E., 341.56 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 8° 26'41", A DISTANCE OF 58.96 FEET;

THENCE S. 72° 29'38" E., 96.43 FEET;

THENCE S. 69° 08'36" E., 192.66 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 5° 44'30", A DISTANCE OF 40.09 FEET;

THENCE S. 74° 53'06" E., 189.25 FEET;

THENCE S. 73° 41'59" E., 176.33 FEET;

THENCE S. 78° 22'05" E., 256.11 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 6° 00'57", A DISTANCE OF 31.50 FEET;

THENCE S. 72° 21'08" E., 202.07 FEET;

THENCE S. 70° 36'23" E., 289.86 FEET;

THENCE ALONG THE ARC OF A 320 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 13° 52'14", A DISTANCE OF 77.47 FEET;

THENCE S. 84° 28'37" E., 160.41 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 18° 57'17", A DISTANCE OF 99.25 FEET;

THENCE S. 65° 31'20" E., 207.43 FEET;

THENCE ALONG THE ARC OF A 380 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 32° 16'51", A DISTANCE OF 214.09 FEET;

THENCE N. 82° 11'49" E., 53.92 FEET;

THENCE ALONG THE ARC OF A 550 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 5° 20'15", A DISTANCE OF 51.24 FEET;

THENCE N. 87° 32'04" E., 21.16 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 12° 04'01", A DISTANCE OF 42.12 FEET;

THENCE N. 75° 28'04" E., 109.14 FEET;

THENCE ALONG THE ARC OF A 600 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 18° 19'30", A DISTANCE OF 191.90 FEET;

THENCE S. 86° 12'27" E., 264.77 FEET;

THENCE ALONG THE ARC OF A 800 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 21° 37'50", A DISTANCE OF 302.02 FEET;

THENCE S. 64° 34'37" E., 288.94 FEET;

THENCE ALONG THE ARC OF A 240 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 27° 50'56", A DISTANCE OF 116.65 FEET;

THENCE N. 87° 34'27" E., 111.84 FEET;

THENCE ALONG THE ARC OF A 250 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 30° 27'37", A DISTANCE OF 132.91 FEET;

THENCE S. 61° 57'55" E., 64.45 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 24° 37'43", A DISTANCE OF 171.94 FEET;

THENCE S. 37° 20'12" E., 110.07 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 18° 11'20", A DISTANCE OF 95.24 FEET;

THENCE S. 55° 31'32" E., 243.85 FEET;

THENCE ALONG THE ARC OF A 310 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 1° 55'18", A DISTANCE OF 10.40 FEET;

THENCE S. 57° 26'50" E., 55.62 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 31° 55'41", A DISTANCE OF 111.45 FEET;

THENCE S. 25° 31'09" E., 85.79 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 14° 22'18", A DISTANCE OF 75.25 FEET;

THENCE S. 39° 53'26" E., 55.38 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 16° 16'30", A DISTANCE OF 85.22 FEET;

THENCE S. 23° 36'56" E., 242.51 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 9° 18'13", A DISTANCE OF 16.24 FEET;

THENCE S. 14° 18'43" E., 351.99 FEET;

THENCE S. 5° 37'32" E., 60.91 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 41° 49'50", A DISTANCE OF 146.02 FEET;

THENCE S. 47° 27'21" E., 91.68 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 35° 26'53", A DISTANCE OF 61.87 FEET;

THENCE S. 82° 54'14" E., 26.99 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 31° 40'54", A DISTANCE OF 55.29 FEET;

THENCE S. 51° 13'20" E., 106.45 FEET;

THENCE ALONG THE ARC OF A 120 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 65° 19'31", A DISTANCE OF 136.82 FEET;

THENCE N. 63° 27'09" E. 54.64 FEET;

THENCE ALONG THE ARC OF A 80 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 57° 41'28", A DISTANCE OF 80.55 FEET;

THENCE S. 58° 51'23" E., 47.24 FEET;

THENCE ALONG THE ARC OF A 125 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 35° 49'16", A DISTANCE OF 78.15 FEET;

THENCE N. 85° 19'21" E., 7.71 FEET;

THENCE ALONG THE ARC OF A 150 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 16'58", A DISTANCE OF 40.01 FEET TO A POINT ON THE EAST LINE OF SAID SECTION 33, THENCE LEAVING SAID CENTERLINE ALONG SAID EAST LINE N. 3° 57'16" E., 1009.13 FEET TO THE NORTHEAST CORNER OF SAID SECTION, THENCE ALONG THE EAST LINE OF SAID SECTION 28 N. 5° 05'46" E. 1292.84 FEET TO THE NORTHEAST CORNER OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION; THENCE ALONG THE NORTH LINE OF SAID SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER N. 84° 11'43" W., 1372.84 FEET TO THE NORTHWEST CORNER OF SAID SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER; THENCE ALONG THE WEST LINE OF SAID SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF S.3° 36'19" W., 310.77 FEET; THENCE LEAVING SAID WEST LINE N. 88° 12'17" W., 1366.21 FEET TO A 1/2" IRON PIPE AS SHOWN ON THAT RECORD OF SURVEY FILED FOR RECORD IN MAP CASE 2, DRAWER 22, AT PAGE 3, MENDOCINO COUNTY RECORDS, THENCE N. 75° 43'37" W., 2754.51 FEET TO A 1/2" IRON PIPE AS SHOWN ON THAT PARCEL MAP FILED FOR RECORD IN MAP CASE 2, DRAWER 17, AT PAGE 83, MENDOCINO COUNTY RECORDS, MARKING THE NORTHEAST CORNER OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 29; THENCE ALONG THE NORTH LINE OF SAID SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF N. 89° 46'28" W. 1340.79 FEET TO THE POINT OF BEGINNING.

THE BEARINGS AND DISTANCES USED IN THE ABOVE DESCRIPTION ARE GRID, BASED ON THE CALIFORNIA COORDINATE SYSTEM OF 1983 (CCS83), ZONE 2 NAD 83 (NSRS 2007) (EPOCH 2011.00). TO OBTAIN GEODETIC BEARINGS, ROTATE GRID BEARINGS 0° 40'40"

COUNTERCLOCKWISE. TO OBTAIN GROUND DISTANCES, DIVIDE GRID DISTANCES BY A COMBINED SCALE FACTOR OF 0.9998297205.

ALSO EXCEPTING THEREFROM ALL THAT PORTION OF THE AFORE DESCRIBED LANDS LYING SOUTHERLY OF A LINE PARALLEL WITH, AND DISTANT 30 FEET NORTHERLY, AS MEASURED AT RIGHT ANGLES, FROM THE CENTERLINE OF AFORE DESCRIBED EEL RIVER ROAD.

SAID PARALLEL LINE IS TO BE LENGTHENED OR SHORTENED AS TO BEGIN ON THE NORTHERLY LINE OF THE SOUTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 29, AND END ON THE EASTERLY LINE OF THE NORTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 33.

ALSO EXCEPTING THEREFROM ALL THAT PORTION OF SECTION 26, 34, AND 35, TOWNSHIP 18 NORTH, RANGE 11 WEST, OF THE MOUNT DIABLO BASE AND MERIDIAN IN THE COUNTIES OF MENDOCINO AND LAKE, STATE OF CALIFORNIA, LYING NORTHERLY OF A LINE PARALLEL WITH, AND DISTANT 30 FEET NORTHERLY, AS MEASURED AT RIGHT ANGLES, FROM THE CENTERLINE OF THE EEL RIVER ROAD (SO NAMED WITHIN MENDOCINO COUNTY, SAID ROAD BEING NAMED ELK MOUNTAIN ROAD WITHIN LAKE COUNTY), THE CENTERLINE OF WHICH BEING MORE GENERALLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHWEST CORNER OF SAID SECTION 34, THENCE SOUTHERLY ALONG THE WEST LINE OF SAID SECTION S. 3° 57'16"W., 1009.13 FEET TO THE CENTERLINE OF EEL RIVER ROAD (SAID ROAD BEING NAMED ELK MOUNTAIN ROAD WITHIN LAKE COUNTY), THENCE ALONG THE CENTERLINE OF SAID ROAD THE FOLLOWING COURSES:

S.79° 23'41"E., 28.84 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 38'39", A DISTANCE OF 30.79 FEET;

THENCE S.61° 45'02"E., 100.55 FEET;

THENCE S.67° 12'50"E., 87.31 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 38° 38'41", A DISTANCE OF 67.45 FEET;

THENCE S.28° 34'09"E., 66.81 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 27° 39'02", A DISTANCE OF 48.26 FEET;

THENCE S.56° 13'11"E., 66.09 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 11° 56'08", A DISTANCE OF 41.66 FEET TO THE NORTHERLY LINE OF SAID SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 34, BEING THE TRUE

POINT OF BEGINNING, THENCE CONTINUING ALONG THE CENTERLINE OF SAID ROAD S.44° 17'03"E., 311.73 FEET;

THENCE ALONG THE ARC OF A 125 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 49° 41'34", A DISTANCE OF 108.41 FEET;

THENCE S.5° 24'31"W., 200.50 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 21° 10'54", A DISTANCE OF 73.94 FEET;

THENCE S.15° 46'22"E., 52.38 FEET;

THENCE ALONG THE ARC OF A 150 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 73° 24'50", A DISTANCE OF 192.20 FEET;

THENCE S.89° 11'13"E., 94.64FEET;

THENCE N.84° 35'45"E., 79.12 FEET;

THENCE S.85° 43'50"E., 120.41 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 37° 28'15", A DISTANCE OF 130.80 FEET;

THENCE N.56° 47'56"E., 181.50 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 41'56", A DISTANCE OF 82.20 FEET;

THENCE N.72° 29'52"E., 226.17 FEET;

THENCE ALONG THE ARC OF A 75 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 47° 07'21", A DISTANCE OF 61.68 FEET;

THENCE N.25° 22'31"E., 80.62 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 82° 02'48", A DISTANCE OF 143.20 FEET;

THENCE S.72° 34'42"E., 72.69 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 57° 57'38", A DISTANCE OF 101.16 FEET;

THENCE N.49° 27'40"E., 221.43 FEET;

THENCE N.45° 30'03"E., 342.16 FEET;

THENCE N.48° 07'37"E., 236.28 FEET;

THENCE N.47° 51'31"E., 319.26 FEET;

THENCE N.52° 38'18"E., 227.20 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 48° 41'42", A DISTANCE OF 169.98 FEET;

THENCE S.78° 40'00"E., 215.67 FEET;

THENCE S.85° 31'18"E., 196.81 FEET;

THENCE S.84° 57'48"E., 133.77 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 29'00", A DISTANCE OF 135.12 FEET;

THENCE S.69° 28'48"E., 253.56 FEET;

THENCE S76° 16'11"E., 242.40 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 17° 45'40", A DISTANCE OF 154.99 FEET;

THENCE N.85° 58'10"E., 112.91 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 9° 07'43", A DISTANCE OF 63.73 FEET;

THENCE S.84° 54'07"E., 72.20 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 17° 11'05", A DISTANCE OF 119.97 FEET;

THENCE N.77° 54'47" E., 224.51 FEET;

THENCE ALONG THE ARC OF A 400 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 15° 24'00", A DISTANCE OF 107.51 FEET;

THENCE S.86° 41'13"E., 84.37 FEET;

THENCE N.85° 54'13"E., 135.18 FEET;

THENCE N.85° 45'19"E., 269.61 FEET;

THENCE ALONG THE ARC OF A 500 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 29° 00'37", A DISTANCE OF 253.16 FEET;

THENCE N.56° 44'42"E., 135.71 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 07'47", A DISTANCE OF 86.69 FEET;

THENCE N.73° 52'28"E., 182.07 FEET;

THENCE N.71° 01'41"E., 225.64 FEET;

THENCE ALONG THE ARC OF A 250 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 19° 46'36", A DISTANCE OF 86.29 FEET;

THENCE N.51° 15'05"E., 53.82 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 25° 50'31", A DISTANCE OF 90.21 FEET;

THENCE N.25° 24'34"E., 15.25 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 27° 41'06", A DISTANCE OF 48.32 FEET;

THENCE N.53° 05'40"E., 144.67 FEET;

THENCE N.55° 07'35"E., 75.08 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 12'12", A DISTANCE OF 70.52 FEET;

THENCE N.34° 55'23"E., 58.18 FEET;

THENCE N.21° 53'09"E., 56.35 FEET;

THENCE N.27° 13'04"E., 111.53 FEET;

THENCE N.29° 44'08"E., 196.00 FEET;

THENCE N.33° 55'54"E., 113.95 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 37° 47'07", A DISTANCE OF 197.84 FEET;

THENCE N.3° 51'13"W., 37.41 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 26° 45'41", A DISTANCE OF 93.42 FEET;

THENCE N.22° 54'28"E., 32.35 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 16° 13'00", A DISTANCE OF 84.91 FEET;

THENCE N.6° 41'29"E., 13.27 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 49° 15'08", A DISTANCE OF 171.92 FEET;

THENCE N.55° 56'37"E., 49.24 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 23° 16'56", A DISTANCE OF 81.27 FEET;

THENCE N.32° 39'41"E., 183.14 FEET;

THENCE N.30° 25'27"E., 235.16 FEET;

THENCE ALONG THE ARC OF A 150 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 50° 25'05", A DISTANCE OF 131.99 FEET;

THENCE N.80° 50'32"E., 49.72 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 24° 44'45", A DISTANCE OF 43.19 FEET;

THENCE N.56° 05'47"E., 129.12 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 58'18", A DISTANCE OF 73.21 FEET;

THENCE N.35° 07'29"E., 250.33 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 27° 57'05", A DISTANCE OF 97.57 FEET;

THENCE N.7° 10'24"E., 111.84 FEET;

THENCE ALONG THE ARC OF A 200 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 24° 03'55", A DISTANCE OF 84.00 FEET;

THENCE N.31° 14'19"E., 147.14 FEET;

THENCE ALONG THE ARC OF A 300 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 26° 30'23", A DISTANCE OF 138.79 FEET;

THENCE N.4° 43'56"E., 165.63 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 70° 28'10", A DISTANCE OF 122.99 FEET;

THENCE N.75° 12'07"E., 43.14 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 42° 37'10", A DISTANCE OF 74.39 FEET;

THENCE N.32° 34'56"E., 202.60 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 67° 13'02", A DISTANCE OF 117.32 FEET;

THENCE S.80° 12'01"E., 40.45 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 44° 03'36", A DISTANCE OF 76.90 FEET;

THENCE N.55° 44'23"E., 70.33 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 66° 40'27", A DISTANCE OF 116.37 FEET;

THENCE N.10° 56'04"W., 261.41 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 20° 55'48", A DISTANCE OF 36.53 FEET;

THENCE N.31° 51'52"W., 72.93 FEET;

THENCE ALONG THE ARC OF A 100 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 17° 21'55", A DISTANCE OF 30.31 FEET;

THENCE N.14° 29'58"W., 129.97 FEET;

THENCE ALONG THE ARC OF A 75 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 85° 30'59", A DISTANCE OF 111.94 FEET;

THENCE N.71° 01'01"E., 13.82 FEET;

THENCE ALONG THE ARC OF A 50 FOOT RADIUS CURVE TO THE RIGHT, THROUGH A CENTRAL ANGLE OF 63° 02'47", A DISTANCE OF 55.02 FEET;

THENCE S.45° 56'12"E., 70.60 FEET;

THENCE ALONG THE ARC OF A 350 FOOT RADIUS CURVE TO THE LEFT, THROUGH A CENTRAL ANGLE OF 40° 55'35", A DISTANCE OF 250 FEET, TO A POINT ON THE EAST LINE OF THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 26, FROM WHICH THE NORTHEAST CORNER OF THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION BEARS N.70° 58'07"E., A DISTANCE OF 1344.07 FEET MORE OR LESS, SAID POINT BEING THE POINT OF TERMINATION FOR THIS DESCRIPTION.

THE BEARINGS AND DISTANCES USED IN THE ABOVE DESCRIPTION ARE GRID, BASED ON THE CALIFORNIA COORDINATE SYSTEM OF 1983 (CCS83), ZONE 2 NAD 83 (NSRS 2007) (EPOCH 2011.00). TO OBTAIN GEODETIC BEARINGS, ROTATE GRID BEARINGS BY 0° 40'40" COUNTERCLOCKWISE. TO OBTAIN GROUND DISTANCES, DIVIDE GRID DISTANCES BY A COMBINED SCALE FACTOR OF 0.9998297205.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED JULY 24, 2019 AS INSTRUMENT NO. 2019-08481 OFFICIAL RECORDS OF MENDOCINO COUNTY.

APN: 171-170-02, 171-180-04, 171-200-03, 171-200-11 PORTION, 171-210-03 PORTION, 171-210-10 PORTION, 171-230-20, 171-240-08, 171-260-09

PARCEL 8:
LCP ID# 0749

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 29, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JULY 21, 1910 IN BOOK 117, PAGE 118 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTH ONE-HALF OF THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 29.
A.P.N. 171-200-11 PORTION

PARCEL 9:
LCP ID# 0750

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 31, TOWNSHIP 18 NORTH, RANGE 11 WEST, AND SECTION 6, TOWNSHIP 17 NORTH, RANGE 11 WEST, BOTH OF MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 28, 1906 IN LIBER 99, PAGE 546 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

A STRIP OF LAND OF 100 FEET LYING 50 FEET ON EACH SIDE OF THE FOLLOWING DESCRIBED LINE:

BEGINNING AT A POINT ON THE SOUTH LINE OF THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 31, FROM WHICH POINT THE QUARTER SECTION

CORNER ON THE WEST BOUNDARY OF SAID SECTION 31, BEARS WEST 2239.05 FEET DISTANT, SAID POINT OF BEGINNING BEING AT STATION 49 + 47 OF THE LOCATED CENTER LINE OF THE EEL RIVER POWER AND IRRIGATION COMPANY'S TUNNEL; THENCE
SOUTH 22° 02' WEST 986.4 FEET ALONG THE CENTER LINE OF SAID TUNNEL TO STATION 0 + 00 OF THE PIPE LINE LEADING AWAY FROM SAID TUNNEL; THENCE FOLLOWING THE CENTER LINE OF SAID PIPE LINE, AS FOLLOWS:

SOUTH 22° 02' WEST 396.4 FEET; THENCE
SOUTHWESTERLY ON A CURVE TO THE RIGHT WITH A RADIUS 301.6 FEET AND AN ARC DISTANCE OF 103.5 FEET; THENCE
SOUTH 41° 42' WEST 745.8 FEET; THENCE
SOUTHERLY ON A CURVE TO THE LEFT WITH A RADIUS 301.6 FEET AND AN ARC DISTANCE OF 198.8 FEET; THENCE
SOUTH 3° 57' WEST 532.54 FEET TO A POINT ON THE TOWNSHIP LINE BETWEEN SAID TOWNSHIPS 17 AND 18 NORTH, RANGE 11 WEST, FROM WHICH THE QUARTER SECTION CORNER ON THE NORTH BOUNDARY OF SAID SECTION 6, BEARS EAST 1510.83 FEET; THENCE SOUTH 3° 57' WEST 1157.52 FEET CONTINUING TO A POINT DISTANT 150 FEET WEST OF THE NORTHEAST CORNER OF THE "POWER HOUSE TRACT", HEREINAFTER DESCRIBED, BEING A PART OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 31, AND A PART OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 6.

ALSO INCLUDING A PORTION OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 6, BEGINNING AT A POST MARKED "E" 150 FEET EAST OF THE SOUTH END OF THE FOREGOING DESCRIBED CENTER LINE, FROM WHICH THE QUARTER SECTION CORNER ON THE NORTH BOUNDARY OF SAID SECTION 6, BEARS NORTH 51° 17' EAST 1846.10 FEET DISTANT; THENCE WEST 788.70 FEET TO A POST MARKED "F", FROM WHICH A WHITE OAK TREE 40 INCHES IN DIAMETER MARKED "F.B.T." BEARS SOUTH 2° 35' WEST AND IS DISTANT 90.10 FEET; THENCE SOUTH 400 FEET TO A POINT 50 FEET SOUTH OF A STAKE MARKED "A", FROM WHICH A BLACK OAK TREE 1 FOOT IN DIAMETER MARKED "ABT" BEARS NORTH 10° 00' EAST AND IS DISTANT 84.16 FEET; THENCE
EAST 350 FEET TO A POINT 50 FEET SOUTH OF A STAKE MARKED "B", FROM WHICH A WHITE OAK TREE 3 FEET IN DIAMETER MARKED "BBT" BEARS NORTH 60° 00' WEST AND IS DISTANT 120.47 FEET; THENCE
NORTH 125 FEET TO A STAKE MARKED "C"; THENCE
EAST 438.70 FEET TO A STAKE MARKED "D"; THENCE
NORTH 275 FEET TO THE POINT OF BEGINNING.
A.P.N. 173-030-02, 172-300-02 PORTION, 173-030-03 PORTION

PARCEL 10:
LCP ID# 0751

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN THE SOUTHWEST ONE-QUARTER OF SECTION 31, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED IN BOOK 114, PAGE 432 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

BEGINNING AT A POINT ON THE BOUNDARY OF THE PIPE LINE TRACT OF THE SNOW MOUNTAIN WATER & POWER COMPANY, FROM WHICH THE QUARTER SECTION CORNER OF THE WEST

BOUNDARY OF SAID SECTION 31 BEARS NORTH 54° 36' WEST DISTANT 2177.20 FEET, AND WHICH POINT IS NORTH 67° 58' WEST 50 FEET DISTANT FROM A POINT ON THE CENTER LINE OF SAID TRACT, SOUTH 22° 02' WEST 1380.8 FEET DISTANT FROM THE POINT OF BEGINNING OF SAID CENTER LINE AS DESCRIBED IN THE ORIGINAL DEED AND IS THE POINT OF CURVATURE OF A 19° CURVE RIGHT; THENCE
ON A CURVE TO THE RIGHT WITH A RADIUS OF 252.94 FEET AND AN ARC DISTANCE OF 86. 25 FEET ALONG SAID BOUNDARY OF SAID TRACT; THENCE
SOUTH 41° 42' WEST 745.8 FEET CONTINUING ALONG SAID BOUNDARY; THENCE
ON A CURVE TO THE LEFT WITH A RADIUS OF 352.94 FEET AND AN ARC DISTANCE OF 231.77 FEET CONTINUING ON SAID BOUNDARY; THENCE
NORTH 3° 57' EAST 273.28 FEET; THENCE
NORTH 48° 18' WEST 155 FEET; THENCE
NORTH 41° 42' EAST 210 FEET; THENCE
SOUTH 48° 18' EAST 130 FEET; THENCE
NORTH 51° 11' EAST 629.52 FEET, MORE OR LESS, TO THE POINT OF BEGINNING.
A.P.N. 172-300-02 PORTION

PARCEL 11
LCP ID#0752

LL THAT CERTAIN PARCEL OF LAND SITUATE IN THE NORTHWEST ONE-QUARTER OF SECTION 6, TOWNSHIP 17 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JUNE 14, 1909 IN BOOK 86, PAGE 548 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

TRACT NO. 1

BEGINNING AT AN IRON STAKE MARKED "E" FROM WHICH THE QUARTER SECTION CORNER ON THE NORTH BOUNDARY OF SAID SECTION 6 BEARS NORTH 51° 17' EAST 1846.10 FEET DISTANT; SAME BEING THE NORTHEAST CORNER OF THE POWER HOUSE TRACT; THENCE
EAST 108.9 FEET TO A REDWOOD STAKE MARKED "I"; THENCE
SOUTH 400 FEET TO A REDWOOD STAKE MARKED "H"; THENCE
WEST 108.9 FEET TO A REDWOOD STAKE MARKED "G"; THENCE
NORTH 400 FEET TO THE POINT OF BEGINNING.

TRACT NO. 3

ALSO BEGINNING AT AN IRON STAKE MARKED "D" BEING THE SOUTHEAST CORNER OF THE POWER HOUSE TRACT OF THE SNOW MOUNTAIN WATER AND POWER CO., AND FROM WHICH THE QUARTER SECTION CORNER ON THE NORTH BOUNDARY OF SAID SECTION 6 BEARS NORTH 45° 13' EAST 2029.46 FEET DISTANT; THENCE
SOUTH 125 FEET TO A POINT MARKED "G"; THENCE
WEST 438.7 FEET TO AN IRON STAKE MARKING A CORNER OF SAID POWER HOUSE TRACT; THENCE
NORTH 125 FEET TO AN IRON STAKE MARKING A CORNER OF SAID POWER HOUSE TRACT; THENCE
EAST 438.7 FEET TO THE POINT OF BEGINNING.
A.P.N. 173-030-03 PORTION

PARCEL 12
LCP ID#0753

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN THE WEST ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SECTION 6, TOWNSHIP 17 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED FEBRUARY 27, 2925 IN VOLUME 3, PAGE 109 OF OFFICIAL RECORDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

PARCEL NO. 1

BEGINNING AT THE NORTHWEST CORNER OF THE POWER HOUSE TRACT BELONGING TO THE SNOW MOUNTAIN WATER & POWER COMPANY, AND RUNNING THENCE
WESTERLY 154.75 FEET TO THE TOWNSHIP LINE; THENCE
SOUTH ALONG SAID TOWNSHIP LINE 400 FEET; THENCE
EAST TO THE SOUTHWEST CORNER OF SAID POWER HOUSE TRACT; THENCE
NORTHERLY ALONG SAID POWER HOUSE TRACT TO THE POINT OF BEGINNING.

PARCEL NO. 2

BEGINNING AT THE SOUTHEAST CORNER OF THE POWER HOUSE TRACT OF SNOW MOUNTAIN WATER & POWER COMPANY, BEING ALSO THE SOUTHEAST CORNER OF TRACT NO. 1 DESCRIBED IN THE DEED FROM OTTO R. HUGHES TO SNOW MOUNTAIN WATER AND POWER COMPANY DATED JUNE 12, 1909 AND RECORDED JUNE 14, 1909 IN BOOK 86 OF DEEDS AT PAGE 548, MENDOCINO COUNTY RECORDS, AND RUNNING THENCE
SOUTH 300. 8 FEET; THENCE
WESTERLY PARALLEL TO THE SOUTH LINE OF SAID POWER HOUSE TRACT 1242.35 FEET TO THE TOWNSHIP LINE; THENCE
NORTHERLY 300.8 FEET TO THE SOUTHWEST CORNER OF PARCEL NO. 1 ABOVE DESCRIBED; THENCE
EASTERLY ALONG THE SOUTHERLY LINE OF SAID TRACT NO. 1 TO THE POINT OF BEGINNING.
A. P.N. 173-030-02 PORTION, 173-030-03 PORTION

PARCEL 13:
LCP ID#0754

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN THE NORTHWEST ONE-QUARTER OF SECTION 6, TOWNSHIP 17 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JUNE 14, 1909 IN BOOK 86, PAGE 548 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

TRACT NO. 2: BEGINNING AT AN IRON STAKE MARKED "A" FROM WHICH THE QUARTER SECTION CORNER ON THE NORTH BOUNDARY OF SAID SECTION 6 BEARS NORTH 43° 26 1/2' EAST 2676.55 FEET DISTANT, SAME BEING THE SOUTHWEST CORNER OF THE POWER HOUSE TRACT, THENCE WEST 190 FEET TO A REDWOOD STAKE MARKED "E"; THENCE
NORTH 400 FEET TO A REDWOOD STAKE MARKED "J"; THENCE
EAST 190 FEET TO AN IRON PIN MARKED "F"; THENCE

SOUTH ALONG THE WEST BOUNDARY OF THE POWER HOUSE TRACT TO THE POINT OF BEGINNING.
A.P.N. 173-030-02 PORTION

PARCEL 14:
LCP ID#0755

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 32, TOWNSHIP 18 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED DECEMBER 8, 1925 IN BOOK 10, PAGE 11 OF OFFICIAL RECORDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

ALL THAT PORTION OF THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 32 THAT LIES WESTERLY OF THE OLD COUNTY ROAD LEADING FROM POTTER VALLEY TO GRAVELLY VALLEY.

THIS DEED IS INTENDED TO CONVEY AND THE GRANTOR DOES HEREBY GRANT AND CONVEY TO THE GRANTEE ALL OF THE LAND IN WHICH THEY HAVE AN INTEREST IN THE SAID SOUTHEAST ONE-QUARTER OF NORTHWEST ONE-QUARTER OF SAID SECTION 32, WHETHER INCLUDED IN THE ABOVE DESCRIPTION OR NOT.
PGE# 2418-11-0151

A.P.N. 171-210-03 PORTION, 171-210-10 PORTION

PARCEL 15:
LCP ID#0756

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 15, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED FEBRUARY 28, 1924 IN BOOK 65, PAGE 448 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE EAST ONE-HALF OF THE NORTHWEST ONE-QUARTER, THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, AND THE SOUTH ONE-HALF OF SAID SECTION 15.
EXCEPTING THEREFROM ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SAID SECTION 15, AS DESCRIBED IN THE UNRECORDED DEED FROM PACIFIC GAS AND ELECTRIC COMPANY TO GEORGE R. WINKLER DATED MAY 11 1945, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE NORTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, THE SOUTH HALF OF THE NORTHWEST QUARTER, THE NORTH ONE-HALF OF THE SOUTHWEST QUARTER, AND THE NORTH ONE-HALF OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 15.
A.P.N. 001-035-004 PORTION

PARCEL 16:
LCP ID#0757

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 23, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED SEPTEMBER 2, 1907 IN BOOK 39, PAGE 585 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE WEST ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 23.
A.P.N. 001-035-003 PORTION

PARCEL 17:
LCP ID#0758

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 23, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE NORTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 23.
A.P.N. 001-035-003 PORTION

PARCEL 18:
LCP ID# 0759

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 23, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED DECEDMBER 21, 1928 IN BOOK 49, PAGE 208 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

1) THAT PORTION OF THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 23 BEGINNING AT A POINT ON THE NORTH LINE OF THE SAID SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SECTION 23 FROM WHICH THE NORTHEAST CORNER OF THE SAID SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SECTION 23, BEARS NORTH 89° 45' EAST 742.3 FEET, AND RUNS THENCE THE FOLLOWING COURSES AND DISTANCES:
SOUTH 39° 21' EAST 58.2 FEET
SOUTH 13° 36' EAST 66.7 FEET
SOUTH 11° 44' EAST 150.3 FEET
SOUTH 16° 32' EAST 196.1 FEET
SOUTH 18° 19' EAST 167.1 FEET
SOUTH 4° 53' EAST 93.0 FEET
SOUTH 60° 24' WEST 207.3 FEET
SOUTH 26° 16' EAST 55.4 FEET
NORTH 82° 11' EAST 118.5 FEET
SOUTH 58° 18' EAST 131.2 FEET
SOUTH 32° 46' EAST 116.7 FEET
SOUTH 11° 26' EAST 110.6 FEET
SOUTH 18° 33' WEST 90.6 FEET
NORTH 89° 39' EAST 60.9 FEET
SOUTH 51° 57' EAST 42.2 FEET

SOUTH 0° 18' EAST 96.2 FEET
SOUTH 89° 58' EAST 335.4 FEET
NORTH 0° 41' WEST 1330.3 FEET
TO THE NORTHEAST CORNER OF THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 23; THENCE
SOUTH 89° 54' WEST 742.3 FEET TO THE POINT OF BEGINNING.

2) THAT PORTION OF THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 23 BEGINNING AT A POINT ON THE NORTH LINE OF THE SAID NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SECTION 23 FROM WHICH THE NORTHEAST CORNER OF THE NORTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 23 BEARS SOUTH 89° 58' EAST 335.4 FEET, AND RUNNING THENCE THE FOLLOWING COURSES AND DISTANCES:

SOUTH 0° 18' EAST 23.9 FEET
SOUTH 66° 40' EAST 70.7 FEET
SOUTH 21° 57' EAST 137.1 FEET
SOUTH 3° 41' EAST 200.9 FEET
SOUTH 31° 49' WEST 190. 0 FEET
SOUTH 2° 16' EAST 348.2 FEET
SOUTH 47° 56' WEST 265.8 FEET
SOUTH 31° 45' EAST 161.2 FEET
NORTH 59° 19' EAST 248.0 FEET
SOUTH 77° 13' EAST 38.7 FEET
SOUTH 25° 41' EAST 98.3 FEET
SOUTH 1° 26' WEST 150.8 FEET
NORTH 71° 52' EAST 128.0 FEET
SOUTH 63° 29' EAST 31.6 FEET
NORTH 0° 41' WEST 1298.6 FEET
TO THE NORTHEAST CORNER OF THE NORTHWEST ONE-QUARTER OF SOUTHEAST ONE-QUARTER OF SAID SECTION 23, THENCE
NORTH 89° 58' WEST 335.4 FEET TO THE POINT OF BEGINNING.
A.P.N. 001-035-003 PORTION

PARCEL 19:
LCP ID#0760

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 14 AND 23, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN VOLUME 36, PAGE 588 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE WEST ONE-HALF OF SOUTHEAST ONE-QUARTER AND SOUTHEAST ONE-QUARTER OF SOUTHWEST ONE-QUARTER OF SAID SECTION 14; AND THE NORTHWEST ONE-QUARTER OF NORTHEAST ONE-QUARTER OF SAID SECTION 23.
A.P.N. 001-035-003 PORTION

PARCEL 20:

LCP ID#0761

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 14, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHWEST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 14.
A.P.N. 001-035-035

PARCEL 21:
LCP ID#0762

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 14, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 14.
A.P.N. 001-035-003 PORTION

PARCEL 22:
LCP ID#0763

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 12, 13 AND 14, TOWNSHIP 18 NORTH, RANGE 10 WEST; AND SECTIONS 7 AND 18, TOWNSHIP 18 NORTH, RANGE 9 WEST; ALL MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 588 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHEAST ONE-QUARTER OF NORTHEAST ONE-QUARTER, THE NORTH ONE-HALF OF SOUTHEAST ONE-QUARTER, AND THE SOUTHWEST ONE-QUARTER OF SOUTHEAST ONE-QUARTER OF SAID SECTION 12; THE SOUTHEAST ONE-QUARTER OF NORTHEAST ONE-QUARTER, THE WEST ONE-HALF OF NORTHEAST ONE-QUARTER, AND THE SOUTH ONE-HALF OF NORTHWEST ONE-QUARTER OF SAID SECTION 13; THE SOUTHEAST ONE-QUARTER OF NORTHEAST ONE-QUARTER OF SAID SECTION 14; AND THE SOUTHWEST ONE-QUARTER OF NORTHWEST ONE-QUARTER AND THE NORTH ONE-HALF OF SOUTHWEST ONE-QUARTER OF SAID SECTION 18.

ALSO INCLUDING ALL THAT PORTION OF THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 14, DESCRIBED AS FOLLOWS:

COMMENCING AT THE CENTER OF THE WELL LOCATED ABOUT 20 FEET WEST OF THE RESIDENCE FORMERLY OCCUPIED BY R. F. MCMATH, AND RUNNING THENCE
NORTH 25° WEST 86 FEET; THENCE
SOUTH 85° EAST 396 FEET TO THE WESTERN BOUNDARY LINE OF R. F. MCMATH'S PRE-EMPTION; THENCE

SOUTH 980 FEET ALONG SAID LINE OF THE LAND PRE-EMPTED BY THE SAID R. F. MCMATH; THENCE
WEST 242 FEET; THENCE
NORTH 25° WEST 978 FEET TO THE POINT OF BEGINNING.
A.P.N. 001-033-006, 001-035-003 PORTION, 001-040-001

PARCEL 23:
LCP ID#0764

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 14, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED DECEMBER 1, 1996 IN BOOK 39, PAGE 262 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

SOUTHEAST ONE-QUARTER OF NORTHWEST ONE-QUARTER OF SECTION 14.
A.P.N. 001-035-003 PORTION

PARCEL 24:
LCP ID#0765

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 11 AND 14, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JULY 16, 1908 IN BOOK 41, PAGE 316 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

SOUTHEAST ONE-QUARTER OF SAID SECTION 11; AND THE NORTH ONE-HALF OF THE NORTHEAST ONE-QUARTER AND THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 14.

LESS AND EXCEPT THAT PORTION OF THE SAID SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, BEING 7 ACRES FORMERLY OWNED BY R. F. MCMATH AND NOW BELONGING TO SNOW MOUNTAIN WATER & POWER COMPANY.
A.P.N. 001-033-005 PORTION, 001-035-003 PORTION

PARCEL 25:
LCP ID#0766

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 2 AND 11, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED MARCH 22, 1906 IN BOOK 36, PAGE 590 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE WEST ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 2; THE SOUTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE NORTH ONE-HALF OF THE NORTHWEST ONE-QUARTER, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER, THE NORTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER; AND THE SOUTH ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 11.
A.P.N. 001-033-003 PORTION, 001-033-004 PORTION, 001-033-005 PORTION

PARCEL 26:
LCP ID#0767

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 10 AND 11, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED AUGUST 13, 1906 IN BOOK 39, PAGE 95 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE EAST ONE-HALF OF THE NORTHEAST ONE-QUARTER OF SECTION 10; AND SOUTHWEST ONE-QUARTER OF NORTHWEST ONE-QUARTER AND NORTHWEST ONE-QUARTER OF SOUTHWEST ONE-QUARTER OF SECTION 11.
A.P.N. 001-033-004 PORTION

PARCEL 27:
LCP ID#0768

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 11 AND 12, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED DECEMBER 10, 1906 IN BOOK 39, PAGE 272 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE EAST ONE-HALF OF THE NORTHEAST ONE-QUARTER, AND THE NORTHWEST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 11; AND THE NORTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 12.
A.P.N. 001-033-005 PORTION

PARCEL 28A:
LCP ID # 769 PORTION

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 3, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO MERIDIAN, AS DESCRIBED IN A DOCUMENT RECORDED MAY 6, 1907, IN BOOK 39 AT PAGE 484 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, MORE PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTH ONE-HALF OF THE NORTHEAST ONE-QUARTER, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE- QUARTER, THE NORTHEAST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER AND THE SOUTH ONE-HALF OF THE SOUTH ONE-HALF OF SAID SECTION 3.

EXCEPTING THEREFROM, ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SAID SECTION 3, AS DESCRIBED IN AN UNRECORDED DEED FROM PACIFIC GAS AND ELECTRIC COMPANY TO GEORGE R. WINKLER, DATED MAY 11, 1945, MORE PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTH ONE-HALF OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 3.

ALSO EXCEPTING THEREFROM, THE SOUTHEAST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 3.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED OCTOBER 1, 2021 AS DOCUMENT NO. 2021016686 OFFICIAL RECORDS OF LAKE COUNTY.

APN: 001-033-003 PORTION

PARCEL 29:
LCP ID#0770

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 2, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JULY 10, 1906 IN BOOK 39, PAGE 80 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

SOUTHEAST ONE-QUARTER OF THE SOUTHWEST ONE-QUARTER OF SAID SECTION 2.
A.P.N. 001-033-003 PORTION

PARCEL 30A:
LCP ID # 771 PORTION

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 1 AND 2, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JULY 16, 1908, IN BOOK 41 AT PAGE 311 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER, THE EAST ONE-HALF OF THE SOUTHEAST ONE-QUARTER, AND THE SOUTHWEST ONE-QUARTER OF THE SOUTHEAST ONE-QUARTER OF SAID SECTION 2; AND 10 ACRES OFF THE WESTERLY SIDE OF THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 1.

EXCEPTING THEREFROM THAT PORTION LYING NORTHERLY OF THE FOLLOWING DESCRIBED LINE:

COMMENCING AT THE WEST QUARTER CORNER OF SAID SECTION 1 FROM WHICH THE EAST QUARTER CORNER OF SAID SECTION BEARS S.88° 36'40"E. A DISTANCE OF 5176.79 FEET; THENCE ALONG THE SOUTH LINE OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 1, S.88° 36'40"E. A DISTANCE OF 1277.48 FEET TO THE SOUTHEAST CORNER OF THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER; THENCE ALONG THE EAST LINE OF SAID SOUTHWEST ONE-QUARTER N.0° 36'09"E. A DISTANCE OF 1144.15 FEET, MORE OR LESS, TO A POINT ON A LINE PARALLEL WITH AND 60 FEET NORTHWESTERLY OF, MEASURED AT RIGHT ANGLES, THE CENTERLINE OF FOREST ROAD NO. M1 (FORMERLY "SIMMONS ROAD"), SAID POINT BEING THE TRUE POINT OF BEGINNING;

THENCE ALONG SAID PARALLEL LINE THE FOLLOWING COURSES:

S.43° 56'49"W. 174.80 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1440.00 FEET, THROUGH A CENTRAL ANGLE OF 17° 46'10", AN ARC DISTANCE OF 446.60 FEET TO THE BEGINNING OF A COMPOUND CURVE TO THE RIGHT, HAVING A RADIUS OF 4940.00 FEET, THROUGH A CENTRAL ANGLE OF 2° 36'50", AN ARC DISTANCE OF 225.36 FEET;

THENCE S.64° 19'49"W. 429.46 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1940.00 FEET, THROUGH A CENTRAL ANGLE OF 7° 07'10", AN ARC DISTANCE OF 241.06 FEET;

THENCE S.71° 26'58"W. 565.47 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1090. 00 FEET, THROUGH A CENTRAL ANGLE OF 27° 52'08", AN ARC DISTANCE OF 530.18 FEET;

THENCE N.80° 40'54"W. 326.25 FEET TO A POINT ON THE WEST LINE OF THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 2, SAID POINT BEING THE POINT OF TERMINATION.

THE BEARINGS AND DISTANCES USED IN THE FOREGOING DESCRIPTION ARE ON THE CALIFORNIA COORDINATE SYSTEM OF 1983, ZONE 2, NAD 83(NSRS 2007)(EPOCH 2011.00). TO OBTAIN GEODETIC BEARINGS, ROTATE ALL CALIFORNIA COORDINATE BEARINGS 0° 35'43" COUNTERCLOCKWISE. DISTANCES HEREIN ARE GRID. TO OBTAIN GROUND DISTANCES, DIVIDE ALL DISTANCES BY A COMBINED SCALE FACTOR OF 0.9998508.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED OCTOBER 1, 2021 AS DOCUMENT NO. 2021016690 OFFICIAL RECORDS OF LAKE COUNTY.

APN: 001-033-003 PORTION

PARCEL 31A:
LCP ID # 772 PORTION

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTIONS 1 AND 2, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED APRIL 26, 1906 IN BOOK 37 AT PAGE 623 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

LOT 4 AND THE EAST 30 ACRES OF THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 1, AND LOT 1 OF SAID SECTION 2.

EXCEPTING THEREFROM THAT PORTION LYING NORTHERLY OF THE FOLLOWING DESCRIBED LINE:

COMMENCING AT THE WEST ONE-QUARTER CORNER OF SAID SECTION 1 FROM WHICH THE EAST ONE-QUARTER CORNER OF SAID SECTION BEARS S.88° 36'40"E. A DISTANCE OF 5176.79 FEET; THENCE ALONG THE SOUTH LINE OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 1,

S.88° 36'40"E. A DISTANCE OF 1277.48 FEET TO THE SOUTHEAST CORNER OF THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER; THENCE ALONG THE EAST LINE OF SAID SOUTHWEST ONE-QUARTER N.0° 36'09"E. A DISTANCE OF 1144.15 FEET, MORE OR LESS, TO A POINT ON A LINE PARALLEL WITH AND 60 FEET NORTHWESTERLY OF, MEASURED AT RIGHT ANGLES, THE CENTERLINE OF FOREST ROAD NO. M1 (FORMERLY "SIMMONS ROAD"), SAID POINT BEING THE TRUE POINT OF BEGINNING;

THENCE ALONG SAID PARALLEL LINE THE FOLLOWING COURSES:

S.43° 56'49"W. 174.80 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1440.00 FEET, THROUGH A CENTRAL ANGLE OF 17° 46'10", AN ARC DISTANCE OF 446.60 FEET TO THE BEGINNING OF A COMPOUND CURVE TO THE RIGHT, HAVING A RADIUS OF 4940.00 FEET, THROUGH A CENTRAL ANGLE OF 2° 36'50", AN ARC DISTANCE OF 225.36 FEET;

THENCE S.64° 19'49"W. 429.46 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1940.00 FEET, THROUGH A CENTRAL ANGLE OF 7° 07'10", AN ARC DISTANCE OF 241.06 FEET;

THENCE S.71° 26'58"W. 565.47 FEET;

THENCE ALONG A TANGENT CURVE TO THE RIGHT, HAVING A RADIUS OF 1090.00 FEET, THROUGH A CENTRAL ANGLE OF 27° 52'08", AN ARC DISTANCE OF 530.18 FEET;

THENCE N.80° 40'54"W. 326.25 FEET TO A POINT ON THE WEST LINE OF THE SOUTHEAST ONE-QUARTER OF THE NORTHEAST ONE-QUARTER OF SAID SECTION 2, SAID POINT BEING THE POINT OF TERMINATION.

THE BEARINGS AND DISTANCES USED IN THE FOREGOING DESCRIPTION ARE ON THE CALIFORNIA COORDINATE SYSTEM OF 1983, ZONE 2, NAD 83(NSRS 2007)(EPOCH 2011.00). TO OBTAIN GEODETIC BEARINGS, ROTATE ALL CALIFORNIA COORDINATE BEARINGS 0° 35'43" COUNTERCLOCKWISE. DISTANCES HEREIN ARE GRID. TO OBTAIN GROUND DISTANCES, DIVIDE ALL DISTANCES BY A COMBINED SCALE FACTOR OF 0.9998508.

BEING ALL THAT PROPERTY AS DESCRIBED IN THAT CERTAIN CERTIFICATE OF COMPLIANCE RECORDED OCTOBER 1, 2021 AS DOCUMENT NO. 2021016688 OFFICIAL RECORDS OF LAKE COUNTY.

APN: 001-033-003 PORTION

PARCEL 32:
LCP ID#0773

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN SECTION 12, TOWNSHIP 18 NORTH, RANGE 10 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED JULY 16, 1908 IN BOOK 41, PAGE

316 OF DEEDS OF THE COUNTY OF LAKE, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

THE SOUTHWEST ONE-QUARTER OF THE NORTHWEST ONE-QUARTER OF SAID SECTION 12.
A.P.N. 001-033-005 PORTION

PARCEL 33:
LCP ID#0774

ALL THAT CERTAIN PARCEL OF LAND SITUATE IN THE NORTH ONE-HALF OF THE NORTHWEST ONE-QUARTER OF SECTION 7, TOWNSHIP 17 NORTH, RANGE 11 WEST, MOUNT DIABLO BASE AND MERIDIAN, AS RECORDED APRIL 16, 1920 IN BOOK 161, PAGE 113 OF DEEDS OF THE COUNTY OF MENDOCINO, STATE OF CALIFORNIA, PARTICULARLY DESCRIBED THEREIN AS FOLLOWS:

A STRIP OF LAND TWENTY FIVE FEET WIDE THROUGH THE PREMISE OF THE FIRST PARTIES ALONG THE EASTERLY SIDE OF THE 100 FOOT STRIP DESCRIBED IN THAT CERTAIN CONVEYANCE FROM THE SAID FIRST PARTIES TO THE PARTY OF SECOND PART, DATED MAY 31, 1907 AND RECORDED IN BOOK 86, PAGE 251 OF DEEDS OF SAID COUNTY, SAID CONVEYANCE DESCRIBED AS FOLLOWS:

A STRIP OF LAND OF THE UNIFORM WIDTH OF 100 FEET WIDE, BEING 50 FEET ON EACH SIDE OF THE FOLLOWING DESCRIBED LINE:

BEGINNING AT A POINT ON THE LINE BETWEEN SECTIONS 6 AND 7 OF SAID TOWNSHIP AND RANGE, FROM WHICH THE QUARTER SECTION CORNER BETWEEN SAID SECTIONS BEARS NORTH 88° 46' EAST AND IS DISTANT 857.75 FEET; THENCE
FROM SAID POINT OF BEGINNING SOUTHERLY ON A 5° CURVE TO THE LEFT, RADIUS 1146.01 FEET, A DISTANCE OF 273.75 FEET; THENCE
SOUTH 52° 33' EAST 758.07 FEET TO A POINT ON THE NORTH BOUNDARY LINE OF THE LAND OF HATTIE W. ENGLISH, SAID POINT BEING WEST 68.30 FEET OF THE SOUTHEAST CORNER OF GRANTOR'S LAND AS THE SAME IS AT PRESENT ESTABLISHED, FROM WHICH SOUTHEAST CORNER OF GRANTOR'S LAND THE QUARTER SECTION CORNER BETWEEN SAID SECTIONS 6 AND 7 BEARS NORTH 0° 43' WEST AND IS DISTANT 664.30 FEET.

A.P.N. 173-130-23

APN:

# EXHIBIT B

Description of Hydro Project Activities
and
Hydroelectric Facilities and Associated Water Delivery Facilities

As used in this Easement, "**Hydro Project Activities**" are those existing and future uses of the Property, and the existing and future Hydroelectric Facilities and Associated Water Delivery Facilities (as defined below) now or hereafter located on, above, or under the Property, associated with the operation of the Potter Valley hydroelectric project (FERC Project No. 77), located primarily on the Eel River, and shall include any future uses of the Property, and the existing and future Hydroelectric Facilities and Associated Water Delivery Facilities now or hereafter located on, above, or under the Property, associated with compliance with any future FERC License, FERC License renewal or other regulatory requirements.

As used in this Easement, **"Hydroelectric Facilities and Associated Water Delivery Facilities"** are those existing and future facilities, structures, buildings, and improvements now or hereafter located on, above, or under the Property, and associated with the operation of the Potter Valley hydroelectric project (FERC Project No. 77), including, but not limited to, the following existing and future improvements: Lake Pillsbury, Van Arsdale Reservoir, Scott Dam, Cape Horn Dam, Potter Valley Powerhouse, Powerhouse canal, penstocks, tunnels, staff gages, stream gauges, dams, bridges, canals, spillways, gage houses, aerial tramways, helipads, substations, electric transmission lines, electric distribution lines, communication lines, and associated infrastructure facilities; facilities necessary for the operation of powerhouses; improvements for existing and future water delivery and other requirements of power generation, transmission, distribution, and storage, for nonconsumptive and consumptive water use.

# EXHIBIT C

## Hydro Reserved Rights

Grantor's reserved rights on the Property include the following, which are expressly excluded from the transfer and conveyance of the easement granted in this Easement and reserved to Grantor:

Subject to the provisions of Section 7, the right to conduct Hydro Project Activities on the Property, including construction, operation, repair, alteration, maintenance, removal, replacement and expansion of existing and future Hydroelectric Facilities and Associated Water Delivery Facilities, including project replacements and improvements required for existing and future water delivery and other requirements for power generation, transmission, distribution, and storage, for nonconsumptive and consumptive water, and for communications in connection with the foregoing and for compliance with any future FERC License, FERC License renewal or other regulatory requirements. In furtherance of and without in any way limiting the generality of the foregoing, the following rights are expressly reserved:

(1) The right to conduct any and all uses and activities now or at any time in the future deemed necessary or appropriate by Grantor in Grantor's sole discretion exercised in good faith in connection with the generation of hydroelectric energy, including, but not limited to the construction, operation, repair, alteration, maintenance, removal, replacement and expansion of existing Hydroelectric Facilities and Associated Water Delivery Facilities, and the construction, operation, repair, alteration, maintenance, removal, replacement and expansion of new Hydroelectric Facilities and Associated Water Delivery Facilities; and

(2) The right to use, maintain, establish, construct, alter, expand and improve water sources, courses, and bodies within the Property, and to take, divert, store, convey and appropriate water; and

(3) The right to increase or otherwise modify water diversion, storage and transmission capacities of Hydroelectric Facilities and Associated Water Delivery Facilities; and

(4) The right to exercise: all riparian water rights inherent in and part and parcel of the Property; all appropriative surface water rights (including, but not limited to, any appropriative surface water rights having a point of diversion, place of storage, or place of use on the Property); all prescriptive surface water rights; and all other right, title and interest of any nature whatsoever in and to the surface waters which are now or hereafter located or flowing on, under or abutting the Property; and

(5) The right to decommission all or any portion of existing and future Hydroelectric Facilities and Water Delivery Facilities in accordance with the applicable license issued by the FERC, or as otherwise allowed by Applicable Law; and

(6) The right to enlarge, improve, reconstruct, relocate and replace said Grantor's existing facilities and additional facilities with any other number, size, or type of transformers, poles, towers, or structures, or underground wires, cables, pipelines and conduits, or other devices and equipment either in the original location or at any location or locations within the Property; and

(7) The right to construct, operate, use, repair, alter, maintain, remove, replace and expand Grantor's existing and future facilities for transformation, transmission and distribution of electric energy and for communication purposes and also the rights to reconstruct, replace, remove, maintain and use the same as Grantor shall at any time and from time to time deem necessary in Grantor's sole discretion exercised in good faith, together with the rights to excavate for, construct, install, repair, reconstruct, replace, remove, maintain and use, at any time and from time to time, additional facilities for the transformation, transmission and distribution of electric energy and for communication purposes, consisting of such devices and equipment with suitable concrete pads and adequate protection therefore necessary for transforming electric energy, one or more lines of underground wires and cables (enclosed at Grantor's option within conduits), and one or more lines of towers, poles and/or other structures, wires and cables, including both underground and overhead ground wires, and all necessary and proper foundations, footings, cross arms and other appliances and fixtures for use in connection with said towers, poles and/or other structures, wires and cables.

# EXHIBIT D

## Beneficial Public Values

The Purpose of the Conservation Easement for the Property is to protect the Beneficial Public Values of the Property, as summarized below and described in more detail in the Baseline Documentation Report:

(a) Habitat for fish, wildlife, and plants that are native to the area, including species protected under the California Endangered Species Act and/or the federal Endangered Species Act. The term "habitat" includes vegetation along banks and shorelines that contribute to maintaining watershed health. The term "native" refers to plants and animals that occur naturally on the Property, and are defined as "native" by the California Department of Fish & Wildlife and its successors.

(b) Forest resources on the Property. Forest resources are primarily classified as Douglas-fir and Montane hardwood. Trees on-site consist of mainly Douglas-fir, ponderosa pine, sugar pine, knobcone pine, black oak, white oak, coast live oak, big leaf maple and madrone with chaparral components of manzanita, deerbrush, coyote bush, and annual and perennial grasses.

(c) The scenic viewshed of the Property in keeping with the surrounding environment, providing a contiguous forested landscape visible to passersby on the nearby roads and highways.

(d) Outdoor recreation in the form of passive recreational pursuits such as hiking, fishing, hunting, birding, boating, and sightseeing.

(e) Identified historical and cultural values, to the extent they are protected by state and federal law.

(f) Agricultural values, such as limited grazing and limited row cropping.

# EXHIBIT E

## Grantee Insurance Requirements

Grantee shall procure, carry and maintain the following insurance coverage:

A. Workers' Compensation and Employers' Liability

1. Workers' Compensation insurance or self-insurance indicating compliance with any applicable labor codes, acts, laws or statutes, state or federal.

2. Employers' Liability insurance shall not be less than One Hundred Thousand Dollars ($100,000) for injury or death each accident.

B. Commercial General Liability

1. Coverage shall be at least as broad as the Insurance Services Office (ISO) Commercial General Liability Coverage "occurrence" form, with no coverage deletions.

2. The limit shall not be less than One Million Dollars ($1,000,000) each occurrence/ Two Million Dollars ($2,000,000) aggregate for bodily injury, property damage and personal injury. The foregoing coverage minimums shall include excess/"umbrella" coverage that are in force and that supplement underlying insurance coverage. In addition, such insurance shall insure the performance by Grantee of its indemnity and other contractual obligations under the Easement.

3. Coverage shall: a) By "Additional Insured" endorsement add as insureds Grantor, its directors, officers, agents and employees with respect to liability arising out of work performed by or for Grantee; b) Be endorsed to specify that Grantee's insurance is primary.

C. Business Auto

1. Coverage shall be at least as broad as the Insurance Services Office (ISO) Business Auto Coverage form covering Automobile Liability, code 1 "any auto."

2. The limit shall not be less than One Million Dollars ($1,000,000) each accident for bodily injury and property damage.

D. Additional Insurance Provisions

1. Upon change in carrier or coverage, or otherwise upon Grantor's request, Grantee shall furnish Grantor with certificates of insurance and endorsements of all required insurance for Grantee.

2. The documentation shall state that coverage shall not be canceled except after thirty (30) days prior written notice has been given to Grantor.

3. The documentation must be signed by a person authorized by that insurer to bind coverage on its behalf and shall be submitted to:

Pacific Gas and Electric Company
Insurance Department - B24H
Post Office Box 770000
San Francisco, CA 94177

A copy of all such insurance documents shall be sent to Grantor's Land Agent as specified under Notices in the body of this Easement.

4. Upon request, not to exceed once annually, Grantee shall furnish Grantor complete copies of policies.

5. Upon request, not to exceed once annually, Grantee shall furnish Grantor the same evidence of insurance for Grantee's agents or contractors as Grantor requires of Grantee.

EXHIBIT F

Prohibited Uses

As provided in Section 6 of this Easement, Grantor will not engage in, or permit others to engage in, the following Prohibited Uses:

1. Number of Fee Owners; Subdivision.

(a) Limit on Number of Fee Owners. Except for Specified Required Actions, notwithstanding the fact that the Property, at any time, might be comprised of more than two (2) separate legal parcels, fee title to the Property shall be held by no more than two (2) separate owners at any given time, provided, however, that the foregoing shall not prohibit undivided ownership of the Property by multiple owners (e.g. tenants in common), subject to the restrictions on the rights of undivided owners provided below, and the terms and conditions of this Easement shall perpetually apply to the Property as a whole. The existence of any separate legal parcels shall not be interpreted to permit any use or activity on an individual legal parcel that would not have been permitted on said parcel under the terms and conditions of this Easement as applied to the Property as a whole. This section only applies to conveyances of fee ownership and not to conveyances of any property interests other than fee ownership (e.g. leasehold interests). In respect to ownership of the Property or permitted separate legal parcels, as the case may be, ownership may be (among others) in the form of a partnership, limited partnership, limited liability company, corporation or other legal entity or as undivided interests such as tenants in common, whether by choice or by operation of any Applicable Laws, but no owner of an undivided interest shall thereby have (i) the right of exclusive occupancy or exclusive use of any separate portion of the Property (or permitted separate legal parcel), or (ii) any right to have the Property (or permitted separate legal parcel), partitioned in kind, whether pursuant to California Code of Civil Procedure §872.010 et seq. ("**CCP**") or any successor statute or otherwise. In the event that a partition action is brought and a court determines that the remedy of partition must be granted, Grantor, on behalf of itself and its successors and assigns hereby irrevocably agrees the remedy shall not be a physical subdivision of the Property (or permitted separate legal parcel), but instead may be a partition by appraisal pursuant to CCP §873.910 or any successor statute or a judicially supervised sale of Grantor's entire estate in the Property (or permitted separate legal parcel) pursuant to CCP §873.510 or any successor statute, subject, however, to this Easement, followed by a division of sales proceeds among the parties entitled thereto. Grantor recognizes that Grantee will incur direct and indirect costs for monitoring and administration of the Conservation Easement in the event fee title to a portion of the Property is transferred under this provision. Accordingly, upon Grantor's sale, transfer or conveyance of fee title to less than all of the Property in accordance with this subsection (a), Grantor shall pay, or cause to be paid, to Grantee a one-time payment of a sum representing the increased cost of such Conservation Easement stewardship, as reasonably determined at such time by Grantee. Such one-time payment shall be in addition to any reimbursements required pursuant to Section 13.2.4 or Section 17 of this Easement.

(b) Limit on Subdivision. Except for Specified Required Actions, Grantor shall not subdivide the Property with the result of frustrating the ownership restrictions set forth in subsection (a) above. For example, the following actions would not frustrate the ownership restrictions in subsection (a) above: (i) merger and reduction of the number of separate legal parcels comprising the Property; or (ii) reconfiguring by lot line adjustment the existing internal boundaries of legal parcels within the outer boundaries of the Property; or (iii) clarifying boundary lines with adjacent landowners; or (iv) subdivisions to facilitate Hydro Project Activities. Grantor shall (i) as part of the Annual Work Plan review in accordance with Section 7, or at least ninety (90) days prior to any Grantor subdivision activity (whether or not prohibited hereunder), furnish Grantee with the subdivision application or filings; and (ii) provide to Grantee reasonably sufficient information to identify the boundaries of each legal parcel. This information will become part of the Baseline Documentation Report. At the election of either party, the parties shall execute and record an amendment of this Easement to reflect any change to the legal description of the Property set forth in Exhibit A or any other changes and allocations resulting from permitted subdivision that are not established to the reasonable satisfaction of the parties by recordation in the Public Records of the plan of subdivision approved under Applicable Law.

2. Development Rights. Except for Specified Required Actions provided in Section 7, the development rights associated with all or any portion of the Property may not be transferred to, or used or exercised in connection with, any property other than the Property, such rights of transfer, use and exercise being hereby terminated and extinguished in perpetuity. The phrase "development rights" means any and all rights, however designated, now or hereafter associated with the Property or any portion thereof that may be used pursuant to applicable zoning laws, land use laws or other governmental laws or regulations, to compute permitted size, height, bulk, or number of structures, development density, lot yield, or any similar development variable on or pertaining to the Property or any other property.

3. Mining and Drilling. There shall be no mining, drilling, removing, fracking, or exploring for or extracting of minerals, oil, gas, coal, or other hydrocarbons, soils, sands, gravel, loam, rocks or any other material on, under, or at the Property. Notwithstanding the foregoing, the following shall not be Prohibited Uses:

(a) Specified Required Actions provided in Section 7;

(b) Drilling, removal and extraction of soils, sands, gravel, loam, rocks or any other material on, under, or at the Property in connection with studies and testing to the extent related to Grantor's exercise of the Hydro Reserved Rights;

(c) Testing, drilling and operating groundwater wells, and construction or placement of any structures or improvements within the Hydro Operating Zone to the extent related to Grantor's exercise of the Hydro Reserved Rights; and

(d) The use of soil, sand, gravel and other similar material located on the Property as appropriate for road maintenance, erosion control and in connection

with a Required Action subject to the following limitations: (i) such disturbance shall be kept to the minimum necessary to exercise such rights, (ii) any such soils, sands, and other materials shall not be removed from the Property, and (iii) all such utilization activities shall be conducted in a manner that minimizes to the greatest extent practicable impacts to the Beneficial Public Values.

4. Construction and Placement of Structures and Improvements. There shall be no construction or placement of any structures or improvements on the Property, including (but not limited to) residential, industrial, office, or other buildings, underground or aboveground tanks. Notwithstanding the foregoing, the following shall not be Prohibited Uses:

(a) Required Actions provided in Section 7;

(b) Permitted Uses under Exhibit I;

(c) Construction or placement of any structures or improvements within the Hydro Operating Zone which Grantor has determined relate to Grantor's exercise of the Hydro Reserved Rights; and

(d) Structures and improvements made in the course of prudent and customary land management activities and/or to protect, preserve, or enhance the Beneficial Public Values (including, for example, garbage enclosures, benches, interpretive kiosks, and appropriately located and sized caretaker structure).

5. Vehicles. Except for Required Actions provided in Section 7, or in the case of an emergency or other occurrence affecting the safety of persons and/or property, there shall be no use of any motorized vehicles off of existing roadways on the Property except vehicles used as necessary to carry out prudent and customary land management activities and/or to protect, preserve, or enhance the Beneficial Public Values. Motorized off-road recreational use shall not constitute an activity "related to the protection or preservation of the Beneficial Public Values" as provided in the preceding sentence.

6. Dumping or Salvage. Except for Required Actions provided in Section 7, there shall be no dumping, storage or other disposal on the Property of soil, trash or garbage except for (a) refuse generated on the Property which may be disposed of on the Property on a temporary basis prior to its removal from the Property in areas where the Beneficial Public Values of the Property are not significantly impaired, or (b) compostable refuse generated on the Property which may be disposed of on the Property in a responsible manner which does not significantly impair the Beneficial Public Values of the Property. Except for Required Actions provided in Section 7, there shall be no dumping, storage (other than on a temporary basis) or other disposal of ashes, sludge, Hazardous Substances, or other unsightly or dangerous materials outside of the Hydro Operating Zone. Except for Required Actions provided in Section 7, there shall be no storage or disassembly of inoperable automobiles, trucks, or other

vehicles or equipment for purposes of sale, or rental of space for that purpose outside of the Hydro Operating Zone.

7. Non-Native Animal Species. Except for Required Actions provided in Section 7, there shall be no release anywhere on the Property of non-native animal species other than livestock without Grantee's prior written approval in accordance with Section 7, as required.

8. Vegetation. Except for Required Actions provided in Section 7, there shall be no removal, cutting or destruction on the Property of native vegetation except (a) in an emergency and/or for purposes of disease or insect control or (b) to prevent property damage, personal injury, or flooding or (c) as permitted in Exhibit I, Sections 4 and 9, or (d) with Grantee's prior written approval in accordance with Section 7, as required.

9. Roads and Trails. Except for Required Actions provided in Section 7 or as required to implement a Required Action, or in the case of an emergency or other occurrence affecting the safety of persons and/or property, there shall be no construction of any new roads or trails on the Property; provided, however, the construction of new roads and trails (or the relocation of existing road and trails) on the Property to protect, preserve or enhance the Beneficial Public Values shall be permitted with Grantee's prior written approval in accordance with Section 7, as required. As used herein, the term "construction" shall not include the creation of roads or trails through repeated use, although such activities shall be governed by this Easement.

10. Commercial Uses. There shall be no office, industrial, or other commercial use on the Property that is likely to significantly impair Beneficial Public Values. Notwithstanding the foregoing, the following shall not be Prohibited Uses:

(a) Required Actions provided in Section 7;

(b) Uses permitted by Third Party Use Agreements; and

(c) Office, industrial, or other commercial uses within the Hydro Operating Zone which Grantor has determined relate to Grantor's exercise of the Hydro Reserved Rights.

11. Alteration of Land or Excavation. Except for Required Actions provided in Section 7 or as otherwise explicitly permitted by the terms of this Easement, there shall be no filling, excavating, grading, draining or dredging outside of the Hydro Operating Zone, nor any change in the general topography of the Property; provided, however, such activities shall be permitted outside of the Hydro Operating Zone in the course of prudent and customary land management activities and/or to protect, preserve, or enhance the Beneficial Public Values.

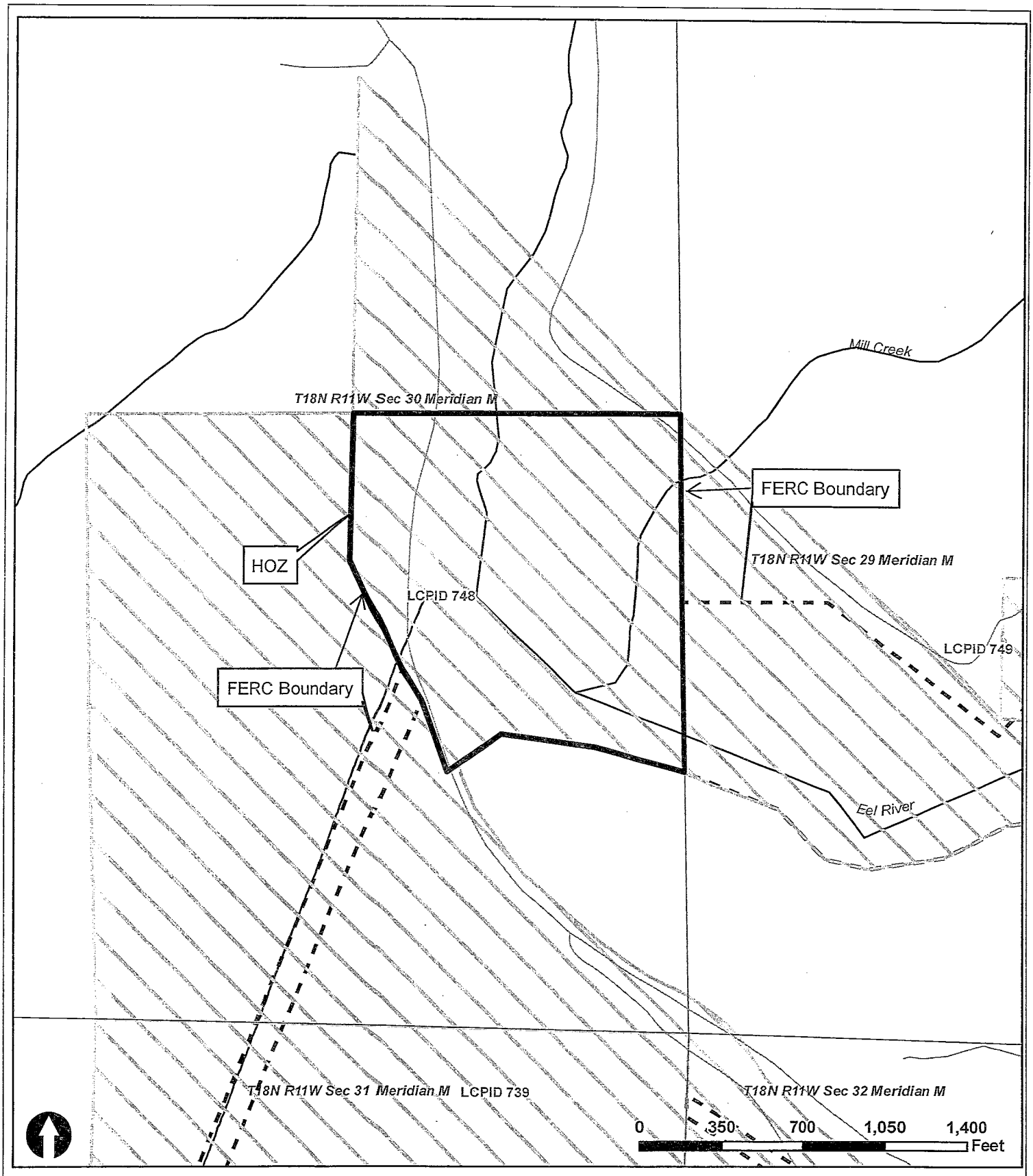
12. Billboards. Except for Required Actions provided in Section 7 or permitted uses under Exhibit I or Grantee's signs permitted under Section 5.3, there shall be no placement of billboards or advertising facilities. The use of Grantor's logo and/or trade

style on a sign will not in and of itself constitute a billboard or advertising facility under this provision.

EXHIBIT G

Hydro Operating Zone

[Follows this page]




HOZ

FERC Boundary

DLP

# Exhibit G
# Eel River

## Hydo Operating Zone (HOZ)

Sheet 1 of 1
Revised 4/11/2016

# EXHIBIT H

Express Third Party Uses and Third Party Use Agreements

1. Express Third Party Uses

The Express Third Party Uses on the Property are all uses permitted by and pursuant to the Third Party Use Agreements.

2. Third Party Use Agreements

The Third Party Use Agreements on the Property are those agreements and rights disclosed by the following:

(a) Those agreements identified on Attachment 1 hereto.

(b) Those agreements and rights disclosed by the following:

1. RIGHTS OF THE PUBLIC AND OF THE COUNTY OF LAKE, AS TO THAT PORTION OF THE HEREIN DESCRIBED PROPERTY LYING WITHIN ANY PUBLIC ROADS.
2. RIGHTS AND EASEMENTS, INCLUDING BUT NOT LIMITED TO, RECREATION, NAVIGATION AND FISHERIES, WHICH MAY EXIST OVER THAT PORTION OF SAID LAND LYING BENEATH THE WATERS OF ANY CREEK, SPRING, RIVER, RESERVOIR, CANAL OR LAKE.
3. AN EASEMENT OVER SAID LAND FOR CONSTRUCTION, MAINTENANCE AND FULL FREE QUIET USE AND ENJOYMENT AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED DATED MAY 13, 1926.

   PGE# 2418-10-0080

   AFFECTS A.P.N. 001-033-3 PORTION

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.
4. AN EASEMENT OVER SAID LAND FOR TRANSMISSION OF ELECTRICITY, HEAT, POWER AND LIGHT, FOR TELEPHONE AND TELEGRAPH LINES AND A PERPETUAL RIGHT OF WAY TO TRAVEL WITH WHEELED VEHICLES AND ALL OTHER MODES OF CONVEYANCE AND TRANSPORTION AND INCIDENTAL PURPOSES, AS GRANTED TO CALIFORNIA TELEPHONE AND LIGHT COMPANY, A CORPORATION, IN DEED RECORDED JULY 08, 1929, IN BOOK 42 PAGE 381 MENDOCINO COUNTY, OFFICIAL RECORDS.

   PGE# 2417-11-0001

   AFFECTS A.P.N. 173-030-2 AND 3, 173-130-23

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

5. AN EASEMENT OVER SAID LAND FOR THE CONSTRUCTION, MAINTENANCE AND USE OF A HIGHWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED DATED JANUARY 30, 1931

   PGE# 2418-11-0082

   AFFECTS: A.P.N. 171-230-19 AND 20; 171-200-011 AND 04; 001-031-02 AND 024

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

6. AN EASEMENT OVER SAID LAND FOR HIGHWAY PURPOSES, 60 FEET IN WIDTH AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED DATED JUNE 18, 1931, (NO RECORDING INFORMATION SHOWN).

   PGE# 2418-11-0071

   AFFECTS A.P.N. 001-030-009, 001-0361-11 AND 02

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

7. AN EASEMENT OVER SAID LAND FOR CONSTRUCTION, MAINTENANCE, AND USE OF PUBLIC HIGHWAY, 60 FEET IN WIDTH AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED DATED APRIL 24, 1936, (NO RECORDING INFORMATION SHOWN).

   PGE# 2418-10-0037

   AFFECTS A.P.N. 001-035-03, 02, 04 PORTION

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

8. AN EASEMENT OVER SAID LAND FOR CONSTRUCTION, MAINTENANCE AND USE OF A PUBLIC HIGHWAY, 60 FEET IN WIDTH AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED DATED AUGUST 24, 1936, (NO RECORDING INFORMATION SHOWN)

   PGE# 2418-10-0040

   AFFECTS A.P.N. 001-035,04, 02 PORTION

   NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

9. AN EASEMENT OVER SAID LAND FOR THE CONSTRUCTION, MAINTENANCE AND USE OF A HIGHWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED DATED DECEMBER 12, 1939 (NO RECORDING INFORMATION SHOWN)

   PGE# 2418-11-0010

AFFECTS: A.P.N. 171-170-02, 171-200-11

NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

10. AN EASEMENT OVER SAID LAND FOR A BRIDGE AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED RECORDED MARCH 12, 1956, IN BOOK 423 PAGE 546 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2417-11-0034

    AFFECTS A.P.N. 173-130-23

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

11. AN EASEMENT OVER SAID LAND FOR A PUBLIC ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED JANUARY 30, 1956, IN BOOK 260 PAGE 163 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0075

    AFFECTS A.P.N. 001-033-03 PORTION

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

12. AN EASEMENT OVER SAID LAND TO CONSTRUCT, RECONSTRUCT , MAINTAIN, AND USE A PUBLIC ROAD, 66 FEET IN WIDTH OR MORE, IF NECESSARY TO ACCOMMODATE CUTS AND FILLS AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED JUNE 08, 1956, IN BOOK 432 PAGE 580 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0146

    AFFECTS A.P.N. 171-230-019, 020; 171-200-14 AND 04; 001-035-02, 04, 001-033-003

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

    A CORRECTION DEED WAS RECORDED ON MARCH 6, 1958, IN BOOK 290 OF OFFICIAL RECORDS AT PAGE 145, LAKE COUNTY ROADS, AND FEBRUARY 20, 1958, IN BOOK 478 OF OFFICIAL RECORDS PAGE 491, MENDOCINO COUNTY RECORDS.

    A PARTIAL QUITCLAIM DEED THEREUNDER RECORDED MARCH 31, 2009 AS INSTRUMENT NO. 2009-04380 OFFICIAL RECORDS OF MENDOCINO COUNTY AND INSTRUMENT NO. 2009004923 OFFICIAL RECORDS OF LAKE COUNTY.

    PACIFIC GAS AND ELECTRIC COMPANY# 2418-11-0150

13. AN EASEMENT OVER SAID LAND TO CONSTRUCT, MAINTAIN AND USE A ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED RECORDED MAY 29, 1964, IN BOOK 429 PAGE 317 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0035

    AFFECTS A.P.N. 001-037-01

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

14. AN EASEMENT OVER SAID LAND FOR RIGHT OF WAY FOR HIGHWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED RECORDED MAY 12, 1966, IN BOOK 491 PAGE 504 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0151

    AFFECTS A.P.N. 001-030-09, 001-031-02

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

15. AN EASEMENT OVER SAID LAND TO MAINTAIN AND USE A ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO ROBERT G. SEQUEIRA, ET AL, IN DEED RECORDED SEPTEMBER 01, 1966, IN BOOK 502 PAGE 577 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0107

    AFFECTS A.P.N. 001-035-003

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

16. AN EASEMENT OVER SAID LAND FOR CONSTRUCTION, MAINTENANCE AND USE OF A HIGHWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED RECORDED JUNE 19, 1967, IN BOOK 742 PAGE 220 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0153

    AFFECTS A.P.N. 171-260-02

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

17. AN EASEMENT OVER SAID LAND TO CONSTRUCT, MAINTAIN, AND USE A ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO ROBERT G. SEQUEIRA, ET UX, IN DEED RECORDED NOVEMBER 13, 1968, IN BOOK 571 PAGE 634 LAKE COUNTY, OFFICIAL RECORDS.

PGE# 2418-10-0110

AFFECTS A.P.N. 001-035-03

NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

18. AN EASEMENT OVER SAID LAND TO RECONSTRUCT , MAINTAIN AND USE THE EXISTING ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO DASHIELL STATION, A PARTNERSHIP, IN DEED RECORDED OCTOBER 14, 1975, IN BOOK 810 PAGE 31 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0193

    AFFECTS A.P.N. 001-030-009

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

19. AN EASEMENT OVER SAID LAND FOR NON-EXCLUSIVE 70 FEET IN WIDTH FOR INGRESS AND EGRESS OVER THE EXISTING ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO WILMOT G. PRESLEY, IN DEED RECORDED JANUARY 04, 1978, IN BOOK 914 PAGE 270 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# NONE SHOWN

    AFFECTS A.P.N. 001-031-24 WITH OTHER PROPERTY

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

20. AN EASEMENT OVER SAID LAND FOR PROMOTING AND PROTECTING WATER QUAILITY, NATURAL STREAMS, FLOOD CONTROL, SOIL CONSERVATION, SCENIC CONSERVATION AND THE PRESERVATION OF NATURAL AMENITIES AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED OCTOBER 05, 1979, IN BOOK 1010 PAGE 265 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0056

    AFFECTS A.P.N. 001-033-03

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

21. AN EASEMENT OVER SAID LAND FOR RECREATION FACILITIES AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED OCTOBER 05, 1979, IN BOOK 1010 PAGE 270 LAKE COUNTY, OFFICIAL RECORDS.

    AMENDED IN DOCUMENT RECORDED JULY 29, 1986 IN BOOK 1323 PAGE 113, LAKE COUNTY OFFICIAL RECORDS.

PGE# 2418-10-0055

AFFECTS A.P.N. 001-033-03

NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

22. THE TERMS, CONDITIONS AND PROVISIONS AS CONTAINED IN THE INSTRUMENT ENTITLED "CONSENT AGREEMENT ", BY AND BETWEEN PACIFIC GAS AND ELECTRIC COMPANY , A CALIFORNIA CORPORATION, AND EUGENE J. M. MCFADDEN, RECORDED MAY 20, 1981, IN BOOK 1306 PAGE 303 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2417-11-0187

    AFFECTS A.P.N. 173-030-002, 003

23. AN EASEMENT OVER SAID LAND FOR NON-EXCLUSIVE EASEMENT AND RIGHT OF WAY FOR PUBLIC UTILITIES AND ROAD PURPOSES, 40 FEET IN WIDTH AND INCIDENTAL PURPOSES, AS GRANTED TO ROGER DIXON, ET AL, IN DEED RECORDED NOVEMBER 09, 1983, IN BOOK 1203 PAGE 200 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# NONE SHOWN

    AFFECTS A.P.N. 001-031-024

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

24. AN EASEMENT OVER SAID LAND FOR CONSTRUCTING, RECONSTRUCTING , RELOCATING, OPERATING AND MAINTAINING ALL RECREATIONAL FACILITIES AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED JULY 29, 1986, IN BOOK 1323 PAGE 115 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0170

    AFFECTS A.P.N. 001-033-005

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

25. THE TERMS, CONDITIONS AND PROVISIONS AS CONTAINED IN THE INSTRUMENT ENTITLED "AGREEMENT GRANTING RECIPROCAL ROAD USE EASEMENTS", BY AND BETWEEN PACIFIC GAS AND ELECTRIC COMPANY, AND STATE OF CALIFORNIA, RECORDED AUGUST 15, 1987, IN BOOK 1640 PAGE 685 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0207

    AFFECTS A.P.N. 171-250-013, 171-230-019, 171-260-002

26. AN EASEMENT OVER SAID LAND FOR COUNTY ROAD PURPOSES AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED RECORDED DECEMBER 13, 1988, IN BOOK 1723 PAGE 698 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0209

    AFFECTS A.P.N. 171-200-011

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

27. AN EASEMENT OVER SAID LAND FOR COUNTY ROAD PURPOSES AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF MENDOCINO, IN DEED RECORDED DECEMBER 13, 1988, IN BOOK 1723 PAGE 700 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0210

    AFFECTS A.P.N. 171-200-011

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

28. AN EASEMENT OVER SAID LAND TO CONSTRUCT, MAINTAIN AND USE A ROAD AND INCIDENTAL PURPOSES, AS GRANTED TO DENNIS OLE WIK, ET UX, IN DEED RECORDED MAY 25, 1990, IN BOOK 1832 PAGE 001 MENDOCINO COUNTY, OFFICIAL RECORDS.

    PGE# 2418-11-0229

    AFFECTS A.P.N. 171-170-002

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

29. AN EASEMENT OVER SAID LAND FOR RECREATIONAL HIKING TRAIL 10 FEET IN WIDTH, MORE OR IF NECESSARY TO ACCOMMODATE CUTS AND FILLS AND INCIDENTAL PURPOSES, AS GRANTED TO UNITED STATES OF AMERICA, IN DEED RECORDED JULY 11, 1991, INSTRUMENT NO. 013932 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0173

    AFFECTS A.P.N. 001-033-034

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

30. AN EASEMENT OVER SAID LAND TO CONSTRUCT, RECONSTRUCT , MAINTAIN AND USE A PUBLIC ROAD 66 FEET IN WIDTH MORE IF NECESSARY TO ACCOMMODATE CUTS AND FILLS AND INCIDENTAL PURPOSES, AS GRANTED TO

UNITED STATES OF AMERICA, IN DEED RECORDED AUGUST 01, 1991, INSTRUMENT NO. 91-015537 LAKE COUNTY, OFFICIAL RECORDS.

PGE# 2418-10-0174

AFFECTS A.P.N. 001-035-003 AND 001-033-006

NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

31. AN EASEMENT OVER SAID LAND FOR COMMUNICATION FACILITIES AND INCIDENTAL PURPOSES, AS GRANTED TO PACIFIC BELL, IN INSTRUMENT RECORDED MAY 11, 1999, IN INSTRUMENT NO. 99-007994, OFFICIAL RECORDS.

    AFFECTS: APN 001-031-002

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT. REFERENCE IS MADE TO SAID DOCUMENT FOR FULL PARTICULARS.

32. AN EASEMENT OVER SAID LAND FOR PUBLIC ROADWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED RECORDED AUGUST 06, 2001, INSTRUMENT NO. 01-015385 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-0204

    AFFECTS: A.P.N. 001-035-004

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

33. AN EASEMENT OVER SAID LAND FOR PUBLIC ROADWAY AND INCIDENTAL PURPOSES, AS GRANTED TO COUNTY OF LAKE, IN DEED RECORDED AUGUST 06, 2001, INSTRUMENT NO. 01-15386 LAKE COUNTY, OFFICIAL RECORDS.

    PGE# 2418-10-205

    AFFECTS: A.P.N. 001-035-004

    NO REPRESENTATION IS MADE AS TO THE CURRENT OWNERSHIP OF SAID EASEMENT.

# ATTACHMENT 1
to
Exhibit H

1. Row Crop/Grazing License between Pacific Gas and Electric Company and Eugene J. McFadden, dated February 28, 1998.

2. [Intentionally Deleted].

3. [Intentionally Deleted].

4. [Intentionally Deleted].

5. Letter Agreement between Pacific Gas and Electric Company and the State of California, Department of Forestry and Fire Protection, dated December 17, 1997.

6. Boat Dock Agreement between Pacific Gas and Electric Company and Duane D. Goebel, Jr. and Gina M. Goebel, dated August 7, 2013.

7. Boat dock agreement between Pacific Gas and Electric Company and Donald Chase and Karol Chase, dated September 4, 1998.

8. Boat dock agreement between Pacific Gas and Electric Company and Joel deSilva and Vicki deSilva, dated October 20, 1998.

9. Boat dock agreement between Pacific Gas and Electric Company and Danny L. Fish, Jr. and Randa L. Fish dated September 30, 1998.

10. Boat dock agreement between Pacific Gas and Electric Company and Dennis Fuller, Linda Fuller, Robert Luhrs, Cheryl Luhrs, Jolane Luhrs, Steven Luhrs, and Carolyn Luhrs, dated August 18, 1998.

11. Boat dock agreement between Pacific Gas and Electric Company and William Garringer and Deborah Garringer, dated August 18, 1998.

12. Boat Dock agreement between Pacific Gas and Electric Company and Alfonso C. Gomar, dated July 21, 1988.

13. Boat dock agreement between Pacific Gas and Electric Company and Kenneth L. Thompson Sally J. Thompson, Deborah L. Hanssen, Lawrence J. Thompson, and Nancy A. Horton, dated July 21, 1998.

14. [Intentionally Deleted].

15. License Agreement (Boat Dock Facilities – Lake Pillsbury), between Pacific Gas and Electric Company and Patrick Yount and Deborah Yount, dated August 7, 2019.

16. Boat dock agreement between Pacific Gas and Electric Company and Loren G. Hardisty and Joanne Hardisty, dated July 21, 1998.

17. Boat dock agreement between Pacific Gas and Electric Company and William Healey and Mary J. Healey, dated July 27, 1998.

18. Boat dock license agreement between Pacific Gas and Electric Company and Kazuo Kunitani and Suzu Kunitani, dated July 27, 1998.

19. Boat dock agreement between Pacific Gas and Electric Company and Robert I. Lawrence, Jr. dated July 21, 1998.

20. Boat dock agreement between Pacific Gas and Electric Company and Orville A. Lowe and Bonnie A. Lowe, dated October 20, 1998.

21. License Agreement (Boat Dock Facilities – Lake Pillsbury), between Pacific Gas and Electric Company and Joseph H. Masciorini, Jr. and Matthew H. Masciorini, dated July 9, 2019.

22. License Agreement (Boat Dock Facilities – Lake Pillsbury), between Pacific Gas and Electric Company and Joseph H. Masciorini, Jr. and Matthew H. Masciorini, dated July 4, 2019.

23. [Intentionally Deleted].

24. Boat dock agreement between Pacific Gas and Electric Company and Robert C. Meyer, Barbara A. Meyer and James D. Meyer, dated March 16, 1999.

25. Boat dock agreement between Pacific Gas and Electric Company and Shirley Blanche Stone, dated July 21, 1998.

26. Boat dock license agreement between Pacific Gas and Electric Company and Lake Pillsbury Homesite Association, dated May 1, 2013.

27. Boat ramp license between Pacific Gas and Electric Company and Lake Pillsbury Homesite Association, dated October 1, 2014.

28. Log boom license agreement between Pacific Gas and Electric Company and Lake Pillsbury Homesite Association, dated June 15, 2000.

29. License agreement between Pacific Gas and Electric Company and Lake Pillsbury Resort, Inc., dated July 1, 2013.

30. First Amendment to License Agreement (For A Right of Entry For Use) between Pacific Gas and Electric Company and Lake Pillsbury Resort, Inc., dated March 29, 2017.

31. Second Amendment to License Agreement (For A Right of Entry For Use) between Pacific Gas and Electric Company and Lake Pillsbury Resort, Inc., dated June 19, 2019.

32. Lease agreement between Pacific Gas and Electric Company and Westshore Campers Association, dated December 21, 1985.

33. License Agreement, to install and maintain a radio reporting weather station between Pacific Gas and Electric Company and United States of America, Department of the Army, Corps of Engineers, dated October 12, 1995, as amended by a First Amendment to License Agreement to install and maintain a radio reporting weather station between Pacific Gas and Electric Company and United States of America, Department of the Army, Corps of Engineers, dated May 1, 2009.

34. License Agreement to install and maintain a rain gauge between Pacific Gas and Electric Company and United States of America, Department of Commerce, dated December 8, 1964.

35. Permission agreement to install, maintain and use egg collecting station and an egg eyeing station for development and conservation of game fish between Pacific Gas and Electric

Company and State of California, Department of Natural Resources, Division of Fish & Game, dated August 3, 1938.

36. Site License Agreement For Seismic Monitoring by U.S. Geological Survey, dated June 18, 2019, between Pacific Gas and Electric Company and the U.S. Geological Survey.

# EXHIBIT I

## Permitted Uses

The following are Permitted Uses:

1. The Express Third Party Uses.

2. The uses and activities expressly permitted under Exhibit F.

3. Except as otherwise limited by this Easement, the right to sell, encumber, or otherwise transfer the Property, portions thereof, or interests therein, to anyone Grantor chooses.

4. The right to trim and cut down and clear away any and all trees, brush and vegetation (a) which constitute a hazard to persons or property, and/or (b) for purposes of fire management, disease or insect control or otherwise as necessary or appropriate for prudent land management (i.e., not motivated by commercial benefit), and/or (c) for other vegetation management operations, including but not limited to fuel reduction projects, thinning of tree stands and meadow restoration projects. The foregoing may include pesticide use to control vegetation (brush, grass, weeds, etc.) and/or insects.

5. Consistent with Section 9.2, the right to install, maintain, repair, replace and maintain gates and fences.

6. The right to perform all activities required to comply with any and all Applicable Laws.

7. The right to maintain, repair, restore, replace and reconstruct all structures and improvements now or hereafter located on the Property, provided any replacement structures or improvements shall be located in substantially the same location and within the same footprint as the structure or improvement being replaced, and shall be substantially the same height as the structure or improvement being replaced.

8. The right to install minor, temporary structures necessary or appropriate in connection with the performance of prudent and customary land management activities, Hydro Project Activities, or the protection, preservation, or enhancement of the Beneficial Public Values.

9. (a) The right to undertake commercial and/or non-commercial forest management activities on the Property for any of the following purposes: (1) to promote the health and sustainability of the Property's natural resources; (2) to protect and enhance the Property's riparian resources; (3) to maintain an ecologically appropriate species mix of overstory and understory vegetation allowing for a full and balanced range of stand ages and characteristics that is allowed to move across the landscape over time, including early-seral, mid-seral and late-seral forest conditions that provide adequate amounts of snags and cavity trees; (4) to protect and enhance wildlife habitat

for native species historically present on the Property; (5) to protect cultural resources on the Property; (6) to control invasive and non-native vegetation; and (7) to prevent, mitigate, and/or respond to any natural disaster (such as wildfire, significant insect and disease outbreak, or significant wind damage).

(b) Forest management activities for the purposes outlined in Paragraph (a) may include, but shall not be limited to, timber harvesting; salvage logging; conversion of vegetation types; prescribed fire; pre-commercial and commercial thinning of conifer and hardwood trees; fuels management; tree planting; control of undesirable vegetation and pests; habitat maintenance and enhancement; and road and watercourse crossing construction, maintenance, repair, and enhancement. Grantor shall carry out all such forest management activities in compliance with Applicable Law.

(c) Grantor shall provide to Grantee any existing or future forest management plan as part of the Annual Work Plan review process pursuant to Section 7.3 of this Easement. Grantor and Grantee shall review and discuss such plan (along with any proposals Grantee may have regarding this Permitted Use by Grantor pursuant to Section 7.3.4(b) of this Easement) periodically as part of such Annual Work Plan review process. Said forest management plan shall be updated periodically, as appropriate.

10. The right to maintain, repair, improve, replace and reconstruct, the existing cabins, camping facilities, boat docks, log booms and associated improvements on the Property in their existing location, within their existing footprints and without a substantial increase in height.

11. The right to construct, reconstruct, replace, remove, maintain and use the types of facilities and improvements described in paragraph (7) of Exhibit C that are unrelated to Hydro Project Activities and do not constitute a Required Action provided that such facilities shall be subject to Grantee's approval in the manner provided for Discretionary Actions.

12. In accordance with Applicable Laws, the right to control or eliminate noxious weeds and non-native plant species on the Property, and the right to control animals that (a) pose or threaten to pose a hazard to persons or property, including Hydroelectric Facilities and Associated Water Delivery Facilities, or (b) adversely impact or threaten to adversely impact (i) one or more of the Beneficial Public Values, (ii) Grantor's Hydro Reserved Rights or Hydro Project Activities, (iii) the Hydroelectric Facilities and Associated Water Delivery Facilities, or (iv) an Express Third Party Use.

13. The right to erect reasonably sized signs (illuminated and non-illuminated) to support and manage safety and permitted uses of the Property, including signs regarding authorized and unauthorized entry and uses or other appropriate markers in prominent locations on the Property, such as boundary fences, trails, and access roads.

# EXHIBIT J

## New FERC Project License – Potential Removal of Project Dams

On January 28, 2019 Grantor withdrew its Notice of Intent ("**NOI**") and Pre-Application Document ("**PAD**") seeking renewal of its FERC license for the Potter Valley Project - FERC Project No. 77 ("**Project**"). On June 28, 2019, pursuant to 18 C.F.R. § 5.5 (2018) and the FERC March 1, 2019 "Notice Soliciting Applications," Mendocino County Inland Water and Power Commission ("**Mendocino County IWPC**"), Sonoma County Water Agency ("**Sonoma Water**"), California Trout, Inc. ("**Cal Trout**"), and the County of Humboldt, California (collectively, "**NOI Parties**") submitted their NOI to file an application for a new FERC project ("**New Project**"). The NOI Parties filed the NOI as proxies for a yet to be formed regional entity ("**Regional Entity**") to serve as license applicant and future owner of the New Project. The NOI Parties and Regional Entity are hereinafter referred to collectively as the "**New License Applicant**" or "**New Licensee.**" As noted in Recital I and more definitively described in Exhibit B, Grantor continues to use the Property for Hydro Project Activities.

The New License Applicant anticipates the new license application, which is required to be submitted to FERC on or before April 14, 2022, will propose a modified New Project (including works and operations) as appropriate to achieve certain "shared objectives" of the NOI Parties. In furtherance of achieving such shared objectives, the New License Applicant is anticipated to evaluate a wide range of options for New Project facilities, including additions, modifications, and/or removal to achieve fish passage as appropriate for restoration of native anadromous fisheries in the Eel River Basin. The NOI Parties, as New License Applicant, have indicated an intent to evaluate facility modifications to achieve fish passage objectives, including the possibility for removal of Scott Dam and Van Arsdale Dam ("**Project Dams**").

Although the outcome of the New License Applicant's relicensing process is unknown as of the Effective Date, Grantor acknowledges that the issuance of a new FERC license authorizing or requiring the New Licensee to remove Project Dams could result in material changes to the baseline condition and use of the Property. Grantor further acknowledges that such material changes to the baseline condition and use of the Property resulting from the removal of Project Dams could increase the cost of Conservation Easement stewardship to Grantee or potentially cause an extinguishment of the Easement pursuant to Section 14.1 above.

As provided in Section 13.1 of this Easement, upon Grantor's sale, transfer or conveyance of fee title of the Property, the one-time payment of a sum representing the increased cost of such Conservation Easement stewardship, determined at such time by Grantee, shall include an amount representing the increased costs associated with the New Project. Grantor and Grantee acknowledge that conveyance of fee title to the Property to the New Licensee could occur in advance of issuance of a new FERC license, and that impacts of the New Project on Grantee's Conservation Easement stewardship may be uncertain at the time Grantor conveys fee title.

In recognition of the unique FERC relicensing circumstances associated with the Property as of the Effective Date, and in furtherance of the intent to ensure Grantee is reimbursed for incremental Conservation Stewardship costs resulting from Grantor's transfer of fee interest in the Property as described in Section 13.1, Grantor and Grantee agree to the following:

Upon receipt of the notice 90-day notice contemplated by Section 13.1 above, and in accordance with the provisions of Section 13.1, Grantee shall make a good faith effort, in consultation with Grantor and New Licensee, to determine the increased Conservation Easement stewardship costs that will be incurred by Grantee as a result of Grantor's sale, transfer or conveyance of fee title of the Property associated with the subsequent removal of Project Dams. Grantor's payment obligation pursuant to Section 13.1 above shall include the increased cost of such Conservation Easement stewardship associated with removal of Project Dams, as reasonably determined at such time by Grantee. Such one-time payment shall be made concurrently with the transfer of fee title and in addition to any reimbursements required pursuant to Section 13.2.4 and Section 17 of this Easement.

The provisions of this Exhibit J shall be limited to requirements of the FERC license issued to New Licensee as contemplated as of the Effective Date. Grantee shall not be entitled to reimbursement for increased costs of Conservation Easement stewardship for other provisions of said FERC license issued to New licensee or for the implementation of conditions of any future FERC license beyond reimbursements otherwise contemplated within this Easement.